#### ARKANSAS STATE HIGHWAY AND TRANSPORTATION DEPARTMENT



#### SUBSURFACE INVESTIGATION

STATE JOB NO		009814	
FEDERAL AID PROJE	CT NO	NHPP-0003(50)	
	E. PIGE	ON CREEK STR. & APPRS. (S	3)
STATE HIGHWAY	201	SECTION	11
IN		BAXTER	COUNTY
LETTING OF		SEPTEMBER 21, 2016	

The information contained herein was obtained by the Department for design and estimating purposes only. It is being furnished with the express understanding that said information does not constitute a part of the Proposal or Contract and represents only the best knowledge of the Department as to the location, character and depth of the materials encountered. The information is only included and made available so that bidders may have access to subsurface information obtained by the Department and is not intended to be a substitute for personal investigation, interpretation and judgment of the bidder. The bidder should be cognizant of the possibility that conditions affecting the cost and/or quantities of work to be performed may differ from those indicated herein.

#### ARKANSAS STATE HIGHWAY AND TRANSPORTATION DEPARTMENT

May 15, 2014

TO:

Mr. Trinity Smith, Engineer of Roadway Design

SUBJECT:

Job No. 009814

E. Pigeon Creek Str. & Apprs. (S)

Route 201 Section 1

**Baxter County** 

Transmitted herewith is the requested Soil Survey, Strength Data and Resilient Modulus test results for the above referenced job. The project consists of replacing the bridge crossing Pigeon Creek on Highway 201. Samples were obtained in the existing travel lanes, and ditch line. There were no paved shoulders within the project limits.

Based on laboratory results of samples obtained, the subgrade soils consist primarily of moderately plastic clay with sand and varying amounts of limestone fragments. The subgrade soils are expected to provide a stable working platform with conventional processing if the weather is favorable during construction.

Based on currently available cross-sections the maximum embankment height is approximately 12 feet. All soft unstable organic material should be undercut prior to embankment construction, anticipated to be no more than two feet. The embankment may be constructed using locally available unspecified material utilizing a 3:1 slope configuration.

The maximum cut depth is approximately 17 feet. The 3:1 cut slope configuration is acceptable as shown in the currently available cross-sections.

Listed below is the additional information requested for use in developing the plans:

1. The Qualified Products List (QPL) indicates that Aggregate Base Course (Class CL-7) is available from commercial producers in the vicinity of Mountain Home.

2. Asphalt Concrete Hot Mix

Type	Asphalt Cement %	Mineral Aggregate %
Surface Course	5.3	94.7
Binder Course	4.3	95.7
Base Course	4.1	95.9

Michael C. Benson

Materials Engineer

MCB:pt:bjj Attachment

cc: State Constr. Eng. - Master File Copy

District 9 Engineer

Transportation Planning and Policy Div.

G. C. File

# ARKANSAS STATE HIGHWAY AND TRANSPORTATION DEPARTMENT - LITTLE ROCK, ARKANSAS MATERIALS DIVISION

# MICHAEL BENSON, MATERIALS ENGINEER \*\*\* SOIL SURVEY STRENGTH TEST REPORT \*\*\*

DATE - 05/14/2014

SEQUENCE NO. - 1

JOB NUMBER - 009814

MATERIAL CODE - SSRVPS

SPEC. YEAR - 2003

SUPPLIER ID. - 1

COUNTY/STATE - 03

DISTRICT NO. - 09

JOB NAME - E. PIGEON CREEK STRS. & APPRS. (S)

\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*

\* STATION LIMITS R-VALUE AT 240 psi

\*

BEGIN OB - END JOB

LESS THAN 5

RESILIENT MODULUS

STA.112+00

7758

REMARKS -

AASHTO TESTS : T190

# ARKANSAS STATE HIGHWAY AND TRANSPORTATION DEPARTMENT MATERIALS DIVISION

# AASHTO T 307-99 - RESILIENT MODULUS OF SUBGRADE SOILS RECOMPACTED SAMPLES

Job No. Date Sampled: Date Tested: Name of Project:	009814 04/30/14 April 30, 2014 E.PIGEON CREEK STRS. & APPRS.(S)	Material Code Station No.: Location:	SSRVPS 112+00 17'LT
County: Sampled By:	Code: 3 Name: BAXTER FAULKNER	Depth:	0-5
Lab No.:	20141194	AASHTO Class:	A-6(13)
Sample ID: LATITUDE:	RV362	Material Type (1 or 2): LONGITUDE:	2
1. Testing Inform	ation:		
	Preconditioning - Permanent Strain > 5% (Y		N
	Testing - Permanent Strain > 5% (Y=Yes or Number of Load Sequences Completed (0-1	•	N 15
2. Specimen Info	rmation:		
	Specimen Diameter (in):		
	Top Middle		3.96
	Bottom		3.94 3.95
	Average		3.95
	Membrane Thickness (in):		0.01
	Height of Specimen, Cap and Base (in):		8.03
	Height of Cap and Base (in):		0.00
	Initial Length, Lo (in): Initial Area, Ao (sq. in):		8.03 12.18
	Initial Volume, AoLo (cu. in):		97.80
	(ou. 11).		01.00
3. Soil Specimen	_		
	Weight of Wet Soil Used (g):		3230.00
4. Soil Properties	::		
	Optimum Moisture Content (%): Maximum Dry Density (pcf): 95% of MDD (pcf):		15.9 110.4 104.9
	In-Situ Moisture Content (%):		N/A
5. Specimen Prop	perties:		
	Wet Weight (g):		3230.00
	Compaction Moisture content (%):		16.6
	Compaction Wet Density (pcf): Compaction Dry Density (pcf):		125.83 107.92
	Moisture Content After Mr Test (%):		16.6
6. Quick Shear Te	est (Y=Yes, N=No, N/A=Not Applicable):		#VALUE!
7. Resilient Modu	ılus, Mr:	10241(Sc	e)^-0.22815(S3)^0.29219
8. Comments		<u> </u>	
9. Tested By:	MW/DT	Date: April 30, 2014	

# ARKANSAS STATE HIGHWAY AND TRANSPORTATION DEPARTMENT MATERIALS DIVISION

# AASHTO T 307-99 - RESILIENT MODULUS OF SUBGRADE SOILS RECOMPACTED SAMPLES

SSRVPS 112+00

17'LT

AASHTO Class: A-6(13)
Material Type (1 or 2): 2 0-5 Material Code Station No.: Location: Depth: BAXTER E.PIGEON CREEK STRS. & APPRS.(S) Name: April 30, 2014 Code: 3 FAULKNER 20141194 04/30/14 009814 Name of Project: Date Sampled: Date Tested: Sampled By: Lab No.: County: Job No.

LONGITUDE:

RV362

LATITUDE:

Sample ID:

Resilient Modulus			M	psi	15,228	14,220	12,814	10,942	9,531	12,392	11,559	10,766	9,841	8,936	10,492	9,562	8,929	8,334	7,758
Resilient Strain			చ్	in/in	0.00012	0.00025	0.00042	0.00064	0.00089	0.00015	0.00031	0.00049	0.000070	0.00094	0.00017	0.00037	0.00058	0.00082	0.00107
Average Recov Def.	LVDT 1	ğ	Havg	Ē	96000.0	0.00203	0.00335	0.00510	0.00713	0.00118	0.00247	0.00393	0.00563	0.00758	0.00138	0.00295	0.00466	0.00655	0.00860
Actual Applied	Contact	200	Scontact	psi	0.2	0.2	0.3	0.5	0.7	0.2	0.2	0.2	0.4	9.0	0.2	0.2	0.2	0.4	9.0
Actual Applied	Cyclic	22200	Scyclic	psi	1.8	3.6	5.3	7.0	8.5	1.8	3.6	5.3	6.9	8.4	1.8	3.5	5.2	8.9	8.3
Actual Applied	Max. Axial	Stress	Smax	psi	2.1	3.8	5.6	7.5	9.2	2.0	3.8	5.5	7.3	9.1	2.0	3.7	5.4	7.2	8.9
Actual Applied	Contact		Pcontact	sql	2.7	2.8	3.7	6.1	9.6	2.7	2.8	2.8	5.2	9.7	2.7	2.8	2.8	4.3	6.9
Actual Applied	Cyclic Load		Peydic	lbs	22.3	43.9	65.0	84.7	103.1	22.1	43.4	64.2	84.0	102.8	22.0	42.8	63.1	82.8	101.2
Actual Applied	Ē		Р <sub>мах</sub>	lps	25.0	46.7	68.7	8.06	111.7	24.9	46.2	67.1	89.3	110.4	24.7	45.6	0.99	87.1	108.1
Nominal Maximum	Axial	2000	Seyclic	psi	2.0	4.0	0.9	8.0	10.0	2.0	4.0	0.9	8.0	10.0	2.0	4.0	6.0	8.0	10.0
Chamber Confining	Pressure		S	psi	6.0	6.0	0.9	0.9	0.9	4.0	4.0	4.0	4.0	4.0	2.0	2.0	2.0	2.0	2.0
	PARAMETER		DESIGNATION	UNIT	Sequence 1	Sequence 2	Sequence 3	Sequence 4	Sequence 5	Sequence 6	Sequence 7	Sequence 8	Sednence 9	Sequence 10	Sequence 11	Sequence 12	Sequence 13	Sequence 14	Sequence 15

April 30, 2014

DATE DATE

MW/DT

REVIEWED BY

TESTED BY

#### ARKANSAS STATE HIGHWAY AND TRANSPORTATION DEPARTMENT MATERIALS DIVISION

#### AASHTO T 307-99 - RESILIENT MODULUS OF SUBGRADE SOILS RECOMPACTED / THINWALL TUBE SAMPLES

Name: BAXTER

Job No.

009814

Material Code SSRVPS

Date Sampled:

04/30/14

**Station No.:** 112+00

Date Tested:

April 30, 2014

Location: 17'LT

Name of Project: E.PIGEON CREEK STRS. & APPRS.(S)

County:

Code: 3

**FAULKNER** 

Depth: 0-5

Sampled By:

Lab No.:

20141194

AASHTO Class: A-6(13)

Sample ID:

RV362

Material Type (1 or 2): 2

LATITUDE:

LONGITUDE:

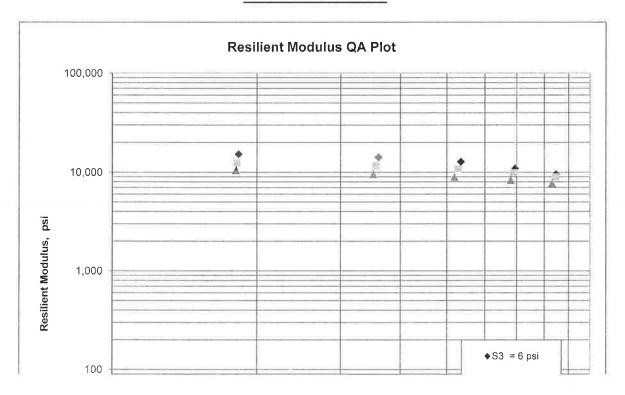
$$M_R = K1 (S_C)^{K2} (S_3)^{K5}$$

$$K1 = 10,241$$

$$K2 = -0.22815$$

$$K5 = 0.29219$$

$$R^2 = 0.92$$



JOB: 009814

Arkansas State Highway Transporation Department

JOB NAME: E. PIGEON CREEK STRS. & APPRS. (S)

Materials Division

COUNTY NO. 3 DATE TESTED

4/23/2014

Michael Benson, Materials Engineer

STA.#	LOC.	DEPTH	COLOR	#4	#10		#80	#200	L.L.	<i>P.I.</i>	SOIL CLASS	<i>LAB</i> #:	%MOISTURE
112+00	17LT	0-5	BR/GR		(3)	<b>E</b> (((°))	(//5)	E S	36	25	A-6(13)	RV362	
103+00	05RT	0-5	BR/GR	( <b>((0)</b> 0)	(8,8)	SH	(3(9)	(a)	35	22	A-6(17)	S358	25.4
103+00	17RT	0-5	BR/GR		(13)	(3)	162	13/16	34	21	A-6(8)	S359	22.2
112+00	05LT	0-5	BR/GR	je (e)/	. CAs	7.6%	(6(6)	1076	37	24	A-6(10)	S360	20.5
112+00	17LT	0-5	BR/GR	, ·V.	( v/O		ીકા .	- 4 (a)	40	27	A-6(11)	S361	19.9

DATE TESTED

4/23/2014

Arkansas State Highway Transporation Department

Materials Division

 $JOB\ NAME$ : E. PIGEON CREEK STRS. & APPRS. (S)

COUNTY NO.

009814

JOB:

Michael Benson, Materials Engineer

PAVEMENT SOUNDINGS AGG.BASE CRS CL-7 AGG.BASE CRS CL-7 AGG.BASE CRS CL-7 AGG.BASE CRS CL-7 ACHMSC ACHMSC ACHMSC ACHIMSC 3.0 CHIP SEAL CHIP SEAL CHIP SEAL CHIP SEAL .25 05RT 17RT 17LT STA.# LOC. 05LT 103+00 112+00 112+00 103+00

Friday, May 02, 2014

Page 1 of 1

comments:

# ARKANSAS STATE HIGHWAY AND TRANSPORTATION DEPARTMENT - LITTLE ROCK, ARKANSAS MATERIALS DIVISION

#### MICHAEL BENSON, MATERIALS ENGINEER

\*\*\* SOIL SURVEY / PAVEMENT SOUNDING TEST REPORT \*\*\*

DATE - 04/	24/14	, ,				GEOTIENGE	NO 1	
JOB NUMBER - 009							CODE - S	
FEDERAL AID NO TO		CNED						
							AR - 2	
PURPOSE - SOI							ID 1	
SPEC. REMARKS - NO		CATION CH	ECK			·	TATE - C	
SUPPLIER NAME - STA					a (a)	DISTRICT	NO 0	19
NAME OF PROJECT - E			TRS. & AL	PR	.S. (S)			
PROJECT ENGINEER - N		ICABLE						
PIT/QUARRY - ARKAN								
LOCATION - BAXTE	•	Ϋ́					IPLED - C	
SAMPLED BY - S.FAUI							EIVED - C	
SAMPLE FROM - TEST							TED - C	14/23/14
MATERIAL DESC SO	CL SURVE	EY - R VA	LUE- PAVI	EME	ENT SOUNDIN	1GS		
LAB NUMBER	-	20141190		-	20141191	-	20141192	
SAMPLE ID	_	S358		_	S359	_	S360	
TEST STATUS	_	INFORMAT	ION ONLY	-	INFORMATI	ON ONLY -	INFORMAT	CION ONLY
STATION -	_	103+00		-	103+00	-	112+00	
LOCATION	, I	05RT		-	17RT	-	05LT	
DEPTH IN FEET	-	0-5		-	0-5	_	0-5	
MAT'L COLOR	_	BR/GR		-	BR/GR	-	BR/GR	
MAT'L TYPE	_			_		-		
LATITUDE DEG-MIN-					36 27		=	46.20
LONGITUDE DEG-MIN-	SEC -	92 21	44.70		92 21	44.70	92 21	34.50
% PASSING 2	IN			=		1=		
1 1/2	IN			=		275		
3/4	IN			-	100	7.5	100	
•	IN			-	97	-	99	
	4 -	100		-	93	-	97	
NO.	10 -	99		-	88	-	84	
NO.		91		_	83	15 72	76	
NO.	80 -	89		-	72	_	66	
	200 -	86			57		57	
LIQUID LIMIT	_	35		_	34	_	37	
PLASTICITY INDEX	_	22		_	21	-	24	
AASHTO SOIL	_	A-6(17)		-		-	A-6(10)	
UNIFIED SOIL	_	11 0 (17)		-	(-,	-		
% MOISTURE CONTENT	_	25.4		-	22.2	-	20.5	
	(IN) -	.25		-		_	.25	
ACHMSC	(IN) -	3.5		-	· · · · ·	_	3.0	
AGG.BASE CRS CL-7	(IN) -	5.0		-	-	-	5.0	
rico. Bribli etto eli 7	( 111)	3.0		-		-		
	-			-		-		
	77			7		-		
	-			-		-		
	-					_		
	<u></u>			-		_		

REMARKS -

AASHTO TESTS : T24 T88 T89 T90 T265

2

#### ARKANSAS STATE HIGHWAY AND TRANSPORTATION DEPARTMENT - LITTLE ROCK, ARKANSAS MATERIALS DIVISION

# MICHAEL BENSON, MATERIALS ENGINEER \*\*\* SOIL SURVEY / PAVEMENT SOUNDING TEST REPORT \*\*\*

DATE - 04/2 JOB NUMBER - 0098 FEDERAL AID NO TO B PURPOSE - SOIL SPEC. REMARKS - NO S SUPPLIER NAME - STAT NAME OF PROJECT - E. PROJECT ENGINEER - NO PIT/QUARRY - ARKANS LOCATION - BAXTER SAMPLE BY - S.FAULK SAMPLE FROM - TEST H MATERIAL DESC SOIL	14 E ASS SURV PECIF E PIGE T APP AS , COUN WER OLE	EY SAMPLE ICATION CH ON CREEK S LICABLE IY	ECK	RS. (S)	SEQUENCE NO. MATERIAL CODE SPEC. YEAR SUPPLIER ID. COUNTY/STATE DISTRICT NO.  DATE SAMPLED DATE RECEIVED DATE TESTED	- SSRVPS - 2003 - 1 - 03 - 09 - 04/10/14 - 04/11/14
LAB NUMBER	_			_	_	
SAMPLE ID	_	20141193 S361		_	_	
TEST STATUS			TON ONLY -	-	-	
STATION				-	_	
LOCATION		17LT	-	-	-	
DEPTH IN FEET	_	0 – 5	•	-	_	
MAT'L COLOR	_	BR/GR	_	-	_	
MAT'L TYPE	-		-	_		
LATITUDE DEG-MIN-S				-	_	
LONGITUDE DEG-MIN-S	EC -	92 21	34.60			
% PASSING 2	IN		_		923	
1 1/2					-	
·		100			V <del>55</del>	
		90	-		· -	
		82	-		:#	
NO.			-		236	
NO.					:= :	
NO.						
		56				
T TOUTD I TMT		40				
LIQUID LIMIT PLASTICITY INDEX		27		-	7 <u>m</u> 7 <u>m</u>	
AASHTO SOIL		A-6(11)		-		
UNIFIED SOIL	_	A-0 (11)	-	-		
% MOISTURE CONTENT	_	19.9	-	-	\mathrew{m}	
		10.5				
	(IN) -	7.7		-	-	
	(IN) -			-	_	
AGG.BASE CRS CL-7	(IN) -			_	_	
				_	_	
	-			_	-	
	-			_	-	
	:=			_	-	
	_			-	-	

REMARKS -

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AASHTO TESTS : T24 T88 T89 T90 T265

### ARKANSAS STATE HIGHWAY AND TRANSPORTATION DEPARTMENT - LITTLE ROCK, ARKANSAS MATERIALS DIVISION

#### MICHAEL BENSON, MATERIALS ENGINEER

\*\*\* SOIL SURVEY / PAVEMENT SOUNDING TEST REPORT \*\*\*

DATE - 04/24/14  JOB NUMBER - 009814  FEDERAL AID NO TO BE AMPURPOSE - SOIL SUMMER  SPEC. REMARKS - NO SPECTOR SUPPLIER NAME - STATE  NAME OF PROJECT - E. PICTOR PROJECT ENGINEER - NOT AMPUT	SSIG RVEY IFIC GEON PPLI UNTY	SAMPI ATION CREE CABLE	CHE		APPRS.	(S)	SEQUENCE NO. MATERIAL CODE SPEC. YEAR SUPPLIER ID. COUNTY/STATE DISTRICT NO.  DATE SAMPLED	 RV 2003 1 03 09
SAMPLED BY - S.FAULKNER SAMPLE FROM - TEST HOLE MATERIAL DESC SOIL SU		7 - RE	STS'	TANCE R	-VAT <sub>I</sub> UF	ACTUAL	DATE RECEIVED DATE TESTED RESULTS	04/11/14
				IANCE K	- VALUE	ACTUAL	REBORID	
LAB NUMBER		20141	194		-		=	
SAMPLE ID		RV362			-		-	
TEST STATUS	-	INFORI	TAN.	ON ONLY	. –		)意 7位	
		112+00	)				100	
		17LT			_		::=	
					-		3 <del>5.</del>	
1111 = 00=011		BR/GR			-		9	
MAT'L TYPE	-	2.6	0.7	16.20	-		122	
LATITUDE DEG-MIN-SEC LONGITUDE DEG-MIN-SEC				46.30 34.60	-		-	
% PASSING 2 IN.	_				-		-	
1 1/2 IN.	_				-		-	
3/4 IN.					_		-	
3/8 IN.		100			-		-	
NO. 4		99			-		-	
NO. 10	_	96			_		_	
NO. 40	-	89			_		_	
NO. 80	_	75			_		-	
NO. 200	-	65						
LIQUID LIMIT	-	36			-		-	
PLASTICITY INDEX	-	25			-		-	
AASHTO SOIL	-	A-6(	13)		-		-	
UNIFIED SOIL	_				-		-	
% MOISTURE CONTENT	=				-			
	-				-		-	
	-				-		_	
	-				_		-	
	_				_		_	
	_				_		_	
	-				-		_	
	-				-		-	
	-				-		-	
	-				-		-	

REMARKS -

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AASHTO TESTS : T24 T88 T89 T90 T265

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#### ARKANSAS STATE HIGHWAY AND TRANSPORTATION DEPARTMENT

August 12, 2014

TO:

Mr. Rick Ellis, Bridge Engineer

SUBJECT:

Job No. 009814

E. Pigeon Creek Strs. & Apprs. (S)

Route 201 Section 1 Baxter County

Transmitted herewith are a brief summary of the geology and site conditions, unconfined compressive strength test results, D50 analysis test results, and the logs of the borings conducted for the structures and approaches of the above referenced project. The samples obtained by the Standard Penetration Tests were brought to the laboratory and visually classified by experienced lab personnel to confirm the field identifications. The rock cores are available for inspection at the Materials Division.

Based on the depth at which bedrock was encountered, it is anticipated that the interior bents will be founded on spread footings. Spread footings should be sized based on the values provided in Table 1.

TABLE 1 – Bearing Capacity Recommendations for Interior bents

Foundation Description	Nominal Bearing Resistance (ksf)	Resistance Factor	Factored Bearing Resistance (ksf)	Presumptive Bearing Resistance at the Service Limit State (ksf)
Spread Footings	155	0.45	70	40

If you have any questions concerning these recommendations, please contact the Geotechnical Section.

Michael C. Benson Materials Engineer

MCB:rpt

CC:

State Construction Engineer - Master File Copy

District 9 Engineer

G.C. File

# GEOLOGY AND SITE CONDITIONS Job No. 009814

# E. Pigeon Creek Str. & Apprs. (S) Baxter County Route 201 Section 1

#### **Site Conditions**

There are two existing bridges at the jobsite: one over Pigeon Creek, and one over a relief for Pigeon Creek. These bridges are to be replaced by a single bridge. The relief bridge (**Bridge 1**) is constructed of concrete deck with rock and mortar bents. The guardrail is constructed of concrete on the bridge and steel leading up to the bridge.

The Pigeon Creek Bridge (**Bridge 2**) is a 4 span bridge and is constructed similarly to Bridge 1. The end bents and the middle bent are constructed of rock and mortar. The second and fourth bents are constructed of steel beams with spread footings. The bridge has a concrete deck supported by seven steel beams with concrete guardrail on the bridge with steel guardrail leading up to the bridge.

Pigeon Creek flows to the southeast into Norfork Lake approximately two miles downstream from the existing bridge. Rock is exposed in the channel and consists of dolostone. Overhead power lines cross the roadway at the jobsite at a diagonal. A gravel road intersects the highway upstation from the bridges on the right side of the highway. The area on the downstation side of the creek channel is moderately to heavily wooded. The area on the upstation side of the creek is tree-lined with pastureland beyond.

#### **Site Geology**

The project alignment is located in the mapped outcrop of the Ordovician Jefferson City and Cotter Dolostone (map symbols Ojc and Oc, respectively). The Jefferson City consists of light- to dark-tan, fine-grained, crystalline dolostone and considerable chert with some rare thin beds of sandstone, shale, and oolite. The Jefferson City Dolomite unconformably overlies the Roubidoux Formation. This lower contact is not exposed in Arkansas. The Cotter Dolostone is composed of dolostone of predominantly two types: a fine-grained, argillaceous, earthy textured, relatively soft, white to buff or gray dolostone called "cotton rock", and a more massive, medium-grained, gray dolostone that weathers to a somewhat hackly surface texture and becomes dark on exposure. The formation contains chert, some minor beds of greenish shale, and occasional thin interbedded sandstone. The chert nodules associated with the Cotter frequently have concentric light and dark bands. To date, there has been no success in differentiating the Cotter Formation from the underlying Jefferson City Formation in Arkansas, due to the Jefferson City having a similar composition. The thickness is about 340 feet in the vicinity of Cotter, but the interval may range up to 500 feet thick in places.

#### **Subsurface Conditions**

Based on the results of the borings, the subsurface stratigraphy may be generalized as follows:

0 to 9.0 Feet: Varies f

Varies from dry to moist, medium to very dense, brown sand to sand with clay, gravel (dolostone fragments) and some organic matter to hard, slightly weathered, gray and brown dolostone. Some dolostone in this zone is fractured.

9.0 to 32.6 Feet:

Consists of hard, slightly weathered, gray and brown **dolostone**. Some dolostone in this zone is fractured. Some rock in this zone contains vugs.

# Rock Core Unconfined Compression Test Summary

Project Number:

009814

Project Name:

E. PIGEON CREKK STR. & APPRS.

Date Tested:

8/6/2014

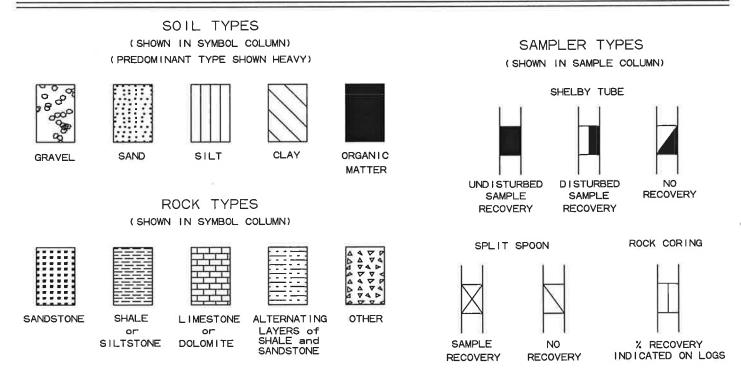
Station	Location	Sample No.	Depth (ft)	Diameter (in)	Height (in)	Total Load (lbs)	Correction Factor	Stress (psi)	Remarks
107+22	23' RT	1	12.0	1.75	3.65	19,190	1.00	7,978	
107+63	18' RT	2	6.5	1.75	3.66	15,330	1.00	6,373	
108+10	C.L.	3	3.0	1.75	3.65	6,330	1.00	2,632	
108+64	20' RT	4	9.0	1.75	3.72	15,990	1.00	6,648	
			6						
			-						

<sup>\*</sup> Please note any broken samples, fractures or other characteristics of sample in Remarks.

# D<sub>50</sub> AGGREGATE ANALYSIS FOR SCOUR CALCULATIONS

		Job No.	009814		
Creek Name	Station	Sample Type	Location	Depth (FT)	Aggregate Size (D50) (IN)
E. Pigeon Creek	108+15	Creek Bank	5' Rt. C.L. Construction	NA	0.0083

# LEGEND



#### TERMS DESCRIBING CONSISTENCY OR CONDITION

GRANU	LAR SOIL		CLAY	CL/	AY-SHALE	SHALE		
'N' Value	Density	'N' Value	Consistency	'N' Value	Consistency	'N' Value	Consistency	
0-4	Very Loose	0-1	Very Soft	0-1	Very Soft			
5-10	Loose	2-4	Soft	2-4	Soft	31-60	Soft	
11-30	Medium Dense	5-8	Medium Stiff	5-8	Medium Stiff	0ver 60		
31-50	Dense	9-15	Stiff	9-15	Stiff	More than	2'	
0ver 50	Very Dense	16-30	Very Stiff	16-30	Very Stiff	Penetrati	on	
		31-60	Hard	31-60	Hard	in 60 Blow	vs: Medium Hard	
ŀ		0ver 60	Very Hard	0ver 60	Very Hard	Less than	2'	
						Penetrati	on	
						in 60 Blow	vs∎ Hard	

- 1. Ground water elevations indicated on boring logs represent ground water elevations at date or time shown on boring log. Absence of water surface implies that no ground water data is available but does not necessarily mean that ground water will not be encountered at locations or within the vertical reaches of these borings.
- 2. Borings represent subsurface conditions at their respective locations for their respective depths. Variations in conditions between or adjacent to boring locations may be encountered.
- 3. Terms used for describing soils according to their texture or grain size distribution are in accordance with the Unified Soil Classification System.

Standard Penetration Test – Driving a 2.0" O.D., 1-3/8" I.D. sampler a distance of 1.0 foot into undisturbed soil with a 140 pound hammer free falling a distance of 30 inches. It is customary to drive the spoon 6.0 inches to seat into undisturbed soil, then perform the test. The number of hammer blows for seating the spoon and performing the test are recorded for each 6 inches of penetration on the drill log. The field "N" Value (N<sub>f</sub>) can be obtained by

adding the bottom two numbers for example:  $\frac{6}{8-9} \Rightarrow 8+9=17 blows/ft$ . The "N" Value corrected to 60% efficiency (N<sub>60</sub>) can be obtained by multiplying N<sub>f</sub> by the hammer correction factor published on the boring log.

ARKANSAS HWY. & TRANS. DEPARTMENT					BORIN			г <b>1</b>								
MATERIALS DIVISION - GEOTECHNICAL SEC.					PAGE 1 OF 1											
	JOB NO. 009814 Baxter County JOB NAME: E. Pigeon Creek Strs. & Apprs.						DATE: July 29, 2014  TYPE OF DRILLING: Hollow Stem Auger &									
10D I	Alvic.		S.H. 201		TYPE OF DRILLING: Hollow Stem Auger & Diamond Coring											
STATI	ION;		107+22		EQUIPMENT: CME 850 w/ CME											
LOCA			23' Right of Center Line of Construction		Automatic Hammer											
			Stanley Bates		HAMMI	ER C	ORREC	TION	FAC	TOR:		1.23				
COMPLETION DEPTH: 32.6																
D	S	S														
E P	Υ	A M						Kora .	Ę	S		%	%			
-	М	P	DESCRIPTION OF MATERIAL	SOIL		42		GH	CO.	OW		S	R			
н	ВО	L		GROUF	IΞ	IST,	l <sub>A.</sub>	WEI	ER	F BI	Ζį	C R	Q D			
	L	E			PLASTIC LIMIT	% MOIST	LIQUID	DRY WEIGHT	LBS PER CU.FT	NO. OF BLOWS	PER 6-IN					
FT.	v. J. J. V	S	SURFACE ELEVATION: 613.8		<u> </u>	%	133	Q	그	ž	H <sub>P</sub>					
_ =																
	666															
	0.0															
	8		Dry, Very Dense, Brown Sand with Clay, Gravel							9						
_ 5	18/3	X	(Dolostone Fragments) and some Organic Matter							26-3	36					
	8. 8. E.															
	130															
10	出	$\top$								60 (1"	<u> </u>					
10	斑		DOLOSTONE - Gray, Medium Bedded, Slightly							(1	- 1	100	71			
	斑		Weathered, Hard, with Slight Dip									100	/4			
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	斑															
15	斑		DOLOSTONE WITH DARK GRAY SHALE									20	20			
	进		PARTINGS - Gray, Medium Bedded, Slightly Weathered, Hard, with Slight Dip									98	88			
	莊莊		Weathered, Hard, with Slight Dip													
	器										-					
	莊															
20	莊		DOLOSTONE - Gray and Brown, Medium									100	90			
	斑		Bedded, Slightly Weathered, Vuggy, Hard, with Slight Dip									100	OU			
	莊		Chight Dip													
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	跷															
25	斑											96	88			
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	癍		DOLOSTONE - Gray, Medium Bedded, Slightly													
<u> </u>			Weathered, Hard, with Slight Dip								İ					
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ARKANSAS HWY. & TRANS. DEPARTMENT					BORING NO. 2										
MATERIALS DIVISION - GEOTECHNICAL SEC.					PAGE 1 OF 1										
JOB NO. 009814 Baxter County						DATE: July 29, 2014									
JOB N.	AME:		E. Pigeon Creek Strs. & Apprs.		TYPE OF DRILLING: Hollow Stem Auger & Diamond Coring										
			S.H. 201		· ·										
STATI			107+63		EQUIPMENT: CME 850 w/ CME Automatic Hammer										
LOCA'			18' Right of Center Line of Construction		HAMMER CORRECTION FACTOR: 1.23										
_			Stanley Bates N DEPTH: 28.3		HAMM	ER C	JKKEC	HON	FAC	IUK:		1.23	-		
$\overline{}$	PLEI		V DEPTH: 28.3			1		T		T					
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	Y	м	DESCRIPTION OF MATERIAL						FT	WS	1	%	%		
T	M B	Р	DESCRIPTION OF WATERIAL	SOIL GROUI	, , ,	ے ا		IGH HE	CI	I.o		SC	К Q		
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FT.	Ĺ	E S	CUREAGE ELEVATION. C44.0		PLASTIC LIMIT	% MOIST	LIQUID	DRY WEIGHT	LBS PER CU.FT	NO. OF BLOWS	PER 6-IN.				
F 1 8	:	3	SURFACE ELEVATION: 611.8			%	111	10		Z	<u>P</u>	$\vdash$			
<u> </u>	0.06														
<u> </u>	80 80		Dry, Very Dense, Brown Sand with Gravel												
<u> </u>	7 00 C		(Dolostone Fragments) and some Organic Matter	1											
<u> </u>	φ. 60 σ														
5	品	T		ĺ						6 (1	30 1")				
	田田		DOLOSTONE - Gray and Brown, Medium							1,	,	91	48		
	辯問		Bedded, Slightly Weathered, Hard, with Slight Dip and some Fractured Layers					20				91	40		
L -	斑		DIP and 30mo i radialed Eayore								-				
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10	曲														
	部		DOLOSTONE - Gray, Medium Bedded, Slightly									98	86		
	出出		Weathered, Hard, with Slight Dip												
	斑										ļ				
	盘			1							!				
15	盘		DOLOSTONE - Gray and Brown, Medium								-				
	部		Bedded, Slightly Weathered, Vuggy, Hard, with								ļ	100	74		
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	田田		DOLOSTONE - Gray, Medium Bedded, Slightly												
		+	Weathered, Hard, with Slight Dip								1				
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					BORING NO. 3												
MATERIALS DIVISION - GEOTECHNICAL SEC.					PAGE 1 OF 1												
JOB N	Ο.		009814 Baxter County		DATE: July 28, 2014												
JOB N	AME:		E. Pigeon Creek Strs. & Apprs.	- 1	TYPE OF DRILLING: Hollow Stem Auger &												
l			S.H. 201	- 1	Diamond Coring												
	STATION: 108+10							EQUIPMENT: CME 850 w/ CME									
LOCA			Center Line of Construction	- 1	Automatic Hammer												
			Stanley Bates		HAMMI	ER C	ORREC	TION	FAC	TOR:	_	1.23					
	COMPLETION DEPTH: 23.9							_	-								
D E	s	S A															
P	Υ	М	DECORPTION OF MATERIAL					۱,	FT.	X.S		%	%				
T	M B	Р	DESCRIPTION OF MATERIAL	SOIL GROUP				H	CU	[CO		S C	R Q				
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FT.	Ĺ	E			PLASTIC LIMIT	% MOIST.	LIQUID	DRY WEIGHT	LBS PER CU.FT	NO. OF BLOWS	PER 6-IN.						
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	斑症																
	莊											100	53				
	斑																
			DOLOSTONE - Brown and Gray, Medium														
5	斑		Bedded, Slightly Weathered, Hard, with Slight Dip														
	斑語		ыр										00				
	쓮											94	90				
<u> </u>	部																
	辯																
10																	
	辯		DOLOSTONE - Brown and Gray, Medium														
	斑		Bedded, Slightly Weathered, Vuggy, Hard, with									96	64				
	螆		Slight Dip and some Fractured Layers														
	莊																
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	斑白																
	莊莊											98	94				
	螆																
	斑斑	Ш	DOLOSTONE - Gray, Medium Bedded, Slightly														
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	踣											100	98				
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	斑																
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REM	ARK	 3:				-											

					BORING NO. 4										
					PAGE 1 OF 1										
JOB NO. 009814 Baxter County						DATE: July 22, 2014									
JOB N	AME:		E. Pigeon Creek Strs. & Apprs.		TYPE OF DRILLING: Hollow Stem Auger & Diamond Coring										
OTATI	ONT		S.H. 201 108+64		EQUIPMENT: CME 850 w/ CME										
STATION: 108+64  LOCATION: 20' Right of Center Line of Construction						Automatic Hammer									
			Stanley Bates		HAMMER CORRECTION FACTOR: 1.23										
			N DEPTH: 22.9												
D		S													
E	S Y	Α							1	co.			2/		
P	М	М	DESCRIPTION OF MATERIAL	SOIL				Ħ	U.F	OW.		% S	% R		
T H	В	P L		GROUP	ျှ	ST.		EIG	RC	BL	z l	C R	Q		
	0	Е			PLASTIC LIMIT	"WOIST	LIQUID	DRY WEIGHT	LBS PER CU.FT	NO. OF BLOWS	PER 6-IN.	K	D		
FT.	L	S	SURFACE ELEVATION: 614.8		PL/ LIN	1%	1919	DR	LB	0X	PEI				
			Moist, Medium Dense, Brown Sand												
			Wolst, Wedidin Donse, Drown Cana												
		<b>&gt;</b>	DOLOSTONE - Gray, Weathered, Moderately							6	0				
5			\Hard							(3	)")				
	斑		DOLOSTONE - Gray and Brown, Medium									100	18		
			Bedded, Slightly Weathered, Hard, with Slight Dip and Fractured Layers												
	莊		DIP and Fractured Layers												
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15	斑		DOLOSTONE - Gray, Medium Bedded, Slightly									ام	20		
	盎		Weathered, Hard, with Slight Dip									96	88		
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