#### ARKANSAS DEPARTMENT OF TRANSPORTATION



#### SUBSURFACE INVESTIGATION

IN		DREW	COUNTY
STATE HIGHWAY	138	SECTION	3
	ABLES CR	EEK STR. & APPRS. (S)	
FEDERAL AID PROJE	CT NO. N	NHPP-0022(37)	
STATE JOB NO.		020587	

The information contained herein was obtained by the Department for design and estimating purposes only. It is being furnished with the express understanding that said information does not constitute a part of the Proposal or Contract and represents only the best knowledge of the Department as to the location, character and depth of the materials encountered. The information is only included and made available so that bidders may have access to subsurface information obtained by the Department and is not intended to be a substitute for personal investigation, interpretation and judgment of the bidder. The bidder should be cognizant of the possibility that conditions affecting the cost and/or quantities of work to be performed may differ from those indicated herein.



#### ARKANSAS DEPARTMENT OF TRANSPORTATION

ARDOT.gov | IDriveArkansas.com | Scott E. Bennett, P.E., Director

#### MATERIALS DIVISION

11301 West Baseline Road | P.O. Box 2261 | Little Rock, AR 72203-2261 | Phone: 501.569.2185 | Fax: 501.569.2368

Michael C. Benson Materials Engineer

October 6, 2017

TO:

Mr. Rick Ellis, Bridge Engineer

SUBJECT:

Job No. 020587

Ables Creek Str. & Apprs. (S)

Route 138 Section 3

Drew County

Transmitted herewith are a brief summary of the geology and site conditions, summary of percent material passing #200 sieve and Atterberg Limits test results (for liquefaction susceptibility analysis), D50 scour analysis, and the logs of the borings conducted for the structures and approaches of the above referenced project. The samples obtained by the Standard Penetration Tests were brought to the laboratory and visually classified by experienced lab personnel to confirm the field identifications.

This project consists of replacing the bridge crossing Ables Creek, on Highway 138, northeast of Monticello. The new bridge will be constructed on the existing alignment and a temporary detour bridge will be constructed to maintain traffic. Three of the five requested borings, all intermediate bents, were inaccessible due to steep slopes, low bridge clearance, and high water levels in the channel. The three borings that were not obtained were located at: 218+32 C.L. Construction, 218+82 C.L. Construction, and 219+32 C.L. Construction.

Based on plans provided by Bridge Division and the findings from this subsurface investigation, it is anticipated that all bents will be founded on concrete filled steel shell piles.

The project is located in a seismic area with a horizontal acceleration coefficient of 0.236, as provided by Bridge Design, A global stability analysis was performed for this embankment configuration and provides for a satisfactory Factor of Safety for seismic and static conditions. However, if the embankment geometry is altered in any way the embankment will need to be reanalyzed for seismic and static conditions.

If you have any questions concerning these recommendations, please contact the Geotechnical Section.

MCB:rpt:mlg

cc: State Construction Engineer - Master File Copy

District 2 Engineer

G.C. File

# GEOLOGY AND SITE CONDITIONS Job No. 020587

# Ables Creek Str. & Apprs. (S) Route 138 Section 3 Drew County

#### **Site Conditions**

The existing bridge is located over Ables Creek. The existing bridge is an 8 span structure constructed of concrete deck, timber pilings with concrete caps, and timber end walls. The bridge has steel guardrail supported by concrete posts on the bridge and steel posts leading up to the bridge. Overhead power lines parallel the right side of the existing roadway. The area on the right side of the roadway is moderately to heavily wooded. The channel to the left of the bridge is lined with trees and brush with agricultural fields beyond. There are trees growing in the channel, both up and downstream from the bridge. The main part of the channel is located under spans 3 and 4.

#### **Site Geology**

The area around the bridge is mapped as point bar deposits of a paleo-meander belt of the Arkansas River. These deposits consist of clay, silt, and sand. The top layer at the jobsite varies in thickness from 26.7 to 56.1 feet below ground level and consists primarily of clay with some layers of silt. This layer overlies sand and sand with silt. Some of the samples in the lower part of the lower layer contain a trace to some gravel.

#### **Subsurface Conditions**

Based on the results of the borings, the subsurface stratigraphy may be generalized as follows:

0 to 31.7 Feet:	Consists of moist to wet, soft to very stiff, reddish brown clay, silty clay,
	and sandy clay.

31.7 to 56.1 Feet:	Varies from wet, soft to medium stiff, brown to gray clay to very loose to
	medium dense, brown silty sand to sand.

56.1 to 61.7 Feet:	Consists of wet, very loose to medium dense, brown clayey sand to sand
	with silt to sand.

61.7 to 91.7 Feet:	Consists of wet, medium dense to dense, brown silty sand with som	е
	gravel to sand with some gravel.	

91.7 to 99.6 Feet:	Consists of wet,	medium dense	to very dense	, brown <b>silty</b>	sand to sand
	with gravel				

# Lab Test Summary Project: 020587

Project:

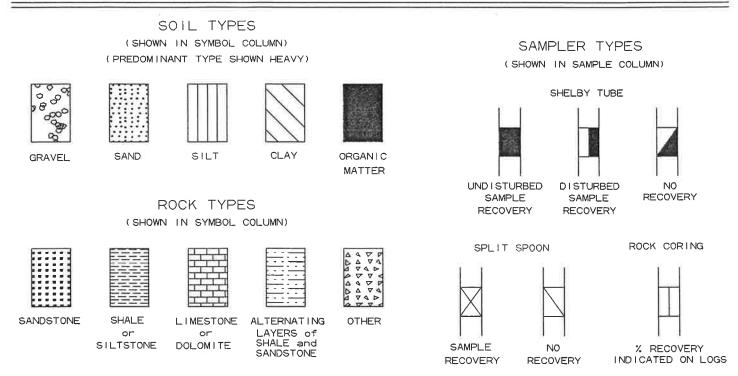
Ctation	Location	Depth	Plastic	Liquid	Plasticity	% Passing
Station	Location	(Ft.)	Limit	Limit	Index	No. 200
217+96.5	7' LT.	4.6	NT			95
217+96.5	7' LT.	9.6	NT			93
217+96.5	7' LT.	11.1	NT			97
217+96.5	7' LT.	16.1	18	25	7	93
217+96.5	7' LT.	21.1	21	63	42	99
217+96.5	7' LT.	26.1	22	61	39	98
217+96.5	7' LT.	31.1	17	35	18	99
217+96.5	7' LT.	36.1	NP			82
217+96.5	7' LT.	41.1	16	32	16	93
217+96.5	7' LT.	46.1	30	87	57	99
217+96.5	7' LT:	51.1	27	53	26	92
217+96.5	7' LT.	56.1	NP			7
217+96.5	7' LT.	61.1	NP			4
217+96.5	7' LT.	66.1	NP			4
217+96.5	7' LT.	71.1	NP			6
217+96.5	7' LT.	76.1	NP			5
217+96.5	7' LT.	81.1	NP			14
217+96.5	7' LT.	86.1	NP			5
217+96.5	7' LT.	91.1	NP			6
217+96.5	7' LT.	98.1	NP			4
219+67.5	7' RT.	4.2	NT			77
219+67.5	7' RT.	9.2	NT			95
219+67.5	7' RT.	11.7	NT			98
219+67.5	7' RT.	16.7	NT			99
219+67.5	7' RT.	21.7	17	35	18	94
219+67.5	7' RT.	26.7	20	28	8	91
219+67.5	7' RT.	31.7	NP			38
219+67.5	7' RT.	36.7	NP			36
219+67.5	7' RT.	41.7	NP			3
219+67.5	7' RT.	46.7	NP			5
219+67.5	7' RT.	51.7	NP			4
219+67.5	7' RT.	56.7	NP			23
219+67.5	7' RT.	61.7	NP			3
219+67.5	7' RT.	66.7	NP			2
219+67.5	7' RT.	71.7	NP			3
219+67.5	7' RT.	76.7	NP			21
219+67.5	7' RT.	81.7	NP			3
219+67.5	7' RT.	86.7	NP			3
219+67.5	7' RT.	91.7	NP			1
219+67.5	7' RT.	96.7	NP			2

NT = NO TEST

# D<sub>50</sub> AGGREGATE ANALYSIS FOR SCOUR CALCULATIONS

<b>Job No.</b> 020587												
Creek Name	Station	Sample Type	Location	Depth (FT)	Aggregate Size (D50) (IN)							
Ables Creek	218+47	Creek Bank	20' Lt. C.L. Construction	N/A	Less Than 0.0029							

# I FGFND



#### TERMS DESCRIBING CONSISTENCY OR CONDITION

GRANL	LAR SOIL		CLAY	CLA	Y-SHALE	SHALE				
*N* Value	Density	N' Value	Consistency	Nº Value	Consistency	'N' Value	Consistency			
0-4	Very Loose	0-1	Very Soft	0-1	Very Soft					
5-10	Loose	2-4	Soft	2-4	Soft	31-60	Soft			
11-30	Medium Dense	5-8	Medium Stiff	5-8	Medium Stiff	0ver 60				
31-50	Dense	9-15	Stiff	9-15	Stiff	More than	2'			
0ver 50	Very Dense	16-30	Very Stiff	16-30	Very Stiff	Penetrati	on			
		31-60	Hard	31-60	Hard	in 60 Blow	vs: Medium Har			
		0ver 60	Very Hard	Over 60	Very Hard	Less than	2"			
						Penetrati	on			
						in 60 Blov	vsı Hard			

- 1. Ground water elevations indicated on boring logs represent ground water elevations at date or time shown on boring log. Absence of water surface implies that no ground water data is available but does not necessarily mean that ground water will not be encountered at locations or within the vertical reaches of these borings.
- 2. Borings represent subsurface conditions at their respective locations for their respective depths. Variations in conditions between or adjacent to boring locations may be encountered.
- 3. Terms used for describing soils according to their texture or grain size distribution are in accordance with the Unified Soil Classification System.

Standard Penetration Test – Driving a  $2.0^\circ$  O.D.,  $1\text{-}3/8^\circ$  I.D. sampler a distance of 1.0 foot into undisturbed soil with a 140 pound hammer free falling a distance of 30 inches. It is customary to drive the spoon 6.0 inches to seat into undisturbed soil, then perform the test. The number of hammer blows for seating the spoon and performing the test are recorded for each 6 inches of penetration on the drill log. The field "N" Value ( $N_f$ ) can be obtained by

adding the bottom two numbers for example:  $\frac{6}{8-9} \Rightarrow 8+9=17 blows/ft$ . The "N" Value corrected to 60% efficiency (N<sub>60</sub>) can be obtained by multiplying N<sub>f</sub> by the hammer correction factor published on the boring log.

	1						BORING NO. 1 PAGE 1 OF 3							
JOB N	DATE: August 16, 2017													
JOB N	TYPE OF DRILLING:													
1	Hollow Stem Auger -Rotary Wash													
STATI	ION:		219+67.5		EQUIPMENT: Acker									
LOCA			7' Right of Construction Centerline		1									
		_	/inston Buie		HAMN	ÆR (	CORREC	CTION	V FAC	CTOR:		N/A		
	PLET		DEPTH: 98.2			_								
DEPTH T.	S Y M B O L	SAMPLES	DESCRIPTION OF MATERIAL SURFACE ELEVATION: 158.6	SOIL GROUP	PLASTIC LIMIT	% MOIST.	LIQUID LIMIT	DRY WEIGHT	LBS PER CU.FT.	NO. OF BLOWS	PER 6-IN.	% T C R	% R Q D	
-1-1-0	7:7	3	SURFACE ELEVATION: 136.6	-	LP	- %	ココ	Δ_	ı,	Z	Ъ		-	
5		X	Moist, Stiff, Brown Sandy Clay with Trace Gravel							3-4-	-6			
10		X	Moist, Very Stiff, Reddish Brown Clay							7-	10			
15		X	Wet, Stiff, Reddish Brown Clay							5-	-8 1 -7			
<del> </del>		$\vee$		CL	17		35				<u>2</u> -4			
			Wet, Medium Stiff, Reddish Brown Clay	*						3-	-4			
	1	X		CL	20		28				2			
30			Wet, Soft, Reddish Brown Clay	-										
_		X		SM	NP						3-8			
35			Wet, Medium Dense, Brown Silty Sand								-			
REM	ARKS	 S:												
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			HWY. & TRANS. DEPARTMENT DIVISION - GEOTECHNICAL SEC.		BORING NO. 1 PAGE 2 OF 3								
JOB N	IO.		DATE: August 16, 2017										
JOB N	IAME:	TYPE OF DRILLING: Hollow Stem Auger -Rotary Wash											
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STAT	IUN: .TION:		219+67.5 7' Right of Construction Centerline		EQUII	MEN	Τ:			Acke	er		ı
			/inston Buie		HAMN	ИER (	CORRE	CTION	v FAC	CTOR:		N/A	1
		_	DEPTH: 98.2										
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			HWY. & TRANS. DEPARTMENT	BORING NO. 1 PAGE 3 OF 3									
$\overline{}$			DIVISION - GEOTECHNICAL SEC.		DATE: August 16, 2017								
JOB N			020587 Drew County		TYPE OF DRILLING:								
JOB N	AME:		Ables Creek Str. & Apprs. (S) Route 138 Section 3		1								
GEL A TO	· O		219+67.5		Hollow Stem Auger -Rotary Wash								
STATI					EQUIPMENT: Acker								
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STATION			7' Left of Construction Centerline		EQUIP	MEN	Т:			Acke	r		
9			tanley Bates		HAMN	MER C	ORRE	CTION	V FAC	TOR:		N/A	
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REMAR	RKS:												

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REM	ARKS	3:											
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			IWY. & TRANS. DEPARTMENT DIVISION - GEOTECHNICAL SEC.		BORI PAGE		io. 2	OF 3					
JOB N			020587 Drew County		DATE:			Aug		22, 20	17		
JOB N	AME:		Ables Creek Str. & Apprs. (S) Route 138 Section 3				RILLIN		or 1	Diama	nd (	7050	ı
STATI	ON:		217+96.5		Hollow Stem Auger - Diamond Core EQUIPMENT: Acker								
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LOGG	ED BY	7: S	tanley Bates		HAMMER CORRECTION FACTOR: N/A								
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DEPTH FT.	S M B O L	SAMPLES	DESCRIPTION OF MATERIAL SURFACE ELEVATION: 158.6	SOIL GROUP	PLASTIC LIMIT	% MOIST.	LIQUID LIMIT	DRY WEIGHT	LBS PER CU.FT.	NO. OF BLOWS	PER 6-IN.	% T C R	% R Q D
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#### ARKANSAS STATE HIGHWAY AND TRANSPORTATION DEPARTMENT

May 16, 2017

TO:

Mr. Trinity Smith, Engineer of Roadway Design

SUBJECT:

Job No. 020587

Ables Creek Str. & Apprs. (S)

Route 138 Section 3

**Drew County** 

Transmitted herewith is the requested Soil Survey, strength data and Resilient Modulus test results for the above referenced job. The project consists of replacing the bridge crossing Ables Creek on Highway 138. Samples were obtained in the existing travel lanes and ditch line. There were no paved shoulders within the project limits.

Based on laboratory results of samples obtained, the subgrade soils consist primarily of highly plastic sandy clay. The subgrade soils are expected to provide a stable working platform with normal drying and compactive efforts, if the weather is favorable during construction.

The maximum embankment height is approximately 15 feet. Cross-sections indicate that the embankment will be placed within the existing ditch line. Based on seasonal conditions, water may be present during the time of construction. The ditches should be drained of all water and the soft unstable organic material should be undercut to a maximum depth of two feet. The embankment may be constructed with locally available unspecified material utilizing the slope configuration shown in the cross-sections.

The proposed cut slopes are acceptable as shown in the currently available cross sections.

Listed below is the additional information requested for use in developing the plans:

1. The Qualified Products List (QPL) indicates that Aggregate Base Course (Class CL-7) is available from commercial producers located at the river port near Dumas.

2. Asphalt Concrete Hot Mix

TypeAsphalt Cement %Mineral Aggregate %Surface Course5.294.8Binder Course4.495.6Base Course4.096.0

Michael C. Benson Materials Engineer

MCB:pt:bjj Attachment

cc: State Constr. Eng. – Master File Copy District 2 Engineer

System Information and Research Div.

G. C. File

# MICHAEL BENSON, MATERIALS ENGINEER \*\*\* SOIL SURVEY STRENGTH TEST REPORT \*\*\*

DATE - 05/01/2017

SEQUENCE NO. - 1

JOB NUMBER - 020587

MATERIAL CODE - SSRV

SPEC. YEAR - 2014

SUPPLIER ID. - 1

COUNTY/STATE - 22 DISTRICT NO. - 02

JOB NAME - ABLES CREEK STR. & APPRS.(S)

\*

BEGIN JOB - END JOB LESS THAN 5

RESILIENT MODULUS

STA. 224+00 6443

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REMARKS =

AASHTO TESTS : T190

# ARKANSAS STATE HIGHWAY AND TRANSPORTATION DEPARTMENT MATERIALS DIVISION

# AASHTO T 307-99 - RESILIENT MODULUS OF SUBGRADE SOILS RECOMPACTED SAMPLES

Job No. Date Sampled: Date Tested: Name of Project: County: Sampled By:	020587 3/20/17 April 27, 2017 ABLES CREEK STR. & APPRS. (S) Code: 22 Name: DREW THORNTON/BATES	Material Code Station No.: Location: Depth:	SSRVPS 224+55 20LT
Lab No.: Sample ID: LATITUDE:	20171040 RV302	AASHTO Class: Material Type (1 or 2): LONGITUDE:	A-6(17) 2
1. Testing Inforn	nation:		<del></del>
	Preconditioning - Permanent Strain > 5% (\text{Y=Yes or Number of Load Sequences Completed (0-1)}	N=No)	N N 15
2. Specimen Info	ormation:		
	Specimen Diameter (in):		
	Тор		3.94
	Middle		3.94
	Bottom		3.94
	Average		3.94
	Membrane Thickness (in):		0.01
	Height of Specimen, Cap and Base (in):		8.05
	Height of Cap and Base (in):		0.00
	Initial Length, Lo (in):		8.05
	Initial Area, Ao (sq. in):		12.12
	Initial Volume, AoLo (cu. in):		97.55
3. Soil Specimer	. Weight:		
3. don opecimer	Weight of Wet Soil Used (g):		3031.70
4. Soil Properties			
	Optimum Moisture Content (%):		16.6
	Maximum Dry Density (pcf):		105.3
	95% of MDD (pcf):		100.0
	In-Situ Moisture Content (%):		N/A
5. Specimen Pro	perties:		
	Wet Weight (g):		3031.70
	Compaction Moisture content (%):		16.7
	Compaction Wet Density (pcf):		118.42
	Compaction Dry Density (pcf):		101.47
	Moisture Content After Mr Test (%):		16.7
6. Quick Shear T	est (Y=Yes, N=No, N/A=Not Applicable):		#VALUE!
7. Resilient Mod	ulus, Mr:	10752(S	c)^-0.29096(S3)^0.17416
8. Comments	ş		
9. Tested By:	GW	Date: April 27, 2017	

# ARKANSAS STATE HIGHWAY AND TRANSPORTATION DEPARTMENT **MATERIALS DIVISION**

# AASHTO T 307-99 - RESILIENT MODULUS OF SUBGRADE SOILS RECOMPACTED SAMPLES

SSRVPS

224+55 20LT

Material Code Station No.: Location: ABLES CREEK STR. & APPRS. (S) April 27, 2017 3/20/17 020587 Name of Project: Date Sampled: Date Tested: Job No.

Name: THORNTON/BATES Code: 22 20171040 RV302 Sampled By: Sample ID: Lab No.: County:

LATITUDE:

Material Type (1 or 2): 2 LONGITITIE 0-5

Depth:

DADAMETED	Chamber Confining	Nominal Maximum	Actual Applied	Actual Applied	Actual Applied	Actual Applied	Actual Applied	Actual Applied	Average Recov Def.	Resilient Strain	Resilient Modulus
	omeest	Stress		Cyclic Load	Load	Axial	Stress	Stress	and 2		
						Stress					
DESIGNATION	Š	Scyclic	Р <sub>мах</sub>	P <sub>cyclic</sub>	Pcontact	S <sub>max</sub>	Scyclic	Scontact	H <sub>avg</sub>	3ر	M
NIT	psi	psi	sql	sql	sql	psi	psi	psi	<u>.</u> ⊑	in/in	psi
Sequence 1	0.9	2.0	25.2	22.3	2.8	2.1	1.8	0.2	0.00121	0.00015	12,242
Sequence 2	0.9	4.0	47.2	44.3	2.8	3.9	3.7	0.2	0.00262	0.00033	11,216
Sequence 3	0.9	0.9	69.2	9.59	3.7	5.7	5.4	0.3	0.00447	0.00056	9,747
Sequence 4	0.9	8.0	91.1	85.1	6.1	7.5	7.0	0.5	0.00699	0.00087	8,085
Sequence 5	0.9	10.0	111.9	103.4	8.5	9.2	8.5	0.7	0.00980	0.00122	7,007
Sequence 6	4.0	2.0	25.0	22.2	2.8	2.1	1.8	0.2	0.00138	0.00017	10,670
Sequence 7	4.0	4.0	46.8	43.9	2.9	3.9	3.6	0.2	0.00299	0.00037	9,740
Sequence 8	4.0	0.9	67.9	65.0	2.9	5.6	5.4	0.2	0.00496	0.00062	8,710
Sednence 9	4.0	8.0	90.1	85.0	5.2	7.4	7.0	0.4	0.00727	0.00090	7,765
Sequence 10	4.0	10.0	111.5	103.9	7.6	9.2	9.8	9.0	0.00997	0.00124	6,923
Sequence 11	2.0	2.0	25.0	22.1	2.8	2.1	1.8	0.2	0.00154	0.00019	9,530
Sequence 12	2.0	4.0	46.5	43.6	2.9	3.8	3.6	0.2	0.00336	0.00042	8,629
Sequence 13	2.0	0.9	67.2	64.4	2.8	5.5	5.3	0.2	0.00550	0.00068	7,772
Sequence 14	2.0	8.0	88.4	84.1	4.3	7.3	6.9	0.4	0.00790	0.00098	7,074
Sequence 15	2.0	10.0	109.9	103.3	9.9	9.1	8.5	0.5	0.01065	0.00132	6,443

April 27, 2017 DATE DATE REVIEWED BY TESTED BY

#### ARKANSAS STATE HIGHWAY AND TRANSPORTATION DEPARTMENT MATERIALS DIVISION

#### **AASHTO T 307-99 - RESILIENT MODULUS OF SUBGRADE SOILS** RECOMPACTED / THINWALL TUBE SAMPLES

Job No. 020587 **Material Code SSRVPS** 

Date Sampled: 3/20/17 **Station No.: 224+55** 

**Date Tested:** April 27, 2017 Location: 20LT

Name of Project: ABLES CREEK STR. & APPRS. (S)

County:

Code: 22 Name: DREW

Sampled By:

THORNTON/BATES

Depth: 0-5

Lab No.:

20171040

AASHTO Class: A-6(17)

Sample ID: RV302

Material Type (1 or 2): 2

LATITUDE:

LONGITUDE:

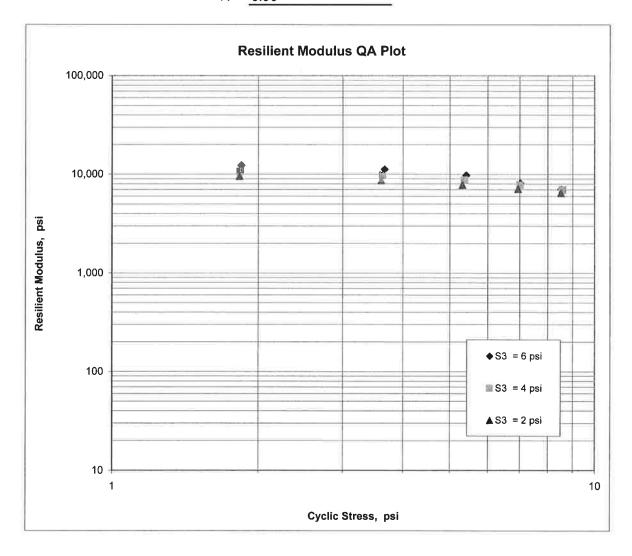
$$M_R = K1 (S_C)^{K2} (S_3)^{K5}$$

$$K1 = 10,752$$

$$K2 = -0.29096$$

$$K5 = 0.17416$$

$$R^2 = 0.90$$



JOB: 020587

Arkansas State Highway Transporation Department

JOB NAME: ABLES CREEK STR. & APPRS.(S)

**Materials Division** 

COUNTY NO. 22 DATE TESTED 4/28/2017 Michael Benson, Materials Engineer

STA.#	LOC.	DEPTH	COLOR	#4	#10	#40	#80	#200	L.L.	<i>P.I.</i>	SOIL CLASS	<i>LAB</i> #:	%MOISTURE
224+55	20 LT	0-5	BROWN	94	93	89	87	82	35	23	A-6(17)	RV302	
214+00	06 RT	0-5	BROWN	84	80	72	67	62	36	24	A-6(11)	S298	15
214+00	18 RT	0-5	BROWN	99	97	95	92	88	35	22	A-6(18)	S299	24
224+59	06 LT	0-5	BROWN	97	95	87	83	78	33	21	A-6(14)	S300	21
224+59	18 LT	0-5	BROWN	100		Sul les		91	42	29	A-7-6(26)	S301	23

comments: W-MULTIPLE LAYERS

Tuesday, May 02, 2017

JOB: JOB NAME: ABLES CREEK STR. & APPRS.(S) 020587

Arkansas State Highway Transporation Department

Materials Division

DATE TESTED 4/28/2017

STA.# LOC. COUNTY NO. 22

Michael Benson, Materials Engineer

STA.# LOC.	LOC.			PAVEMENT SOUNDINGS
214+00	06 RT	214+00 06 RT ACHMSC	ACHMSC	SOIL CEMENT
		3.0W	8.0	1
214+00	18 RT	214+00 18 RT ACHMSC	ACHMSC	SOIL CEMENT
		1	9	1
224+59	06 LT	ACHMSC	ACHMSC	SOIL CEMENT
		2.5W	Î	5.51

comments: W=MULTIPLE LAYERS

Tuesday, May 02, 2017

#### MICHAEL BENSON, MATERIALS ENGINEER

\*\*\* SOIL SURVEY / PAVEMENT SOUNDING TEST REPORT \*\*\*

DATE - 05/ JOB NUMBER - 020 FEDERAL AID NO TO PURPOSE - SOI SPEC. REMARKS - NO SUPPLIER NAME - STAI NAME OF PROJECT - A PROJECT ENGINEER - N PIT/QUARRY - ARKAN LOCATION - DREW SAMPLED BY - THORNT SAMPLE FROM - TEST MATERIAL DESC SOI	587 BE ASSI L SURVE SPECIFI TE BLES CR OT APPL SAS COUNTY ON/BATE HOLE	Y SAMPLE CATION CHEC EEK STR. & ICABLE	CK APPRS.(S)		MATERIA SPEC. Y SUPPLIE COUNTY/ DISTRIC  DATE SA DATE RE DATE TE	R ID 1 STATE - 2	SRVPS 014 2 2 3/20/17 3/22/17
LAB NUMBER  SAMPLE ID  TEST STATUS  STATION  LOCATION  DEPTH IN FEET  MAT'L COLOR  MAT'L TYPE  LATITUDE DEG-MIN-:	- - - - - - - - SEC -	20171036 \$298 INFORMATIO 214+00 06 RT 0-5 BROWN	ON ONLY -	20171037 S299 INFORMATIO 214+00 18 RT 0-5 BROWN	ON ONLY	- 224+59 - 06 LT - 0-5 - BROWN - 33 44	
% PASSING 2 1 1/2 3/4	IN IN IN IN 4 - 10 - 40 - 80 -	100 86 84 80 72 67 62	- - - - - - - -	100 99 97 95 92 88		- 100 - 99 - 97 - 95 - 87 - 83 - 78	
LIQUID LIMIT PLASTICITY INDEX AASHTO SOIL UNIFIED SOIL % MOISTURE CONTENT ACHMSC ACHMSC SOIL CEMENT	(IN) - (IN) - (IN) - - -	36 24 A-6(11) 15.0 3.0W 8.0		35 22 A-6(18) 24.0		33 21 A-6(14) 21.0 2.5W 	

REMARKS - W=MULTIPLE LAYERS

-

AASHTO TESTS : T24 T88 T89 T90 T265

#### MICHAEL BENSON, MATERIALS ENGINEER

\*\*\* SOIL SURVEY / PAVEMENT SOUNDING TEST REPORT \*\*\*

DATE - 05/02/ JOB NUMBER - 020587  FEDERAL AID NO TO BE .  PURPOSE - SOIL S:  SPEC. REMARKS - NO SPE.  SUPPLIER NAME - STATE  NAME OF PROJECT - ABLE  PROJECT ENGINEER - NOT .  PIT/QUARRY - ARKANSAS  LOCATION - DREW COU.  SAMPLED BY - THORNTON/  SAMPLE FROM - TEST HOL  MATERIAL DESC SOIL S	ASSIGNED JRVEY SAMPLE CIFICATION CHECK S CREEK STR. & APPRS.( APPLICABLE NTY BATES E	S)	SEQUENCE NO 2  MATERIAL CODE - SSRVPS  SPEC. YEAR - 2014  SUPPLIER ID 1  COUNTY/STATE - 22  DISTRICT NO 02  DATE SAMPLED - 03/20/17  DATE RECEIVED - 03/22/17  DATE TESTED - 04/28/17  GS
LAB NUMBER SAMPLE ID TEST STATUS STATION LOCATION DEPTH IN FEET	- 20171039 - S301 - INFORMATION ONLY - 224+59 - 18 LT - 0-5		** ** ** ** ** ** ** ** ** ** ** ** **
MAT'L COLOR MAT'L TYPE LATITUDE DEG-MIN-SEC LONGITUDE DEG-MIN-SEC	- 33 44 14.60	= = = =	- - - -
% PASSING 2 IN 1 1/2 IN 3/4 IN 3/8 IN NO. 4 NO. 10 NO. 40 NO. 80 NO. 200	   . 100 - -	- - - - - -	
LIQUID LIMIT PLASTICITY INDEX AASHTO SOIL UNIFIED SOIL % MOISTURE CONTENT	- 42 - 29 - A-7-6(26) - 23.0 		

REMARKS - W-MULTIPLE LAYERS

30 40

AASHTO TESTS : T24 T88 T89 T90 T265

:

# MICHAEL BENSON, MATERIALS ENGINEER \*\*\* SOIL SURVEY / PAVEMENT SOUNDING TEST REPORT \*\*\*

DATE - 05/01/1  JOB NUMBER - 020587  FEDERAL AID NO TO BE A  PURPOSE - SOIL SU  SPEC. REMARKS - NO SPEC  SUPPLIER NAME - STATE  NAME OF PROJECT - ABLES  PROJECT ENGINEER - NOT A  PIT/QUARRY - ARKANSAS  LOCATION - DREW COUN	SSI RVE IFI CR	Y SAMPLE CATION CHECK EEK STR. & APPRS.		SUPPLIER ID COUNTY/STATE -	RV 2014 1 22 02
SAMPLED BY - THORNTON/E	BATE	S		DATE RECEIVED -	03/22/17
SAMPLE FROM - TEST HOLE MATERIAL DESC SOIL SU		Y - RESISTANCE R-	VALUE ACTUAL	DATE TESTED - RESULTS	04/28/17
LAB NUMBER	_	20171040	_	=	
SAMPLE ID	-	RV302	_		
TEST STATUS	-	INFORMATION ONLY	<u> </u>	=	
STATION	_	224+55	_	-	
LOCATION	_	20 LT	_	S#	
DEPTH IN FEET	_	0-5	-		
MAT'L COLOR	_	BROWN	_	-	
MAT'L TYPE	_		_		
LATITUDE DEG-MIN-SEC	_	33 44 14.60	_	<u>_</u>	
LONGITUDE DEG-MIN-SEC	_	91 33 37.20			
% PASSING 2 IN.	_		<del>16</del> 2	=	
1 1/2 IN.	_			· ·	
3/4 IN.	_	100	<del></del>		
3/8 IN.		97	至()	\ <u>~</u>	
NO. 4	_	94	==	=	
NO. 10	_	93	=×: =-:	=	
NO. 40		89	=70 =20		
NO. 80	_	87	##C	-	
NO. 200		82			
LIQUID LIMIT	-	35	-	(35	
PLASTICITY INDEX	-	23	-	<u> </u>	
AASHTO SOIL	_	A-6(17)	_	124	
UNIFIED SOIL	-		-	? <del>=</del>	
% MOISTURE CONTENT	-		_		
	-		-	-	
	-		-	~~	
	-		_	-	
	-		_	-	
	_		_	_	
	_		_	_	
			_	_	
	_		-	-	
	-		-	-	

REMARKS - W=MULTIPLE LAYERS

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AASHTO TESTS : T24 T88 T89 T90 T265