

ARKANSAS STATE HIGHWAY AND TRANSPORTATION DEPARTMENT



**SUBSURFACE INVESTIGATION**

STATE JOB NO. 040623

FEDERAL AID PROJECT NO. NHPP-0017(33)

NATURAL DAM-NORTH STRS. & APPRS. (S)

STATE HIGHWAY 59 SECTION 5

IN CRAWFORD COUNTY

LETTING OF AUGUST 10, 2016

The information contained herein was obtained by the Department for design and estimating purposes only. It is being furnished with the express understanding that said information does not constitute a part of the Proposal or Contract and represents only the best knowledge of the Department as to the location, character and depth of the materials encountered. The information is only included and made available so that bidders may have access to subsurface information obtained by the Department and is not intended to be a substitute for personal investigation, interpretation and judgment of the bidder. The bidder should be cognizant of the possibility that conditions affecting the cost and/or quantities of work to be performed may differ from those indicated herein.

ARKANSAS STATE HIGHWAY AND TRANSPORTATION DEPARTMENT

May 15, 2014

TO: Mr. Trinity Smith, Engineer of Roadway Design

SUBJECT: Job No. 040623  
Natural Dam - North Strs. & Apprs. (S)  
Route 59 Section 5  
Crawford County

Transmitted herewith is the requested Soil Survey, Strength Data and Resilient Modulus test results for the above referenced job. The project consists of replacing three bridges on Highway 59. Two of the bridges are on new location. Samples were obtained in the existing travel lanes, shoulders, ditch line and along the new location. Sample locations were measured from centerline of construction and should be noted as such on the logs.

Based on laboratory results of samples obtained, the subgrade soils consist primarily of low plasticity sandy clay with varying amounts of sandstone fragments. Cross-sections are not currently available but it is anticipated that the construction grade line will closely match that of the existing grade line. Rock was encountered at stations 102+00 at 4 feet, 13 feet, and 22 feet right of centerline at depths of 3.0 feet, 3.0 feet, and 2.5 feet respectively; and 201+00 5 feet right of centerline at a depth of 5.0 feet. The subgrade soils are expected to provide a stable working platform with conventional processing if the weather is favorable during construction.

Undercut requirements may vary based on seasonal conditions. If embankment is to be placed within the existing ditch line, the soft organic material should be undercut a maximum depth of two feet. Further recommendations will be made when plans are more developed and cross-sections become available.

Listed below is the additional information requested for use in developing the plans:

1. The Qualified Products List (QPL) indicates that Aggregate Base Course (Class CL-7) is available from commercial producers in the vicinity of Van Buren.
2. Asphalt Concrete Hot Mix

Type	Asphalt Cement %	Mineral Aggregate %
Surface Course	5.5	94.5
Binder Course	4.6	95.4
Base Course	4.1	95.9

  
Michael C. Benson  
Materials Engineer

MCB:pt:bjj  
Attachment

cc: State Constr. Eng. – Master File Copy  
District 4 Engineer  
Transportation Planning and Policy Div.  
G. C. File

ARKANSAS STATE HIGHWAY AND TRANSPORTATION DEPARTMENT - LITTLE ROCK, ARKANSAS  
MATERIALS DIVISION  
MICHAEL BENSON, MATERIALS ENGINEER  
\*\*\* SOIL SURVEY STRENGTH TEST REPORT \*\*\*

DATE - 05/12/2014  
JOB NUMBER - 040623

SEQUENCE NO. - 1  
MATERIAL CODE - SSRVPS  
SPEC. YEAR - 2003  
SUPPLIER ID. - 1  
COUNTY/STATE - 17  
DISTRICT NO. - 04

JOB NAME - NATURAL DAM - NORTH STRS. & APPRS. (S)

\*\*\*\*\*  
\* STATION LIMITS R-VALUE AT 240 psi \*  
\*\*\*\*\*

BEGIN JOB - END JOB 8

RESILIENT MODULUS  
STA.101+00 8551  
STA.201+00 9006

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REMARKS -

-  
AASHTO TESTS : T190

**ARKANSAS STATE HIGHWAY AND TRANSPORTATION DEPARTMENT  
MATERIALS DIVISION**

**AASHTO T 307-99 - RESILIENT MODULUS OF SUBGRADE SOILS  
RECOMPACTED SAMPLES**

<b>Job No.</b>	040623	<b>Material Code</b>	SSRVPS
<b>Date Sampled:</b>	5/8/14	<b>Station No.:</b>	110+00
<b>Date Tested:</b>	May 8, 2014	<b>Location:</b>	20'LT
<b>Name of Project:</b>	NATURAL DAM-NORTH STRS. & APPRS (S)		
<b>County:</b>	<b>Code:</b> 17	<b>Name:</b>	CRAWFORD
<b>Sampled By:</b>	FAULKNER	<b>Depth:</b>	0-5
<b>Lab No.:</b>	20141217	<b>AASHTO Class:</b>	A-6(6)
<b>Sample ID:</b>	RV385	<b>Material Type (1 or 2):</b>	2
<b>LATITUDE:</b>		<b>LONGITUDE:</b>	

**1. Testing Information:**

Preconditioning - Permanent Strain > 5% (Y=Yes or N= No)	N
Testing - Permanent Strain > 5% (Y=Yes or N=No)	N
Number of Load Sequences Completed (0-15)	15

**2. Specimen Information:**

Specimen Diameter (in):	
Top	3.94
Middle	3.93
Bottom	3.93
Average	3.93
Membrane Thickness (in):	0.01
Height of Specimen, Cap and Base (in):	8.04
Height of Cap and Base (in):	0.00
Initial Length, Lo (in):	8.04
Initial Area, Ao (sq. in):	12.08
Initial Volume, AoLo (cu. in):	97.10

**3. Soil Specimen Weight:**

Weight of Wet Soil Used (g):	3170.90
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**4. Soil Properties:**

Optimum Moisture Content (%):	15.4
Maximum Dry Density (pcf):	110.8
95% of MDD (pcf):	105.3
In-Situ Moisture Content (%):	N/A

**5. Specimen Properties:**

Wet Weight (g):	3170.90
Compaction Moisture content (%):	14.7
Compaction Wet Density (pcf):	124.43
Compaction Dry Density (pcf):	108.48
Moisture Content After Mr Test (%):	14.7

**6. Quick Shear Test (Y=Yes, N=No, N/A=Not Applicable):**

#VALUE!

**7. Resilient Modulus, Mr:**

12227(Sc)<sup>-0.25857</sup>(S3)<sup>0.26469</sup>

**8. Comments**

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**9. Tested By:**

M.W.

**Date:** May 8, 2014

**ARKANSAS STATE HIGHWAY AND TRANSPORTATION DEPARTMENT  
MATERIALS DIVISION**

**AASHTO T 307-99 - RESILIENT MODULUS OF SUBGRADE SOILS  
RECOMPACTED SAMPLES**

**Job No.** 040623      **Material Code** SSRVPS  
**Date Sampled:** 5/8/14      **Station No.:** 110+00  
**Date Tested:** May 8, 2014      **Location:** 20LT  
**Name of Project:** NATURAL DAM-NORTH STRS. & APPRS (S)  
**County:** Code: 17      **Name:** CRAWFORD  
**Sampled By:** FAULKNER      **Depth:** 0-5  
**Lab No.:** 20141217      **AASHTO Class:** A-6(6)  
**Sample ID:** RV385      **Material Type (1 or 2):** 2  
**LATITUDE:** LONGITUDE:

PARAMETER	Chamber Confining Pressure	Nominal Maximum Axial Stress	Actual Applied Max. Axial Load		Actual Applied Cyclic Load	Actual Applied Contact Load	Actual Applied Max. Axial Stress	Actual Applied Cyclic Stress	Actual Applied Contact Stress	Average Recov Def. LVDT 1 and 2	Resilient Strain	Resilient Modulus
			P <sub>max</sub> lbs	P <sub>cyclic</sub> lbs								
Sequence 1	6.0	2.0	24.8	22.1	2.7	2.1	1.8	0.2	0.0088	0.0011	16,728	
Sequence 2	6.0	4.0	46.8	44.1	2.7	3.9	3.6	0.2	0.0194	0.0024	15,130	
Sequence 3	6.0	6.0	68.7	65.2	3.5	5.7	5.4	0.3	0.0319	0.0040	13,583	
Sequence 4	6.0	8.0	90.6	84.6	5.9	7.5	7.0	0.5	0.0491	0.0061	11,472	
Sequence 5	6.0	10.0	112.4	104.1	8.3	9.3	8.6	0.7	0.0674	0.0084	10,277	
Sequence 6	4.0	2.0	24.8	22.1	2.7	2.1	1.8	0.2	0.0101	0.0013	14,565	
Sequence 7	4.0	4.0	46.4	43.6	2.8	3.8	3.6	0.2	0.0219	0.0027	13,270	
Sequence 8	4.0	6.0	67.0	64.2	2.8	5.6	5.3	0.2	0.0362	0.0045	11,820	
Sequence 9	4.0	8.0	89.0	83.9	5.1	7.4	6.9	0.4	0.0529	0.0066	10,555	
Sequence 10	4.0	10.0	110.8	103.2	7.6	9.2	8.5	0.6	0.0710	0.0088	9,683	
Sequence 11	2.0	2.0	24.6	21.9	2.7	2.0	1.8	0.2	0.0124	0.0015	11,768	
Sequence 12	2.0	4.0	45.8	43.1	2.7	3.8	3.6	0.2	0.0269	0.0034	10,646	
Sequence 13	2.0	6.0	66.2	63.5	2.7	5.5	5.3	0.2	0.0427	0.0053	9,897	
Sequence 14	2.0	8.0	86.8	82.6	4.2	7.2	6.8	0.3	0.0602	0.0075	9,132	
Sequence 15	2.0	10.0	108.8	102.1	6.7	9.0	8.5	0.6	0.0795	0.0099	8,551	

**TESTED BY** \_\_\_\_\_ **M.W.** \_\_\_\_\_ **DATE** May 8, 2014  
**REVIEWED BY** \_\_\_\_\_ **DATE** \_\_\_\_\_

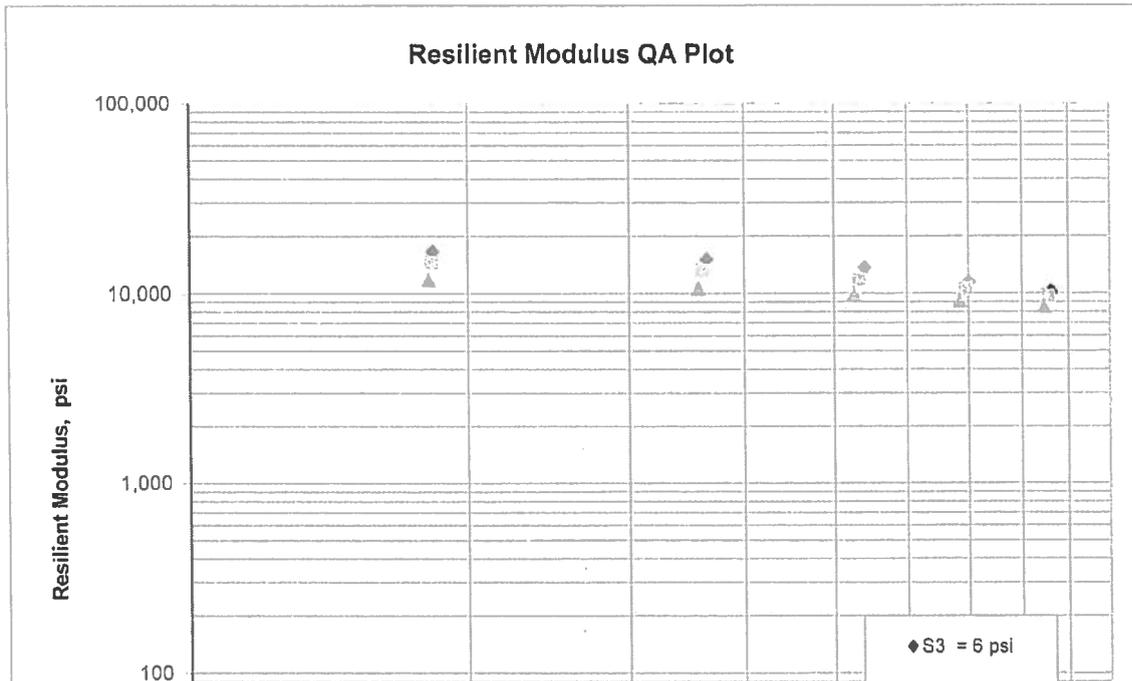
**ARKANSAS STATE HIGHWAY AND TRANSPORTATION DEPARTMENT  
MATERIALS DIVISION**

**AASHTO T 307-99 - RESILIENT MODULUS OF SUBGRADE SOILS  
RECOMPACTED / THINWALL TUBE SAMPLES**

<b>Job No.</b>	040623	<b>Material Code</b>	SSRVPS
<b>Date Sampled:</b>	5/8/14	<b>Station No.:</b>	110+00
<b>Date Tested:</b>	May 8, 2014	<b>Location:</b>	20'LT
<b>Name of Project:</b>	NATURAL DAM-NORTH STRS. & APPRS (S)		
<b>County:</b>	<b>Code:</b> 17	<b>Name:</b>	CRAWFORD
<b>Sampled By:</b>	FAULKNER	<b>Depth:</b>	0-5
<b>Lab No.:</b>	20141217	<b>AASHTO Class:</b>	A-6(6)
<b>Sample ID:</b>	RV385	<b>Material Type (1 or 2):</b>	2
<b>LATITUDE:</b>		<b>LONGITUDE:</b>	

$$M_R = K_1 (S_c)^{K_2} (S_3)^{K_5}$$

$K_1 = \underline{12,227}$   
 $K_2 = \underline{-0.25857}$   
 $K_5 = \underline{0.26469}$   
 $R^2 = \underline{0.94}$



**ARKANSAS STATE HIGHWAY AND TRANSPORTATION DEPARTMENT  
MATERIALS DIVISION**

**AASHTO T 307-99 - RESILIENT MODULUS OF SUBGRADE SOILS  
RECOMPACTED SAMPLES**

<b>Job No.</b>	040623	<b>Material Code</b>	SSRVPS
<b>Date Sampled:</b>	5/9/14	<b>Station No.:</b>	201+00
<b>Date Tested:</b>	May 9, 2014	<b>Location:</b>	20'RT
<b>Name of Project:</b>	NATURAL DAM-NORTH STRS. & APPRS(S)		
<b>County:</b>	<b>Code:</b> 17	<b>Name:</b>	CRAWFORD
<b>Sampled By:</b>	FAULKNER	<b>Depth:</b>	0-5
<b>Lab No.:</b>	20141218	<b>AASHTO Class:</b>	A-4(0)
<b>Sample ID:</b>	RV386	<b>Material Type (1 or 2):</b>	2
<b>LATITUDE:</b>		<b>LONGITUDE:</b>	

**1. Testing Information:**

Preconditioning - Permanent Strain > 5% (Y=Yes or N= No)	N
Testing - Permanent Strain > 5% (Y=Yes or N=No)	N
Number of Load Sequences Completed (0-15)	15

**2. Specimen Information:**

Specimen Diameter (in):	
Top	3.94
Middle	3.93
Bottom	3.95
Average	3.94
Membrane Thickness (in):	0.01
Height of Specimen, Cap and Base (in):	8.04
Height of Cap and Base (in):	0.00
Initial Length, Lo (in):	8.04
Initial Area, Ao (sq. in):	12.12
Initial Volume, AoLo (cu. in):	97.43

**3. Soil Specimen Weight:**

Weight of Wet Soil Used (g):	3252.20
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**4. Soil Properties:**

Optimum Moisture Content (%):	11.2
Maximum Dry Density (pcf):	117.7
95% of MDD (pcf):	111.8
In-Situ Moisture Content (%):	N/A

**5. Specimen Properties:**

Wet Weight (g):	3252.20
Compaction Moisture content (%):	11.0
Compaction Wet Density (pcf):	127.19
Compaction Dry Density (pcf):	114.58
Moisture Content After Mr Test (%):	10.9

**6. Quick Shear Test (Y=Yes, N=No, N/A=Not Applicable):**

#VALUE!

**7. Resilient Modulus, Mr:**

8486(Sc)^-0.16851(S3)^0.50151

**8. Comments**

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**9. Tested By:**

M.W.

**Date:** May 9, 2014

**ARKANSAS STATE HIGHWAY AND TRANSPORTATION DEPARTMENT  
MATERIALS DIVISION**

**AASHTO T 307-99 - RESILIENT MODULUS OF SUBGRADE SOILS  
RECOMPACTED SAMPLES**

**Job No.** 040623      **Material Code** SSRVPS  
**Date Sampled:** 5/9/14      **Station No.:** 201+00  
**Date Tested:** May 9, 2014      **Location:** 20RT  
**Name of Project:** NATURAL DAM-NORTH STRS. & APPRS(S)  
**County:** Code: 17      **Name:** CRAWFORD  
**Sampled By:** FAULKNER      **Depth:** 0-5  
**Lab No.:** 20141218      **AASHTO Class:** A-4(0)  
**Sample ID:** RV386      **Material Type (1 or 2):** 2  
**LATITUDE:**      **LONGITUDE:**

PARAMETER	Chamber Confining Pressure		Nominal Maximum Axial Stress		Actual Applied Max. Axial Load		Actual Applied Cyclic Load		Actual Applied Contact Load		Actual Applied Max. Axial Stress		Actual Applied Cyclic Stress		Actual Applied Contact Stress		Average Recov Def. LVD1 and 2		Resilient Strain		Resilient Modulus	
	S <sub>g</sub>	psi	S <sub>cyclic</sub>	psi	P <sub>max</sub>	lbs	P <sub>cyclic</sub>	lbs	P <sub>contact</sub>	lbs	S <sub>max</sub>	psi	S <sub>cyclic</sub>	psi	S <sub>contact</sub>	psi	H <sub>avg</sub>	in	ε <sub>r</sub>	in/in	M <sub>r</sub>	psi
Sequence 1	6.0	2.0	2.0	25.2	22.7	22.7	2.6	2.6	2.1	1.9	0.2	0.0070	0.0009	25,249								
Sequence 2	6.0	4.0	4.0	47.1	44.6	44.6	2.5	2.5	3.9	3.7	0.2	0.0183	0.0023	16,144								
Sequence 3	6.0	6.0	6.0	69.3	65.9	65.9	3.4	3.4	5.7	5.4	0.3	0.0283	0.0035	15,425								
Sequence 4	6.0	8.0	8.0	92.7	86.9	86.9	5.8	5.8	7.7	7.2	0.5	0.0403	0.0050	14,297								
Sequence 5	6.0	10.0	10.0	116.4	108.0	108.0	8.3	8.3	9.6	8.9	0.7	0.0516	0.0064	13,886								
Sequence 6	4.0	2.0	2.0	24.7	22.1	22.1	2.6	2.6	2.0	1.8	0.2	0.0106	0.0013	13,839								
Sequence 7	4.0	4.0	4.0	45.8	43.2	43.2	2.7	2.7	3.8	3.6	0.2	0.0227	0.0028	12,598								
Sequence 8	4.0	6.0	6.0	67.2	64.5	64.5	2.7	2.7	5.5	5.3	0.2	0.0357	0.0044	11,983								
Sequence 9	4.0	8.0	8.0	90.9	85.8	85.8	5.1	5.1	7.5	7.1	0.4	0.0477	0.0059	11,944								
Sequence 10	4.0	10.0	10.0	114.4	106.9	106.9	7.5	7.5	9.4	8.8	0.6	0.0588	0.0073	12,067								
Sequence 11	2.0	2.0	2.0	24.1	21.4	21.4	2.7	2.7	2.0	1.8	0.2	0.0139	0.0017	10,220								
Sequence 12	2.0	4.0	4.0	44.6	41.8	41.8	2.8	2.8	3.7	3.4	0.2	0.0295	0.0037	9,382								
Sequence 13	2.0	6.0	6.0	65.1	62.2	62.2	2.8	2.8	5.4	5.1	0.2	0.0459	0.0057	9,006								
Sequence 14	2.0	8.0	8.0	87.4	83.1	83.1	4.4	4.4	7.2	6.9	0.4	0.0601	0.0075	9,173								
Sequence 15	2.0	10.0	10.0	110.7	103.9	103.9	6.7	6.7	9.1	8.6	0.6	0.0704	0.0088	9,791								

**TESTED BY** \_\_\_\_\_ **DATE** May 9, 2014  
**REVIEWED BY** \_\_\_\_\_ **DATE** \_\_\_\_\_

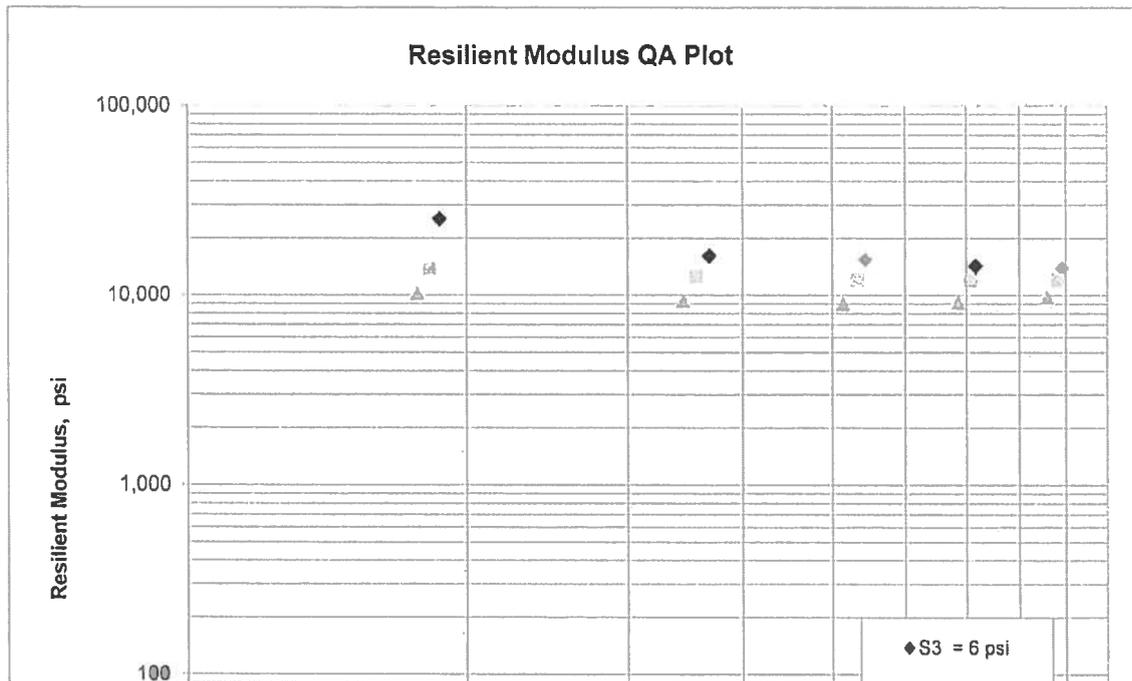
**ARKANSAS STATE HIGHWAY AND TRANSPORTATION DEPARTMENT  
MATERIALS DIVISION**

**AASHTO T 307-99 - RESILIENT MODULUS OF SUBGRADE SOILS  
RECOMPACTED / THINWALL TUBE SAMPLES**

<b>Job No.</b>	040623	<b>Material Code</b>	SSRVPS
<b>Date Sampled:</b>	5/9/14	<b>Station No.:</b>	201+00
<b>Date Tested:</b>	May 9, 2014	<b>Location:</b>	20'RT
<b>Name of Project:</b>	NATURAL DAM-NORTH STRS. & APPRS(S)		
<b>County:</b>	<b>Code:</b> 17	<b>Name:</b>	CRAWFORD
<b>Sampled By:</b>	FAULKNER	<b>Depth:</b>	0-5
<b>Lab No.:</b>	20141218	<b>AASHTO Class:</b>	A-4(0)
<b>Sample ID:</b>	RV386	<b>Material Type (1 or 2):</b>	2
<b>LATITUDE:</b>		<b>LONGITUDE:</b>	

$$M_R = K_1 (S_c)^{K_2} (S_3)^{K_5}$$

K1 = 8,486  
 K2 = -0.16851  
 K5 = 0.50151  
 R<sup>2</sup> = 0.85





ARKANSAS STATE HIGHWAY AND TRANSPORTATION DEPARTMENT - LITTLE ROCK, ARKANSAS  
MATERIALS DIVISION

MICHAEL BENSON, MATERIALS ENGINEER

\*\*\* SOIL SURVEY / PAVEMENT SOUNDING TEST REPORT \*\*\*

DATE - 04/24/14 SEQUENCE NO. - 2  
 JOB NUMBER - 040623 MATERIAL CODE - SSRVPS  
 FEDERAL AID NO. - TO BE ASSIGNED SPEC. YEAR - 2003  
 PURPOSE - SOIL SURVEY SAMPLE SUPPLIER ID. - 1  
 SPEC. REMARKS - NO SPECIFICATION CHECK COUNTY/STATE - 17  
 SUPPLIER NAME - STATE DISTRICT NO. - 04  
 NAME OF PROJECT - NATURAL DAM - NORTH STRS. & APPRS. (S)  
 PROJECT ENGINEER - NOT APPLICABLE  
 PIT/QUARRY - ARKANSAS  
 LOCATION - CRAWFORD, COUNTY DATE SAMPLED - 04/08/14  
 SAMPLED BY - S.FAULKNER DATE RECEIVED - 04/11/14  
 SAMPLE FROM - TEST HOLE DATE TESTED - 04/22/14  
 MATERIAL DESC. - SOIL SURVEY - R VALUE- PAVEMENT SOUNDINGS

LAB NUMBER	20141207	20141208	20141209
SAMPLE ID	S375	S376	S377
TEST STATUS	INFORMATION ONLY	INFORMATION ONLY	INFORMATION ONLY
STATION	110+00	110+00	110+00
LOCATION	06LT	14LT	20LT
DEPTH IN FEET	0-5	0-5	0-5
MAT'L COLOR	BR/GR	BR/GR	BR/GR
MAT'L TYPE	-	-	-
LATITUDE DEG-MIN-SEC	35 39 25.30	35 39 25.40	35 39 25.40
LONGITUDE DEG-MIN-SEC	94 23 57.20	94 23 57.20	94 23 57.10
% PASSING			
2 IN.	-	-	-
1 1/2 IN.	100	-	-
3/4 IN.	95	-	-
3/8 IN.	90	100	100
NO. 4	81	94	95
NO. 10	72	85	88
NO. 40	63	75	82
NO. 80	58	71	79
NO. 200	43	58	74
LIQUID LIMIT	27	27	32
PLASTICITY INDEX	12	12	15
AASHTO SOIL	A-6(2)	A-6(4)	A-6(9)
UNIFIED SOIL	-	-	-
% MOISTURE CONTENT	9.3	9.3	15.1
ACHMSC (IN)	7.0W	3.0	--
AGG.BASE CRS CL-7 (IN)	5.0	6.0	--
-	-	-	-
-	-	-	-
-	-	-	-
-	-	-	-
-	-	-	-
-	-	-	-
-	-	-	-
-	-	-	-
-	-	-	-

REMARKS - LOCATIONS MEASURED FROM C.L. OF CONSTRUCTION, W=MULTIPLE LAYERS,  
 - Z=AUGER REFUSAL

AASHTO TESTS : T24 T88 T89 T90 T265









JOB: 040623

Arkansas State Highway Transportation Department

JOB NAME: NATURAL DAM - NORTH STRS. & APPRS. (S)

Materials Division

COUNTY NO. 17 DATE TESTED 4/22/2014

Michael Benson, Materials Engineer

STA.#	LOC.	DEPTH	COLOR	#4	#10	#40	#80	#200	L.L.	P.I.	SOIL CLASS	LAB #:	%MOISTURE
				S	I	E	P	E					
110+00	20LT	0-5	BR/GR	75	70	65	64	57	33	15	A-6(6)	RV385	
201+00	20RT	0-5	BR/GR	61	58	55	51	39	21	04	A-4(0)	RV386	
102+00	04RT	0-3Z	BROWN	96	90	84	79	35	21	04	A-2-4(0)	S372	22.5
102+00	13RT	0-3Z	BROWN	97	88	83	79	32	ND	NP	A-2-4(0)	S373	23.3
102+00	22RT	0-2.5Z	BROWN	96	90	80	74	32	ND	NP	A-2-4(0)	S374	22.6
110+00	06LT	0-5	BR/GR	81	72	63	58	43	27	12	A-6(2)	S375	9.3
110+00	14LT	0-5	BR/GR	94	85	75	71	58	27	12	A-6(4)	S376	9.3
110+00	20LT	0-5	BR/GR	95	88	82	79	74	32	15	A-6(9)	S377	15.1
201+00	05RT	0-5	BR/GR	92	83	76	68	50	17	03	A-4(0)	S378	12.8
201+00	13RT	0-5	BR/GR	93	86	79	71	52	ND	NP	A-4(0)	S379	14.6
201+00	20RT	0-5	BR/GR	98	95	91	85	69	20	05	A-4(1)	S380	21.4
223+00	CL	0-5	BROWN	99	99	98	82	48	22	08	A-4(1)	S381	16.8
228+00	05LT	0-5	BROWN	99	96	90	74	46	21	08	A-4(1)	S382	15.5
228+00	14LT	0-5	BROWN	99	97	93	78	50	21	07	A-4(1)	S383	15.7
228+00	22LT	0-5	BROWN	97	95	91	75	46	21	06	A-4(0)	S384	15.2

comments: LOCATIONS MEASURED FROM C.L. OF CONSTRUCTION, W=MULTIPLE LAYERS  
Z=AUGER REFUSAL

Monday, May 12, 2014

**JOB:** 040623

*Arkansas State Highway Transportation Department*

*DATE TESTED*

**JOB NAME:** NATURAL DAM - NORTH STRS. & APPRS. (S)

*Materials Division*

4/22/2014

**COUNTY NO.** 17

*Michael Benson, Materials Engineer*

**STA.# LOC.**

**PAVEMENT SOUNDINGS**

102+00	04RT	ACHMSC	AGG.BASE CRS CL-7
		5.75W	5.0
102+00	13RT	ACHMSC	AGG.BASE CRS CL-7
		4.0W	5.0
102+00	22RT	ACHMSC	AGG.BASE CRS CL-7
		--	--
110+00	06LT	ACHMSC	AGG.BASE CRS CL-7
		7.0W	5.0
110+00	14LT	ACHMSC	AGG.BASE CRS CL-7
		3.0	6.0
110+00	20LT	ACHMSC	AGG.BASE CRS CL-7
		--	--
201+00	05RT	ACHMSC	AGG.BASE CRS CL-7
		5.5W	8.0
201+00	13RT	ACHMSC	AGG.BASE CRS CL-7
		3.75W	8.0
201+00	20RT	ACHMSC	AGG.BASE CRS CL-7
		--	--
223+00	CL	ACHMSC	AGG.BASE CRS CL-7
		--	--
228+00	05LT	ACHMSC	AGG.BASE CRS CL-7
		7.25W	7.0
228+00	14LT	ACHMSC	AGG.BASE CRS CL-7
		5.0W	5.0
228+00	22LT	ACHMSC	AGG.BASE CRS CL-7
		--	--

**comments:** LOCATIONS MEASURED FROM C.L. OF CONSTRUCTION, W=MULTIPLE LAY  
Z=AUGER REFUSAL

**ARKANSAS STATE HIGHWAY AND TRANSPORTATION DEPARTMENT**

November 20, 2014

**TO:** Mr. Rick Ellis, Bridge Engineer

**SUBJECT:** Job No. 040623  
Natural Dam – North Strs. & Apprs. (S)  
Route 59 Section 5  
Crawford County

Transmitted herewith are a brief summary of the geology and site conditions, D50 analysis test results, unconfined compressive strength results, and the logs of the borings conducted for the structures and approaches of the above referenced project. The samples obtained by the Standard Penetration Tests were brought to the laboratory and visually classified by experienced lab personnel to confirm the field identifications. The rock cores are available for inspection at the Materials Division.

Based on the depth at which bedrock was encountered, it is anticipated that all bents will be founded on piling. Preboring may be necessary in order to achieve minimum penetration requirements. However, spread footings may be considered and should be sized based on the values provided in Table 1.

TABLE 1 – Bearing Capacity Recommendations for Interior bents

Foundation Description	Nominal Bearing Resistance (ksf)	Resistance Factor	Factored Bearing Resistance (ksf)
Spread Footings	24.8	0.45	11

If you have any questions concerning these recommendations, please contact the Geotechnical Section.

  
Michael C. Benson  
Materials Engineer

MCB:rpt

cc: State Construction Engineer - Master File Copy  
District 4 Engineer  
G.C. File

## **GEOLOGY AND SITE CONDITIONS**

**Job No. 040623**

### **Natural Dam – North Strs. & Apprs. (S)**

**Crawford County**

**Route 59 Section 5**

#### **Site Conditions**

The existing bridge crosses over Lee Creek and consists of 8 spans. The bents are composed of concrete wall piers on footings. The deck is constructed of concrete supported by 5 sets of steel beams, except for the northern two spans which are supported by steel trusses. The guardrail is composed of steel on the truss supported part of the bridge, and concrete on the rest of the bridge, with both steel with wooden posts leading up to the bridge. The north end of the bridge sits on a sandstone bluff. A plastic water line runs along the east side of the bridge and is buried north and south of the bridge parallel to the existing bridge. A buried telecommunication line parallels the east side of the roadway. Metal utility pipes run along the west side of the roadway. Power lines parallel the west side of the bridge, then cross the roadway at a diagonal north of the bridge. An intersecting set of power lines crosses the roadway just to the north of the bridge.

Leek Creek flows in a westerly direction. The channel banks are thickly lined with trees and brush, with pastureland beyond on the south and residences beyond (both sides of the roadway) on the north side of the channel. Sandstone is exposed on the east side of the roadway, on the north end of the bridge.

#### **Site Geology**

The project alignment is located in the mapped outcrop of the Atoka Formation (Pa). The Atoka Formation is a sequence of marine, mostly tan to gray silty sandstones and grayish-black shales with some rare calcareous beds and siliceous shales. Locally, the Atoka contains discontinuous streaks of coal and coaly shale. The unit may reach up to 25,000 feet thick in the Ouachita Mountains; however, north of the Arkansas Valley, the thickness decreases rapidly and only in the southern Boston Mountains does the Atoka attain thicknesses greater than 1,500 feet. There are numerous northeast-southwest and east-west trending normal faults in the area. A fault is mapped just to the south of the existing bridge. There may be other unmapped faults in the area. On the south side of the channel, bedrock was encountered at depths of 8.3 to 19.0 feet below ground level.

## Subsurface Conditions

Based on the results of the boring at station 213+39, the subsurface stratigraphy may be generalized as follows:

- 0 to 3.7 Feet: Consists of moist, very dense, brown **sand with gravel (sandstone fragments)**.
- 3.7 to 7.4 Feet: Consists of brown and dark gray, slightly weathered, well cemented **sandstone** with fractured layers.
- 7.4 to 18.2 Feet: Consists of dark gray and brown, medium hard **shale with frequent weathered layers**.
- 18.2 to 23 Feet: Consists of interbedded gray, slightly weathered, cemented **sandstone** and brown and dark gray, weathered, medium hard **shale**.
- 23 to 27.4 Feet: Consists of brown and dark gray, slightly weathered, well cemented **sandstone with shale partings**.

Based on the results of the borings at stations 214+83 to 219+44, the subsurface stratigraphy may be generalized as follows:

- 0 to 8.3 Feet: Consists of moist to wet, medium dense to very dense, brown **sand with gravel (sandstone fragments) to sandstone cobbles and boulders**.
- 8.3 to 19 Feet: Varies from moist to wet, medium dense to very dense, brown **sand with gravel (sandstone fragments) to sandstone cobbles and boulders** to slightly weathered, medium hard **shale** to gray, slightly weathered, cemented **sandstone**.
- 19 to 43.6 Feet: Consists of gray, slightly weathered, cemented **sandstone to sandstone with dark gray shale partings**.

**D<sub>50</sub> AGGREGATE ANALYSIS  
FOR SCOUR CALCULATIONS**

**Job No. 040623**

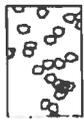
<b>Creek Name</b>	<b>Station</b>	<b>Sample Type</b>	<b>Location</b>	<b>Depth (FT)</b>	<b>Aggregate Size (D50) (IN)</b>
Lee Creek	217+08	Creek Bank	C.L. Construction	N/A	0.110



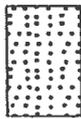
# LEGEND

## SOIL TYPES

(SHOWN IN SYMBOL COLUMN)  
(PREDOMINANT TYPE SHOWN HEAVY)



GRAVEL



SAND



SILT



CLAY



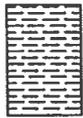
ORGANIC  
MATTER

## ROCK TYPES

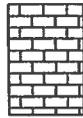
(SHOWN IN SYMBOL COLUMN)



SANDSTONE



SHALE  
or  
SILTSTONE



LIMESTONE  
or  
DOLOMITE



ALTERNATING  
LAYERS of  
SHALE and  
SANDSTONE



OTHER

## SAMPLER TYPES

(SHOWN IN SAMPLE COLUMN)

### SHELBY TUBE



UNDISTURBED  
SAMPLE  
RECOVERY



DISTURBED  
SAMPLE  
RECOVERY



NO  
RECOVERY

### SPLIT SPOON

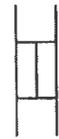


SAMPLE  
RECOVERY



NO  
RECOVERY

### ROCK CORING



% RECOVERY  
INDICATED ON LOGS

## TERMS DESCRIBING CONSISTENCY OR CONDITION

GRANULAR SOIL		CLAY		CLAY-SHALE		SHALE	
'N' Value	Density	'N' Value	Consistency	'N' Value	Consistency	'N' Value	Consistency
0-4	Very Loose	0-1	Very Soft	0-1	Very Soft		
5-10	Loose	2-4	Soft	2-4	Soft	31-60	Soft
11-30	Medium Dense	5-8	Medium Stiff	5-8	Medium Stiff	Over 60	
31-50	Dense	9-15	Stiff	9-15	Stiff	More than 2'	
Over 50	Very Dense	16-30	Very Stiff	16-30	Very Stiff	Penetration	
		31-60	Hard	31-60	Hard	in 60 Blows	Medium Hard
		Over 60	Very Hard	Over 60	Very Hard	Less than 2'	
						Penetration	
						in 60 Blows	Hard

1. Ground water elevations indicated on boring logs represent ground water elevations at date or time shown on boring log. Absence of water surface implies that no ground water data is available but does not necessarily mean that ground water will not be encountered at locations or within the vertical reaches of these borings.
2. Borings represent subsurface conditions at their respective locations for their respective depths. Variations in conditions between or adjacent to boring locations may be encountered.
3. Terms used for describing soils according to their texture or grain size distribution are in accordance with the Unified Soil Classification System.

Standard Penetration Test – Driving a 2.0" O.D., 1-3/8" I.D. sampler a distance of 1.0 foot into undisturbed soil with a 140 pound hammer free falling a distance of 30 inches. It is customary to drive the spoon 6.0 inches to seat into undisturbed soil, then perform the test. The number of hammer blows for seating the spoon and performing the test are recorded for each 6 inches of penetration on the drill log. The field "N" Value ( $N_f$ ) can be obtained by adding the bottom two numbers for example:  $\frac{6}{8-9} \Rightarrow 8+9 = 17 \text{ blows/ft}$ . The "N" Value corrected to 60% efficiency ( $N_{60}$ ) can be obtained by multiplying  $N_f$  by the hammer correction factor published on the boring log.

ARKANSAS HWY. & TRANS. DEPARTMENT MATERIALS DIVISION - GEOTECHNICAL SEC.				BORING NO. 1 PAGE 1 OF 1								
JOB NO. 040623		Crawford County		DATE: October 13, 2014		TYPE OF DRILLING: Hollow Stem Auger & Diamond Coring						
JOB NAME: Natural Dam - North Strs. & Apprs.		U.S. 59		EQUIPMENT: CME 75 w/ CME Automatic Hammer		HAMMER CORRECTION FACTOR: 1.37						
STATION: 213+39		LOCATION: 10' Left of Center Line of Construction										
LOGGED BY: Tracy Henderson		COMPLETION DEPTH: 27.4										
DEPTH FT.	SYMBOL	SAMPLES	DESCRIPTION OF MATERIAL	SOIL GROUP	PLASTIC LIMIT	% MOIST.	LIQUID LIMIT	DRY WEIGHT	LBS PER CU.FT.	NO. OF BLOWS PER 6-IN.	% S C R	% R Q D
			SURFACE ELEVATION: 671.7									
			Moist, Very Dense, Brown Sand with Gravel (Sandstone Fragments)									
5			SANDSTONE - Brown and Dark Gray, Thin Bedded, Slightly Weathered, Well-Cemented, with Slight Dip and Fractured Layers							60 (1")	94	0
10			SHALE WITH FREQUENT WEATHERED SHALE LAYERS - Dark Gray and Brown, Laminated, Medium Hard, with Slight Dip								76	10
15			SHALE - Brown and Dark Gray, Laminated, Highly Weathered, Medium Hard, with Slight Dip								100	32
20			SANDSTONE WITH OCCASIONAL DARK GRAY WEATHERED SHALE LAYERS - Gray, Medium Bedded, Slightly Weathered, Cemented, with Slight Dip								100	28
			SHALE - Brown and Dark Gray, Laminated, Weathered, Medium Hard, with Slight Dip									
25			SANDSTONE WITH DARK GRAY SHALE PARTINGS - Brown and Dark Gray, Thick Bedded, Slightly Weathered, Well-Cemented, with Slight Dip								100	68
30			Boring Terminated									
35												
REMARKS:												

**ARKANSAS HWY. & TRANS. DEPARTMENT  
MATERIALS DIVISION - GEOTECHNICAL SEC.**

BORING NO. 2  
PAGE 1 OF 1

JOB NO. 040623 Crawford County  
JOB NAME: Natural Dam - North Strs. & Apprs.  
U.S. 59  
STATION: 214+83  
LOCATION: Center Line of Construction  
LOGGED BY: David Allen

DATE: October 27, 2014  
TYPE OF DRILLING: Hollow Stem Auger &  
Diamond Coring  
EQUIPMENT: CME 850 w/ CME  
Automatic Hammer  
HAMMER CORRECTION FACTOR: 1.23

COMPLETION DEPTH: 29.8

DEPTH FT.	SYMBOL	SAMPLES	DESCRIPTION OF MATERIAL	SOIL GROUP	PLASTIC LIMIT	% MOIST.	LIQUID LIMIT	DRY WEIGHT	LBS PER CU.FT.	NO. OF BLOWS PER 6-IN.	% S C R	% R O D
			SURFACE ELEVATION: 636.3									
5			Wet, Medium Dense, Brown Sand with Gravel (Sandstone Fragments)							6 8-12		
10			Wet, Very Dense, Brown Sand with Gravel (Sandstone Fragments) Sandstone Gravel and Cobbles							44 60 (2")	78	0
15			SHALE - Dark Gray, Laminated, Slightly Weathered, Medium Hard, with Slight Dip SANDSTONE WITH DARK GRAY SHALE SEAMS AND PARTINGS - Gray, Medium Bedded, Slightly Weathered, Cemented, with Slight Dip								88	22
20			SANDSTONE - Gray, Medium Bedded, Slightly Weathered, Cemented, with Slight Dip								100	70
25			SANDSTONE WITH DARK GRAY SHALE PARTINGS - Gray, Thin Bedded, Slightly Weathered, Cemented, with Slight Dip and Fractured Layers								100	0
30			SANDSTONE WITH DARK GRAY SHALE PARTINGS AND OCCASIONAL LAYERS - Gray, Medium Bedded, Slightly Weathered, Cemented, with Slight Dip								100	18
35			Boring Terminated									

REMARKS:

**ARKANSAS HWY. & TRANS. DEPARTMENT  
MATERIALS DIVISION - GEOTECHNICAL SEC.**

BORING NO. 3  
PAGE 1 OF 1

JOB NO. 040623 Crawford County  
JOB NAME: Natural Dam - North Strs. & Apprs.  
U.S. 59  
STATION: 216+10  
LOCATION: 8' Right of Center Line of Construction  
LOGGED BY: David Allen

DATE: October 28, 2014  
TYPE OF DRILLING: Hollow Stem Auger &  
Diamond Coring  
EQUIPMENT: CME 850 w/ CME  
Automatic Hammer  
HAMMER CORRECTION FACTOR: 1.23

COMPLETION DEPTH: 33.1

DEPTH FT.	SYMBOL	SAMPLES	DESCRIPTION OF MATERIAL	SOIL GROUP	PLASTIC LIMIT	% MOIST.	LIQUID LIMIT	DRY WEIGHT	LBS PER CU.FT.	NO. OF BLOWS PER 6-IN.	% S C R	% R O D
			SURFACE ELEVATION: 637.5									
5		X	Sandstone Gravel							6 7.4		
			SHALE - Gray and Brown, Weathered, Soft									
10			Sandstone Gravel and Cobbles							60 (.01")	100	71
15			SANDSTONE WITH DARK GRAY SHALE PARTINGS - Gray, Medium Bedded, Slightly Weathered, Cemented, with Slight Dip								92	62
20											100	52
25											100	32
30											98	68
35			Boring Terminated									

REMARKS:

ARKANSAS HWY. & TRANS. DEPARTMENT MATERIALS DIVISION - GEOTECHNICAL SEC.				BORING NO. 4 PAGE 1 OF 1								
JOB NO. 040623		Crawford County		DATE: October 22, 2014								
JOB NAME: Natural Dam - North Strs. & Apprs. U.S. 59				TYPE OF DRILLING: Hollow Stem Auger & Diamond Coring								
STATION: 217+08				EQUIPMENT: CME 850 w/ CME Automatic Hammer								
LOCATION: 1' Left of Center Line of Construction				HAMMER CORRECTION FACTOR: 1.23								
LOGGED BY: David Allen												
COMPLETION DEPTH: 28.5												
DEPTH FT.	SYMBOL	SAMPLES	DESCRIPTION OF MATERIAL	SOIL GROUP	PLASTIC LIMIT	% MOIST.	LIQUID LIMIT	DRY WEIGHT	LBS PER CU.FT.	NO. OF BLOWS PER 6-IN.	% S C R	% R O D
			SURFACE ELEVATION: 636.0									
5			Wet, Very Dense, Brown Sand with Gravel (Sandstone Fragments)							60 (2")		
10			SANDSTONE WITH DARK GRAY SHALE PARTINGS - Gray, Medium Bedded, Slightly Weathered, Cemented, with Slight Dip							30 (.01")	95	38
15			SANDSTONE WITH OCCASIONAL POORLY- CEMENTED LAYERS - Gray, Medium Bedded, Slightly Weathered, Cemented, with Slight Dip								96	0
20			SANDSTONE WITH DARK GRAY SHALE PARTINGS - Gray, Medium Bedded, Slightly Weathered, Cemented, with Slight Dip								100	48
25			SANDSTONE WITH DARK GRAY SHALE PARTINGS - Gray, Thick Bedded, Slightly Weathered, Cemented, with Slight Dip								100	58
30			Boring Terminated									
35												
REMARKS:												

ARKANSAS HWY. & TRANS. DEPARTMENT MATERIALS DIVISION - GEOTECHNICAL SEC.				BORING NO. 5 PAGE 1 OF 2								
JOB NO. 040623		Crawford County		DATE: October 21, 2014		TYPE OF DRILLING: Hollow Stem Auger & Diamond Coring						
JOB NAME: Natural Dam - North Strs. & Apprs. U.S. 59		STATION: 218+00		EQUIPMENT: CME 850 w/ CME Automatic Hammer		HAMMER CORRECTION FACTOR: 1.23						
LOCATION: 21' Right of Center Line of Construction		LOGGED BY: David Allen		COMPLETION DEPTH: 43.6								
DEPTH FT.	SYMBOL	SAMPLES	DESCRIPTION OF MATERIAL	SOIL GROUP	PLASTIC LIMIT	% MOIST.	LIQUID LIMIT	DRY WEIGHT	LBS PER CU.FT.	NO. OF BLOWS PER 6-IN.	% S C R	% R O D
			SURFACE ELEVATION: 642.1									
5		X	Moist, Dense, Brown Sand with Gravel (Sandstone Fragments) and some Clay							10 9-22		
10		X	Wet, Very Dense, Brown Sand with Gravel (Sandstone Fragments) % *							60 (5")		
15			Sandstone Gravel, Cobbles and Boulders								32	12
20			SANDSTONE WITH OCCASIONAL POORLY-CEMENTED LAYERS - Gray, Medium Bedded, Slightly Weathered, Cemented, with Slight Dip and Fractured Layers *								30+	0
25											76	24
30											38#	10
35			SANDSTONE WITH DARK GRAY SHALE PARTINGS - Gray, Medium Bedded, Slightly Weathered, Cemented, with Slight Dip *									
REMARKS: % Water stratum at 9.6'. * Total water loss was encountered from 10.9' to 23.6'. + Poor core recovery due to gravel layer. # Pore core recovery due to inner core barrel not locking in.												

ARKANSAS HWY. & TRANS. DEPARTMENT MATERIALS DIVISION - GEOTECHNICAL SEC.				BORING NO. 5 PAGE 2 OF 2								
JOB NO. 040623 Crawford County		DATE: October 21, 2014		TYPE OF DRILLING: Hollow Stem Auger & Diamond Coring								
JOB NAME: Natural Dam - North Strs. & Apprs. U.S. 59		EQUIPMENT: CME 850 w/ CME Automatic Hammer		HAMMER CORRECTION FACTOR: 1.23								
STATION: 218+00		LOGGED BY: David Allen		COMPLETION DEPTH: 43.6								
LOCATION: 21' Right of Center Line of Construction												
DEPTH FT.	SYMBOL	SAMPLES	DESCRIPTION OF MATERIAL	SOIL GROUP	PLASTIC LIMIT	% MOIST.	LIQUID LIMIT	DRY WEIGHT	LBS PER CU.FT.	NO. OF BLOWS PER 6-IN.	% S C R	% R Q D
			SURFACE ELEVATION: 642.1									
40											100	66
											100	54
45			Boring Terminated									
50												
55												
60												
65												
70												
REMARKS: % Water stratum at 9.6'. * Total water loss was encountered from 10.9' to 23.6'. + Poor core recovery due to gravel layer. # Pore core recovery due to inner core barrel not locking in.												

**ARKANSAS HWY. & TRANS. DEPARTMENT  
MATERIALS DIVISION - GEOTECHNICAL SEC.**

BORING NO. 5A  
PAGE 1 OF 1

JOB NO. 040623 Crawford County  
JOB NAME: Natural Dam - North Strs. & Apprs.  
U.S. 59  
STATION: 218+00  
LOCATION: 42' Right of Center Line of Construction  
LOGGED BY: David Allen

DATE: October 28-29, 2014  
TYPE OF DRILLING: Hollow Stem Auger &  
Diamond Coring  
EQUIPMENT: CME 850 w/ CME  
Automatic Hammer  
HAMMER CORRECTION FACTOR: 1.23

COMPLETION DEPTH: 33.3

DEPTH FT.	SYMBOL	SAMPLES	DESCRIPTION OF MATERIAL	SOIL GROUP	PLASTIC LIMIT	% MOIST.	LIQUID LIMIT	DRY WEIGHT	LBS PER CU.FT.	NO. OF BLOWS PER 6-IN.	% S C R	% R Q D
			SURFACE ELEVATION: 642.3									
5			Moist, Medium Dense, Brown Sand with Gravel (Sandstone Fragments)							7 8-8		
10			Wet, Very Dense, Brown Sand with Gravel (Sandstone Fragments)							60 (1")	69	50
			SANDSTONE WITH OCCASIONAL POORLY-CEMENTED LAYERS - Gray, Medium Bedded, Slightly Weathered, Cemented, with Slight Dip *									
15			SANDSTONE WITH POORLY-CEMENTED LAYERS AND OCCASIONAL DARK GRAY SHALE SEAMS - Gray, Thick Bedded, Slightly Weathered, Cemented, with Slight Dip								100	34
20			SANDSTONE WITH OCCASIONAL POORLY-CEMENTED LAYERS AND SHALE PARTINGS - Gray, Thin Bedded, Slightly Weathered, Cemented, with Slight Dip								100	0
25			SANDSTONE WITH DARK GRAY SHALE PARTINGS - Gray, Thick Bedded, Slightly Weathered, Cemented, with Slight Dip								100	54
30			SANDSTONE WITH DARK GRAY SHALE SEAMS - Light Gray, Medium Bedded, Slightly Weathered, Cemented, with Slight Dip								100	86
35			Boring Terminated									

REMARKS: \* Total water loss was encountered at 11.0'.

**ARKANSAS HWY. & TRANS. DEPARTMENT  
MATERIALS DIVISION - GEOTECHNICAL SEC.**

BORING NO. 6  
PAGE 1 OF 1

JOB NO. 040623 Crawford County  
JOB NAME: Natural Dam - North Strs. & Apprs.  
U.S. 59  
STATION: 218+86  
LOCATION: 8' Left of Center Line of Construction  
LOGGED BY: David Allen

DATE: October 20, 2014  
TYPE OF DRILLING: Hollow Stem Auger &  
Diamond Coring  
EQUIPMENT: CME 850 w/ CME  
Automatic Hammer  
HAMMER CORRECTION FACTOR: 1.23

COMPLETION DEPTH: 33.1

DEPTH FT.	SYMBOL	SAMPLES	DESCRIPTION OF MATERIAL	SOIL GROUP	PLASTIC LIMIT	% MOIST.	LIQUID LIMIT	DRY WEIGHT	LBS PER CU.FT.	NO. OF BLOWS PER 6-IN.	% S C R	% R Q D
			SURFACE ELEVATION: 643.4									
			Moist, Medium Dense, Brown Sand with some Clay									
5		X	Moist, Medium Dense, Brown Sand with Gravel (Sandstone Fragments)							10 12-12		
10		X	Gravel (Sandstone Fragments) with some Sandy Clay							60 (4")		
			SANDSTONE WITH DARK GRAY SHALE PARTINGS - Gray, Medium Bedded, Slightly Weathered, Cemented, with Slight Dip								84	60
15			SANDSTONE WITH DARK GRAY SHALE PARTINGS - Gray, Thick Bedded, Slightly Weathered, Cemented, with Slight Dip								100	81
20											100	78
25											100	96
30											98	92
35			Boring Terminated									

REMARKS:

**ARKANSAS HWY. & TRANS. DEPARTMENT  
MATERIALS DIVISION - GEOTECHNICAL SEC.**

BORING NO. 7  
PAGE 1 OF 2

JOB NO. 040623 Crawford County  
JOB NAME: Natural Dam - North Strs. & Apprs.  
U.S. 59  
STATION: 219+44  
LOCATION: 3' Right of Center Line of Construction  
LOGGED BY: David Allen

DATE: October 15-16, 2014  
TYPE OF DRILLING: Hollow Stem Auger &  
Diamond Coring  
EQUIPMENT: CME 850 w/ CME  
Automatic Hammer  
HAMMER CORRECTION FACTOR: 1.23

COMPLETION DEPTH: 37.4

DEPTH FT.	SYMBOL	SAMPLES	DESCRIPTION OF MATERIAL	SOIL GROUP	PLASTIC LIMIT	% MOIST.	LIQUID LIMIT	DRY WEIGHT	LBS PER CU.FT.	NO. OF BLOWS PER 6-IN.	% C C R	% R Q D
			SURFACE ELEVATION: 644.4									
5		X	Moist, Medium Dense, Brown Sand with Gravel (Sandstone Fragments)							9 11-12		
10		X	Wet, Dense, Brown Sand with Gravel (Sandstone Fragments)							3 20-15		
15			SANDSTONE WITH DARK GRAY SHALE PARTINGS - Gray, Medium Bedded, Slightly Weathered, Well-Cemented, with Slight Dip							60 (1")	100	38
20			SANDSTONE WITH DARK GRAY SHALE PARTINGS - Gray, Medium Bedded, Slightly Weathered, Cemented, with Slight Dip								98	60
25			SANDSTONE WITH DARK GRAY SHALE SEAMS - Gray, Medium Bedded, Slightly Weathered, Cemented, with Slight Dip								100	40
30			SANDSTONE WITH DARK GRAY SHALE SEAMS - Gray, Medium Bedded, Slightly Weathered, Cemented, with Slight Dip								100	78
35			SANDSTONE WITH DARK GRAY SHALE SEAMS - Gray, Medium Bedded, Slightly Weathered, Cemented, with Slight Dip									

REMARKS:

**ARKANSAS HWY. & TRANS. DEPARTMENT  
MATERIALS DIVISION - GEOTECHNICAL SEC.**

BORING NO. 7  
PAGE 2 OF 2

JOB NO. 040623 Crawford County  
JOB NAME: Natural Dam - North Strs. & Apprs.  
U.S. 59  
STATION: 219+44  
LOCATION: 3' Right of Center Line of Construction  
LOGGED BY: David Allen

DATE: October 15-16, 2014  
TYPE OF DRILLING: Hollow Stem Auger &  
Diamond Coring  
EQUIPMENT: CME 850 w/ CME  
Automatic Hammer  
HAMMER CORRECTION FACTOR: 1.23

COMPLETION DEPTH: 37.4

DEPTH FT.	SYMBOL	SAMPLES	DESCRIPTION OF MATERIAL	SOIL GROUP	PLASTIC LIMIT	% MOIST.	LIQUID LIMIT	DRY WEIGHT	LBS PER CU.FT.	NO. OF BLOWS PER 6-IN.	% S C R	% R O D
			SURFACE ELEVATION: 644.4								100	84
			Boring Terminated									
40												
45												
50												
55												
60												
65												
70												

REMARKS:

# ARKANSAS STATE HIGHWAY AND TRANSPORTATION DEPARTMENT

September 16, 2015

**TO:** Mr. Rick Ellis, Bridge Engineer

**SUBJECT:** Job No. 040623  
Natural Dam – North Strs. & Apprs. (S)  
Route 59 Section 5  
Crawford County

The foundation recommendations included in this memo shall be supplementary to those included in the IOM dated November 20, 2014. At that time it was anticipated that all bents would be founded on piling. After discussions with Bridge Design, it is anticipated that all intermediate bents will be founded on drilled shafts. Drilled shafts socketed a minimum of two times the shaft diameter into the competent, gray, cemented sandstone with dark gray shale partings and seams should be sized based on the values provided in Table 1.

TABLE 1 – Bearing Capacity Recommendations for Drilled Shafts

Foundation Description	Nominal Shaft Side Resistance (ksf)	Factored Shaft Side Resistance (ksf)	Nominal Shaft Tip Resistance (ksf)	Factored Shaft Tip Resistance (ksf)
Drilled Shafts	11.7	6.4	183	92

It is anticipated that Bent 1, station 213+65, will be founded on a spread footing and should be sized based on the values provided in Table 2. It is anticipated that Bent 7, station 219+67, will be founded on piling. Preboring may be necessary to achieve minimum penetration requirements.

TABLE 2 – Bearing Capacity Recommendations for Spread Footings

Foundation Description	Nominal Bearing Resistance (ksf)	Resistance Factor	Factored Bearing Resistance (ksf)
Spread Footings	24.8	0.45	11

If you have any questions concerning these recommendations, please contact the Geotechnical Section.



Michael C. Benson  
Materials Engineer

MCB:rpt:mlg

cc: State Construction Engineer - Master File Copy  
District 4 Engineer  
G.C. File