

ARKANSAS DEPARTMENT OF TRANSPORTATION



SUBSURFACE INVESTIGATION

STATE JOB NO. 061509

FEDERAL AID PROJECT NO. CMF-9065(24)

HWY. 367 – HWY. 89 (CABOT) (S)

STATE HIGHWAY 321 SECTION 1

IN LONOKE COUNTY

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ARKANSAS DEPARTMENT OF TRANSPORTATION

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MATERIALS DIVISION

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April 30, 2019

TO: Mr. Rick Ellis, Bridge Engineer

SUBJECT: Job No. 061509
Hwy. 367 – Hwy. 89 (Cabot) (S)
Lonoke County
Route 321, Section 1

Transmitted herewith is a brief summary of the geology and site conditions, D50 analysis test results, summary of percent material passing #200 sieve and Atterberg Limits test results (for liquefaction susceptibility analysis), rock mass rating summary (RMR), unconfined compressive strength test results, and the logs of the borings conducted for the structures and approaches of the above referenced project. The samples obtained by the Standard Penetration Tests were brought to the laboratory and visually classified by experienced lab personnel. The rock cores are available for inspection at the Materials Division.

This project contains three structures on Highway 321, west of Highway 67 in Cabot. The bridges included in this project are: White Oak Branch, Bayou Two Prairie, and Drain Two & UPRR.

White Oak Branch

The proposed plan for the White Oak Branch Bridge is to widen the existing by adding a new bridge section on each side. Based on plans provided by Bridge Design and the depth at which bedrock was encountered it is anticipated that all end bents will be founded on pilings. Preboring may be necessary in order to achieve minimum pile penetration requirements. No borings were obtained at the intermediate bents of the proposed bridges due to steep slopes, high water levels, and access limitations. However, utilizing the data obtained from the end bent borings and correlating the depths at which bedrock was encountered, it is anticipated that competent rock will be slightly deeper than 15 feet below ground level. Based on this information all interior bents could be founded on piling or drilled shafts. Piling should be tipped into competent Shale and preboring may be necessary in order to achieve minimum penetration requirements. Drilled shafts should be founded in competent slightly weathered to unweathered Shale and should be designed based on the values provided in Table 1.

TABLE 1 – Bearing Capacity Recommendations for Drilled Shafts

Nominal Tip Resistance (ksf)	Factored Tip Resistance (ksf)	Nominal Side Resistance (ksf)	Factored Side Resistance (ksf)
196	98	22.9	12.6

Embankment analysis included global stability with seismic design consideration. The proposed embankment geometry provides for a satisfactory Factor of Safety for seismic and static conditions.

Bayou Two Prairie

The proposed Bayou Two Bridge is to be constructed adjacent to the existing, on the south side. Two of the four requested borings were inaccessible due to steep slopes. The two borings that were not obtained were located at: 260+91.50 21.75 Ft. Right of C.L. Construction and 261+16.50 21.75 Ft. Right of C.L. Construction. The two borings that were obtained had to be offset due to conflicts with utilities and steep slopes. The obtained borings are anticipated to represent uniform site conditions and should be adequate to design the proposed pile foundations.

Embankment analysis included global stability with seismic design consideration. The proposed embankment geometry provides for a satisfactory Factor of Safety for seismic and static conditions.

Drain Two & UPRR

The proposed Drain Two & UPRR Bridge is to be constructed adjacent to the existing, on the south side. The obtained borings are anticipated to represent uniform site conditions and should be adequate to design the proposed pile supported footings. Based on plans provided by Bridge Design and the findings from this subsurface investigation, it is anticipated that end bents will be founded on piling and all interior bents will be founded on pile supported footings. The existing east bridge end embankment has settled, exposing the bottom of the pile cap and piles. This area should be repaired during the construction project.

This project is located in a seismic zone with a mild horizontal acceleration of 0.241. Due to the presence of a very soft clay layer in the borings for the east bridge end embankment, reinforcement will be required to satisfy seismic conditions. The soft clay layer is likely responsible for settlement of the existing embankment as discussed above and in the Geology section of this report. The east bridge end embankment shall be strengthened by excavating existing material down to an approximate elevation of 255.5 Ft. A layer of Type 10 separation fabric should be placed followed by a 4 feet thick layer of stone backfill. The remaining embankment material shall be internally reinforced with geogrid. Temporary shoring will be required for this work. Geogrid

placement and specification recommendations are detailed in the attached draft Special Provision: Geosynthetic Internal Reinforced Embankment Construction, along with Figures 1 and 2.

Embankment analysis was based on an embankment height of 25 feet with 3H:1V bridge end slopes. Seismic analysis included a coefficient of horizontal acceleration of 0.241 as provided by Bridge Design. FHWA publication NHI-10-025 Volume II indicates that a value of one-half the horizontal coefficient may be utilized in the design of reinforced embankments. Therefore, a value of 0.1205 was utilized in this design. This configuration provides for a satisfactory Factor of Safety for seismic and static conditions.

The proposed embankment geometry for the west bridge end embankment provides for a satisfactory Factor of Safety for seismic and static conditions.

If you have any questions concerning these recommendations, please contact the Geotechnical Section.



Michael C. Benson
Materials Engineer

MCB:rpt:mlg

cc: State Construction Engineer - Master File Copy
District 6 Engineer
G.C. File

ARKANSAS STATE HIGHWAY AND TRANSPORTATION DEPARTMENT**SPECIAL PROVISION****JOB NO. 061509****GEOSYNTHETIC INTERNAL REINFORCED EMBANKMENT CONSTRUCTION**

DESCRIPTION: This item shall consist of furnishing and installing a geosynthetic internal reinforcement for embankment construction in accordance with the plans and specifications. The geosynthetic internal reinforcement shall be placed as described herein, from the Drain Two & UPRR East Bridge End Slope to Station 275+54.

MATERIALS: Geogrid shall be manufactured as a single layer regular network of integrally connected longitudinal and transverse polymer tensile elements with a geometry that permits significant mechanical interlock with the backfill material. The geogrid structure shall remain dimensionally stable under construction stresses and have high resistance to damage during construction, to ultraviolet degradation, and to all forms of chemical and biological degradation encountered in the soil being reinforced. Geogrid must be evaluated by NTPEP with test results included in the Datamine database.

The geogrid shall also conform in all respects to the following physical requirements:

Provide a geogrid with a minimum tensile strength, T_{allow} as specified in the plans and this Special Provision.

Where: $T_{allow} = T_{ult} / RF$

And $RF = FS_{ID} \times FS_{CR} \times FS_D$

Determine T_{ult} (Ultimate Tensile Strength) according to ASTM D 6637 Method B (note, that the same test shall be used for definition of the geogrid creep reduction factor) and ASTM D 4759.

Determine FS_{ID} , FS_{CR} , and FS_D according to the following:

FS_{ID} Determine the Partial Factor of Safety for Installation Damage from the results of full-scale construction damage tests conducted according to ASTM D 5818. If possible, conduct tests using project-specific backfill and construction placement techniques. Use a default value of 3.0 if no installation damage testing has been conducted. The minimum value for FS_{ID} is 1.1.

FS_{CR} Determine the Partial Factor of Safety for Creep Deformation according to ASTM D 5262. Collect test data for a minimum duration of 10,000 hours for both standard and elevated temperatures. Extrapolate the test results to a 75-year design life as provided in Appendix B of FHWA Publication No. FHWA-NHI-10-025, "Design and Construction of Mechanically Stabilized Walls and Reinforced

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Soil Slopes – Volume II”. If testing has not been conducted, default values for F_{SCR} are:

<u>Polymer Type</u>	<u>F_{SCR}</u>
Polyester	3.00
Polypropylene	5.00
Polyethylene	5.00

F_{SD} The Durability Reduction Factor is dependent on the susceptibility of the geogrid to attack from chemicals, thermal oxidation, hydrolysis, stress cracking, and microorganisms. The minimum reduction factor for the combined effects of chemical and biological degradations is:

<u>Polymer Type</u>	<u>F_{SD}</u>
Polyester	1.20
Polypropylene	1.25
Polyethylene	1.10

Identify, store and handle geogrids according to ASTM D 4873. Limit geogrid exposure to ultraviolet radiation to less than 10 days.

The Contractor shall furnish to the Engineer a production certification that the geogrid supplied meets the respective criteria set forth in these specifications. The certification shall state the name of the manufacturer, product name, style number, chemical composition of the filaments, ribs, or yarns and other information to fully describe the geogrid. The Contractor shall supply test data from an independent laboratory to support certified values submitted.

The embankment material placed within the limits of this Special Provision shall not have greater than 35% passing the #200 sieve AND a Liquid Limit greater than 40. The Contractor shall perform quality control and acceptance sampling and testing of the compacted embankment material for density and moisture content in accordance with Subsection 210.02 and 210.10, at the frequencies established in Section 210. The Contractor shall perform quality control and acceptance sampling and testing of the compacted embankment material for gradation and liquid limit in accordance with Section 306, except that the size of the standard lots will be 3000 cubic yards. There will be no direct payment made for fulfilling these material requirements but compensation shall be considered included in the price bid for Compacted Embankment (Special).

CONSTRUCTION METHODS: The geogrid reinforcement shall be placed to the lines and dimensions shown in the plans or as directed by the Engineer. During clearing and grubbing in

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the embankment area, all organic and deleterious materials, and soft or loose compressible soils shall be excavated and removed from the fill area. Prior to fill placement, the exposed foundation soils shall be proof-rolled to detect any unstable locations, which shall subsequently be compacted or excavated and replaced with compacted fill as directed by the Engineer.

Correct orientation (roll direction) of the geogrids shall be verified by the Engineer. All geogrids shall be placed/unrolled per the manufacturer's recommendations. The Contractor shall provide the engineer detailed installation recommendations from the manufacturer. All geogrid shall be placed to lay flat, pulled tight and pinned or weighted down to hold its position until the subsequent soil layer can be placed.

The first layer of seismic reinforcement geogrid shall be placed directly on top of the stone backfill. Each subsequent layer of geogrid shall be placed at intervals as shown on the plans and shall continue to within two feet of the top of finished subgrade. There shall be a minimum of 4 layers of seismic reinforcement geogrid with a minimum T_{allow} of 5000 lb./ft. Seismic reinforcement geogrid shall be placed in continuous longitudinal strips perpendicular to the face of the embankment. It shall extend from side slope face to the temporary shoring. The curved transition from side slope to bridge end slope shall be constructed of rectangular pieces of grid and shall be overlapped so that the entire embankment is covered.

The first layer of slope reinforcement geogrid shall be placed two feet above the first layer of seismic reinforcement grid after the embankment material has been placed and prepared in accordance with Section 210, Excavation and Embankment of the Standard Specifications for Highway Construction, edition of 2014. This grid shall be placed in continuous longitudinal strips perpendicular to the face of the embankment slope. Each strip shall be a minimum of twenty-five feet long and have a minimum T_{allow} of 1500 lb./ft. The curved transition from side slope to bridge end slope shall be constructed of rectangular pieces of grid. Grid shall be overlapped so that the entire embankment is covered.

Overlaps of geogrid between rolls shall be located at no less than 30 feet from the finished slope surface. Geogrid shall be overlapped a minimum of 5 feet. The number of overlaps shall be limited to one per strip of geogrid. Mechanical bar connections shall be placed per manufacturer's recommendations if required. Adjacent strips of geogrid need not be overlapped.

The embankment fill between layers of geogrid reinforcement shall be prepared in accordance with Section 210, Excavation and Embankment of the Standard Specifications for Highway Construction, Edition of 2014. Reinforcement can be placed directly on the prepared embankment. No special surface treatment will be required. If a sheep's-foot roller is utilized, the imprints are acceptable surfaces for geogrid reinforcement placement.

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Tracked construction equipment shall not be operated directly upon the geogrid. A minimum fill thickness of 6 inches is required prior to operation of tracked vehicles over the geogrid. Turning of tracked vehicles shall be kept to a minimum to prevent tracks from displacing the fill and damaging the geogrid.

Rubber-tired equipment may pass over geogrid reinforcement at slow speeds of less than 10 mph. Sudden breaking and sharp turning shall be avoided.

METHOD OF MEASUREMENT: All embankment material including the geogrid reinforcement will be measured in accordance with Section 210 Excavation and Embankment of the Standard Specifications for Highway Construction, Edition of 2014.

BASIS OF PAYMENT: Placement and compaction of embankment material and installing geogrid reinforcement shall be paid for under the item "Compacted Embankment (Special)", which price shall be full compensation for all costs involved in furnishing all material; for proof rolling ground surfaces or subgrade; for constructing the embankments in accordance with Section 210 and this Special Provision; for quality control and acceptance sampling and testing; and for all labor, tools, equipment, and incidentals necessary to complete the work

Payment will be made under:

<u>Pay Item</u>	<u>Pay Unit</u>
Compacted Embankment (Special)	Cubic Yard

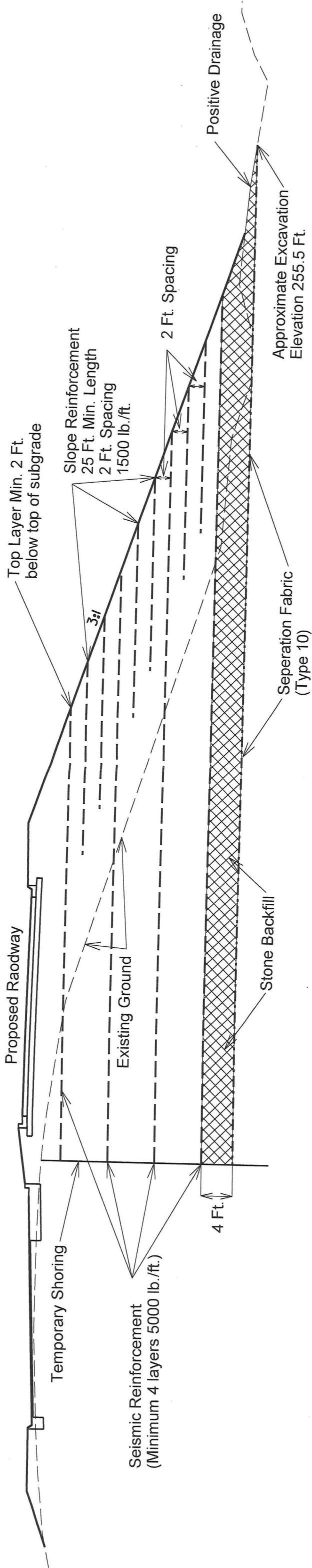
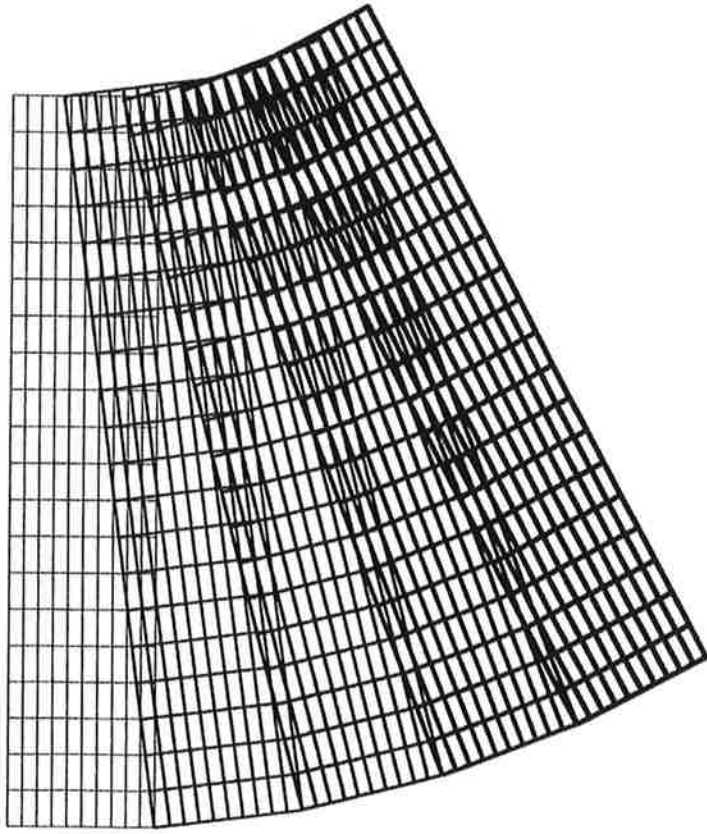


Figure 1 - Reinforced Slope Design

Note: Stone Backfill shall have a 4" to 6" cap of Class 7 as described in Section 207 of the Standard Specifications for Highway Construction, 2014 edition.

Side Slope to End Slope
Geogrid Transition



Geogrid Overlap

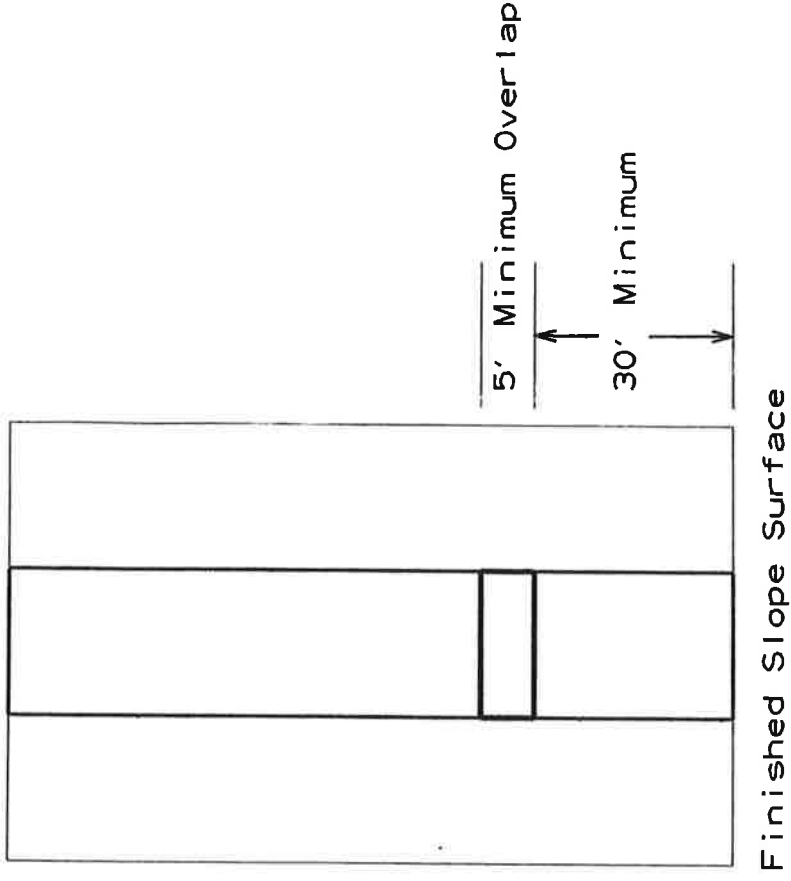


Figure 2 - Geogrid Special Details

GEOLOGY AND SITE CONDITIONS

Job No. 061509

Hwy. 367 – Hwy. 89 (Cabot) (S)

Lonoke County

Route 321 Section 1

Site Conditions

The proposed job contains three bridges. **Bridge 1**, the westernmost bridge, crosses the White Oak Branch of the Bayou Two Prairie, which flows to the south. Bridge 1 is a three span bridge constructed of cast-in-place concrete deck supported by concrete square batter pilings. The guardrail is composed of concrete walls on the bridge and steel leading up to the bridge. Riprap has been placed on the slopes below the bridge abutments. A buried telecommunication line parallels the North side of the roadway and is covered with concrete in the channel. Overhead power lines and a buried gas line parallel the south side of the roadway. The gas line appears to be exposed in the channel. A church and a business with associated parking lots are located on the south and north side of the roadway, down-station of the channel. The area up-station of the channel is moderately to heavily wooded on both sides of the roadway.

Bridge 2 crosses the Bayou Two Prairie and flows to the southeast. The structure is built very similarly to Bridge 1 except, the bents have been placed at a skew to parallel channel flow. Overhead power lines parallel the north side of the bridge. They cross the roadway down-station from the bridge and then parallel the south side of the roadway. Major grid power lines cross the roadway up-station from the bridge. A sewer line and a buried telecommunication line parallel the north side of the roadway. The area around the bridge is moderately to heavily wooded except, along the major grid power line corridor.

Bridge 3 is an 11 span bridge. Railroad tracks run under span 6, an unnamed stream under span 9, and a levee, that parallels the up-station side of the stream, under span 10. The structure is constructed of concrete deck supported by six steel beams and concrete square pilings. The guardrail is composed of concrete walls on the bridge and steel leading up to the bridge. Riprap has been placed on the slopes below the bridge abutments. Overhead power lines, a sewer line, and a buried telecommunication line parallel the north side of the bridge. A buried telecommunication line parallels the down-station side of the railroad tracks. The area around the bridge is moderately to heavily wooded with agricultural fields surrounding the up-station end of the bridge.

Site Geology

The geology of the project alignment changes between Bridge 1 and Bridge 2. **Bridge 1** is located on unconsolidated, primarily clayey deposits mapped as Quaternary alluvial deposits (map symbol Qal). The alluvial deposits overlie shale from the middle part of the Atoka Formation.

The Atoka Formation is a sequence of marine, mostly tan to gray silty sandstones and grayish-black shales. Some rare calcareous beds and siliceous shales are known.

This unit has the largest areal extent of any of the Paleozoic formations in the state. It is the surface rock of the Boston Mountains, dominates the exposures in the Arkansas River Valley and the frontal Ouachita Mountains, and is present in the southern part of the Ouachita Mountains. In the Arkansas River Valley and the frontal Ouachita Mountains, the Atoka Formation has been subdivided into upper, middle, and lower lithic members based on regionally mappable shale or sandstone intervals. The unit locally contains discontinuous streaks of coal and coaly shale in the Boston Mountains and Arkansas River Valley. The Atoka may be up to 25,000 feet thick in the Ouachita Mountains. Shale was encountered in borings at depths ranging from 28 to 32 feet below ground level.

Bridge 2 is also located on alluvial deposits; however, the alluvial deposits overlie the Arkadelphia Formation. In general, the Arkadelphia is mostly a dark-gray to black marl or marly clay with some limy, gray sandstone, gray sandy clay, sandy limestone, concretionary limestone, and white to light-brown impure chalk. The Arkadelphia Formation at the job site consists primarily of marly clay with a few beds of limestone up to 1.2 feet thick. The Arkadelphia was encountered in borings at 45.1 and 50 feet below ground level. The Atoka Formation was encountered below the Arkadelphia Formation at 76.6 and 90 feet below ground level and consisted of shale.

Bridge 3 is located over the same geology as Bridge 2. The Arkadelphia was encountered at depths ranging from approximately 45 to 65 feet below ground (elevation range of 212.5 to 221 feet above MSL). The Atoka Formation was not encountered in the first boring (down-station) due to the higher elevation of the boring location. The Atoka was encountered in the second and third borings at depths ranging from approximately 90.7 to 100.0 feet below ground level. The elevation of the top of the Atoka Formation was too low to be encountered in borings up-station from the third boring. There appeared to be some subsidence under the up-station bridge end cap of Bridge 3. A very soft layer occurs at a depth of 25 feet below ground level in the easternmost boring and may be the cause of the subsidence at the bridge end.



Figure 1. Subsidence under the east bridge end cap.

Scour Potential

The banks of the channel at **Bridge 1** (See Figure 1) consists of lean clay with sand (CL) based on the scour sample taken at the site. The bottom of the channel appears to consist of clay. Due to the cohesion of the clay particles, the sediment has diminished capacity for scour. There is no evidence of scour at the Bridge 1 site.



Figure 2. The stream at Bridge 1 looking upstream.

The channel at **Bridge 2** (See Figure 2) is deeper with taller banks. This may be due to the sediment at this site being slightly coarser. The sample tested for scour consisted of silt with sand (ML). There are some small stream bank collapses, but most of the banks are rather well vegetated. Riprap has been placed on the stream banks under the bridge to prevent erosion (See Figure 3). No evidence of scour was observed at Bridge 2.



Figure 3. The stream at Bridge 2 looking upstream.

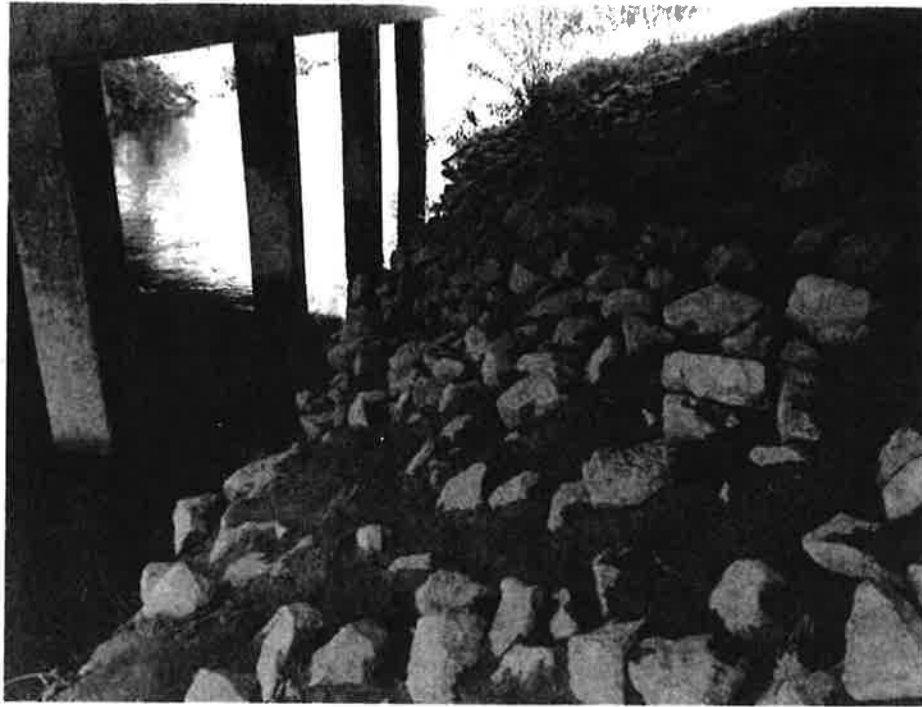


Figure 4. Riprap on stream bank under Bridge 2 looking upstream.

The sediment in the channel at **Bridge 3** (See Figures 4 and 5) is similar to the sediment located at Bridge 1. Based on the scour sample taken at this site, the sediment consists of lean clay (CL). The channel has low banks and the channel is somewhat shallow. Due to the cohesion of the clay particles, the sediment in the channel has a diminished capacity for scour. There is no evidence of scour at the Bridge 3 site.



Figure 5. The stream at Bridge 3 looking upstream.

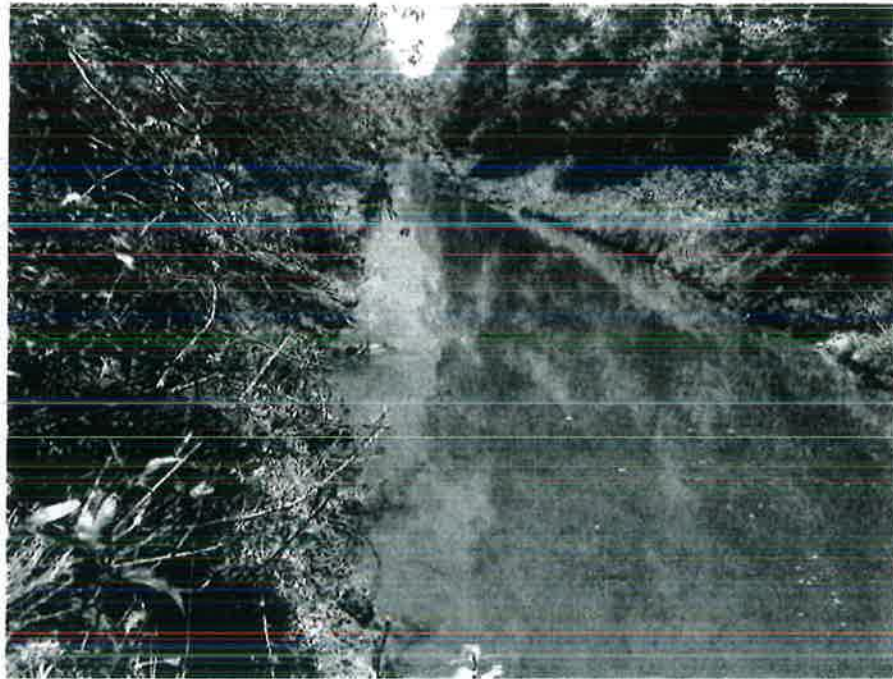


Figure 6. The stream at Bridge 3 looking downstream.

Subsurface Conditions

Based on the results of the borings from Stations 224+28 to 225+64, the subsurface stratigraphy may be generalized as follows:

- 0 to 28 Feet: Consists primarily of moist to wet, soft to hard, brown to gray **clay to sandy clay**.
- 28 to 35 Feet: Varies from wet, medium stiff to hard, brown to gray **sandy clay to sandy clay with gravel** to highly weathered, medium hard, dark gray **shale**.
- 35 to 64.5 Feet: Consists of slightly weathered to unweathered, medium hard, dark gray **shale**.

Based on the results of the borings from Stations 260+74 to 261+65.5, the subsurface stratigraphy may be generalized as follows:

- 0 to 45.1 Feet: Varies from moist to wet, soft to very stiff, brown to gray **sandy clay to clay to very loose to medium dense, brown silty sand to sand with clay**.
- 45.1 to 50 Feet: Varies from wet, stiff, gray **sandy clay to moist, very stiff, dark gray clay with occasional layers of moderately hard, gray limestone**.
- 50 to 76.6 Feet: Consists of moist, very stiff to very hard, dark gray **clay with occasional layers of moderately hard, gray limestone**. One prominent limestone bed occurs in this zone and is approximately one foot thick.

76.6 to 90 Feet: Varies from moist, very stiff to very hard, dark gray **clay with occasional layers of** moderately hard, gray **limestone** to highly weathered to weathered, medium hard, dark gray **shale**.

90 to 101.1 Feet: Consists of highly weathered to slightly weathered, medium hard, dark gray **shale**.

Based on the results of the borings from Stations 265+52.5 to 274+65.5, the subsurface stratigraphy may be generalized as follows:

0 to 45 Feet: Varies from moist to wet, medium stiff to stiff, brown and gray **clay** and **sandy clay** to loose to medium dense, gray **silty sand** to **silty sand with gravel**.

45 to 65 Feet: Varies from moist to wet, medium stiff, brown and gray **sandy clay** to medium dense, gray **silty sand** to moist very stiff to hard, dark gray **clay**.

65 to 90.7 Feet: Consists of moist, very stiff to hard, dark gray **clay**. Some zones within this interval contain thin **limestone** layers.

90.7 to 101.5 Feet: Varies from moist, hard, dark gray **clay** to weathered, medium hard, dark gray **shale to cemented**, dark gray **sandstone**.

**D₅₀ AGGREGATE ANALYSIS
FOR SCOUR CALCULATIONS**

Job No. 061509

Creek Name	Station	Sample Type	Location	Depth (FT)	Soil Description	Aggregate Size (D50) (IN)
White Oak Branch of Bayou Two Prairic	225+00	Creek Bank	Construction Centerline	NA	CL Lean Clay With Sand	Less Than 0.0029
Bayou Two Prairic	261+16.5	Creek Bank	21.75 Ft. Right of Construction Centerline	NA	ML Silt With Sand	Less Than 0.0029
Unnamed Stream	273+00	Creek Bank	21.75 Ft. Left of Construction Centerline	NA	CL Lean Clay	Less Than 0.0029

Lab Test Summary

Project Number: 061509
 Project Name: Hwy. 367 - Hwy. 89 (Cabot) (S)

Station	Location	Depth (ft.)	Plastic Limit	Liquid Limit	Plasticity Index	% Passing No. 200	Unified Soil Classification
274+65.5	6.25' Rt.	4.9	ND				
274+65.5	6.25' Rt.	9.9	15	49	34	62	CL
274+65.5	6.25' Rt.	15.0	14	51	37	76	CH
274+65.5	6.25' Rt.	20.0	12	38	26	70	CL
274+65.5	6.25' Rt.	25.0	17	33	16	89	CL
274+65.5	6.25' Rt.	30.0	15	38	23	92	CL
274+65.5	6.25' Rt.	35.0	14	48	34	89	CL
274+65.5	6.25' Rt.	40.0	11	32	21	77	CL
274+65.5	6.25' Rt.	45.0	12	31	19	72	CL
274+65.5	6.25' Rt.	50.0			NP	42	SC
274+65.5	6.25' Rt.	55.0			NP	28	SM
274+65.5	6.25' Rt.	60.0	24	75	51	98	CH
274+65.5	6.25' Rt.	65.0	21	65	44	99	CH
274+65.5	6.25' Rt.	70.0	23	71	48	98	CH
274+65.5	6.25' Rt.	75.0	24	70	46	93	CH
274+65.5	6.25' Rt.	80.0	23	67	44	97	CH
274+65.5	6.25' Rt.	85.0	19	54	35	95	CH
274+65.5	6.25' Rt.	90.0	22	69	47	98	CH
274+65.5	6.25' Rt.	95.0	21	55	34	96	CH
274+65.5	6.25' Rt.	100.0	20	50	30	90	CH

ROCK MASS RATING SUMMARY
JOB # 080439

SAMPLE #1

Station/Location	224+28/30' LT
Depth (ft)	36.2
Relative Rating	
Uniaxial Compressive Strength	4
RQD	13
Spacing of Joints	20
Condition of Joints	20
Groundwater Conditions	7
Sum	64
Class Number	II
Description	GOOD ROCK

SAMPLE #2

Station/Location	224+28/30' LT
Depth (ft)	39.1
Relative Rating	
Uniaxial Compressive Strength	7
RQD	17
Spacing of Joints	30
Condition of Joints	25
Groundwater Conditions	7
Sum	86
Class Number	I
Description	VERY GOOD ROCK

SAMPLE #3

Station/Location	224+28/30' LT
Depth (ft)	44.5
Relative Rating	
Uniaxial Compressive Strength	0
RQD	20
Spacing of Joints	30
Condition of Joints	25
Groundwater Conditions	7
Sum	82
Class Number	I
Description	VERY GOOD ROCK

SAMPLE #4

Station/Location	224+28/30' LT
Depth (ft)	50
Relative Rating	
Uniaxial Compressive Strength	7
RQD	20
Spacing of Joints	25
Condition of Joints	20
Groundwater Conditions	7
Sum	79
Class Number	II
Description	GOOD ROCK

SAMPLE #5

Station/Location	224+28/30' LT
Depth (ft)	54.2
Relative Rating	
Uniaxial Compressive Strength	0
RQD	20
Spacing of Joints	25
Condition of Joints	20
Groundwater Conditions	7
Sum	72
Class Number	II
Description	GOOD ROCK

SAMPLE #6

Station/Location	224+31/27' RT
Depth (ft)	35.5
Relative Rating	
Uniaxial Compressive Strength	7
RQD	13
Spacing of Joints	25
Condition of Joints	20
Groundwater Conditions	7
Sum	72
Class Number	II
Description	GOOD ROCK

SAMPLE #7

Station/Location	224+31/27' RT
Depth (ft)	37.0
Relative Rating	
Uniaxial Compressive Strength	4
RQD	13
Spacing of Joints	25
Condition of Joints	20
Groundwater Conditions	7
Sum	69
Class Number	II
Description	GOOD ROCK

SAMPLE #8

Station/Location	224+31/27' RT
Depth (ft)	42.8
Relative Rating	
Uniaxial Compressive Strength	7
RQD	17
Spacing of Joints	25
Condition of Joints	20
Groundwater Conditions	7
Sum	76
Class Number	II
Description	GOOD ROCK

SAMPLE #9

Station/Location	224+31/27' RT
Depth (ft)	48.4
Relative Rating	
Uniaxial Compressive Strength	7
RQD	17
Spacing of Joints	25
Condition of Joints	20
Groundwater Conditions	7
Sum	76
Class Number	II
Description	GOOD ROCK

SAMPLE #10

Station/Location	224+31/27' RT
Depth (ft)	52.5
Relative Rating	
Uniaxial Compressive Strength	7
RQD	17
Spacing of Joints	25
Condition of Joints	25
Groundwater Conditions	7
Sum	81
Class Number	I
Description	VERY GOOD ROCK

SAMPLE #11

Station/Location	225+64/27' LT
Depth (ft)	34.2
Relative Rating	
Uniaxial Compressive Strength	4
RQD	17
Spacing of Joints	10
Condition of Joints	20
Groundwater Conditions	7
Sum	58
Class Number	III
Description	FAIR ROCK

SAMPLE #12

Station/Location	225+64/27' LT
Depth (ft)	38.6
Relative Rating	
Uniaxial Compressive Strength	4
RQD	20
Spacing of Joints	20
Condition of Joints	20
Groundwater Conditions	7
Sum	71
Class Number	II
Description	GOOD ROCK

SAMPLE #13

Station/Location	225+64/27' LT
Depth (ft)	42.2
Relative Rating	
Uniaxial Compressive Strength	0
RQD	20
Spacing of Joints	20
Condition of Joints	20
Groundwater Conditions	7
Sum	67
Class Number	II
Description	GOOD ROCK

SAMPLE #14

Station/Location	225+64/27' LT
Depth (ft)	48.6
Relative Rating	
Uniaxial Compressive Strength	7
RQD	20
Spacing of Joints	20
Condition of Joints	20
Groundwater Conditions	7
Sum	74
Class Number	II
Description	GOOD ROCK

SAMPLE #15

Station/Location	225+64/27' LT
Depth (ft)	53.2
Relative Rating	
Uniaxial Compressive Strength	4
RQD	20
Spacing of Joints	25
Condition of Joints	20
Groundwater Conditions	7
Sum	76
Class Number	II
Description	GOOD ROCK

SAMPLE #16

Station/Location	225+53/30' RT
Depth (ft)	42.9
Relative Rating	
Uniaxial Compressive Strength	4
RQD	17
Spacing of Joints	25
Condition of Joints	20
Groundwater Conditions	7
Sum	73
Class Number	II
Description	GOOD ROCK

SAMPLE #17

Station/Location	225+53/30' RT
Depth (ft)	48.4
	Relative Rating
Uniaxial Compressive Strength	7
RQD	13
Spacing of Joints	20
Condition of Joints	20
Groundwater Conditions	7
Sum	67
Class Number	II
Description	GOOD ROCK

SAMPLE #18

Station/Location	225+53/30' RT
Depth (ft)	53.9
	Relative Rating
Uniaxial Compressive Strength	4
RQD	8
Spacing of Joints	20
Condition of Joints	20
Groundwater Conditions	7
Sum	59
Class Number	III
Description	FAIR ROCK

SAMPLE #19

Station/Location	225+53/30' RT
Depth (ft)	58.8
	Relative Rating
Uniaxial Compressive Strength	4
RQD	13
Spacing of Joints	20
Condition of Joints	20
Groundwater Conditions	7
Sum	64
Class Number	II
Description	GOOD ROCK

SAMPLE #20

Station/Location	225+53/30' RT
Depth (ft)	63.2
	Relative Rating
Uniaxial Compressive Strength	4
RQD	13
Spacing of Joints	20
Condition of Joints	20
Groundwater Conditions	7
Sum	64
Class Number	II
Description	GOOD ROCK

SAMPLE #21

Station/Location	
Depth (ft)	
	Relative Rating
Uniaxial Compressive Strength	
RQD	
Spacing of Joints	
Condition of Joints	
Groundwater Conditions	
Sum	
Class Number	
Description	

SAMPLE #22

Station/Location	
Depth (ft)	
	Relative Rating
Uniaxial Compressive Strength	
RQD	
Spacing of Joints	
Condition of Joints	
Groundwater Conditions	
Sum	
Class Number	
Description	

SAMPLE #23

Station/Location	
Depth (ft)	
	Relative Rating
Uniaxial Compressive Strength	
RQD	
Spacing of Joints	
Condition of Joints	
Groundwater Conditions	
Sum	
Class Number	
Description	

SAMPLE #24

Station/Location	
Depth (ft)	
	Relative Rating
Uniaxial Compressive Strength	
RQD	
Spacing of Joints	
Condition of Joints	
Groundwater Conditions	
Sum	
Class Number	
Description	

Rock Core Unconfined Compression Test Summary

Project Number: 061509
 Project Name: Hwy. 367 - Hwy. 89 (Cabot) (S)
 Date Tested: 3/28/2019

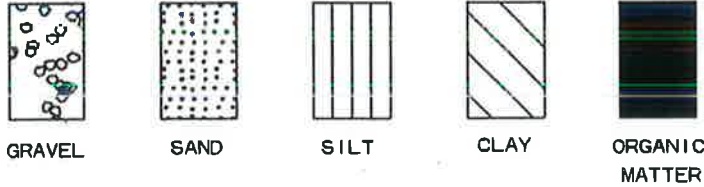
Station	Location	Sample No.	Depth (ft.)	Diameter (in)	Height (in)	Total Load (lbs.)	Correction Factor	Stress (psi)	Remarks
224+28	30' Lt.	1	36.2	1.75	3.57	12,040	1.00	5,006	
224+28	30' Lt.	2	39.1	1.75	3.80	20,850	1.00	8,668	
224+28	30' Lt.	3	44.5	1.75	-				Broke in Saw
224+28	30' Lt.	4	50.0	1.75	4.00	19,020	1.00	7,908	
224+28	30' Lt.	5	54.2	1.75	-				Broke in Saw
224+31	27' Rt.	6	35.5	1.75	3.95	18,340	1.00	7,625	
224+31	27' Rt.	7	37.0	1.75	3.53	17,800	1.00	7,400	
224+31	27' Rt.	8	42.8	1.75	3.45	24,290	1.00	10,098	
224+31	27' Rt.	9	48.4	1.75	3.55	27,760	1.00	11,541	
224+31	27' Rt.	10	52.5	1.75	4.65	24,140	1.00	10,036	
225+64	27' Lt.	11	34.2	1.75	4.05	16,450	1.00	6,839	
225+64	27' Lt.	12	38.6	1.75	4.85	9,960	1.00	4,141	
225+64	27' Lt.	13	42.2	1.75	-				Broke in Saw
225+64	27' Lt.	14	48.6	1.75	4.50	20,240	1.00	8,415	
225+64	27' Lt.	15	53.2	1.75	4.05	16,270	1.00	6,764	
225+53	30' Rt.	16	42.9	1.75	3.95	12,020	1.00	4,997	
225+53	30' Rt.	17	48.4	1.75	4.30	18,820	1.00	7,824	
225+53	30' Rt.	18	53.9	1.75	4.70	8,840	1.00	3,675	
225+53	30' Rt.	19	58.8	1.75	3.90	10,930	1.00	4,544	
225+53	30' Rt.	20	63.2	1.75	3.40	10,630	1.00	4,419	

* Please note any broken samples, fractures or other characteristics of sample in Remarks.

LEGEND

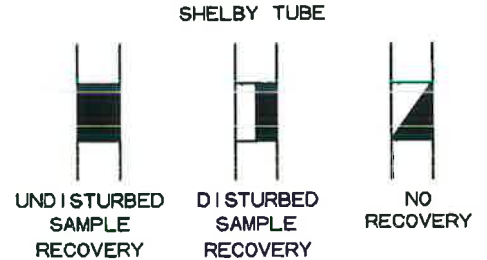
SOIL TYPES

(SHOWN IN SYMBOL COLUMN)
(PREDOMINANT TYPE SHOWN HEAVY)



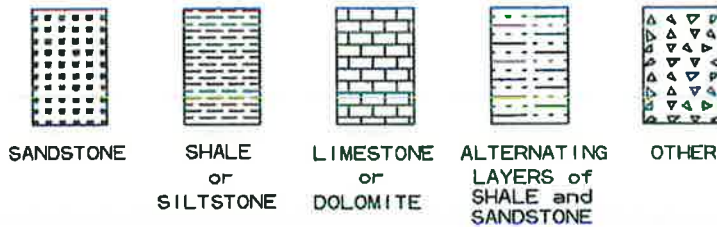
SAMPLER TYPES

(SHOWN IN SAMPLE COLUMN)



ROCK TYPES

(SHOWN IN SYMBOL COLUMN)



SPLIT SPOON

ROCK CORING



TERMS DESCRIBING CONSISTENCY OR CONDITION

GRANULAR SOIL		CLAY		CLAY-SHALE		SHALE	
*N' Value	Density	*N' Value	Consistency	*N' Value	Consistency	*N' Value	Consistency
0-4	Very Loose	0-1	Very Soft	0-1	Very Soft		
5-10	Loose	2-4	Soft	2-4	Soft	31-60	Soft
11-30	Medium Dense	5-8	Medium Stiff	5-8	Medium Stiff	Over 60	
31-50	Dense	9-15	Stiff	9-15	Stiff	More than 2'	
Over 50	Very Dense	16-30	Very Stiff	16-30	Very Stiff	Penetration	
		31-60	Hard	31-60	Hard	in 60 Blows	Medium Hard
		Over 60	Very Hard	Over 60	Very Hard	Less than 2'	
						Penetration	
						in 60 Blows	Hard

1. Ground water elevations indicated on boring logs represent ground water elevations at date or time shown on boring log. Absence of water surface implies that no ground water data is available but does not necessarily mean that ground water will not be encountered at locations or within the vertical reaches of these borings.
2. Borings represent subsurface conditions at their respective locations for their respective depths. Variations in conditions between or adjacent to boring locations may be encountered.
3. Terms used for describing soils according to their texture or grain size distribution are in accordance with the Unified Soil Classification System.

Standard Penetration Test – Driving a 2.0” O.D., 1-3/8” I.D. sampler a distance of 1.0 foot into undisturbed soil with a 140-pound hammer free falling a distance of 30 inches. It is customary to drive the spoon 6.0 inches to seat into undisturbed soil, and then perform the test. The number of hammer blows for seating the spoon and performing the test are recorded for each 6 inches of penetration on the drill log. The field “N” Value (N_f) can be obtained by

adding the bottom two numbers for example: $\frac{6}{8-9} \Rightarrow 8+9 = 17 \text{ blows/ft}$. The “N” Value corrected to 60% efficiency (N_{60}) can be obtained by multiplying N_f by the hammer correction factor published on the boring log.

KEY TO SYMBOLS

Symbol Description

Symbol Description

Strata symbols



clay



sandy clay



shale with clay seams



shale/siltstone



silt/cemented silt



sandy clay with gravel



silty sand



limestone/dolomite



claystone with frequent
limestone layers
limestone with frequent
claystone layers



silty clay



clayey sand



cavity



silty sand with gravel



cemented sand/sandstone

Soil Samplers



Split spoon -
sample recovery



Rock coring

Notes:

1. Exploratory borings were drilled on March 14 and 18, 2018 using a 4-inch diameter continuous flight power auger.
2. No free water was encountered at the time of drilling or when re-checked the following day.
3. Boring locations were taped from existing features and elevations extrapolated from the final design schematic plan.
4. These logs are subject to the limitations, conclusions, and recommendations in this report.
5. Results of tests conducted on samples recovered are reported on the logs.

**ARKANSAS DEPARTMENT OF TRANSPORTATION
MATERIALS DIVISION - GEOTECHNICAL SEC.**

BORING NO. 1
PAGE 1 OF 2

JOB NO. 061509 Lonoke County
JOB NAME: Hwy. 367 - Hwy. 89 (Cabot)(S)
Route 321 Section 1
STATION: 224+28
LOCATION: 30' Left of Construction Centerline
LOGGED BY: Austin Dillman / Donnie Thornton

DATE: February 12, 2019
TYPE OF DRILLING:
Hollow Stem Auger - Diamond Core
EQUIPMENT: Acker 2094
HAMMER CORRECTION FACTOR: N/A

COMPLETION DEPTH: 55.5

DEPTH FT.	SYMBOL	SAMPLES	DESCRIPTION OF MATERIAL	SOIL GROUP	PLASTIC LIMIT	% MOIST.	LIQUID LIMIT	DRY WEIGHT	LBS PER CU.FT.	NO. OF BLOWS PER 6-IN.	% T C R	% R Q D
			SURFACE ELEVATION: 265.3									
5			Moist, Stiff, Brown and Gray Clay							4 5-4		
10			Moist, Very Stiff, Brown Clay							6 7-9		
15			Wet, Soft, Light Brown Clay							2 2-2		
20			Wet, Stiff, Brown and Gray Sandy Clay*							4 5-6		
25			Wet, Very Stiff, Brown Sandy Clay with Trace Gravel							4 8-13		
30			SHALE									
			SHALE - Highly Weathered, Medium Hard, Dark Gray							47 62-27 (7")		
35			SHALE - Highly Weathered, Medium Hard, Dark								50	0

REMARKS: * A water stratum was encountered at 23.1' below ground level.
White Oak Branch of the Bayou Two Prairie

**ARKANSAS DEPARTMENT OF TRANSPORTATION
MATERIALS DIVISION - GEOTECHNICAL SEC.**

BORING NO. 1
PAGE 2 OF 2

JOB NO. 061509 Lonoke County
JOB NAME: Hwy. 367 - Hwy. 89 (Cabot)(S)
Route 321 Section 1
STATION: 224+28
LOCATION: 30' Left of Construction Centerline
LOGGED BY: Austin Dillman / Donnie Thornton

DATE: February 12, 2019
TYPE OF DRILLING:
Hollow Stem Auger - Diamond Core
EQUIPMENT: Acker 2094

HAMMER CORRECTION FACTOR: N/A

COMPLETION DEPTH: 55.5

DEPTH FT.	SYMBOL	SAMPLES	DESCRIPTION OF MATERIAL	SOIL GROUP	PLASTIC LIMIT	% MOIST.	LIQUID LIMIT	DRY WEIGHT	LBS PER CU.FT.	NO. OF BLOWS PER 6-IN.	% T C R	% R Q D
			SURFACE ELEVATION: 265.3									
			Gray									
40			SHALE - Slightly Weathered, Medium Hard, Dark Gray								94	64
45											100	90
50			SHALE - Unweathered, Medium Hard, Dark Gray								95	95
55											100	98
60			Boring Terminated									
65												
70												

REMARKS: * A water stratum was encountered at 23.1' below ground level.
White Oak Branch of the Bayou Two Prairie

**ARKANSAS DEPARTMENT OF TRANSPORTATION
MATERIALS DIVISION - GEOTECHNICAL SEC.**

BORING NO. 2
PAGE 1 OF 2

JOB NO. 061509 Lonoke County
JOB NAME: Hwy. 367 - Hwy. 89 (Cabot)(S)
Route 321 Section 1
STATION: 224+31
LOCATION: 27' Right of Construction Centerline
LOGGED BY: Austin Dillman

DATE: February 6, 2019
TYPE OF DRILLING:
Hollow Stem Auger - Diamond Core
EQUIPMENT: Acker 2094

HAMMER CORRECTION FACTOR: N/A

COMPLETION DEPTH: 54.7

DEPTH FT.	SYMBOL	SAMPLES	DESCRIPTION OF MATERIAL	SOIL GROUP	PLASTIC LIMIT	% MOIST.	LIQUID LIMIT	DRY WEIGHT	LBS PER CU.FT.	NO. OF BLOWS PER 6-IN.	% F C R	% R O D
			SURFACE ELEVATION: 265.2									
5			Moist, Stiff, Brown and Gray Clay							7 7-6		
10			Moist, Very Stiff, Brown and Gray Clay							4 9-13		
15			Moist, Loose, Light Brown Silt							3 4-5		
20			Moist, Stiff, Brown Sandy Clay							3 4-7		
25			Wet, Very Stiff, Brown Sandy Clay with Gravel							5 7-11		
30			Wet, Hard, Brown Sandy Clay with Gravel							6 18-29		
35			SHALE									

REMARKS: White Oak Branch of the Bayou Two Prairie

ARKANSAS DEPARTMENT OF TRANSPORTATION MATERIALS DIVISION - GEOTECHNICAL SEC.	BORING NO. 2 PAGE 2 OF 2
JOB NO. 061509 Lonoke County JOB NAME: Hwy. 367 - Hwy. 89 (Cabot)(S) Route 321 Section 1 STATION: 224+31 LOCATION: 27' Right of Construction Centerline LOGGED BY: Austin Dillman	DATE: February 6, 2019 TYPE OF DRILLING: Hollow Stem Auger - Diamond Core EQUIPMENT: Acker 2094 HAMMER CORRECTION FACTOR: N/A

COMPLETION DEPTH: 54.7

DEPTH FT.	SYMBOL	SAMPLES	DESCRIPTION OF MATERIAL	SOIL GROUP	PLASTIC LIMIT	% MOIST.	LIQUID LIMIT	DRY WEIGHT	LBS PER CU.FT.	NO. OF BLOWS PER 6-IN.	% T C R	% R Q D
			SURFACE ELEVATION: 265.2									
40			SHALE - Unweathered, Medium Hard, Dark Gray							10 (0")	84	64
45											97	74
50											96	88
55			SHALE - Unweathered, Medium Hard, Occasional Fractures, Dark Gray								97	86
60			Boring Terminated									
65												
70												

REMARKS: White Oak Branch of the Bayou Two Prairie

**ARKANSAS DEPARTMENT OF TRANSPORTATION
MATERIALS DIVISION - GEOTECHNICAL SEC.**

BORING NO. 3
PAGE 1 OF 2

JOB NO. 061509 Lonoke County
JOB NAME: Hwy. 367 - Hwy. 89 (Cabot)(S)
Route 321 Section 1
STATION: 225+64
LOCATION: 27' Left of Construction Centerline
LOGGED BY: Austin Dillman / Donnie Thornton

DATE: February 13, 2019
TYPE OF DRILLING:
Hollow Stem Auger - Diamond Core
EQUIPMENT: Acker 2094
HAMMER CORRECTION FACTOR: N/A

COMPLETION DEPTH: 54.1

DEPTH FT.	SYMBOL	SAMPLES	DESCRIPTION OF MATERIAL	SOIL GROUP	PLASTIC LIMIT	% MOIST.	LIQUID LIMIT	DRY WEIGHT	LBS PER CU.FT.	NO. OF BLOWS PER 6-IN.	% F C R	% R Q D
			SURFACE ELEVATION: 265.0									
5			Moist, Soft, Gray Clay							0 2-2		
10			Moist, Very Stiff, Brown and Gray Sandy Clay							5 11-12		
15			Wet, Medium Stiff, Brown Clay with Sand							1 2-3		
20			Wet, Medium Stiff, Brown Sandy Clay*							2 3-3		
25			Wet, Stiff, Brown Sandy Clay							4 5-6		
30			SHALE - Medium Hard, Highly Weathered, Dark Gray							47 10 (0")		
			SHALE - Medium Hard, Weathered with Highly Weathered Layers, Dark Gray								53	0
35												

REMARKS: *A water stratum was encountered at 24.1' below ground level.
White Oak Branch of the Bayou Two Prairie

**ARKANSAS DEPARTMENT OF TRANSPORTATION
MATERIALS DIVISION - GEOTECHNICAL SEC.**

BORING NO. 3
PAGE 2 OF 2

JOB NO. 061509 Lonoke County
JOB NAME: Hwy. 367 - Hwy. 89 (Cabot)(S)
Route 321 Section 1
STATION: 225+64
LOCATION: 27' Left of Construction Centerline
LOGGED BY: Austin Dillman / Donnie Thornton

DATE: February 13, 2019
TYPE OF DRILLING:
Hollow Stem Auger - Diamond Core
EQUIPMENT: Acker 2094
HAMMER CORRECTION FACTOR: N/A

COMPLETION DEPTH: 54.1

DEPTH FT.	SYMBOL	SAMPLES	DESCRIPTION OF MATERIAL	SOIL GROUP	PLASTIC LIMIT	% MOIST.	LIQUID LIMIT	DRY WEIGHT	LBS PER CU.FT.	NO. OF BLOWS PER 6-IN.	% F C R	% R Q D
			SURFACE ELEVATION: 265.0									
			SHALE - Medium Hard, Slightly Weathered, Frequent Fractures, Dark Gray								100	80
40											100	99
45			SHALE - Medium Hard, Unweathered, Occasional Fractures, Dark Gray								96	88
50											100	100
55			Boring Terminated									
60												
65												
70												

REMARKS: *A water stratum was encountered at 24.1' below ground level.
White Oak Branch of the Bayou Two Prairie

**ARKANSAS DEPARTMENT OF TRANSPORTATION
MATERIALS DIVISION - GEOTECHNICAL SEC.**

BORING NO. 4
PAGE 1 OF 2

JOB NO. 061509 Lonoke County
JOB NAME: Hwy. 367 - Hwy. 89 (Cabot)(S)
Route 321 Section 1
STATION: 225+53
LOCATION: 30' Right of Construction Centerline
LOGGED BY: Austin Dillman

DATE: February 5
TYPE OF DRILLING:
Hollow Stem Auger - Diamond Core
EQUIPMENT: Acker 2094

HAMMER CORRECTION FACTOR: N/A

COMPLETION DEPTH: 64.5

DEPTH FT.	SYMBOL	SAMPLES	DESCRIPTION OF MATERIAL	SOIL GROUP	PLASTIC LIMIT	% MOIST.	LIQUID LIMIT	DRY WEIGHT	LBS PER CU. FT.	NO. OF BLOWS PER 6-IN.	% T C R	% R C D
			SURFACE ELEVATION: 265.2									
5			Wet, Medium Stiff, Gray Clay							3 2-3		
10			Moist, Stiff, Brown and Gray Clay							2 6-7		
15			Moist, Medium Dense, Light Brown Silt with Trace Gravel							7 8-10		
20			Wet, Medium Stiff, Gray Clay							3 3-4		
25			Wet, Stiff, Reddish Brown Sandy Clay with Some Gravel							3 6-6		
30			Wet, Hard, Brown and Gray Sandy Clay with Gravel							9 16-20		
35												

REMARKS: White Oak Branch of the Bayou Two Prairie

ARKANSAS DEPARTMENT OF TRANSPORTATION MATERIALS DIVISION - GEOTECHNICAL SEC.		BORING NO. 4 PAGE 2 OF 2
JOB NO. 061509 Lonoke County		DATE: February 5
JOB NAME: Hwy. 367 - Hwy. 89 (Cabot)(S) Route 321 Section 1		TYPE OF DRILLING: Hollow Stem Auger - Diamond Core
STATION: 225+53		EQUIPMENT: Acker 2094
LOCATION: 30' Right of Construction Centerline		HAMMER CORRECTION FACTOR: N/A
LOGGED BY: Austin Dillman		

COMPLETION DEPTH: 64.5

DEPTH FT.	SYMBOL	SAMPLES	DESCRIPTION OF MATERIAL	SOIL GROUP	PLASTIC LIMIT	% MOIST.	LIQUID LIMIT	DRY WEIGHT	LBS PER CU.FT.	NO. OF BLOWS PER 6-IN.	% T C R	% R Q D
			SURFACE ELEVATION: 265.2									
40		X	SHALE - Highly Weathered, Medium Hard, Dark Gray							22 60 (2")		
			SHALE - Slightly Weathered, Hard, Dark Gray							20 (0")		
45			SHALE - Unweathered, Medium Hard, Dark Gray								89	77
50			SHALE - Unweathered, Medium Hard, Occasional Fractures, Dark Gray								98	86
55			SHALE - Unweathered, Medium Hard, Frequent Fractures, Dark Gray								74	55
60			SHALE - Unweathered, Medium Hard, Occasional Fractures, Dark Gray								96	45
65			Boring Terminated								94	60
70												

REMARKS: White Oak Branch of the Bayou Two Prairie

**ARKANSAS DEPARTMENT OF TRANSPORTATION
MATERIALS DIVISION - GEOTECHNICAL SEC.**

BORING NO. 5
PAGE 1 OF 3

JOB NO. 061509 Lonoke County
JOB NAME: Hwy. 367 - Hwy. 89 (Cabot)(S)
Route 321 Section 1
STATION: 260+74
LOCATION: 27' Right of Construction Centerline
LOGGED BY: Austin Dillman

DATE: February 20, 2019
TYPE OF DRILLING:
Hollow Stem Auger - Diamond Core
EQUIPMENT: Acker 2094
HAMMER CORRECTION FACTOR: N/A

COMPLETION DEPTH: 79.7

DEPTH FT.	SYMBOL	SAMPLES	DESCRIPTION OF MATERIAL	SOIL GROUP	PLASTIC LIMIT	% MOIST.	LIQUID LIMIT	DRY WEIGHT	LBS PER CU. FT.	NO. OF BLOWS PER 6-IN.	% T C R	% R Q D
			SURFACE ELEVATION: 259.8									
5			Moist, Stiff, Brown and Gray Sandy Clay							4 6-8		
10			Moist, Very Stiff, Gray Sandy Clay*							6 8-10		
15			Moist, Stiff, Brown Sandy Clay							3 5-7		
20			Wet, Stiff, Brown Sandy Clay							3 5-5		
25			Wet, Loose, Brown Silty Sand							2 2-5		
30			Wet, Medium Stiff, Brown Sandy Clay							1 2-3		
35												

REMARKS: * A water stratum was encountered at 12.8' below ground level.
Bayou Two Prairie

**ARKANSAS DEPARTMENT OF TRANSPORTATION
MATERIALS DIVISION - GEOTECHNICAL SEC.**

BORING NO. 5
PAGE 2 OF 3

JOB NO. 061509 Lonoke County
JOB NAME: Hwy. 367 - Hwy. 89 (Cabot)(S)
Route 321 Section 1
STATION: 260+74
LOCATION: 27' Right of Construction Centerline
LOGGED BY: Austin Dillman

DATE: February 20, 2019
TYPE OF DRILLING:
Hollow Stem Auger - Diamond Core
EQUIPMENT: Acker 2094
HAMMER CORRECTION FACTOR: N/A

COMPLETION DEPTH: 79.7

DEPTH FT.	SYMBOL	SAMPLES	DESCRIPTION OF MATERIAL	SOIL GROUP	PLASTIC LIMIT	% MOIST.	LIQUID LIMIT	DRY WEIGHT	LBS PER CU.FT.	NO. OF BLOWS PER 6-IN.	% T C R	% R Q D
			SURFACE ELEVATION: 259.8									
40			Wet, Very Loose, Brown Silty Sand							1 2-2		
45			Wet, Very Stiff, Brown and Gray Sandy Clay with Some Gravel							6 8-8		
50			LIMESTONE - Unweathered, Moderately Hard, Gray (Arkadelphia Formation)							18 (1")	46	7
55			Moist, Very Stiff to Hard, Dark Gray Clay								50	0
60			CLAY WITH OCCASIONAL LIMESTONE LAYERS - Unweathered, Medium Hard, Dark Gray								80	14
65			LIMESTONE - Unweathered, Moderately Hard, Gray									
65			CLAY WITH FREQUENT LIMESTONE LAYERS - Unweathered, Medium Hard, Dark Gray								56	0
70			Moist, Very Stiff to Hard, Dark Gray Clay								56	0

REMARKS: * A water stratum was encountered at 12.8' below ground level.
Bayou Two Prairie





**ARKANSAS DEPARTMENT OF TRANSPORTATION
MATERIALS DIVISION - GEOTECHNICAL SEC.**

BORING NO. 5
PAGE 3 OF 3

JOB NO. 061509 Lonoke County
JOB NAME: Hwy. 367 - Hwy. 89 (Cabot)(S)
Route 321 Section 1
STATION: 260+74
LOCATION: 27' Right of Construction Centerline
LOGGED BY: Austin Dillman

DATE: February 20, 2019
TYPE OF DRILLING:
Hollow Stem Auger - Diamond Core
EQUIPMENT: Acker 2094
HAMMER CORRECTION FACTOR: N/A

COMPLETION DEPTH: 79.7

DEPTH FT.	SYMBOL	SAMPLES	DESCRIPTION OF MATERIAL	SOIL GROUP	PLASTIC LIMIT	% MOIST.	LIQUID LIMIT	DRY WEIGHT	LBS PER CU. FT.	NO. OF BLOWS PER 6-IN.	% T C R	% R Q D
			SURFACE ELEVATION: 259.8									
			Moist, Very Stiff to Hard, Dark Gray Clay with Occasional Limestone Concretions								100	7
75			LIMESTONE WITH FREQUENT CLAY LAYERS - Unweathered, Moderately Hard, Dark Gray									
			SANDY CLAY WITH FREQUENT LIMESTONE LAYERS - Unweathered, Soft Layers Interbedded with Moderately Hard Layers, Dark Gray								86	26
80			SHALE - Highly Weathered, Soft, Moderate Dip, Dark Gray (Atoka Formation)									
			SHALE - Weathered, Medium Hard, Moderate Dip, Dark Gray									
			Boring Terminated									
85												
90												
95												
100												
105												

REMARKS: * A water stratum was encountered at 12.8' below ground level.
Bayou Two Prairie

**ARKANSAS DEPARTMENT OF TRANSPORTATION
MATERIALS DIVISION - GEOTECHNICAL SEC.**

BORING NO. 6
PAGE 1 OF 3

JOB NO. 061509 Lonoke County
JOB NAME: Hwy. 367 - Hwy. 89 (Cabot)(S)
Route 321 Section 1
STATION: 261+65.5
LOCATION: 9' Centerline of Construction
LOGGED BY: Don McCollum

DATE: February 25, 2019
TYPE OF DRILLING:
Hollow Stem Auger - Rotary Wash
EQUIPMENT: Acker 2094

HAMMER CORRECTION FACTOR: N/A

COMPLETION DEPTH: 100.1

DEPTH FT.	SYMBOL	SAMPLES	DESCRIPTION OF MATERIAL	SOIL GROUP	PLASTIC LIMIT	% MOIST.	LIQUID LIMIT	DRY WEIGHT	LBS PER CU.FT.	NO. OF BLOWS PER 6-IN.	% T C R	% R Q D
			SURFACE ELEVATION: 265.5									
5		X	Moist, Stiff, Brown and Gray Clay							2 5-6		
10		X	Moist, Medium Stiff, Brown and Gray Clay							3 3-5		
15		X	Wet, Soft, Light Gray Clay							1 2-2		
20		X	Wet, Medium Stiff, Light Gray Silty Clay							2 3-4		
25		X	Wet, Stiff, Gray Sandy Clay							3 5-4		
30		X	Wet, Medium Dense, Light Brown Sand with Clay							3 11-14		
35		X										

REMARKS: * Sampler blocked off by a cobble
Bayou Two Prairie

**ARKANSAS DEPARTMENT OF TRANSPORTATION
MATERIALS DIVISION - GEOTECHNICAL SEC.**

BORING NO. **6**
PAGE **2** OF **3**

JOB NO. **061509** Lonoke County
JOB NAME: **Hwy. 367 - Hwy. 89 (Cabot)(S)**
Route 321 Section 1

DATE: **February 25, 2019**
TYPE OF DRILLING:
Hollow Stem Auger - Rotary Wash

STATION: **261+65.5**



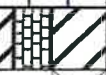

EQUIPMENT:
Acker 2094

LOCATION: **9' Centerline of Construction**

LOGGED BY: **Don McCollum**

HAMMER CORRECTION FACTOR: **N/A**

COMPLETION DEPTH: **100.1**

DEPTH FT.	SYMBOL	SAMPLES	DESCRIPTION OF MATERIAL	SOIL GROUP	PLASTIC LIMIT	% MOIST.	LIQUID LIMIT	DRY WEIGHT LBS PER CU.FT.	NO. OF BLOWS PER 6-IN.	% T C R	% R Q D
40		X	Wet, Stiff, Brown Sandy Clay						5 5-5		
45		X	Wet, Medium Stiff, Light Brown and Light Gray Sandy Clay						0 2-3		
50		X	No Sample Recovered						10 5-4		
55		X	Moist, Very Stiff, Dark Gray Clay (Arkadelphia Formation)						3 9-13		
60		X	Moist, Very Hard, Dark Gray Clay (Sampler Refusal at 61.4')						8 12-18		
65			LIMESTONE						8 13-23 (11")		
70			Moist, Very Stiff, Dark Gray Clay						6 14-16		

REMARKS: * Sampler blocked off by a cobble
Bayou Two Prairie

**ARKANSAS DEPARTMENT OF TRANSPORTATION
MATERIALS DIVISION - GEOTECHNICAL SEC.**

BORING NO. 6
PAGE 3 OF 3

JOB NO. 061509 Lonoke County
JOB NAME: Hwy. 367 - Hwy. 89 (Cabot)(S)
Route 321 Section 1
STATION: 261+65.5
LOCATION: 9' Centerline of Construction
LOGGED BY: Don McCollum

DATE: February 25, 2019
TYPE OF DRILLING:
Hollow Stem Auger - Rotary Wash
EQUIPMENT: Acker 2094
HAMMER CORRECTION FACTOR: N/A

COMPLETION DEPTH: 100.1

DEPTH FT.	SYMBOL	SAMPLES	DESCRIPTION OF MATERIAL	SOIL GROUP	PLASTIC LIMIT	% MOIST.	LIQUID LIMIT	DRY WEIGHT	LBS PER CU.FT.	NO. OF BLOWS PER 6-IN.	% T C R	% R O D
			SURFACE ELEVATION: 265.5									
75			Moist, Hard, Dark Gray Clay							15 15-16		
80			Moist, Very Hard, Dark Gray Clay							9 14-17		
85			LIMESTONE INTERBEDDED WITH CLAY							22 42-60 (8")		
90			Moist, Very Hard, Sandy Clay with Some Gravel							60 (2")		
95			SHALE - Weathered to Highly Weathered, Medium Hard, Dark Gray (Atoka Formation)							60 (4")		
100			SHALE - Slightly Weathered, Medium Hard, Dark Gray							44 60 (1")		
			Boring Terminated							20 (1")		
105												

REMARKS: * Sampler blocked off by a cobble
Bayou Two Prairie

**ARKANSAS DEPARTMENT OF TRANSPORTATION
MATERIALS DIVISION - GEOTECHNICAL SEC.**

BORING NO. 7
PAGE 1 OF 3

JOB NO. 061509 Lonoke County
JOB NAME: Hwy. 367 - Hwy. 89 (Cabot)(S)
Route 321 Section 1

DATE: February 27, 2019

STATION: 265+78

TYPE OF DRILLING:
Hollow Stem Auger - Rotary Wash
EQUIPMENT: Acker 2094

LOCATION: 13.5' Right of Construction Centerline
LOGGED BY: Don McCollum

HAMMER CORRECTION FACTOR: N/A

COMPLETION DEPTH: 101.5

DEPTH FT.	SOIL TYPE	DESCRIPTION OF MATERIAL	SOIL GROUP	PLASTIC LIMIT	% MOIST.	LIQUID LIMIT	DRY WEIGHT LBS PER CU.FT.	NO. OF BLOWS PER 6-IN.	% T C R	% R Q D
5		Moist, Stiff, Brown and Gray Clay						2 4-6		
10		Moist, Medium Stiff, Brown and Gray Clay						3 3-5		
15		Moist, Stiff, Brown and Gray Sandy Clay						6 4-6		
20		Wet, Medium Stiff, Light Gray Clay						0 2-4		
25		Wet, Stiff, Brown and Gray Clay						0 3-10		
30		Wet, Stiff, Brown and Gray Clay						6 4-5		
35										

REMARKS: Drain Two & UPRR








**ARKANSAS DEPARTMENT OF TRANSPORTATION
MATERIALS DIVISION - GEOTECHNICAL SEC.**

BORING NO. 7
PAGE 2 OF 3

JOB NO. 061509 Lonoke County
JOB NAME: Hwy. 367 - Hwy. 89 (Cabot)(S)
Route 321 Section 1
STATION: 265+78
LOCATION: 13.5' Right of Construction Centerline
LOGGED BY: Don McCollum

DATE: February 27, 2019
TYPE OF DRILLING:
Hollow Stem Auger - Rotary Wash
EQUIPMENT: Acker 2094
HAMMER CORRECTION FACTOR: N/A

COMPLETION DEPTH: 101.5

DEPTH FT.	SYMBOL	DESCRIPTION OF MATERIAL	SOIL GROUP	PLASTIC LIMIT	% MOIST.	LIQUID LIMIT	DRY WEIGHT LBS PER CU.FT.	NO. OF BLOWS PER 6-IN.	% T C R	% R Q D
		SURFACE ELEVATION: 281.2								
40								2 4-3		
		Wet, Medium Stiff, Brown and Gray Clay								
45								3 4-8		
		Wet, Stiff, Brown and Gray Clay								
50								0 2-3		
		Wet, Medium Stiff, Brown and Gray Clay								
55								0 3-5		
		Wet, Medium Stiff, Gray Sandy Clay								
60								3 3-9		
		Wet, Medium Dense, Brown Silty Sand with Trace Gravel								
65								3 9-11		
		Moist, Very Stiff, Dark Gray Clay (Arkadelphia Formation)								
70										

REMARKS: Drain Two & UPRR

ARKANSAS DEPARTMENT OF TRANSPORTATION MATERIALS DIVISION - GEOTECHNICAL SEC.		BORING NO. 7 PAGE 3 OF 3
JOB NO. 061509 Lonoke County	DATE: February 27, 2019	
JOB NAME: Hwy. 367 - Hwy. 89 (Cabot)(S) Route 321 Section 1	TYPE OF DRILLING: Hollow Stem Auger - Rotary Wash	
STATION: 265+78	EQUIPMENT: Acker 2094	
LOCATION: 13.5' Right of Construction Centerline	HAMMER CORRECTION FACTOR: N/A	
LOGGED BY: Don McCollum		

COMPLETION DEPTH: 101.5												
DEPTH FT.	SYMBOL	SAMPLES	DESCRIPTION OF MATERIAL	SOIL GROUP	PLASTIC LIMIT	% MOIST.	LIQUID LIMIT	DRY WEIGHT	LBS PER CU.FT.	NO. OF BLOWS PER 6-IN.	% TCR	% RQD
			SURFACE ELEVATION: 281.2									
			Moist, Hard, Dark Gray Clay							9 15-18		
			Clay with Frequent Limestone Layers									
75										9 12-14		
			Moist, Very Stiff, Dark Gray Clay									
80										10 15-19		
			Moist, Hard, Dark Gray Clay									
85			Moist, Very Hard, Dark Gray Clay LIMESTONE INTERBEDDED WITH CLAY							12 60 (0")		
90			Moist, Hard, Dark Gray Clay							9 15-19		
95			Moist, Very Stiff, Dark Gray Clay							5 13-17		
100										6 12-15		
			Boring Terminated									
105												

REMARKS: Drain Two & UPRR

**ARKANSAS DEPARTMENT OF TRANSPORTATION
MATERIALS DIVISION - GEOTECHNICAL SEC.**

BORING NO. 8
PAGE 1 OF 3

JOB NO. 061509 Lonoke County
JOB NAME: Hwy. 367 - Hwy. 89 (Cabot)(S)
Route 321 Section 1

DATE: February 28 and March 5 and 6, 2019
TYPE OF DRILLING:
Hollow Stem Auger - Rotary Wash

STATION: 266+60.5

EQUIPMENT:
Acker 2094

LOCATION: 22' Right of Construction Centerline

LOGGED BY: Don McCollum

HAMMER CORRECTION FACTOR: N/A

COMPLETION DEPTH: 100

DEPTH FT.	SOIL TYPE	SOIL SAMPLE S	DESCRIPTION OF MATERIAL	SOIL GROUP	PLASTIC LIMIT	% MOIST.	LIQUID LIMIT	DRY WEIGHT LBS PER CU.FT.	NO. OF BLOWS PER 6-IN.	% T C R	% R Q D
5			Wet, Medium Stiff, Gray Clay					0	2-3		
10			Wet, Stiff, Brown and Gray Silty Clay					3	4-5		
15			Wet, Stiff, Reddish Brown Clay					5	5-7		
20			Wet, Medium Dense, Lt Brown and Light Gray Clayey Sand					3	5-6		
25			Wet, Medium Stiff, Lt Brown and Light Gray with Sand					3	3-3		
30			Wet, Medium Stiff, Reddish Brown Sandy Clay					0	2-3		
35											

REMARKS: Drain Two & UPRR

**ARKANSAS DEPARTMENT OF TRANSPORTATION
MATERIALS DIVISION - GEOTECHNICAL SEC.**

BORING NO. 8

PAGE 2 OF 3

JOB NO. 061509 Lonoke County
 JOB NAME: Hwy. 367 - Hwy. 89 (Cabot)(S)
 Route 321 Section 1
 STATION: 266+60.5
 LOCATION: 22' Right of Construction Centerline
 LOGGED BY: Don McCollum

DATE: February 28 and March 5 and 6, 2019
 TYPE OF DRILLING:
 Hollow Stem Auger - Rotary Wash
 EQUIPMENT: Acker 2094
 HAMMER CORRECTION FACTOR: N/A

COMPLETION DEPTH: 100

DEPTH FT.	SYMBOL	SAMPLES	DESCRIPTION OF MATERIAL	SOIL GROUP	PLASTIC LIMIT	% MOIST.	LIQUID LIMIT	DRY WEIGHT	LBS PER CUFT.	NO. OF BLOWS PER 6-IN.	% F C R	% R Q D
			SURFACE ELEVATION: 258.1									
40			Wet, Loose, Reddish Brown Silty Sand							2 3-3		
45			Wet, Medium Dense, Reddish Brown Silty Sand with Gravel							9 13-11		
50			Moist, Very Stiff, Dark Gray Clay (Arkadelphia Formation)							7 12-16		
55			Moist, Hard, Dark Gray Clay with Some Sand Seams							9 15-19		
60			Moist, Very Hard, Dark Gray Clay LIMESTONE							8 30 (5")		
65			Moist, Hard, Dark Gray Clay							11 20-22		
70										11 15-19		

REMARKS: Drain Two & UPRR

**ARKANSAS DEPARTMENT OF TRANSPORTATION
MATERIALS DIVISION - GEOTECHNICAL SEC.**

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JOB NO. 061509 Lonoke County
JOB NAME: Hwy. 367 - Hwy. 89 (Cabot)(S)
Route 321 Section 1
STATION: 266+60.5
LOCATION: 22' Right of Construction Centerline
LOGGED BY: Don McCollum

DATE: February 28 and March 5 and 6, 2019
TYPE OF DRILLING:
Hollow Stem Auger - Rotary Wash
EQUIPMENT: Acker 2094
HAMMER CORRECTION FACTOR: N/A

COMPLETION DEPTH: 100

DEPTH FT.	SYMBOL	SAMPLES	DESCRIPTION OF MATERIAL	SOIL GROUP	PLASTIC LIMIT	% MOIST.	LIQUID LIMIT	DRY WEIGHT	LBS PER CU.FT.	NO. OF BLOWS PER 6-IN.	% T C R	% R O D
			SURFACE ELEVATION: 258.1									
75			Moist, Hard, Dark Gray Clay with Some Sand Seams							10 17-16		
80										10 17-20		
85			Moist, Hard, Dark Gray Clay							8 15-18		
90										7 14-20		
			Moist, Very Hard, Dark Gray Clay							17 60 (2")		
95			SHALE - Weathered, Medium Hard, Dark Gray (Atoka Formation)							60 (2")		
100			Boring Terminated							10 (0")		
105												

REMARKS: Drain Two & UPRR

**ARKANSAS DEPARTMENT OF TRANSPORTATION
MATERIALS DIVISION - GEOTECHNICAL SEC.**

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PAGE 1 OF 3

JOB NO. 061509 Lonoke County
JOB NAME: Hwy. 367 - Hwy. 89 (Cabot)(S)
Route 321 Section 1
STATION: 269+16.5
LOCATION: 22' Right of Construction Centerline
LOGGED BY: Austin Dillman and Donnie Thornton

DATE: March 11 and 12, 2019
TYPE OF DRILLING:
Hollow Stem Auger - Rotary Wash
EQUIPMENT: Acker 2094

HAMMER CORRECTION FACTOR: N/A

COMPLETION DEPTH: 100.2

DEPTH FT.	SYMBOL	SAMPLES	DESCRIPTION OF MATERIAL	SOIL GROUP	PLASTIC LIMIT	% MOIST.	LIQUID LIMIT	DRY WEIGHT	LBS PER CU.FT.	NO. OF BLOWS PER 6-IN.	% F C R	% R Q D
			SURFACE ELEVATION: 257.5									
5			Moist, Stiff, Brown and Gray Clay							1 4-5		
10			Moist, Stiff, Light Gray Clay							2 4-5		
15			Moist, Stiff, Reddish Brown and Gray Sandy Clay							2 4-7		
20			Wet, Stiff, Reddish Brown and Gray Sandy Clay							2 4-6		
25			Wet, Loose, Light Brown Silty Sand							3 3-2		
30			Wet, Medium Stiff, Gray Sandy Clay							1 3-4		
35												

REMARKS: Drain Two & UPRR

**ARKANSAS DEPARTMENT OF TRANSPORTATION
MATERIALS DIVISION - GEOTECHNICAL SEC.**

BORING NO. 9
PAGE 2 OF 3

JOB NO. 061509 Lonoke County
JOB NAME: Hwy. 367 - Hwy. 89 (Cabot)(S)
Route 321 Section 1

STATION: 269+16.5

DATE: March 11 and 12, 2019
TYPE OF DRILLING: Hollow Stem Auger - Rotary Wash
EQUIPMENT: Acker 2094

LOCATION: 22' Right of Construction Centerline

LOGGED BY: Austin Dillman and Donnie Thornton

HAMMER CORRECTION FACTOR: N/A

COMPLETION DEPTH: 100.2

DEPTH FT.	SYMBOL	DESCRIPTION OF MATERIAL	SOIL GROUP	PLASTIC LIMIT	% MOIST.	LIQUID LIMIT	DRY WEIGHT LBS PER CU.FT.	NO. OF BLOWS PER 6-IN.	% T C R	% R Q D
		SURFACE ELEVATION: 257.5								
40		Wet, Stiff, Light Brown and Light Gray Sandy Clay						4 5-7		
45		Wet, Very Stiff, Gray Sandy Clay with Some Gravel						4 6-10		
50		Moist, Very Stiff, Dark Gray Clay (Arkadelphia Formation)						7 10-18		
55		Moist, Hard, Dark Gray Clay						10 15-17		
60		Moist, Hard, Dark Gray Clay						11 16-20		
65		LIMESTONE						11 15-17		
70		Moist, Hard, Dark Gray Clay						12 16-20		

REMARKS: Drain Two & UPRR

**ARKANSAS DEPARTMENT OF TRANSPORTATION
MATERIALS DIVISION - GEOTECHNICAL SEC.**

BORING NO. 9
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JOB NO. 061509 Lonoke County
JOB NAME: Hwy. 367 - Hwy. 89 (Cabot)(S)
Route 321 Section 1
STATION: 269+16.5
LOCATION: 22' Right of Construction Centerline
LOGGED BY: Austin Dillman and Donnie Thornton
COMPLETION DEPTH: 100.2

DATE: March 11 and 12, 2019
TYPE OF DRILLING:
Hollow Stem Auger - Rotary Wash
EQUIPMENT: Acker 2094
HAMMER CORRECTION FACTOR: N/A

DEPTH FT.	SYMBOL	SAMPLES	DESCRIPTION OF MATERIAL	SOIL GROUP	PLASTIC LIMIT	% MOIST.	LIQUID LIMIT	DRY WEIGHT	LBS PER CU.FT.	NO. OF BLOWS PER 6-IN.	% T C R	% R Q D
			SURFACE ELEVATION: 257.5									
75			CLAY INTERBEDDED WITH LIMESTONE							13 19-24		
80										12 16-20		
85			Moist, Hard, Dark Gray Clay							11 15-20		
90										11 16-20		
95			Moist, Very Hard, Dark Gray Clay							81 60 (0")		
100			SANDSTONE (Atoka Formation)							79 (2")		
105			Boring Terminated									

REMARKS: Drain Two & UPRR

**ARKANSAS DEPARTMENT OF TRANSPORTATION
MATERIALS DIVISION - GEOTECHNICAL SEC.**

BORING NO. 10
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JOB NO. 061509 Lonoke County
JOB NAME: Hwy. 367 - Hwy. 89 (Cabot)(S)
Route 321 Section 1
STATION: 273+24.5
LOCATION: 27.75' Right of Construction Centerline
LOGGED BY: Austin Dillman

DATE: March 18 and 19, 2019
TYPE OF DRILLING:
Hollow Stem Auger - Rotary Wash
EQUIPMENT: Acker 2094

HAMMER CORRECTION FACTOR: N/A

COMPLETION DEPTH: 101.5

DEPTH FT.	SYMBOL	SAMPLES	DESCRIPTION OF MATERIAL	SOIL GROUP	PLASTIC LIMIT	% MOIST.	LIQUID LIMIT	DRY WEIGHT	LBS PER CU.FT.	NO. OF BLOWS PER 6-IN.	% T C R	% R Q D
			SURFACE ELEVATION: 264.0									
5			Wet, Soft, Light Brown Clay with Trace Gravel							0 2-2		
10										0 0-2		
15			Moist, Stiff, Light Brown and Gray Clay							4 5-8		
20										3 5-7		
25			Moist, Stiff, Light Brown and Gray Clay with Sand							2 5-6		
30			Moist, Stiff, Light Brown and Gray Sandy Clay							1 3-5		
35			Moist, Medium Stiff, Light Brown and Gray Clay									

REMARKS: Drain Two & UPRR

**ARKANSAS DEPARTMENT OF TRANSPORTATION
MATERIALS DIVISION - GEOTECHNICAL SEC.**

BORING NO. 10
PAGE 2 OF 3

JOB NO. 061509 Lonoke County
JOB NAME: Hwy. 367 - Hwy. 89 (Cabot)(S)
Route 321 Section 1

DATE: March 18 and 19, 2019
TYPE OF DRILLING:
Hollow Stem Auger - Rotary Wash







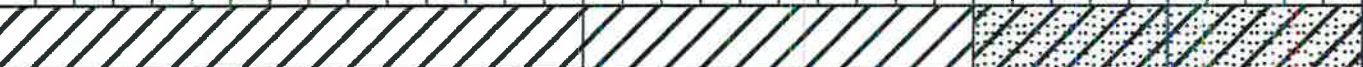
STATION: 273+24.5

EQUIPMENT: Acker 2094

LOCATION: 27.75' Right of Construction Centerline
LOGGED BY: Austin Dillman

HAMMER CORRECTION FACTOR: N/A

COMPLETION DEPTH: 101.5

DEPTH FT.	SOIL TYPE	SOIL GROUP	PLASTIC LIMIT	% MOIST.	LIQUID LIMIT	DRY WEIGHT LBS PER CU.FT.	NO. OF BLOWS PER 6-IN.	% T C R	% R Q D
45							5 7-11		
40							3 5-5		
50							10 13-17		
55							11 14-19		
60							10 16-21		
65							11 16-20		
70									

REMARKS: Drain Two & UPRR

**ARKANSAS DEPARTMENT OF TRANSPORTATION
MATERIALS DIVISION - GEOTECHNICAL SEC.**

BORING NO. 10
PAGE 3 OF 3

JOB NO. 061509 Lonoke County
JOB NAME: Hwy. 367 - Hwy. 89 (Cabot)(S)
Route 321 Section 1
STATION: 273+24.5
LOCATION: 27.75' Right of Construction Centerline
LOGGED BY: Austin Dillman

DATE: March 18 and 19, 2019
TYPE OF DRILLING:
Hollow Stem Auger - Rotary Wash
EQUIPMENT: Acker 2094.

HAMMER CORRECTION FACTOR: N/A

COMPLETION DEPTH: 101.5

DEPTH FT.	SYMBOL	SAMPLES	DESCRIPTION OF MATERIAL	SOIL GROUP	PLASTIC LIMIT	% MOIST.	LIQUID LIMIT	DRY WEIGHT	LBS PER CU.FT.	NO. OF BLOWS PER 6-IN.	% T C R	% R Q D
			SURFACE ELEVATION: 264.0									
75										12 16-21		
80			Moist, Very Hard, Dark Gray Clay LIMESTONE							5 12-19		
85			Moist, Hard, Dark Gray Clay							10 34 (5")		
90										12 16-22		
95			CLAY WITH FREQUENT LIMESTONE LAYERS Moist, Very Hard, Dark Gray Clay							22 29-23		
100			Moist, Hard, Dark Gray Clay							12 75 (5")		
105			Boring Terminated							12 14-21		

REMARKS: Drain Two & UPRR

**ARKANSAS DEPARTMENT OF TRANSPORTATION
MATERIALS DIVISION - GEOTECHNICAL SEC.**

BORING NO. 11
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JOB NO. 061509 Lonoke County
JOB NAME: Hwy. 367 - Hwy. 89 (Cabot)(S)
Route 321 Section 1
STATION: 274+65.5
LOCATION: 6.25' Right of Construction Centerline
LOGGED BY: Austin Dillman

DATE: March 14 and 18, 2018
TYPE OF DRILLING:
Hollow Stem Auger - Rotary Wash
EQUIPMENT: Acker 2094
HAMMER CORRECTION FACTOR: N/A

COMPLETION DEPTH: 101

DEPTH FT.	SYMBOL	SAMPLES	DESCRIPTION OF MATERIAL	SOIL GROUP	PLASTIC LIMIT	% MOIST.	LIQUID LIMIT	DRY WEIGHT	LBS PER CU.FT.	NO. OF BLOWS PER 6-IN.	% T C C R	% R O D
			SURFACE ELEVATION: 281.0									
5			Moist, Soft, Light Brown Sandy Clay with Some Gravel	ND	15	49				1 1-1		
10			Moist, Medium Stiff, Brown Sandy Lean Clay with Trace Gravel	CL						1 3-4		
15			Moist, Medium Stiff, Brown and Gray Fat Clay with Sand and Trace Gravel	CH	14	51				0 2-3		
20			Wet, Medium Stiff, Brown and Gray Sandy Lean Clay with Some Gravel	CL	12	38				0 2-4		
25			Wet, Very Soft, Brown Lean Clay	CL	17	33				0 0-0		
30			Wet, Stiff, Brown Lean Clay with Trace Organic Matter	CL	15	38				5 8-7		

REMARKS: Drain Two & UPRR

**ARKANSAS DEPARTMENT OF TRANSPORTATION
MATERIALS DIVISION - GEOTECHNICAL SEC.**

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JOB NO. 061509 Lonoke County
JOB NAME: Hwy. 367 - Hwy. 89 (Cabot)(S)
Route 321 Section 1
STATION: 274+65.5
LOCATION: 6.25' Right of Construction Centerline
LOGGED BY: Austin Dillman

DATE: March 14 and 18, 2018
TYPE OF DRILLING:
Hollow Stem Auger - Rotary Wash
EQUIPMENT: Acker 2094
HAMMER CORRECTION FACTOR: N/A

COMPLETION DEPTH: 101

DEPTH FT.	SYMBOL	SAMPLES	DESCRIPTION OF MATERIAL	SOIL GROUP	PLASTIC LIMIT	% MOIST.	LIQUID LIMIT	DRY WEIGHT LBS PER CU.FT.	NO. OF BLOWS PER 6-IN.	% T C R	% R Q D
			SURFACE ELEVATION: 281.0								
40			Moist, Stiff, Brown and Gray Lean Clay	CL	14		48		4 7-8		
45			Moist, Stiff, Brown and Gray Lean Clay with Sand	CL	11		32		4 5-7		
50				CL	12		31		4 5-5		
55			Wet, Medium Dense, Light Brown and Gray Clayey Sand	SC	NP				4 7-6		
60			Wet, Medium Dense, Brown and Gray Silty Sand	SM	NP				6 11-9		
65			Moist, Very Stiff, Dark Gray Fat Clay (Arkadelphia Formation)	CH	24		75		5 7-9		
			CLAY WITH FREQUENT LIMESTONE LAYERS								
70				CH	21		65		8 13-18		

REMARKS: Drain Two & UPRR

ARKANSAS DEPARTMENT OF TRANSPORTATION MATERIALS DIVISION - GEOTECHNICAL SEC.	BORING NO. 11 PAGE 3 OF 3
JOB NO. 061509 Lonoke County JOB NAME: Hwy. 367 - Hwy. 89 (Cabot)(S) Route 321 Section 1 STATION: 274+65.5 LOCATION: 6.25' Right of Construction Centerline LOGGED BY: Austin Dillman	DATE: March 14 and 18, 2018 TYPE OF DRILLING: Hollow Stem Auger - Rotary Wash EQUIPMENT: Acker 2094 HAMMER CORRECTION FACTOR: N/A

COMPLETION DEPTH: 101

DEPTH FT.	SYMBOL	SAMPLES	DESCRIPTION OF MATERIAL	SOIL GROUP	PLASTIC LIMIT	% MOIST.	LIQUID LIMIT	DRY WEIGHT	LBS PER CU. FT.	NO. OF BLOWS PER 6-IN.	% T C R	% R Q D
			SURFACE ELEVATION: 281.0									
75		X	Moist, Hard, Dark Gray Fat Clay	CH	23		71			10 14-17		
		X		CH	24		70			11 17-18		
80		X		CH	23		67			11 15-19		
		X		CH	19		54			13 16-21		
85		X		CH	22		69			11 16-17		
90		X		CH	21		55			11 18-19		
95		X		CH	20		50			15 18-20 (6')		
100		X		Moist, Very Hard, Dark Gray Clay	CH	20		50				
			Boring Terminated									
105												

REMARKS: Drain Two & UPRR



ARKANSAS DEPARTMENT OF TRANSPORTATION

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MATERIALS DIVISION

11301 West Baseline Road | P.O. Box 2261 | Little Rock, AR 72203-2261 | Phone: 501.569.2185 | Fax: 501.569.2368

May 10, 2018

TO: Mr. Trinity Smith, Engineer of Roadway Design

SUBJECT: Job No. 061509
Hwy. 367 – Hwy. 89 (Cabot) (S)
Route 321 Section 1
Lonoke County

Transmitted herewith is the requested Soil Survey, strength data and Resilient Modulus test results for the above referenced job. The project consists of widening approximately 3.7 miles of Highway 321 in Cabot. Samples were taken in the existing travel lanes, shoulders and ditch line.

Based on laboratory results of samples obtained, the subgrade soils consist primarily of moderately to highly plastic sandy clay. Cross-sections are not currently available, but it is assumed the construction grade line will closely match that of the existing roadway. The subgrade soils are expected to provide a stable working platform with conventional processing if the weather is favorable during construction. If embankment is to be placed within the existing ditch line all soft unstable organic material should be undercut prior to construction, anticipated to be no more than two feet.

Additional earthwork recommendations will be made upon request when plans are further developed and cross-sections become available.

Listed below is the additional information requested for use in developing the plans:

- 1. The Qualified Products List (QPL) indicates that Aggregate Base Course (Class CL-7) is available from commercial producers located in the vicinity of Little Rock.

- 2. Asphalt Concrete Hot Mix

Table with 3 columns: Type, Asphalt Cement %, Mineral Aggregate %. Rows include Surface Course, Binder Course, and Base Course.

Handwritten signature of Michael C. Benson, Materials Engineer

MCB:pt:bjj
Attachment

cc: State Constr. Eng. – Master File Copy
District 6 Engineer
System Information and Research Div.
G. C. File

ARKANSAS STATE HIGHWAY AND TRANSPORTATION DEPARTMENT - LITTLE ROCK, ARKANSAS
MATERIALS DIVISION
MICHAEL BENSON, MATERIALS ENGINEER
*** SOIL SURVEY STRENGTH TEST REPORT ***

DATE - 04/23/2018
JOB NUMBER - 061509

SEQUENCE NO. - 1
MATERIAL CODE - SSRV
SPEC. YEAR - 2014
SUPPLIER ID. - 1
COUNTY/STATE - 43
DISTRICT NO. - 06

JOB NAME - HWY. 367 - HWY. 89 (CABOT) (S)

* STATION LIMITS R-VALUE AT 240 psi *

BEGIN JOB - END JOB <5

RESILIENT MODULUS
STA. 230+00 7375
STA. 255+00 6998
STA. 288+00 5846
STA. 346+00 5898

REMARKS -

AASHTO TESTS : T190

**ARKANSAS STATE HIGHWAY AND TRANSPORTATION DEPARTMENT
MATERIALS DIVISION**

**AASHTO T 307-99 - RESILIENT MODULUS OF SUBGRADE SOILS
RECOMPACTED SAMPLES**

Job No.	061509	Material Code	SSRVPS
Date Sampled:	3/6/18	Station No.:	230+00
Date Tested:	April 18, 2018	Location:	27RT
Name of Project:	HWY. 367 - HWY. 89 (CABOT)(S)		
County:	Code: 43	Name:	LONOKE
Sampled By:	THORNTON/FRAZIER/JORDAN		
Lab No.:	20180529	Depth:	0-5
Sample ID:	RV134	AASHTO Class:	A-4 (4)
LATITUDE:		Material Type (1 or 2):	2
		LONGITUDE:	

1. Testing Information:

Preconditioning - Permanent Strain > 5% (Y=Yes or N= No)	N
Testing - Permanent Strain > 5% (Y=Yes or N=No)	N
Number of Load Sequences Completed (0-15)	15

2. Specimen Information:

Specimen Diameter (in):	
Top	3.95
Middle	3.95
Bottom	3.95
Average	3.95
Membrane Thickness (in):	0.01
Height of Specimen, Cap and Base (in):	8.02
Height of Cap and Base (in):	0.00
Initial Length, Lo (in):	8.02
Initial Area, Ao (sq. in):	12.18
Initial Volume, AoLo (cu. in):	97.68

3. Soil Specimen Weight:

Weight of Wet Soil Used (g):	3187.20
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4. Soil Properties:

Optimum Moisture Content (%):	14.6
Maximum Dry Density (pcf):	112.1
95% of MDD (pcf):	106.5
In-Situ Moisture Content (%):	N/A

5. Specimen Properties:

Wet Weight (g):	3187.20
Compaction Moisture content (%):	14.9
Compaction Wet Density (pcf):	124.32
Compaction Dry Density (pcf):	108.20
Moisture Content After Mr Test (%):	14.7

6. Quick Shear Test (Y=Yes, N=No, N/A=Not Applicable):

#VALUE!

7. Resilient Modulus, Mr:

7608(Sc)^{-0.15157}(S3)^{0.37310}

8. Comments

9. Tested By:

GW

Date: April 18, 2018

**ARKANSAS STATE HIGHWAY AND TRANSPORTATION DEPARTMENT
MATERIALS DIVISION**

**AASHTO T 307-99 - RESILIENT MODULUS OF SUBGRADE SOILS
RECOMPACTED SAMPLES**

Job No. 061509 **Material Code** SSRVPS
Date Sampled: 3/6/18 **Station No.:** 230+00
Date Tested: April 18, 2018 **Location:** 27'RT

Name of Project: HWY. 367 - HWY. 89 (CABOT)(S)
County: Code: 43 **Name:** LONOKE
Sampled By: THORNTON/FRAZIER/JORDAN
Lab No.: 20180529
Sample ID: RV134
LATITUDE:

Depth: 0-5
AASHTO Class: A-4 (4)
Material Type (1 or 2): 2
LONGITUDE:

PARAMETER	Chamber Confining Pressure	Nominal Maximum Axial Stress	Actual Applied Max. Axial Load	Actual Applied Cyclic Load	Actual Applied Contact Load	Actual Applied Max. Axial Stress	Actual Applied Cyclic Stress	Actual Applied Contact Stress	Average Recov Def. LVDT 1 and 2	Resilient Strain	Resilient Modulus
DESIGNATION	S ₃	S _{cyclic}	P _{max}	P _{cyclic}	P _{contact}	S _{max}	S _{cyclic}	S _{contact}	H _{avg}	ε _r	M _r
UNIT	psi	psi	lbs	lbs	lbs	psi	psi	psi	in	in/in	psi
Sequence 1	6.0	2.0	25.5	23.0	2.6	2.1	1.9	0.2	0.00113	0.00014	13,391
Sequence 2	6.0	4.0	47.7	45.1	2.6	3.9	3.7	0.2	0.00236	0.00029	12,568
Sequence 3	6.0	6.0	70.9	67.4	3.5	5.8	5.5	0.3	0.00377	0.00047	11,795
Sequence 4	6.0	8.0	95.0	89.1	5.9	7.8	7.3	0.5	0.00543	0.00068	10,803
Sequence 5	6.0	10.0	119.4	111.0	8.4	9.8	9.1	0.7	0.00708	0.00088	10,322
Sequence 6	4.0	2.0	25.4	22.7	2.7	2.1	1.9	0.2	0.00126	0.00016	11,867
Sequence 7	4.0	4.0	47.4	44.6	2.8	3.9	3.7	0.2	0.00279	0.00035	10,529
Sequence 8	4.0	6.0	69.3	66.5	2.8	5.7	5.5	0.2	0.00447	0.00056	9,803
Sequence 9	4.0	8.0	93.4	88.2	5.2	7.7	7.2	0.4	0.00628	0.00078	9,254
Sequence 10	4.0	10.0	118.1	110.4	7.6	9.7	9.1	0.6	0.00807	0.00101	9,009
Sequence 11	2.0	2.0	25.2	22.4	2.8	2.1	1.8	0.2	0.00169	0.00021	8,713
Sequence 12	2.0	4.0	46.8	44.1	2.8	3.8	3.6	0.2	0.00363	0.00045	8,000
Sequence 13	2.0	6.0	68.1	65.3	2.8	5.6	5.4	0.2	0.00563	0.00070	7,629
Sequence 14	2.0	8.0	90.9	86.7	4.3	7.5	7.1	0.4	0.00774	0.00096	7,375
Sequence 15	2.0	10.0	115.1	108.4	6.7	9.4	8.9	0.5	0.00966	0.00120	7,388

TESTED BY _____ DATE April 18, 2018
 REVIEWED BY _____ DATE _____

GW

**ARKANSAS STATE HIGHWAY AND TRANSPORTATION DEPARTMENT
MATERIALS DIVISION**

**AASHTO T 307-99 - RESILIENT MODULUS OF SUBGRADE SOILS
RECOMPACTED / THINWALL TUBE SAMPLES**

Job No.	061509	Material Code	SSRVPS
Date Sampled:	3/6/18	Station No.:	230+00
Date Tested:	April 18, 2018	Location:	27'RT
Name of Project:	HWY. 367 - HWY. 89 (CABOT)(S)		
County:	Code: 43	Name:	LONOKE
Sampled By:	THORNTON/FRAZIER/JORDAN		
Lab No.:	20180529	Depth:	0-5
Sample ID:	RV134	AASHTO Class:	A-4 (4)
LATITUDE:		Material Type (1 or 2):	2
		LONGITUDE:	

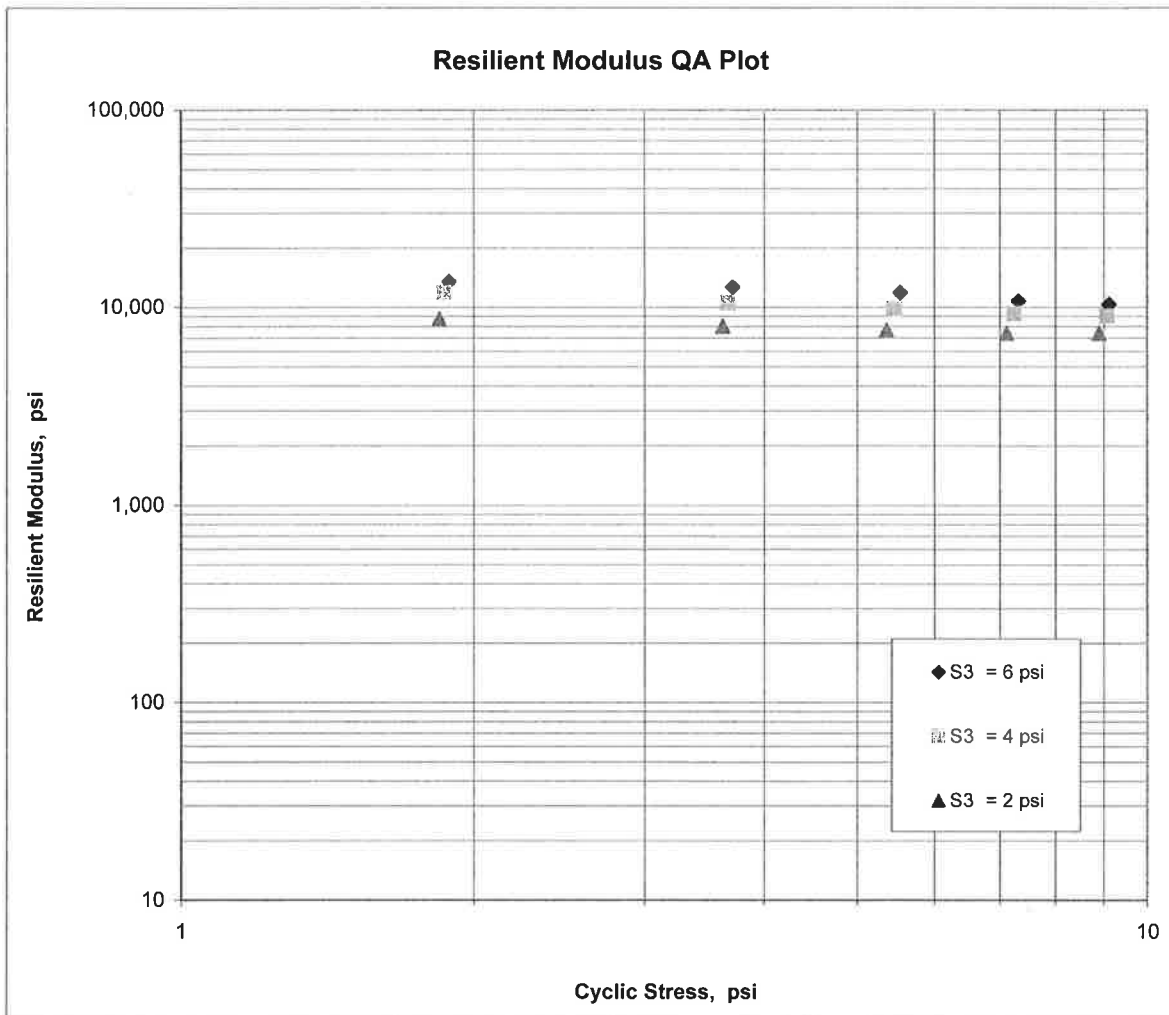
$$M_R = K_1 (S_c)^{K_2} (S_3)^{K_5}$$

$$K_1 = \underline{7,608}$$

$$K_2 = \underline{-0.15157}$$

$$K_5 = \underline{0.37310}$$

$$R^2 = \underline{0.99}$$



**ARKANSAS STATE HIGHWAY AND TRANSPORTATION DEPARTMENT
MATERIALS DIVISION**

**AASHTO T 307-99 - RESILIENT MODULUS OF SUBGRADE SOILS
RECOMPACTED SAMPLES**

Job No.	061509	Material Code	SSRVPS
Date Sampled:	3/6/18	Station No.:	255+00
Date Tested:	April 19, 2018	Location:	27LT
Name of Project:	HWY. 367 - HWY. 89 (CABOT)(S)		
County:	Code: 43	Name: LONOKE	
Sampled By:	THORNTON/FRAZIER/CAMPBELL		Depth: 0-5
Lab No.:	20180530	AASHTO Class:	A-4 (5)
Sample ID:	RV135	Material Type (1 or 2):	2
LATITUDE:		LONGITUDE:	

1. Testing Information:

Preconditioning - Permanent Strain > 5% (Y=Yes or N= No)	N
Testing - Permanent Strain > 5% (Y=Yes or N=No)	N
Number of Load Sequences Completed (0-15)	15

2. Specimen Information:

Specimen Diameter (in):	
Top	3.96
Middle	3.96
Bottom	3.96
Average	3.96
Membrane Thickness (in):	0.01
Height of Specimen, Cap and Base (in):	8.02
Height of Cap and Base (in):	0.00
Initial Length, Lo (in):	8.02
Initial Area, Ao (sq. in):	12.24
Initial Volume, AoLo (cu. in):	98.18

3. Soil Specimen Weight:

Weight of Wet Soil Used (g):	3255.20
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4. Soil Properties:

Optimum Moisture Content (%):	14.4
Maximum Dry Density (pcf):	113.7
95% of MDD (pcf):	108.0
In-Situ Moisture Content (%):	N/A

5. Specimen Properties:

Wet Weight (g):	3255.20
Compaction Moisture content (%):	15.2
Compaction Wet Density (pcf):	126.33
Compaction Dry Density (pcf):	109.66
Moisture Content After Mr Test (%):	14.7

6. Quick Shear Test (Y=Yes, N=No, N/A=Not Applicable): #VALUE!

7. Resilient Modulus, Mr: $8959(S_c)^{-0.23351}(S_3)^{0.33977}$

8. Comments

9. Tested By: GW **Date:** April 19, 2018

**ARKANSAS STATE HIGHWAY AND TRANSPORTATION DEPARTMENT
MATERIALS DIVISION**

**AASHTO T 307-99 - RESILIENT MODULUS OF SUBGRADE SOILS
RECOMPACTED SAMPLES**

Job No. 061509 **Material Code** SSRVPS
Date Sampled: 3/6/18 **Station No.:** 255+00
Date Tested: April 19, 2018 **Location:** 27LT

Name of Project: HWY. 367 - HWY. 89 (CABOT)(S)
County: Code: 43 **Name:** LONOKE
Sampled By: THORNTON/FRAZIER/CAMPBELL
Lab No.: 20180530
Sample ID: RV135
LATTITUDE:

Depth: 0-5
AASHTO Class: A-4 (5)
Material Type (1 or 2): 2
LONGITUDE:

PARAMETER	Chamber Confining Pressure	Nominal Maximum Axial Stress	Actual Applied Max. Axial Load	Actual Applied Cyclic Load	Actual Applied Contact Load	Actual Applied Max. Axial Stress	Actual Applied Cyclic Stress	Actual Applied Contact Stress	Average Recov Def. LVDT 1 and 2	Resilient Strain	Resilient Modulus
DESIGNATION	psi	psi	lbs	lbs	lbs	psi	psi	psi	in	in/in	psi
Sequence 1	6.0	2.0	25.3	22.5	2.8	2.1	1.8	0.2	0.00105	0.00013	14,070
Sequence 2	6.0	4.0	47.5	44.8	2.8	3.9	3.7	0.2	0.00225	0.00028	13,029
Sequence 3	6.0	6.0	70.1	66.4	3.7	5.7	5.4	0.3	0.00373	0.00046	11,666
Sequence 4	6.0	8.0	93.0	86.8	6.1	7.6	7.1	0.5	0.00561	0.00070	10,131
Sequence 5	6.0	10.0	115.6	107.0	8.6	9.4	8.7	0.7	0.00763	0.00095	9,182
Sequence 6	4.0	2.0	25.2	22.4	2.8	2.1	1.8	0.2	0.00118	0.00015	12,407
Sequence 7	4.0	4.0	47.1	44.2	2.9	3.8	3.6	0.2	0.00260	0.00032	11,160
Sequence 8	4.0	6.0	68.0	65.1	2.9	5.6	5.3	0.2	0.00432	0.00054	9,886
Sequence 9	4.0	8.0	90.9	85.7	5.3	7.4	7.0	0.4	0.00622	0.00078	9,017
Sequence 10	4.0	10.0	113.2	105.5	7.8	9.3	8.6	0.6	0.00835	0.00104	8,278
Sequence 11	2.0	2.0	25.0	22.1	2.8	2.0	1.8	0.2	0.00160	0.00020	9,040
Sequence 12	2.0	4.0	46.5	43.7	2.8	3.8	3.6	0.2	0.00330	0.00041	8,668
Sequence 13	2.0	6.0	66.6	63.7	2.9	5.4	5.2	0.2	0.00530	0.00066	7,881
Sequence 14	2.0	8.0	87.9	83.5	4.4	7.2	6.8	0.4	0.00740	0.00092	7,395
Sequence 15	2.0	10.0	110.0	103.1	6.9	9.0	8.4	0.6	0.00965	0.00120	6,998

TESTED BY _____ DATE April 19, 2018
 REVIEWED BY _____ DATE _____

**ARKANSAS STATE HIGHWAY AND TRANSPORTATION DEPARTMENT
MATERIALS DIVISION**

**AASHTO T 307-99 - RESILIENT MODULUS OF SUBGRADE SOILS
RECOMPACTED / THINWALL TUBE SAMPLES**

Job No.	061509	Material Code	SSRVPS
Date Sampled:	3/6/18	Station No.:	255+00
Date Tested:	April 19, 2018	Location:	27'LT
Name of Project:	HWY. 367 - HWY. 89 (CABOT)(S)		
County:	Code: 43	Name:	LONOKE
Sampled By:	THORNTON/FRAZIER/CAMPBELL		
Lab No.:	20180530	Depth:	0-5
Sample ID:	RV135	AASHTO Class:	A-4 (5)
LATITUDE:		Material Type (1 or 2):	2
		LONGITUDE:	

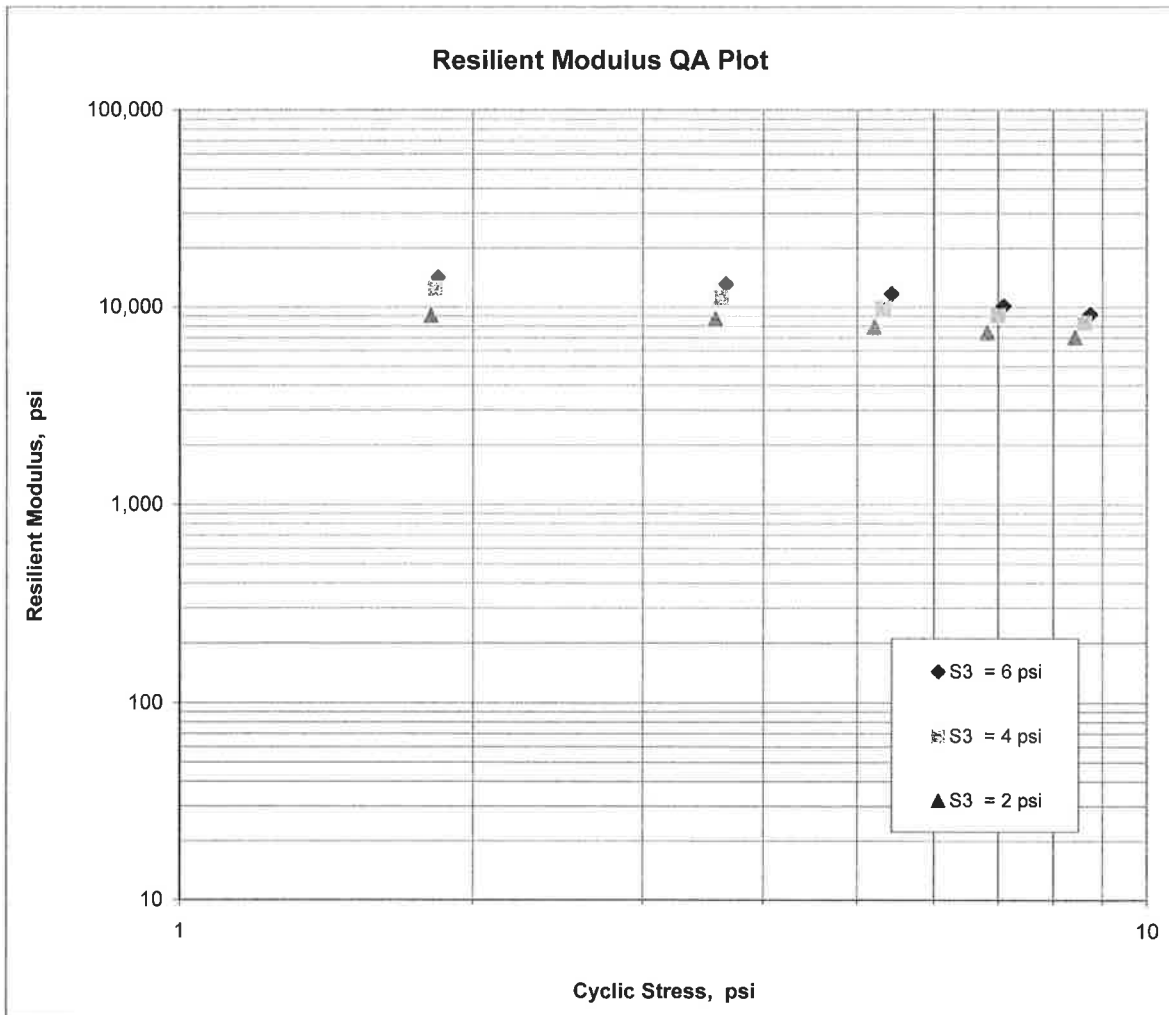
$$M_R = K_1 (S_c)^{K_2} (S_3)^{K_5}$$

$$K_1 = 8,959$$

$$K_2 = -0.23351$$

$$K_5 = 0.33977$$

$$R^2 = 0.95$$



**ARKANSAS STATE HIGHWAY AND TRANSPORTATION DEPARTMENT
MATERIALS DIVISION**

**AASHTO T 307-99 - RESILIENT MODULUS OF SUBGRADE SOILS
RECOMPACTED SAMPLES**

Job No.	061509	Material Code	SSRVPS
Date Sampled:	3/6/18	Station No.:	288+00
Date Tested:	April 19, 2018	Location:	27RT
Name of Project:	HWY. 367 - HWY. 89 (CABOT)(S)		
County:	Code: 43	Name:	LONOKE
Sampled By:	THORNTON/FRAZIER/CAMPBELL		
Lab No.:	20180531	Depth:	0-5
Sample ID:	RV136	AASHTO Class:	A-6 (4)
LATITUDE:		Material Type (1 or 2):	2
		LONGITUDE:	

1. Testing Information:

Preconditioning - Permanent Strain > 5% (Y=Yes or N= No)	N
Testing - Permanent Strain > 5% (Y=Yes or N=No)	N
Number of Load Sequences Completed (0-15)	15

2. Specimen Information:

Specimen Diameter (in):	
Top	3.95
Middle	3.95
Bottom	3.95
Average	3.95
Membrane Thickness (in):	0.01
Height of Specimen, Cap and Base (in):	8.02
Height of Cap and Base (in):	0.00
Initial Length, Lo (in):	8.02
Initial Area, Ao (sq. in):	12.18
Initial Volume, AoLo (cu. in):	97.68

3. Soil Specimen Weight:

Weight of Wet Soil Used (g):	3217.40
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4. Soil Properties:

Optimum Moisture Content (%):	15.9
Maximum Dry Density (pcf):	111.5
95% of MDD (pcf):	105.9
In-Situ Moisture Content (%):	N/A

5. Specimen Properties:

Wet Weight (g):	3217.40
Compaction Moisture content (%):	16.8
Compaction Wet Density (pcf):	125.50
Compaction Dry Density (pcf):	107.45
Moisture Content After Mr Test (%):	16.1

6. Quick Shear Test (Y=Yes, N=No, N/A=Not Applicable): #VALUE!

7. Resilient Modulus, Mr: $9648(S_c)^{-0.30099}(S_3)^{0.21286}$

8. Comments

9. Tested By: GW **Date:** April 19, 2018

**ARKANSAS STATE HIGHWAY AND TRANSPORTATION DEPARTMENT
MATERIALS DIVISION**

**AAASHTO T 307-99 - RESILIENT MODULUS OF SUBGRADE SOILS
RECOMPACTED SAMPLES**

Job No. 061509 **Material Code** SSRVPS
Date Sampled: 3/6/18 **Station No.:** 288+00
Date Tested: April 19, 2018 **Location:** 27'RT

Name of Project: HWY. 367 - HWY. 89 (CABOT)(S)
County: Code: 43 **Name:** LONOKE
Sampled By: THORNTON/FRAZIER/CAMPBELL
Lab No.: 20180531
Sample ID: RV136
LATITUDE:

Depth: 0-5
AAASHTO Class: A-6 (4)
Material Type (1 or 2): 2
LONGITUDE:

PARAMETER	Chamber Confining Pressure	Nominal Maximum Axial Stress	Actual Applied Max. Axial Load	Actual Applied Cyclic Load	Actual Applied Contact Load	Actual Applied Max. Axial Stress	Actual Applied Cyclic Stress	Actual Applied Contact Stress	Average Recov Def. LVDT 1 and 2	Resilient Strain	Resilient Modulus
	S ₃ psi	S _{cyclic} psi	P _{max} lbs	P _{cyclic} lbs	P _{contact} lbs	S _{max} psi	S _{cyclic} psi	S _{contact} psi	H _{avg} in	ε _r in/in	M _r psi
Sequence 1	6.0	2.0	25.2	22.5	2.7	2.1	1.8	0.2	0.00127	0.00016	11,630
Sequence 2	6.0	4.0	47.1	44.4	2.6	3.9	3.6	0.2	0.00275	0.00034	10,629
Sequence 3	6.0	6.0	68.9	65.4	3.5	5.7	5.4	0.3	0.00469	0.00059	9,181
Sequence 4	6.0	8.0	90.8	84.9	5.9	7.5	7.0	0.5	0.00736	0.00092	7,594
Sequence 5	6.0	10.0	111.5	103.2	8.4	9.2	8.5	0.7	0.01043	0.00130	6,513
Sequence 6	4.0	2.0	25.1	22.3	2.8	2.1	1.8	0.2	0.00141	0.00018	10,421
Sequence 7	4.0	4.0	46.8	44.0	2.8	3.8	3.6	0.2	0.00311	0.00039	9,304
Sequence 8	4.0	6.0	67.7	64.8	2.9	5.6	5.3	0.2	0.00520	0.00065	8,212
Sequence 9	4.0	8.0	89.9	84.7	5.2	7.4	7.0	0.4	0.00772	0.00096	7,221
Sequence 10	4.0	10.0	111.1	103.5	7.7	9.1	8.5	0.6	0.01072	0.00134	6,353
Sequence 11	2.0	2.0	25.1	22.2	2.8	2.1	1.8	0.2	0.00176	0.00022	8,327
Sequence 12	2.0	4.0	46.7	43.9	2.8	3.8	3.6	0.2	0.00369	0.00046	7,832
Sequence 13	2.0	6.0	67.3	64.5	2.8	5.5	5.3	0.2	0.00592	0.00074	7,172
Sequence 14	2.0	8.0	88.3	84.0	4.3	7.2	6.9	0.4	0.00854	0.00107	6,472
Sequence 15	2.0	10.0	109.7	102.9	6.8	9.0	8.4	0.6	0.01159	0.00145	5,846

TESTED BY _____ DATE April 19, 2018
 REVIEWED BY _____ DATE _____

**ARKANSAS STATE HIGHWAY AND TRANSPORTATION DEPARTMENT
MATERIALS DIVISION**

**AASHTO T 307-99 - RESILIENT MODULUS OF SUBGRADE SOILS
RECOMPACTED / THINWALL TUBE SAMPLES**

Job No.	061509	Material Code	SSRVPS
Date Sampled:	3/6/18	Station No.:	288+00
Date Tested:	April 19, 2018	Location:	27'RT
Name of Project:	HWY. 367 - HWY. 89 (CABOT)(S)		
County:	Code: 43	Name:	LONOKE
Sampled By:	THORNTON/FRAZIER/CAMPBELL		
Lab No.:	20180531	Depth:	0-5
Sample ID:	RV136	AASHTO Class:	A-6 (4)
LATITUDE:		Material Type (1 or 2):	2
		LONGITUDE:	

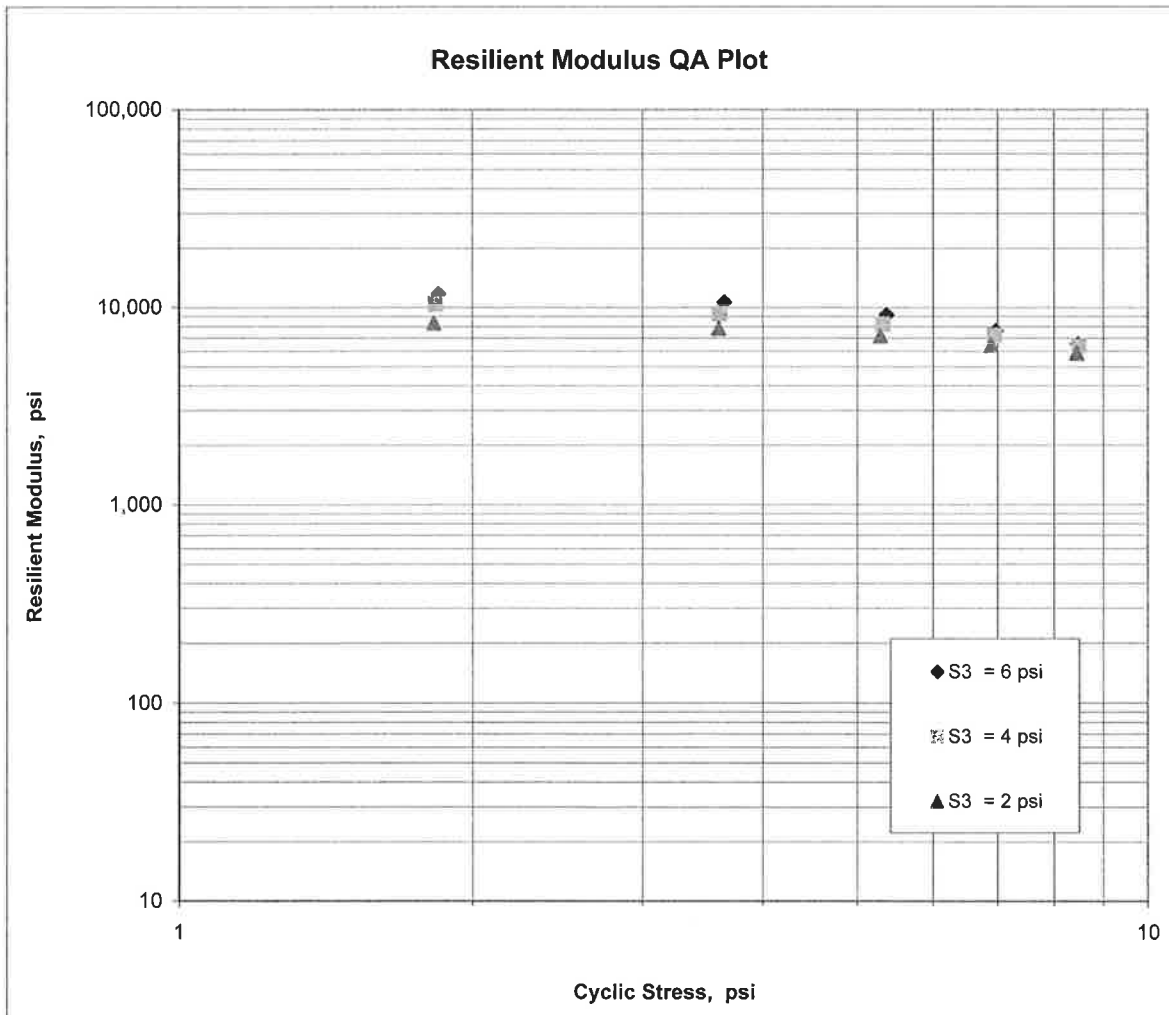
$$M_R = K_1 (S_C)^{K_2} (S_3)^{K_5}$$

$$K_1 = 9,648$$

$$K_2 = -0.30099$$

$$K_5 = 0.21286$$

$$R^2 = 0.89$$



**ARKANSAS STATE HIGHWAY AND TRANSPORTATION DEPARTMENT
MATERIALS DIVISION**

**AASHTO T 307-99 - RESILIENT MODULUS OF SUBGRADE SOILS
RECOMPACTED SAMPLES**

Job No.	061509	Material Code	SSRVPS
Date Sampled:	3/6/18	Station No.:	346+00
Date Tested:	April 19, 2018	Location:	27LT
Name of Project:	HWY. 367 - HWY. 89 (CABOT)(S)		
County:	Code: 43	Name:	LONOKE
Sampled By:	THORNTON/FRAZIER/CAMPBELL		
Lab No.:	20180532	Depth:	0-5
Sample ID:	RV137	AASHTO Class:	A-7-6 (11)
LATITUDE:		Material Type (1 or 2):	2
		LONGITUDE:	

1. Testing Information:

Preconditioning - Permanent Strain > 5% (Y=Yes or N= No)	N
Testing - Permanent Strain > 5% (Y=Yes or N=No)	N
Number of Load Sequences Completed (0-15)	15

2. Specimen Information:

Specimen Diameter (in):	
Top	3.96
Middle	3.96
Bottom	3.96
Average	3.96
Membrane Thickness (in):	0.01
Height of Specimen, Cap and Base (in):	8.02
Height of Cap and Base (in):	0.00
Initial Length, Lo (in):	8.02
Initial Area, Ao (sq. in):	12.24
Initial Volume, AoLo (cu. in):	98.18

3. Soil Specimen Weight:

Weight of Wet Soil Used (g):	3101.50
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4. Soil Properties:

Optimum Moisture Content (%):	17.2
Maximum Dry Density (pcf):	106.3
95% of MDD (pcf):	101.0
In-Situ Moisture Content (%):	N/A

5. Specimen Properties:

Wet Weight (g):	3101.50
Compaction Moisture content (%):	17.4
Compaction Wet Density (pcf):	120.37
Compaction Dry Density (pcf):	102.53
Moisture Content After Mr Test (%):	17.0

6. Quick Shear Test (Y=Yes, N=No, N/A=Not Applicable): #VALUE!

7. Resilient Modulus, Mr: 8074(Sc)^{-0.21806(S3)^{0.20114}}

8. Comments

9. Tested By: GW **Date:** April 19, 2018

**ARKANSAS STATE HIGHWAY AND TRANSPORTATION DEPARTMENT
MATERIALS DIVISION**

**AAASHTO T 307-99 - RESILIENT MODULUS OF SUBGRADE SOILS
RECOMPACTED SAMPLES**

Job No. 061509 **Material Code** SSRVPS
Date Sampled: 3/6/18 **Station No.:** 346+00
Date Tested: April 19, 2018 **Location:** 27'LT
Name of Project: HWY. 367 - HWY. 89 (CABOT)(S)
County: Code: 43 **Name:** LONOKE
Sampled By: THORNTON/FRAZIER/CAMPBELL
Lab No.: 20180532
Sample ID: RV137
LATITUDE:
Depth: 0-5
AAASHTO Class: A-7-6 (11)
Material Type (1 or 2): 2
LONGITUDE:

PARAMETER	Chamber Confining Pressure	Nominal Maximum Axial Stress	Actual Applied Max. Axial Load		Actual Applied Cyclic Load		Actual Applied Max. Axial Stress	Actual Applied Cyclic Stress	Actual Applied Contact Stress	Average Recov Def. LVDT 1 and 2	Resilient Strain	Resilient Modulus
			P _{max} lbs	P _{cyclic} lbs	P _{cyclic} lbs	P _{contact} lbs						
DESIGNATION	S ₃	S _{cyclic} psi	P _{max} lbs	P _{cyclic} lbs	P _{cyclic} lbs	P _{contact} lbs	S _{max} psi	S _{cyclic} psi	S _{contact} psi	H _{avg} in	ε _r in/in	M _r psi
UNIT	psi	psi	lbs	lbs	lbs	lbs	psi	psi	psi	in	in/in	psi
Sequence 1	6.0	2.0	25.3	22.5	22.5	2.8	2.1	1.8	0.2	0.00145	0.00018	10,149
Sequence 2	6.0	4.0	47.6	44.8	44.8	2.8	3.9	3.7	0.2	0.00311	0.00039	9,421
Sequence 3	6.0	6.0	70.0	66.3	66.3	3.7	5.7	5.4	0.3	0.00513	0.00064	8,467
Sequence 4	6.0	8.0	92.7	86.6	86.6	6.1	7.6	7.1	0.5	0.00772	0.00096	7,343
Sequence 5	6.0	10.0	114.3	105.8	105.8	8.5	9.3	8.6	0.7	0.01060	0.00132	6,538
Sequence 6	4.0	2.0	25.3	22.6	22.6	2.8	2.1	1.8	0.2	0.00161	0.00020	9,198
Sequence 7	4.0	4.0	47.2	44.5	44.5	2.7	3.9	3.6	0.2	0.00349	0.00043	8,354
Sequence 8	4.0	6.0	68.3	65.6	65.6	2.7	5.6	5.4	0.2	0.00569	0.00071	7,559
Sequence 9	4.0	8.0	91.4	86.3	86.3	5.1	7.5	7.0	0.4	0.00814	0.00102	6,941
Sequence 10	4.0	10.0	113.8	106.3	106.3	7.5	9.3	8.7	0.6	0.01092	0.00136	6,376
Sequence 11	2.0	2.0	25.1	22.5	22.5	2.7	2.1	1.8	0.2	0.00198	0.00025	7,427
Sequence 12	2.0	4.0	47.0	44.3	44.3	2.7	3.8	3.6	0.2	0.00408	0.00051	7,103
Sequence 13	2.0	6.0	68.0	65.3	65.3	2.6	5.6	5.3	0.2	0.00639	0.00080	6,692
Sequence 14	2.0	8.0	89.7	85.5	85.5	4.2	7.3	7.0	0.3	0.00899	0.00112	6,231
Sequence 15	2.0	10.0	112.2	105.5	105.5	6.7	9.2	8.6	0.5	0.01172	0.00146	5,898

TESTED BY _____ DATE _____
 REVIEWED BY _____ DATE _____

**ARKANSAS STATE HIGHWAY AND TRANSPORTATION DEPARTMENT
MATERIALS DIVISION**

**AASHTO T 307-99 - RESILIENT MODULUS OF SUBGRADE SOILS
RECOMPACTED / THINWALL TUBE SAMPLES**

Job No.	061509	Material Code	SSRVPS
Date Sampled:	3/6/18	Station No.:	346+00
Date Tested:	April 19, 2018	Location:	27'LT
Name of Project:	HWY. 367 - HWY. 89 (CABOT)(S)		
County:	Code: 43	Name:	LONOKE
Sampled By:	THORNTON/FRAZIER/CAMPBELL		
Lab No.:	20180532	Depth:	0-5
Sample ID:	RV137	AASHTO Class:	A-7-6 (11)
LATITUDE:		Material Type (1 or 2):	2
		LONGITUDE:	

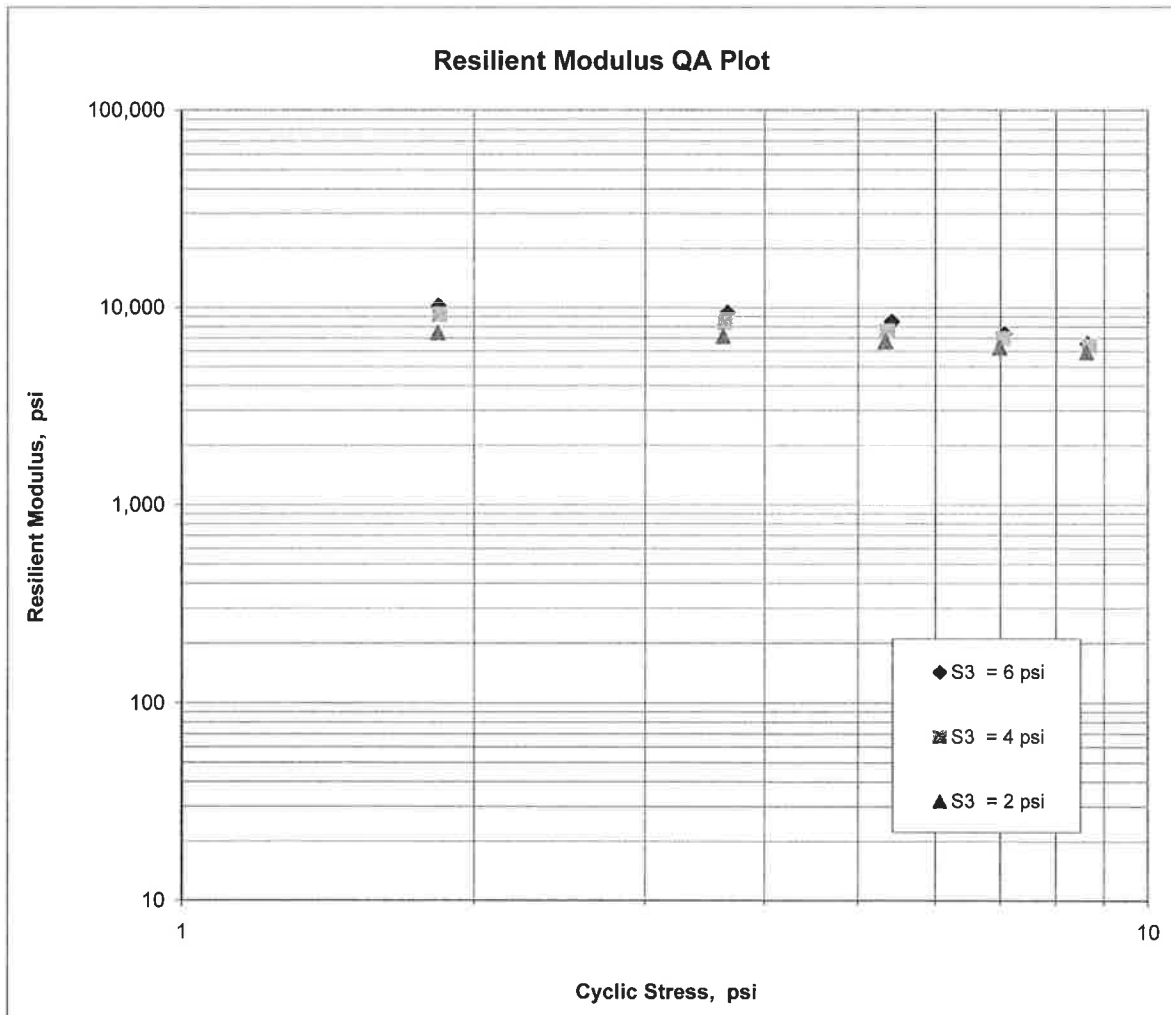
$$M_R = K_1 (S_C)^{K_2} (S_3)^{K_5}$$

$$K_1 = 8,074$$

$$K_2 = -0.21806$$

$$K_5 = 0.20114$$

$$R^2 = 0.91$$



JOB: 061509

Arkansas State Highway Transportation Department

JOB NAME: HWY. 367 - HWY. 89 (CABOT)(S)

Materials Division

COUNTY NO. 43 DATE TESTED 4/10/2018

Michael Benson, Materials Engineer

STA.#	LOC.	DEPTH	COLOR						L.L.	P.I.	SOIL CLASS	LAB #:	%MOISTURE
				#4	#10	#40	#80	#200					
				S	I	E	V	E	S				
230+00	27 RT	0-5	BR/GR	95	94	92	90	87	24	07	A-4(4)	RV134	
255+00	27 LT	0-5	BR/GR	90	88	86	84	78	24	09	A-4(5)	RV135	
288+00	27 RT	0-5	BR/GR	79	72	65	59	38	37	24	A-6(4)	RV136	
346+00	27 LT	0-5	BR/GR	82	81	79	76	54	41	28	A-7-6(11)	RV137	
255+00	18 LT	0-5	BROWN	96	95	94	93	88	28	11	A-6(8)	S100	16.8
255+00	27 LT	0-5	BR/GR	100				94	28	10	A-4(8)	S101	20.3
261+00	06 RT	0-5	BR/GR	99	97	96	95	87	24	08	A-4(5)	S102	17.8
261+00	18 RT	0-5	BROWN	98	97	96	94	86	24	07	A-4(4)	S103	16.9
261+00	27 RT	0-5	BROWN	72	70	67	65	56	25	07	A-4(1)	S104	19.5
280+00	06 LT	0-5	BROWN	100	99	98	95	64	40	22	A-6(12)	S105	26.6
280+00	18 LT	0-5	BROWN	99	97	96	95	51	38	18	A-6(6)	S106	29.1
280+00	27 LT	0-5	BROWN	96	94	91	86	66	39	27	A-6(15)	S107	25.2
288+00	06 RT	0-5	GRAY	99	99	97	96	46	39	25	A-6(8)	S108	21.3
288+00	18 RT	0-5	BROWN	98	91	89	81	56	42	29	A-7-6(12)	S109	21.1
288+00	27 RT	0-5	BR/GR	99	99	98	88	70	37	26	A-6(15)	S110	21.7
296+00	06 LT	0-5	GRAY	98	97	96	92	83	27	10	A-4(6)	S111	23.4
296+00	18 LT	0-5	GRAY	92	83	77	71	61	26	09	A-4(3)	S112	26.8
304+00	06 RT	0-5	GRAY	99	97	96	90	74	26	11	A-6(6)	S113	22.3
304+00	18 RT	0-5	GRAY	100	99	99	93	75	23	04	A-4(1)	S114	23.8
304+00	27 RT	0-5	GRAY	98	98	92	91	74	21	05	A-4(1)	S115	19.9
312+00	12 LT	0-5	BROWN	77	68	63	58	44	26	13	A-6(2)	S116	22.2
312+00	21 LT	0-5	BROWN	99	99	98	82	63				S117	18.6
320+00	06 RT	0-5	GRAY	99	95	91	85	74	35	22	A-6(14)	S118	21.6
320+00	18 RT	0-5	GRAY	99	98	97	92	71	31	18	A-6(10)	S119	21.3
320+00	27 RT	0-5	BROWN	99	97	94	89	72	34	20	A-6(12)	S120	22.4
328+00	06 LT	0-5	BROWN	99	99	95	85	69	28	15	A-6(7)	S121	21.5

comments: W=MULTIPLE LAYERS

Tuesday, April 24, 2018

STA.#	LOC.	DEPTH	COLOR	#4	#10	#40	#80	#200	L.L.	P.I.	SOIL CLASS	LAB #:	%MOISTURE
				S	I	E	V	E					
328+00	18 LT	0-5	BROWN	85	80	77	72	58	36	23	A-6(10)	S122	19.1
328+00	27 LT	0-5	BROWN	99	97	94	86	70	32	19	A-6(11)	S123	20.5
338+00	12 RT	0-5	BR/GR	98	95	93	82	66	31	16	A-6(8)	S124	19.6
338+00	20 RT	0-5	BR/GR	97	95	92	83	66	36	23	A-6(12)	S125	23.1
338+00	27 RT	0-5	BR/GR	85	83	81	71	56	25	11	A-6(3)	S126	19.9
346+00	12 LT	0-5	BROWN	99	99	98	94	58	46	33	A-7-6(15)	S127	29.7
346+00	18 LT	0-5	BROWN	99	99	97	93	66	28	12	A-6(5)	S128	22.1
346+00	27 LT	0-5	BR/GR	99	99	97	94	66	39	22	A-6(12)	S129	19.3
354+00	12 RT	0-5	BR/GR	98	95	75	72	58	22	07	A-4(1)	S130	19.7
354+00	18 RT	0-5	BROWN	99	99	97	76	61	24	11	A-6(4)	S131	17.2
362+00	12 LT	0-5	BROWN	100	99	97	84	68	23	07	A-4(2)	S132	21.5
362+00	18 LT	0-5	BROWN	98	97	94	88	72	31	16	A-6(9)	S133	20.8
203+00	12 RT	0-5	BR/GR	100				93	32	16	A-6(14)	S82	21.2
203+00	21 RT	0-5	BROWN	90	87	83	80	72	23	07	A-4(3)	S83	21.4
203+00	30 RT	0-5	BROWN	98	95	93	90	85	28	12	A-6(8)	S84	23.9
211+00	12 LT	0-5	GRAY	100				93	26	11	A-6(8)	S85	23.2
211+00	21 LT	0-5	GRAY	100				90	27	11	A-6(8)	S86	21.4
219+00	12 RT	0-5	BR/GR	97	94	92	91	87	27	10	A-4(7)	S87	19.2
219+00	21 LT	0-5	BR/GR	96	91	87	84	78	23	08	A-4(4)	S88	14.6
219+00	30 RT	0-5	BR/GR	91	89	87	85	82	24	08	A-4(4)	S89	11.7
230+00	06 RT	0-5	GRAY	100				93	26	09	A-4(7)	S90	18.2
230+00	18 RT	0-5	GRAY	96	95	94	93	90	25	08	A-4(5)	S91	18.7
230+00	27 RT	0-5	BR/GR	100				92	24	06	A-4(4)	S92	20.4
238+00	06 LT	0-5	GRAY	100				95	25	06	A-4(4)	S93	19.1
238+00	18 LT	0-5	GRAY	100				92	25	10	A-4(7)	S94	20.7
238+00	27 LT	0-5	BROWN	92	89	84	81	77	26	10	A-4(5)	S95	16.7
246+00	06 RT	0-5	GRAY	100				91	27	09	A-4(7)	S96	21.1
246+00	18 RT	0-5	GRAY	100				92	27	09	A-4(7)	S97	18.9

comments: W=MULTIPLE LAYERS

Tuesday, April 24, 2018

STA.#	LOC.	DEPTH	COLOR	#4	#10	#40	#80	#200	L.L.	P.I.	SOIL CLASS	LAB #:	%MOISTURE
				S	I	E	V	E					
246+00	27 RT	0-5	BR/GR	73	70	66	63	59	26	08	A-4(2)	S98	17.7
255+00	06 LT	0-5	BROWN	100				90	31	14	A-6(12)	S99	19.6

JOB: 061509

**Arkansas State Highway Transportation Department
Materials Division**

DATE TESTED
4/10/2018

JOB NAME: HWY. 367 - HWY. 89 (CABOT)(S)

COUNTY NO. 43

Michael Benson, Materials Engineer

STA.# LOC.

PAVEMENT SOUNDINGS

203+00	12 RT	ACHMSC	ACHMBC	AGG. BASE CRS CL-7
		3.0W	4.0	6.0
203+00	21 RT	ACHMSC	ACHMBC	AGG. BASE CRS CL-7
		2.25W	---	7.0
203+00	30 RT	ACHMSC	ACHMBC	AGG. BASE CRS CL-7
		---	---	---
211+00	12 LT	ACHMSC	ACHMBC	AGG. BASE CRS CL-7
		4.0W	4.25	8.0
211+00	21 LT	ACHMSC	ACHMBC	AGG. BASE CRS CL-7
		4.0W	---	8.0
219+00	12 RT	ACHMSC	ACHMBC	AGG. BASE CRS CL-7
		2.0W	5.0	6.0
219+00	21 LT	ACHMSC	ACHMBC	AGG. BASE CRS CL-7
		2.0W	4.5	7.0
219+00	30 RT	ACHMSC	ACHMBC	AGG. BASE CRS CL-7
		---	---	---
230+00	06 RT	ACHMSC	ACHMBC	AGG. BASE CRS CL-7
		2.0W	5.0	6.0
230+00	18 RT	ACHMSC	ACHMBC	AGG. BASE CRS CL-7
		4.0W	---	7.0
230+00	27 RT	ACHMSC	ACHMBC	AGG. BASE CRS CL-7
		---	---	---
238+00	06 LT	ACHMSC	ACHMBC	AGG. BASE CRS CL-7
		4.0W	4.0	8.0
238+00	18 LT	ACHMSC	ACHMBC	AGG. BASE CRS CL-7
		4.0W	---	8.0
238+00	27 LT	ACHMSC	ACHMBC	AGG. BASE CRS CL-7
		---	---	---
246+00	06 RT	ACHMSC	ACHMBC	AGG. BASE CRS CL-7
		3.0W	4.0	7.0
246+00	18 RT	ACHMSC	ACHMBC	AGG. BASE CRS CL-7
		3.5W	---	7.0
246+00	27 RT	ACHMSC	ACHMBC	AGG. BASE CRS CL-7
		---	---	---

Comments: W=MULTIPLE LAYERS

PAVEMENT SOUNDINGS

STA.# LOC.

255+00	06 LT	ACHMSC	ACHMBC	AGG. BASE CRS CL-7	8.0
		2.5	4.0W		
255+00	18 LT	ACHMSC	ACHMBC	AGG. BASE CRS CL-7	8.0
		3.5W	---		
255+00	27 LT	ACHMSC	ACHMBC	AGG. BASE CRS CL-7	---
		---	---		
261+00	06 RT	ACHMSC	ACHMBC	AGG. BASE CRS CL-7	7.0
		2.0	4.75W		
261+00	18 RT	ACHMSC	ACHMBC	AGG. BASE CRS CL-7	7.0
		5.0W	---		
261+00	27 RT	ACHMSC	ACHMBC	AGG. BASE CRS CL-7	---
		---	---		
280+00	06 LT	ACHMSC	ACHMBC	AGG. BASE CRS CL-7	7.0
		3.0W	4.5		
280+00	18 LT	ACHMSC	ACHMBC	AGG. BASE CRS CL-7	8.0
		4.5	---		
280+00	27 LT	ACHMSC	ACHMBC	AGG. BASE CRS CL-7	8.0
		---	---		
288+00	06 RT	ACHMSC	ACHMBC	AGG. BASE CRS CL-7	6.0
		2.5	6.0W		
288+00	18 RT	ACHMSC	ACHMBC	AGG. BASE CRS CL-7	7.0
		3.5W	---		
288+00	27 RT	ACHMSC	ACHMBC	AGG. BASE CRS CL-7	---
		---	---		
296+00	06 LT	ACHMSC	ACHMBC	AGG. BASE CRS CL-7	7.0
		3.0	4.5W		
296+00	18 LT	ACHMSC	ACHMBC	AGG. BASE CRS CL-7	8.0
		4.0W	---		
304+00	06 RT	ACHMSC	ACHMBC	AGG. BASE CRS CL-7	6.0
		2.0	6.0		
304+00	18 RT	ACHMSC	ACHMBC	AGG. BASE CRS CL-7	6.0
		4.5	---		
304+00	27 RT	ACHMSC	ACHMSC	AGG. BASE CRS CL-7	AGG. BASE CRS CL-7
		---	---	ACHMBC	---
312+00	12 LT	ACHMSC	ACHMSC	AGG. BASE CRS CL-7	7.0
		4.5W	1.0X	ACHMBC	5.5
312+00	21 LT	ACHMSC	ACHMSC	AGG. BASE CRS CL-7	8.0
		7.0	---	ACHMBC	---

comments: W=MULTIPLE LAYERS

PAVEMENT SOUNDINGS

STA.# LOC.

320+00	06 RT	ACHMSC	ACHMBC	AGG. BASE CRS CL-7
		3.5	5.0	7.0
320+00	18 RT	ACHMSC	ACHMBC	AGG. BASE CRS CL-7
			3.5	7.0
320+00	27 RT	ACHMSC	ACHMBC	AGG. BASE CRS CL-7
328+00	06 LT	ACHMSC	ACHMBC	AGG. BASE CRS CL-7
		3.0	4.0	6.0
328+00	18 LT	ACHMSC	ACHMBC	AGG. BASE CRS CL-7
		3.25		8.0
328+00	27 LT	ACHMSC	ACHMBC	AGG. BASE CRS CL-7
338+00	12 RT	ACHMSC	ACHMBC	AGG. BASE CRS CL-7
		3.0	5.5	7.0
338+00	20 RT	ACHMSC	ACHMBC	AGG. BASE CRS CL-7
		5.5		8.0
338+00	27 RT	ACHMSC	ACHMBC	AGG. BASE CRS CL-7
346+00	12 LT	ACHMSC	ACHMBC	AGG. BASE CRS CL-7
		4.0W	4.5	7.0
346+00	18 LT	ACHMSC	ACHMBC	AGG. BASE CRS CL-7
			4.0	8.0
346+00	27 LT	ACHMSC	ACHMBC	AGG. BASE CRS CL-7
354+00	12 RT	ACHMSC	ACHMBC	AGG. BASE CRS CL-7
		3.0W	5.0	8.0
354+00	18 RT	ACHMSC	ACHMBC	AGG. BASE CRS CL-7
		4.5		7.0
362+00	12 LT	ACHMSC	ACHMBC	AGG. BASE CRS CL-7
362+00	18 LT	ACHMSC	AGG. BASE CRS CL-7	
		3.5W	5.0	

comments: W=MULTIPLE LAYERS

ARKANSAS STATE HIGHWAY AND TRANSPORTATION DEPARTMENT - LITTLE ROCK, ARKANSAS
MATERIALS DIVISION

MICHAEL BENSON, MATERIALS ENGINEER

*** SOIL SURVEY / PAVEMENT SOUNDING TEST REPORT ***

DATE	- 04/23/18	SEQUENCE NO.	- 1
JOB NUMBER	- 061509	MATERIAL CODE	- SSRVPS
FEDERAL AID NO.	- TO BE ASSIGNED	SPEC. YEAR	- 2014
PURPOSE	- SOIL SURVEY SAMPLE	SUPPLIER ID.	- 1
SPEC. REMARKS	- NO SPECIFICATION CHECK	COUNTY/STATE	- 43
SUPPLIER NAME	- STATE	DISTRICT NO.	- 06
NAME OF PROJECT	- HWY. 367 - HWY. 89 (CABOT) (S)		
PROJECT ENGINEER	- NOT APPLICABLE		
PIT/QUARRY	- ARKANSAS		
LOCATION	- LONOKE, COUNTY	DATE SAMPLED	- 03/06/18
SAMPLED BY	- THORNTON/FRAZIER	DATE RECEIVED	- 03/15/18
SAMPLE FROM	- TEST HOLE	DATE TESTED	- 04/10/18
MATERIAL DESC.	- SOIL SURVEY - R VALUE- PAVEMENT SOUNDINGS		

LAB NUMBER	- 20180477	- 20180478	- 20180479
SAMPLE ID	- S82	- S83	- S84
TEST STATUS	- INFORMATION ONLY	- INFORMATION ONLY	- INFORMATION ONLY
STATION	- 203+00	- 203+00	- 203+00
LOCATION	- 12 RT	- 21 RT	- 30 RT
DEPTH IN FEET	- 0-5	- 0-5	- 0-5
MAT'L COLOR	- BR/GR	- BROWN	- BROWN
MAT'L TYPE	-	-	-
LATITUDE DEG-MIN-SEC	- 34 56 46.80	- 34 56 46.80	- 34 56 46.80
LONGITUDE DEG-MIN-SEC	- 92 03 41.50	- 92 03 41.50	- 92 03 41.60
% PASSING	2 IN. -	-	-
	1 1/2 IN. -	- 100	-
	3/4 IN. -	- 91	-
	3/8 IN. -	- 91	- 100
	NO. 4 - 100	- 90	- 98
	NO. 10 -	- 87	- 95
	NO. 40 -	- 83	- 93
	NO. 80 -	- 80	- 90
	NO. 200 - 93	- 72	- 85
LIQUID LIMIT	- 32	- 23	- 28
PLASTICITY INDEX	- 16	- 07	- 12
AASHTO SOIL	- A-6(14)	- A-4(3)	- A-6(8)
UNIFIED SOIL	-	-	-
% MOISTURE CONTENT	- 21.2	- 21.4	- 23.9
ACHMSC (IN)	- 3.0W	- 2.25W	- ---
ACHMBC (IN)	- 4.0	- ---	- ---
AGG. BASE CRS CL-7 (IN)	- 6.0	- 7.0	- ---
	-	-	-
	-	-	-
	-	-	-
	-	-	-
	-	-	-
	-	-	-

REMARKS - W=MULTIPLE LAYERS

ARKANSAS STATE HIGHWAY AND TRANSPORTATION DEPARTMENT - LITTLE ROCK, ARKANSAS
MATERIALS DIVISION

MICHAEL BENSON, MATERIALS ENGINEER

*** SOIL SURVEY / PAVEMENT SOUNDING TEST REPORT ***

DATE - 04/10/18 SEQUENCE NO. - 2
JOB NUMBER - 061509 MATERIAL CODE - SSRVPS
FEDERAL AID NO. - TO BE ASSIGNED SPEC. YEAR - 2014
PURPOSE - SOIL SURVEY SAMPLE SUPPLIER ID. - 1
SPEC. REMARKS - NO SPECIFICATION CHECK COUNTY/STATE - 43
SUPPLIER NAME - STATE DISTRICT NO. - 06
NAME OF PROJECT - HWY. 367 - HWY. 89 (CABOT) (S)
PROJECT ENGINEER - NOT APPLICABLE
PIT/QUARRY - ARKANSAS
LOCATION - LONOKE, COUNTY DATE SAMPLED - 03/06/18
SAMPLED BY - THORNTON/FRAZIER DATE RECEIVED - 03/15/18
SAMPLE FROM - TEST HOLE DATE TESTED - 04/10/18
MATERIAL DESC. - SOIL SURVEY - R VALUE- PAVEMENT SOUNDINGS

LAB NUMBER	-	20180480	-	20180481	-	20180482	
SAMPLE ID	-	S85	-	S86	-	S87	
TEST STATUS	-	INFORMATION ONLY	-	INFORMATION ONLY	-	INFORMATION ONLY	
STATION	-	211+00	-	211+00	-	219+00	
LOCATION	-	12 LT	-	21 LT	-	12 RT	
DEPTH IN FEET	-	0-5	-	0-5	-	0-5	
MAT'L COLOR	-	GRAY	-	GRAY	-	BR/GR	
MAT'L TYPE	-	-	-	-	-	-	
LATITUDE DEG-MIN-SEC	-	34 56 39.80	-	34 56 39.90	-	34 56 34.90	
LONGITUDE DEG-MIN-SEC	-	92 03 37.10	-	92 03 36.80	-	92 03 30.20	
% PASSING	2	IN.	-	-	-	-	
	1 1/2	IN.	-	-	-	-	
	3/4	IN.	-	-	-	-	
	3/8	IN.	-	-	-	100	
	NO. 4	-	100	-	100	-	97
	NO. 10	-	-	-	-	-	94
	NO. 40	-	-	-	-	-	92
	NO. 80	-	-	-	-	-	91
	NO. 200	-	93	-	90	-	87
LIQUID LIMIT	-	26	-	27	-	27	
PLASTICITY INDEX	-	11	-	11	-	10	
AASHTO SOIL	-	A-6(8)	-	A-6(8)	-	A-4(7)	
UNIFIED SOIL	-	-	-	-	-	-	
% MOISTURE CONTENT	-	23.2	-	21.4	-	19.2	
ACHMSC	(IN)	-	4.0W	-	4.0W	-	2.0W
ACHMBC	(IN)	-	4.25	-	---	-	5.0
AGG. BASE CRS CL-7	(IN)	-	8.0	-	8.0	-	6.0
	-	-	-	-	-	-	-
	-	-	-	-	-	-	-
	-	-	-	-	-	-	-
	-	-	-	-	-	-	-
	-	-	-	-	-	-	-
	-	-	-	-	-	-	-
	-	-	-	-	-	-	-
	-	-	-	-	-	-	-

REMARKS - W=MULTIPLE LAYERS

ARKANSAS STATE HIGHWAY AND TRANSPORTATION DEPARTMENT - LITTLE ROCK, ARKANSAS
MATERIALS DIVISION

MICHAEL BENSON, MATERIALS ENGINEER

*** SOIL SURVEY / PAVEMENT SOUNDING TEST REPORT ***

DATE	- 04/10/18	SEQUENCE NO.	- 3
JOB NUMBER	- 061509	MATERIAL CODE	- SSRVPS
FEDERAL AID NO.	- TO BE ASSIGNED	SPEC. YEAR	- 2014
PURPOSE	- SOIL SURVEY SAMPLE	SUPPLIER ID.	- 1
SPEC. REMARKS	- NO SPECIFICATION CHECK	COUNTY/STATE	- 43
SUPPLIER NAME	- STATE	DISTRICT NO.	- 06
NAME OF PROJECT	- HWY. 367 - HWY. 89 (CABOT) (S)		
PROJECT ENGINEER	- NOT APPLICABLE		
PIT/QUARRY	- ARKANSAS		
LOCATION	- LONOKE, COUNTY	DATE SAMPLED	- 03/06/18
SAMPLED BY	- THORNTON/FRAZIER	DATE RECEIVED	- 03/15/18
SAMPLE FROM	- TEST HOLE	DATE TESTED	- 04/10/18
MATERIAL DESC.	- SOIL SURVEY - R VALUE- PAVEMENT SOUNDINGS		

LAB NUMBER	- 20180483	- 20180484	- 20180485
SAMPLE ID	- S88	- S89	- S90
TEST STATUS	- INFORMATION ONLY	- INFORMATION ONLY	- INFORMATION ONLY
STATION	- 219+00	- 219+00	- 230+00
LOCATION	- 21 LT	- 30 RT	- 06 RT
DEPTH IN FEET	- 0-5	- 0-5	- 0-5
MAT'L COLOR	- BR/GR	- BR/GR	- GRAY
MAT'L TYPE	-	-	-
LATITUDE DEG-MIN-SEC	- 34 56 34.90	- 34 56 34.90	- 34 56 35.50
LONGITUDE DEG-MIN-SEC	- 92 03 30.10	- 92 03 30.10	- 92 03 16.90
% PASSING			
2 IN.	-	-	-
1 1/2 IN.	-	-	-
3/4 IN.	-	100	-
3/8 IN.	- 100	- 96	-
NO. 4	- 96	- 91	- 100
NO. 10	- 91	- 89	-
NO. 40	- 87	- 87	-
NO. 80	- 84	- 85	-
NO. 200	- 78	- 82	- 93
LIQUID LIMIT	- 23	- 24	- 26
PLASTICITY INDEX	- 08	- 08	- 09
AASHTO SOIL	- A-4 (4)	- A-4 (4)	- A-4 (7)
UNIFIED SOIL	-	-	-
% MOISTURE CONTENT	- 14.6	- 11.7	- 18.2
ACHMSC (IN)	- 2.0W	- ---	- 2.0W
ACHMBC (IN)	- 4.5	- ---	- 5.0
AGG. BASE CRS CL-7 (IN)	- 7.0	- ---	- 6.0
	-	-	-
	-	-	-
	-	-	-
	-	-	-
	-	-	-
	-	-	-
	-	-	-

REMARKS - W=MULTIPLE LAYERS

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AASHTO TESTS : T24 T88 T89 T90 T265

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ARKANSAS STATE HIGHWAY AND TRANSPORTATION DEPARTMENT - LITTLE ROCK, ARKANSAS
MATERIALS DIVISION

MICHAEL BENSON, MATERIALS ENGINEER

*** SOIL SURVEY / PAVEMENT SOUNDING TEST REPORT ***

DATE - 04/10/18 SEQUENCE NO. - 4
JOB NUMBER - 061509 MATERIAL CODE - SSRVPS
FEDERAL AID NO.- TO BE ASSIGNED SPEC. YEAR - 2014
PURPOSE - SOIL SURVEY SAMPLE SUPPLIER ID. - 1
SPEC. REMARKS - NO SPECIFICATION CHECK COUNTY/STATE - 43
SUPPLIER NAME - STATE DISTRICT NO. - 06
NAME OF PROJECT - HWY. 367 - HWY. 89 (CABOT) (S)
PROJECT ENGINEER - NOT APPLICABLE
PIT/QUARRY - ARKANSAS
LOCATION - LONOKE, COUNTY DATE SAMPLED - 03/06/18
SAMPLED BY - THORNTON/FRAZIER DATE RECEIVED - 03/15/18
SAMPLE FROM - TEST HOLE DATE TESTED - 04/10/18
MATERIAL DESC. - SOIL SURVEY - R VALUE- PAVEMENT SOUNDINGS

LAB NUMBER	-	20180486	-	20180487	-	20180488
SAMPLE ID	-	S91	-	S92	-	S93
TEST STATUS	-	INFORMATION ONLY	-	INFORMATION ONLY	-	INFORMATION ONLY
STATION	-	230+00	-	230+00	-	238+00
LOCATION	-	18 RT	-	27 RT	-	06 LT
DEPTH IN FEET	-	0-5	-	0-5	-	0-5
MAT'L COLOR	-	GRAY	-	BR/GR	-	GRAY
MAT'L TYPE	-		-		-	
LATITUDE DEG-MIN-SEC	-	34 56 35.40	-	34 56 35.40	-	34 56 35.90
LONGITUDE DEG-MIN-SEC	-	92 03 16.90	-	92 03 16.90	-	92 03 7.30
% PASSING	2	IN.	-		-	
	1 1/2	IN.	-		-	
	3/4	IN.	-		-	
	3/8	IN.	-		-	
	NO. 4	-		100	-	100
	NO. 10	-			-	
	NO. 40	-			-	
	NO. 80	-			-	
	NO. 200	-		92	-	95
LIQUID LIMIT	-	25	-	24	-	25
PLASTICITY INDEX	-	08	-	06	-	06
AASHTO SOIL	-	A-4 (5)	-	A-4 (4)	-	A-4 (4)
UNIFIED SOIL	-		-		-	
% MOISTURE CONTENT	-	18.7	-	20.4	-	19.1
ACHMSC	(IN)	4.0W	-	---	-	4.0W
ACHMBC	(IN)	---	-	---	-	4.0
AGG. BASE CRS CL-7	(IN)	7.0	-		-	8.0
			-		-	
			-		-	
			-		-	
			-		-	
			-		-	
			-		-	
			-		-	
			-		-	
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			-		-	

REMARKS - W=MULTIPLE LAYERS

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AASHTO TESTS : T24 T88 T89 T90 T265

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ARKANSAS STATE HIGHWAY AND TRANSPORTATION DEPARTMENT - LITTLE ROCK, ARKANSAS
MATERIALS DIVISION

MICHAEL BENSON, MATERIALS ENGINEER

*** SOIL SURVEY / PAVEMENT SOUNDING TEST REPORT ***

DATE - 04/23/18 SEQUENCE NO. - 5
 JOB NUMBER - 061509 MATERIAL CODE - SSRVPS
 FEDERAL AID NO. - TO BE ASSIGNED SPEC. YEAR - 2014
 PURPOSE - SOIL SURVEY SAMPLE SUPPLIER ID. - 1
 SPEC. REMARKS - NO SPECIFICATION CHECK COUNTY/STATE - 43
 SUPPLIER NAME - STATE DISTRICT NO. - 06
 NAME OF PROJECT - HWY. 367 - HWY. 89 (CABOT) (S)
 PROJECT ENGINEER - NOT APPLICABLE
 PIT/QUARRY - ARKANSAS
 LOCATION - LONOKE, COUNTY DATE SAMPLED - 03/06/18
 SAMPLED BY - THORNTON/FRAZIER DATE RECEIVED - 03/15/18
 SAMPLE FROM - TEST HOLE DATE TESTED - 04/10/18
 MATERIAL DESC. - SOIL SURVEY - R VALUE- PAVEMENT SOUNDINGS

LAB NUMBER	- 20180489	- 20180490	- 20180491
SAMPLE ID	- S94	- S95	- S96
TEST STATUS	- INFORMATION ONLY	- INFORMATION ONLY	- INFORMATION ONLY
STATION	- 238+00	- 238+00	- 246+00
LOCATION	- 18 LT	- 27 LT	- 06 RT
DEPTH IN FEET	- 0-5	- 0-5	- 0-5
MAT'L COLOR	- GRAY	- BROWN	- GRAY
MAT'L TYPE	-	-	-
LATITUDE DEG-MIN-SEC	- 34 56 36.10	- 34 56 36.20	- 34 56 34.90
LONGITUDE DEG-MIN-SEC	- 92 03 7.30	- 92 03 7.30	- 92 02 59.40

% PASSING	2 IN.	-	-	-
	1 1/2 IN.	-	-	-
	3/4 IN.	-	100	-
	3/8 IN.	-	97	-
	NO. 4	- 100	- 92	- 100
	NO. 10	-	- 89	-
	NO. 40	-	- 84	-
	NO. 80	-	- 81	-
	NO. 200	- 92	- 77	- 91

LIQUID LIMIT	- 25	- 26	- 27
PLASTICITY INDEX	- 10	- 10	- 09
AASHTO SOIL	- A-4 (7)	- A-4 (5)	- A-4 (7)
UNIFIED SOIL	-	-	-
% MOISTURE CONTENT	- 20.7	- 16.7	- 21.1

ACHMSC	(IN) - 4.0W	- ---	- 3.0W
ACHMBC	(IN) - ---	- ---	- 4.0
AGG. BASE CRS CL-7	(IN) - 8.0	- ---	- 7.0

REMARKS - W=MULTIPLE LAYERS

AASHTO TESTS : T24 T88 T89 T90 T265

ARKANSAS STATE HIGHWAY AND TRANSPORTATION DEPARTMENT - LITTLE ROCK, ARKANSAS
MATERIALS DIVISION

MICHAEL BENSON, MATERIALS ENGINEER

*** SOIL SURVEY / PAVEMENT SOUNDING TEST REPORT ***

DATE - 04/10/18 SEQUENCE NO. - 6
JOB NUMBER - 061509 MATERIAL CODE - SSRVPS
FEDERAL AID NO. - TO BE ASSIGNED SPEC. YEAR - 2014
PURPOSE - SOIL SURVEY SAMPLE SUPPLIER ID. - 1
SPEC. REMARKS - NO SPECIFICATION CHECK COUNTY/STATE - 43
SUPPLIER NAME - STATE DISTRICT NO. - 06
NAME OF PROJECT - HWY. 367 - HWY. 89 (CABOT) (S)
PROJECT ENGINEER - NOT APPLICABLE
PIT/QUARRY - ARKANSAS
LOCATION - LONOKE, COUNTY DATE SAMPLED - 03/06/18
SAMPLED BY - THORNTON/FRAZIER DATE RECEIVED - 03/15/18
SAMPLE FROM - TEST HOLE DATE TESTED - 04/10/18
MATERIAL DESC. - SOIL SURVEY - R VALUE- PAVEMENT SOUNDINGS

LAB NUMBER	-	20180492	-	20180493	-	20180494
SAMPLE ID	-	S97	-	S98	-	S99
TEST STATUS	-	INFORMATION ONLY	-	INFORMATION ONLY	-	INFORMATION ONLY
STATION	-	246+00	-	246+00	-	255+00
LOCATION	-	18 RT	-	27 RT	-	06 LT
DEPTH IN FEET	-	0-5	-	0-5	-	0-5
MAT'L COLOR	-	GRAY	-	BR/GR	-	BROWN
MAT'L TYPE	-		-		-	
LATITUDE DEG-MIN-SEC	-	34 56 34.70	-	34 56 34.70	-	34 56 31.70
LONGITUDE DEG-MIN-SEC	-	92 02 57.80	-	92 02 57.80	-	92 02 48.10
% PASSING	2	IN. -	-		-	
	1 1/2	IN. -	-	100	-	
	3/4	IN. -	-	86	-	
	3/8	IN. -	-	78	-	
	NO. 4	- 100	-	73	-	100
	NO. 10	-	-	70	-	
	NO. 40	-	-	66	-	
	NO. 80	-	-	63	-	
	NO. 200	- 92	-	59	-	90
LIQUID LIMIT	-	27	-	26	-	31
PLASTICITY INDEX	-	09	-	08	-	14
AASHTO SOIL	-	A-4 (7)	-	A-4 (2)	-	A-6 (12)
UNIFIED SOIL	-		-		-	
% MOISTURE CONTENT	-	18.9	-	17.7	-	19.6
ACHMSC	(IN)	- 3.5W	-	---	-	2.5
ACHMBC	(IN)	- ---	-	---	-	4.0W
AGG. BASE CRS CL-7	(IN)	- 7.0	-	---	-	8.0
	-		-		-	
	-		-		-	
	-		-		-	
	-		-		-	
	-		-		-	
	-		-		-	
	-		-		-	
	-		-		-	
	-		-		-	
	-		-		-	
	-		-		-	
	-		-		-	

REMARKS - W=MULTIPLE LAYERS

AASHTO TESTS : T24 T88 T89 T90 T265

ARKANSAS STATE HIGHWAY AND TRANSPORTATION DEPARTMENT - LITTLE ROCK, ARKANSAS
MATERIALS DIVISION

MICHAEL BENSON, MATERIALS ENGINEER

*** SOIL SURVEY / PAVEMENT SOUNDING TEST REPORT ***

DATE	- 04/23/18	SEQUENCE NO.	- 7
JOB NUMBER	- 061509	MATERIAL CODE	- SSRVPS
FEDERAL AID NO.	- TO BE ASSIGNED	SPEC. YEAR	- 2014
PURPOSE	- SOIL SURVEY SAMPLE	SUPPLIER ID.	- 1
SPEC. REMARKS	- NO SPECIFICATION CHECK	COUNTY/STATE	- 43
SUPPLIER NAME	- STATE	DISTRICT NO.	- 06
NAME OF PROJECT	- HWY. 367 - HWY. 89 (CABOT) (S)		
PROJECT ENGINEER	- NOT APPLICABLE		
PIT/QUARRY	- ARKANSAS		
LOCATION	- LONOKE, COUNTY	DATE SAMPLED	- 03/06/18
SAMPLED BY	- THORNTON/FRAZIER	DATE RECEIVED	- 03/15/18
SAMPLE FROM	- TEST HOLE	DATE TESTED	- 04/10/18
MATERIAL DESC.	- SOIL SURVEY - R VALUE- PAVEMENT SOUNDINGS		

LAB NUMBER	-	20180495	-	20180496	-	20180497
SAMPLE ID	-	S100	-	S101	-	S102
TEST STATUS	-	INFORMATION ONLY	-	INFORMATION ONLY	-	INFORMATION ONLY
STATION	-	255+00	-	255+00	-	261+00
LOCATION	-	18 LT	-	27 LT	-	06 RT
DEPTH IN FEET	-	0-5	-	0-5	-	0-5
MAT'L COLOR	-	BROWN	-	BR/GR	-	BR/GR
MAT'L TYPE	-		-		-	
LATITUDE DEG-MIN-SEC	-	34 56 32.00	-	34 56 32.00	-	34 56 30.30
LONGITUDE DEG-MIN-SEC	-	92 02 47.60	-	92 02 47.60	-	92 02 43.60
% PASSING	2	IN.	-		-	
	1 1/2	IN.	-		-	
	3/4	IN.	-	100	-	
	3/8	IN.	-		-	100
	NO. 4		-	96	-	99
	NO. 10		-		-	97
	NO. 40		-		-	96
	NO. 80		-		-	95
	NO. 200		-	88	-	87
LIQUID LIMIT	-	28	-	28	-	24
PLASTICITY INDEX	-	11	-	10	-	08
AASHTO SOIL	-	A-6(8)	-	A-4(8)	-	A-4(5)
UNIFIED SOIL	-		-		-	
% MOISTURE CONTENT	-	16.8	-	20.3	-	17.8
ACHMSC	(IN)	3.5W	-	---	-	2.0
ACHMBC	(IN)	---	-	---	-	4.75W
AGG. BASE CRS CL-7	(IN)	8.0	-	---	-	7.0
			-		-	
			-		-	
			-		-	
			-		-	
			-		-	
			-		-	
			-		-	
			-		-	
			-		-	
			-		-	

REMARKS - W=MULTIPLE LAYERS
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ARKANSAS STATE HIGHWAY AND TRANSPORTATION DEPARTMENT - LITTLE ROCK, ARKANSAS
MATERIALS DIVISION

MICHAEL BENSON, MATERIALS ENGINEER

*** SOIL SURVEY / PAVEMENT SOUNDING TEST REPORT ***

DATE - 04/10/18 SEQUENCE NO. - 8
JOB NUMBER - 061509 MATERIAL CODE - SSRVPS
FEDERAL AID NO. - TO BE ASSIGNED SPEC. YEAR - 2014
PURPOSE - SOIL SURVEY SAMPLE SUPPLIER ID. - 1
SPEC. REMARKS - NO SPECIFICATION CHECK COUNTY/STATE - 43
SUPPLIER NAME - STATE DISTRICT NO. - 06
NAME OF PROJECT - HWY. 367 - HWY. 89 (CABOT) (S)
PROJECT ENGINEER - NOT APPLICABLE
PIT/QUARRY - ARKANSAS
LOCATION - LONOKE, COUNTY DATE SAMPLED - 03/06/18
SAMPLED BY - THORNTON/FRAZIER DATE RECEIVED - 03/15/18
SAMPLE FROM - TEST HOLE DATE TESTED - 04/10/18
MATERIAL DESC. - SOIL SURVEY - R VALUE- PAVEMENT SOUNDINGS

LAB NUMBER	20180498	20180499	20180500
SAMPLE ID	S103	S104	S105
TEST STATUS	INFORMATION ONLY	INFORMATION ONLY	INFORMATION ONLY
STATION	261+00	261+00	280+00
LOCATION	18 RT	27 RT	06 LT
DEPTH IN FEET	0-5	0-5	0-5
MAT'L COLOR	BROWN	BROWN	BROWN
MAT'L TYPE			
LATITUDE DEG-MIN-SEC	34 56 29.50	34 56 29.50	34 56 25.20
LONGITUDE DEG-MIN-SEC	92 02 41.00	92 02 41.00	92 02 27.20
% PASSING			
2 IN.			
1 1/2 IN.			
3/4 IN.			
3/8 IN.	100	100	
NO. 4	98	72	100
NO. 10	97	70	99
NO. 40	96	67	98
NO. 80	94	65	95
NO. 200	86	56	64
LIQUID LIMIT	24	25	40
PLASTICITY INDEX	07	07	22
AASHTO SOIL	A-4 (4)	A-4 (1)	A-6 (12)
UNIFIED SOIL			
% MOISTURE CONTENT	16.9	19.5	26.6
ACHMSC (IN)	5.0W	---	3.0W
ACHMBC (IN)	---	---	4.5
AGG. BASE CRS CL-7 (IN)	7.0	---	7.0
-			
-			
-			
-			
-			
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-			
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-			

REMARKS - W=MULTIPLE LAYERS
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AASHTO TESTS : T24 T88 T89 T90 T265
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ARKANSAS STATE HIGHWAY AND TRANSPORTATION DEPARTMENT - LITTLE ROCK, ARKANSAS
MATERIALS DIVISION

MICHAEL BENSON, MATERIALS ENGINEER

*** SOIL SURVEY / PAVEMENT SOUNDING TEST REPORT ***

DATE	- 04/23/18	SEQUENCE NO.	- 9
JOB NUMBER	- 061509	MATERIAL CODE	- SSRVPS
FEDERAL AID NO.	- TO BE ASSIGNED	SPEC. YEAR	- 2014
PURPOSE	- SOIL SURVEY SAMPLE	SUPPLIER ID.	- 1
SPEC. REMARKS	- NO SPECIFICATION CHECK	COUNTY/STATE	- 43
SUPPLIER NAME	- STATE	DISTRICT NO.	- 06
NAME OF PROJECT	- HWY. 367 - HWY. 89 (CABOT) (S)		
PROJECT ENGINEER	- NOT APPLICABLE		
PIT/QUARRY	- ARKANSAS		
LOCATION	- LONOKE, COUNTY	DATE SAMPLED	- 03/06/18
SAMPLED BY	- THORNTON/FRAZIER	DATE RECEIVED	- 03/15/18
SAMPLE FROM	- TEST HOLE	DATE TESTED	- 04/10/18
MATERIAL DESC.	- SOIL SURVEY - R VALUE- PAVEMENT SOUNDINGS		

LAB NUMBER	- 20180501	- 20180502	- 20180503
SAMPLE ID	- S106	- S107	- S108
TEST STATUS	- INFORMATION ONLY	- INFORMATION ONLY	- INFORMATION ONLY
STATION	- 280+00	- 280+00	- 288+00
LOCATION	- 18 LT	- 27 LT	- 06 RT
DEPTH IN FEET	- 0-5	- 0-5	- 0-5
MAT'L COLOR	- BROWN	- BROWN	- GRAY
MAT'L TYPE	-	-	-
LATITUDE DEG-MIN-SEC	- 34 56 23.10	- 34 56 23.20	- 34 56 21.30
LONGITUDE DEG-MIN-SEC	- 92 02 19.60	- 92 02 19.60	- 92 02 14.20
% PASSING	2 IN. -	-	-
	1 1/2 IN. -	-	-
	3/4 IN. -	- 100	-
	3/8 IN. - 100	- 99	- 100
	NO. 4 - 99	- 96	- 99
	NO. 10 - 97	- 94	- 99
	NO. 40 - 96	- 91	- 97
	NO. 80 - 95	- 86	- 96
	NO. 200 - 51	- 66	- 46
LIQUID LIMIT	- 38	- 39	- 39
PLASTICITY INDEX	- 18	- 27	- 25
AASHTO SOIL	- A-6 (6)	- A-6 (15)	- A-6 (8)
UNIFIED SOIL	-	-	-
% MOISTURE CONTENT	- 29.1	- 25.2	- 21.3
ACHMSC (IN)	- 4.5	- ---	- 2.5
ACHMBC (IN)	- ---	- ---	- 6.0W
AGG. BASE CRS CL-7 (IN)	- 8.0	- 8.0	- 6.0
	-	-	-
	-	-	-
	-	-	-
	-	-	-
	-	-	-
	-	-	-
	-	-	-

REMARKS - W=MULTIPLE LAYERS

AASHTO TESTS : T24 T88 T89 T90 T265

ARKANSAS STATE HIGHWAY AND TRANSPORTATION DEPARTMENT - LITTLE ROCK, ARKANSAS
MATERIALS DIVISION

MICHAEL BENSON, MATERIALS ENGINEER

*** SOIL SURVEY / PAVEMENT SOUNDING TEST REPORT ***

DATE	- 04/23/18	SEQUENCE NO.	- 10
JOB NUMBER	- 061509	MATERIAL CODE	- SSRVPS
FEDERAL AID NO.	- TO BE ASSIGNED	SPEC. YEAR	- 2014
PURPOSE	- SOIL SURVEY SAMPLE	SUPPLIER ID.	- 1
SPEC. REMARKS	- NO SPECIFICATION CHECK	COUNTY/STATE	- 43
SUPPLIER NAME	- STATE	DISTRICT NO.	- 06
NAME OF PROJECT - HWY. 367 - HWY. 89 (CABOT) (S)			
PROJECT ENGINEER - NOT APPLICABLE			
PIT/QUARRY - ARKANSAS			
LOCATION	- LONOKE, COUNTY	DATE SAMPLED	- 03/06/18
SAMPLED BY	- THORNTON/FRAZIER	DATE RECEIVED	- 03/15/18
SAMPLE FROM	- TEST HOLE	DATE TESTED	- 04/10/18
MATERIAL DESC. - SOIL SURVEY - R VALUE- PAVEMENT SOUNDINGS			

LAB NUMBER	- 20180504	- 20180505	- 20180506
SAMPLE ID	- S109	- S110	- S111
TEST STATUS	- INFORMATION ONLY	- INFORMATION ONLY	- INFORMATION ONLY
STATION	- 288+00	- 288+00	- 296+00
LOCATION	- 18 RT	- 27 RT	- 06 LT
DEPTH IN FEET	- 0-5	- 0-5	- 0-5
MAT'L COLOR	- BROWN	- BR/GR	- GRAY
MAT'L TYPE	-	-	-
LATITUDE DEG-MIN-SEC	- 34 56 20.40	- 34 56 20.40	- 34 56 20.40
LONGITUDE DEG-MIN-SEC	- 92 02 10.60	- 92 02 10.70	- 92 02 4.20
% PASSING			
2 IN.	-	-	-
1 1/2 IN.	-	-	-
3/4 IN.	- 100	-	- 100
3/8 IN.	- 98	- 100	- 99
NO. 4	- 98	- 99	- 98
NO. 10	- 91	- 99	- 97
NO. 40	- 89	- 98	- 96
NO. 80	- 81	- 88	- 92
NO. 200	- 56	- 70	- 83
LIQUID LIMIT	- 42	- 37	- 27
PLASTICITY INDEX	- 29	- 26	- 10
AASHTO SOIL	- A-7-6(12)	- A-6(15)	- A-4(6)
UNIFIED SOIL	-	-	-
% MOISTURE CONTENT	- 21.1	- 21.7	- 23.4
ACHMSC (IN)	- 3.5W	- ----	- 3.0
ACHMBC (IN)	- ----	- ----	- 4.5W
AGG. BASE CRS CL-7 (IN)	- 7.0	- ----	- 7.0
-	-	-	-
-	-	-	-
-	-	-	-
-	-	-	-
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REMARKS - W=MULTIPLE LAYERS
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AASHTO TESTS : T24 T88 T89 T90 T265
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ARKANSAS STATE HIGHWAY AND TRANSPORTATION DEPARTMENT - LITTLE ROCK, ARKANSAS
MATERIALS DIVISION

MICHAEL BENSON, MATERIALS ENGINEER

*** SOIL SURVEY / PAVEMENT SOUNDING TEST REPORT ***

DATE	- 04/23/18	SEQUENCE NO.	- 11
JOB NUMBER	- 061509	MATERIAL CODE	- SSRVPS
FEDERAL AID NO.	- TO BE ASSIGNED	SPEC. YEAR	- 2014
PURPOSE	- SOIL SURVEY SAMPLE	SUPPLIER ID.	- 1
SPEC. REMARKS	- NO SPECIFICATION CHECK	COUNTY/STATE	- 43
SUPPLIER NAME	- STATE	DISTRICT NO.	- 06
NAME OF PROJECT	- HWY. 367 - HWY. 89 (CABOT) (S)		
PROJECT ENGINEER	- NOT APPLICABLE		
PIT/QUARRY	- ARKANSAS		
LOCATION	- LONOKE, COUNTY	DATE SAMPLED	- 03/06/18
SAMPLED BY	- THORNTON/FRAZIER	DATE RECEIVED	- 03/15/18
SAMPLE FROM	- TEST HOLE	DATE TESTED	- 04/10/18
MATERIAL DESC.	- SOIL SURVEY - R VALUE- PAVEMENT SOUNDINGS		

LAB NUMBER	- 20180507	- 20180508	- 20180509
SAMPLE ID	- S112	- S113	- S114
TEST STATUS	- INFORMATION ONLY	- INFORMATION ONLY	- INFORMATION ONLY
STATION	- 296+00	- 304+00	- 304+00
LOCATION	- 18 LT	- 06 RT	- 18 RT
DEPTH IN FEET	- 0-5	- 0-5	- 0-5
MAT'L COLOR	- GRAY	- GRAY	- GRAY
MAT'L TYPE	-	-	-
LATITUDE DEG-MIN-SEC	- 34 56 20.80	- 34 56 20.90	- 34 56 21.00
LONGITUDE DEG-MIN-SEC	- 92 02 1.10	- 92 01 52.90	- 92 01 51.00
% PASSING	2 IN. -	-	-
	1 1/2 IN. -	-	-
	3/4 IN. - 100	-	-
	3/8 IN. - 98	- 100	-
	NO. 4 - 92	- 99	- 100
	NO. 10 - 83	- 97	- 99
	NO. 40 - 77	- 96	- 99
	NO. 80 - 71	- 90	- 93
	NO. 200 - 61	- 74	- 75
LIQUID LIMIT	- 26	- 26	- 23
PLASTICITY INDEX	- 09	- 11	- 04
AASHTO SOIL	- A-4 (3)	- A-6 (6)	- A-4 (1)
UNIFIED SOIL	-	-	-
% MOISTURE CONTENT	- 26.8	- 22.3	- 23.8
ACHMSC (IN)	- 4.0W	- 2.0	- 4.5
ACHMBC (IN)	- ---	- 6.0	- ---
AGG. BASE CRS CL-7 (IN)	- 8.0	- 6.0	- 6.0
	-	-	-
	-	-	-
	-	-	-
	-	-	-
	-	-	-
	-	-	-
	-	-	-
	-	-	-
	-	-	-

REMARKS - W=MULTIPLE LAYERS

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AASHTO TESTS : T24 T88 T89 T90 T265
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ARKANSAS STATE HIGHWAY AND TRANSPORTATION DEPARTMENT - LITTLE ROCK, ARKANSAS
MATERIALS DIVISION

MICHAEL BENSON, MATERIALS ENGINEER

*** SOIL SURVEY / PAVEMENT SOUNDING TEST REPORT ***

DATE	- 04/23/18	SEQUENCE NO.	- 12
JOB NUMBER	- 061509	MATERIAL CODE	- SSRVPS
FEDERAL AID NO.	- TO BE ASSIGNED	SPEC. YEAR	- 2014
PURPOSE	- SOIL SURVEY SAMPLE	SUPPLIER ID.	- 1
SPEC. REMARKS	- NO SPECIFICATION CHECK	COUNTY/STATE	- 43
SUPPLIER NAME	- STATE	DISTRICT NO.	- 06
NAME OF PROJECT	- HWY. 367 - HWY. 89 (CABOT) (S)		
PROJECT ENGINEER	- NOT APPLICABLE		
PIT/QUARRY	- ARKANSAS		
LOCATION	- LONOKE, COUNTY	DATE SAMPLED	- 03/06/18
SAMPLED BY	- THORNTON/FRAZIER	DATE RECEIVED	- 03/15/18
SAMPLE FROM	- TEST HOLE	DATE TESTED	- 04/11/18
MATERIAL DESC.	- SOIL SURVEY - R VALUE- PAVEMENT SOUNDINGS		

LAB NUMBER	- 20180510	- 20180511	- 20180512
SAMPLE ID	- S115	- S116	- S117
TEST STATUS	- INFORMATION ONLY	- INFORMATION ONLY	- INFORMATION ONLY
STATION	- 304+00	- 312+00	- 312+00
LOCATION	- 27 RT	- 12 LT	- 21 LT
DEPTH IN FEET	- 0-5	- 0-5	- 0-5
MAT'L COLOR	- GRAY	- BROWN	- BROWN
MAT'L TYPE	-	-	-
LATITUDE DEG-MIN-SEC	- 34 56 21.00	- 34 56 21.30	- 34 56 21.90
LONGITUDE DEG-MIN-SEC	- 92 01 51.00	- 92 01 43.80	- 92 01 41.60
% PASSING	2 IN. -	-	-
	1 1/2 IN. -	- 100	-
	3/4 IN. - 100	- 86	-
	3/8 IN. - 99	- 84	- 100
	NO. 4 - 98	- 77	- 99
	NO. 10 - 98	- 68	- 99
	NO. 40 - 92	- 63	- 98
	NO. 80 - 91	- 58	- 82
	NO. 200 - 74	- 44	- 63
LIQUID LIMIT	- 21	- 26	-
PLASTICITY INDEX	- 05	- 13	-
AASHTO SOIL	- A-4 (1)	- A-6 (2)	-
UNIFIED SOIL	-	-	-
% MOISTURE CONTENT	- 19.9	- 22.2	- 18.6
ACHMSC (IN)	- ---	- 4.5W	- 7.0
ACHMSC (IN)	- ---	- 1.0X	- ---
ACHMBC (IN)	- ---	- 5.5	- ---
AGG. BASE CRS CL-7 (IN)	- ---	- 7.0	- 8.0
	-	-	-
	-	-	-
	-	-	-
	-	-	-
	-	-	-
	-	-	-
	-	-	-

REMARKS - W=MULTIPLE LAYERS

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ARKANSAS STATE HIGHWAY AND TRANSPORTATION DEPARTMENT - LITTLE ROCK, ARKANSAS
MATERIALS DIVISION

MICHAEL BENSON, MATERIALS ENGINEER

*** SOIL SURVEY / PAVEMENT SOUNDING TEST REPORT ***

DATE - 04/23/18 SEQUENCE NO. - 13
JOB NUMBER - 061509 MATERIAL CODE - SSRVPS
FEDERAL AID NO. - TO BE ASSIGNED SPEC. YEAR - 2014
PURPOSE - SOIL SURVEY SAMPLE SUPPLIER ID. - 1
SPEC. REMARKS - NO SPECIFICATION CHECK COUNTY/STATE - 43
SUPPLIER NAME - STATE DISTRICT NO. - 06
NAME OF PROJECT - HWY. 367 - HWY. 89 (CABOT) (S)
PROJECT ENGINEER - NOT APPLICABLE
PIT/QUARRY - ARKANSAS
LOCATION - LONOKE, COUNTY DATE SAMPLED - 03/06/18
SAMPLED BY - THORNTON/FRAZIER DATE RECEIVED - 03/15/18
SAMPLE FROM - TEST HOLE DATE TESTED - 04/10/18
MATERIAL DESC. - SOIL SURVEY - R VALUE- PAVEMENT SOUNDINGS

LAB NUMBER	-	20180513	-	20180514	-	20180515
SAMPLE ID	-	S118	-	S119	-	S120
TEST STATUS	-	INFORMATION ONLY	-	INFORMATION ONLY	-	INFORMATION ONLY
STATION	-	320+00	-	320+00	-	320+00
LOCATION	-	06 RT	-	18 RT	-	27 RT
DEPTH IN FEET	-	0-5	-	0-5	-	0-5
MAT'L COLOR	-	GRAY	-	GRAY	-	BROWN
MAT'L TYPE	-		-		-	
LATITUDE DEG-MIN-SEC	-	34 56 22.00	-	34 56 22.00	-	34 56 22.00
LONGITUDE DEG-MIN-SEC	-	92 01 32.00	-	92 01 31.90	-	92 01 32.00
% PASSING	2	IN. -	-		-	
	1 1/2	IN. -	-		-	
	3/4	IN. -	-		-	100
	3/8	IN. -	100	100	-	99
	NO. 4	-	99	99	-	99
	NO. 10	-	95	98	-	97
	NO. 40	-	91	97	-	94
	NO. 80	-	85	92	-	89
	NO. 200	-	74	71	-	72
LIQUID LIMIT	-	35	-	31	-	34
PLASTICITY INDEX	-	22	-	18	-	20
AASHTO SOIL	-	A-6 (14)	-	A-6 (10)	-	A-6 (12)
UNIFIED SOIL	-		-		-	
% MOISTURE CONTENT	-	21.6	-	21.3	-	22.4
ACHMSC	(IN)	-	3.5	---	-	---
ACHMBC	(IN)	-	5.0	3.5	-	---
AGG. BASE CRS CL-7	(IN)	-	7.0	7.0	-	---
	-	-	-	-	-	-
	-	-	-	-	-	-
	-	-	-	-	-	-
	-	-	-	-	-	-
	-	-	-	-	-	-
	-	-	-	-	-	-

REMARKS - W=MULTIPLE LAYERS

AASHTO TESTS : T24 T88 T89 T90 T265

ARKANSAS STATE HIGHWAY AND TRANSPORTATION DEPARTMENT - LITTLE ROCK, ARKANSAS
MATERIALS DIVISION

MICHAEL BENSON, MATERIALS ENGINEER

*** SOIL SURVEY / PAVEMENT SOUNDING TEST REPORT ***

DATE	- 04/23/18	SEQUENCE NO.	- 14
JOB NUMBER	- 061509	MATERIAL CODE	- SSRVPS
FEDERAL AID NO.	- TO BE ASSIGNED	SPEC. YEAR	- 2014
PURPOSE	- SOIL SURVEY SAMPLE	SUPPLIER ID.	- 1
SPEC. REMARKS	- NO SPECIFICATION CHECK	COUNTY/STATE	- 43
SUPPLIER NAME	- STATE	DISTRICT NO.	- 06
NAME OF PROJECT	- HWY. 367 - HWY. 89 (CABOT) (S)		
PROJECT ENGINEER	- NOT APPLICABLE		
PIT/QUARRY	- ARKANSAS		
LOCATION	- LONOKE, COUNTY	DATE SAMPLED	- 03/06/18
SAMPLED BY	- THORNTON/FRAZIER	DATE RECEIVED	- 03/15/18
SAMPLE FROM	- TEST HOLE	DATE TESTED	- 04/10/18
MATERIAL DESC.	- SOIL SURVEY - R VALUE- PAVEMENT SOUNDINGS		

LAB NUMBER	-	20180516	-	20180517	-	20180518
SAMPLE ID	-	S121	-	S122	-	S123
TEST STATUS	-	INFORMATION ONLY	-	INFORMATION ONLY	-	INFORMATION ONLY
STATION	-	328+00	-	328+00	-	328+00
LOCATION	-	06 LT	-	18 LT	-	27 LT
DEPTH IN FEET	-	0-5	-	0-5	-	0-5
MAT'L COLOR	-	BROWN	-	BROWN	-	BROWN
MAT'L TYPE	-		-		-	
LATITUDE DEG-MIN-SEC	-	34 56 22.20	-	34 56 22.80	-	34 56 22.90
LONGITUDE DEG-MIN-SEC	-	92 01 28.10	-	92 01 22.40	-	92 01 22.40
% PASSING						
	2	IN.	-		-	
	1 1/2	IN.	-		-	
	3/4	IN.	-	100	-	100
	3/8	IN.	-	93	-	99
	NO. 4		-	85	-	99
	NO. 10		-	80	-	97
	NO. 40		-	77	-	94
	NO. 80		-	72	-	86
	NO. 200		-	58	-	70
LIQUID LIMIT	-	28	-	36	-	32
PLASTICITY INDEX	-	15	-	23	-	19
AASHTO SOIL	-	A-6(7)	-	A-6(10)	-	A-6(11)
UNIFIED SOIL	-		-		-	
% MOISTURE CONTENT	-	21.5	-	19.1	-	20.5
ACHMSC	(IN)	3.0	-	3.25	-	---
ACHMBC	(IN)	4.0	-	---	-	---
AGG. BASE CRS CL-7	(IN)	6.0	-	8.0	-	---
			-		-	
			-		-	
			-		-	
			-		-	
			-		-	
			-		-	

REMARKS - W=MULTIPLE LAYERS

AASHTO TESTS : T24 T88 T89 T90 T265

ARKANSAS STATE HIGHWAY AND TRANSPORTATION DEPARTMENT - LITTLE ROCK, ARKANSAS
MATERIALS DIVISION

MICHAEL BENSON, MATERIALS ENGINEER

*** SOIL SURVEY / PAVEMENT SOUNDING TEST REPORT ***

DATE	- 04/11/18	SEQUENCE NO.	- 15
JOB NUMBER	- 061509	MATERIAL CODE	- SSRVPS
FEDERAL AID NO.	- TO BE ASSIGNED	SPEC. YEAR	- 2014
PURPOSE	- SOIL SURVEY SAMPLE	SUPPLIER ID.	- 1
SPEC. REMARKS	- NO SPECIFICATION CHECK	COUNTY/STATE	- 43
SUPPLIER NAME	- STATE	DISTRICT NO.	- 06
NAME OF PROJECT	- HWY. 367 - HWY. 89 (CABOT) (S)		
PROJECT ENGINEER	- NOT APPLICABLE		
PIT/QUARRY	- ARKANSAS		
LOCATION	- LONOKE, COUNTY	DATE SAMPLED	- 03/06/18
SAMPLED BY	- THORNTON/FRAZIER	DATE RECEIVED	- 03/15/18
SAMPLE FROM	- TEST HOLE	DATE TESTED	- 04/10/18
MATERIAL DESC.	- SOIL SURVEY - R VALUE- PAVEMENT SOUNDINGS		

LAB NUMBER	-	20180519	-	20180520	-	20180521
SAMPLE ID	-	S124	-	S125	-	S126
TEST STATUS	-	INFORMATION ONLY	-	INFORMATION ONLY	-	INFORMATION ONLY
STATION	-	338+00	-	338+00	-	338+00
LOCATION	-	12 RT	-	20 RT	-	27 RT
DEPTH IN FEET	-	0-5	-	0-5	-	0-5
MAT'L COLOR	-	BR/GR	-	BR/GR	-	BR/GR
MAT'L TYPE	-	-	-	-	-	-
LATITUDE DEG-MIN-SEC	-	34 56 21.90	-	34 56 20.90	-	34 56 20.90
LONGITUDE DEG-MIN-SEC	-	92 01 16.40	-	92 01 10.60	-	92 01 10.60
% PASSING	2	IN. -	-	-	-	-
	1 1/2	IN. -	-	-	-	100
	3/4	IN. -	-	100	-	93
	3/8	IN. -	100	99	-	86
	NO. 4	-	98	97	-	85
	NO. 10	-	95	95	-	83
	NO. 40	-	93	92	-	81
	NO. 80	-	82	83	-	71
	NO. 200	-	66	66	-	56
LIQUID LIMIT	-	31	-	36	-	25
PLASTICITY INDEX	-	16	-	23	-	11
AASHTO SOIL	-	A-6(8)	-	A-6(12)	-	A-6(3)
UNIFIED SOIL	-	-	-	-	-	-
% MOISTURE CONTENT	-	19.6	-	23.1	-	19.9
ACHMSC	(IN) -	3.0	-	5.5	-	---
ACHMBC	(IN) -	5.5	-	---	-	---
AGG. BASE CRS CL-7	(IN) -	7.0	-	8.0	-	---
	-	-	-	-	-	-
	-	-	-	-	-	-
	-	-	-	-	-	-
	-	-	-	-	-	-
	-	-	-	-	-	-
	-	-	-	-	-	-
	-	-	-	-	-	-

REMARKS - W=MULTIPLE LAYERS

ARKANSAS STATE HIGHWAY AND TRANSPORTATION DEPARTMENT - LITTLE ROCK, ARKANSAS
MATERIALS DIVISION

MICHAEL BENSON, MATERIALS ENGINEER

*** SOIL SURVEY / PAVEMENT SOUNDING TEST REPORT ***

DATE - 04/23/18 SEQUENCE NO. - 16
JOB NUMBER - 061509 MATERIAL CODE - SSRVPS
FEDERAL AID NO. - TO BE ASSIGNED SPEC. YEAR - 2014
PURPOSE - SOIL SURVEY SAMPLE SUPPLIER ID. - 1
SPEC. REMARKS - NO SPECIFICATION CHECK COUNTY/STATE - 43
SUPPLIER NAME - STATE DISTRICT NO. - 06
NAME OF PROJECT - HWY. 367 - HWY. 89 (CABOT) (S)
PROJECT ENGINEER - NOT APPLICABLE
PIT/QUARRY - ARKANSAS
LOCATION - LONOKE, COUNTY DATE SAMPLED - 03/06/18
SAMPLED BY - THORNTON/FRAZIER DATE RECEIVED - 03/15/18
SAMPLE FROM - TEST HOLE DATE TESTED - 04/10/18
MATERIAL DESC. - SOIL SURVEY - R VALUE- PAVEMENT SOUNDINGS

LAB NUMBER	-	20180522	-	20180523	-	20180524	
SAMPLE ID	-	S127	-	S128	-	S129	
TEST STATUS	-	INFORMATION ONLY	-	INFORMATION ONLY	-	INFORMATION ONLY	
STATION	-	346+00	-	346+00	-	346+00	
LOCATION	-	12 LT	-	18 LT	-	27 LT	
DEPTH IN FEET	-	0-5	-	0-5	-	0-5	
MAT'L COLOR	-	BROWN	-	BROWN	-	BR/GR	
MAT'L TYPE	-	-	-	-	-	-	
LATITUDE DEG-MIN-SEC	-	34 56 18.80	-	34 56 18.80	-	34 56 18.80	
LONGITUDE DEG-MIN-SEC	-	92 01 2.40	-	92 01 1.50	-	92 01 1.50	
% PASSING	2 IN.	-	-	-	-	-	
	1 1/2 IN.	-	-	-	-	-	
	3/4 IN.	-	-	-	-	-	
	3/8 IN.	-	-	-	-	-	
	NO. 4	-	100	-	100	-	100
	NO. 10	-	99	-	99	-	99
	NO. 40	-	99	-	99	-	99
	NO. 80	-	98	-	97	-	97
	NO. 200	-	94	-	93	-	94
		-	58	-	66	-	66
LIQUID LIMIT	-	46	-	28	-	39	
PLASTICITY INDEX	-	33	-	12	-	22	
AASHTO SOIL	-	A-7-6(15)	-	A-6(5)	-	A-6(12)	
UNIFIED SOIL	-	-	-	-	-	-	
% MOISTURE CONTENT	-	29.7	-	22.1	-	19.3	
ACHMSC	(IN)	-	4.0W	-	---	-	---
ACHMBC	(IN)	-	4.5	-	4.0	-	---
AGG. BASE CRS CL-7	(IN)	-	7.0	-	8.0	-	---
	-	-	-	-	-	-	-
	-	-	-	-	-	-	-
	-	-	-	-	-	-	-
	-	-	-	-	-	-	-
	-	-	-	-	-	-	-
	-	-	-	-	-	-	-
	-	-	-	-	-	-	-
	-	-	-	-	-	-	-
	-	-	-	-	-	-	-
	-	-	-	-	-	-	-
	-	-	-	-	-	-	-
	-	-	-	-	-	-	-
	-	-	-	-	-	-	-

REMARKS - W=MULTIPLE LAYERS

AASHTO TESTS : T24 T88 T89 T90 T265

ARKANSAS STATE HIGHWAY AND TRANSPORTATION DEPARTMENT - LITTLE ROCK, ARKANSAS
MATERIALS DIVISION

MICHAEL BENSON, MATERIALS ENGINEER

*** SOIL SURVEY / PAVEMENT SOUNDING TEST REPORT ***

DATE	- 04/23/18	SEQUENCE NO.	- 17
JOB NUMBER	- 061509	MATERIAL CODE	- SSRVPS
FEDERAL AID NO.	- TO BE ASSIGNED	SPEC. YEAR	- 2014
PURPOSE	- SOIL SURVEY SAMPLE	SUPPLIER ID.	- 1
SPEC. REMARKS	- NO SPECIFICATION CHECK	COUNTY/STATE	- 43
SUPPLIER NAME	- STATE	DISTRICT NO.	- 06
NAME OF PROJECT	- HWY. 367 - HWY. 89 (CABOT) (S)		
PROJECT ENGINEER	- NOT APPLICABLE		
PIT/QUARRY	- ARKANSAS		
LOCATION	- LONOKE, COUNTY	DATE SAMPLED	- 03/06/18
SAMPLED BY	- THORNTON/FRAZIER	DATE RECEIVED	- 03/15/18
SAMPLE FROM	- TEST HOLE	DATE TESTED	- 04/11/18
MATERIAL DESC.	- SOIL SURVEY - R VALUE- PAVEMENT SOUNDINGS		

LAB NUMBER	- 20180525	- 20180526	- 20180527
SAMPLE ID	- S130	- S131	- S132
TEST STATUS	- INFORMATION ONLY	- INFORMATION ONLY	- INFORMATION ONLY
STATION	- 354+00	- 354+00	- 362+00
LOCATION	- 12 RT	- 18 RT	- 12 LT
DEPTH IN FEET	- 0-5	- 0-5	- 0-5
MAT'L COLOR	- BR/GR	- BROWN	- BROWN
MAT'L TYPE	-	-	-
LATITUDE DEG-MIN-SEC	- 34 56 17.70	- 34 56 17.70	- 34 56 17.70
LONGITUDE DEG-MIN-SEC	- 92 00 52.60	- 92 00 52.50	- 92 00 45.10
% PASSING	2 IN. -	-	-
	1 1/2 IN. -	-	-
	3/4 IN. - 100	-	-
	3/8 IN. - 99	- 100	-
	NO. 4 - 98	- 99	- 100
	NO. 10 - 95	- 99	- 99
	NO. 40 - 75	- 97	- 97
	NO. 80 - 72	- 76	- 84
	NO. 200 - 58	- 61	- 68
LIQUID LIMIT	- 22	- 24	- 23
PLASTICITY INDEX	- 07	- 11	- 07
AASHTO SOIL	- A-4 (1)	- A-6 (4)	- A-4 (2)
UNIFIED SOIL	-	-	-
% MOISTURE CONTENT	- 19.7	- 17.2	- 21.5
ACHMSC (IN)	- 3.0W	- 4.5	- ---
ACHMBC (IN)	- 5.0	- ---	- ---
AGG. BASE CRS CL-7 (IN)	- 8.0	- 7.0	- ---
	-	-	-
	-	-	-
	-	-	-
	-	-	-
	-	-	-
	-	-	-
	-	-	-

REMARKS - W=MULTIPLE LAYERS

AASHTO TESTS : T24 T88 T89 T90 T265

ARKANSAS STATE HIGHWAY AND TRANSPORTATION DEPARTMENT - LITTLE ROCK, ARKANSAS
 MATERIALS DIVISION

MICHAEL BENSON, MATERIALS ENGINEER

*** SOIL SURVEY / PAVEMENT SOUNDING TEST REPORT ***

DATE	- 04/11/18	SEQUENCE NO.	- 18
JOB NUMBER	- 061509	MATERIAL CODE	- SSRVPS
FEDERAL AID NO.	- TO BE ASSIGNED	SPEC. YEAR	- 2014
PURPOSE	- SOIL SURVEY SAMPLE	SUPPLIER ID.	- 1
SPEC. REMARKS	- NO SPECIFICATION CHECK	COUNTY/STATE	- 43
SUPPLIER NAME	- STATE	DISTRICT NO.	- 06
NAME OF PROJECT	- HWY. 367 - HWY. 89 (CABOT) (S)		
PROJECT ENGINEER	- NOT APPLICABLE		
PIT/QUARRY	- ARKANSAS		
LOCATION	- LONOKE, COUNTY	DATE SAMPLED	- 03/06/18
SAMPLED BY	- THORNTON/FRAZIER	DATE RECEIVED	- 03/15/18
SAMPLE FROM	- TEST HOLE	DATE TESTED	- 04/10/18
MATERIAL DESC.	- SOIL SURVEY - R VALUE- PAVEMENT SOUNDINGS		

LAB NUMBER	-	20180528	-	-
SAMPLE ID	-	S133	-	-
TEST STATUS	-	INFORMATION ONLY	-	-
STATION	-	362+00	-	-
LOCATION	-	18 LT	-	-
DEPTH IN FEET	-	0-5	-	-
MAT'L COLOR	-	BROWN	-	-
MAT'L TYPE	-		-	-
LATITUDE DEG-MIN-SEC	-	34 56 17.90	-	-
LONGITUDE DEG-MIN-SEC	-	92 00 43.00	-	-
% PASSING	2	IN.	-	-
	1 1/2	IN.	-	-
	3/4	IN.	-	-
	3/8	IN.	-	100
	NO. 4		-	98
	NO. 10		-	97
	NO. 40		-	94
	NO. 80		-	88
	NO. 200		-	72
LIQUID LIMIT	-	31	-	-
PLASTICITY INDEX	-	16	-	-
AASHTO SOIL	-	A-6(9)	-	-
UNIFIED SOIL	-		-	-
% MOISTURE CONTENT	-	20.8	-	-
ACHMSC	(IN)	3.5W	-	-
AGG. BASE CRS CL-7	(IN)	5.0	-	-
			-	-
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REMARKS - W=MULTIPLE LAYERS

AASHTO TESTS : T24 T88 T89 T90 T265

ARKANSAS STATE HIGHWAY AND TRANSPORTATION DEPARTMENT - LITTLE ROCK, ARKANSAS
MATERIALS DIVISION

MICHAEL BENSON, MATERIALS ENGINEER

*** SOIL SURVEY / PAVEMENT SOUNDING TEST REPORT ***

DATE	- 04/11/18	SEQUENCE NO.	- 1
JOB NUMBER	- 061509	MATERIAL CODE	- RV
FEDERAL AID NO.	- TO BE ASSIGNED	SPEC. YEAR	- 2014
PURPOSE	- SOIL SURVEY SAMPLE	SUPPLIER ID.	- 1
SPEC. REMARKS	- NO SPECIFICATION CHECK	COUNTY/STATE	- 43
SUPPLIER NAME	- STATE	DISTRICT NO.	- 06
NAME OF PROJECT	- HWY. 367 - HWY. 89 (CABOT) (S)		
PROJECT ENGINEER	- NOT APPLICABLE		
PIT/QUARRY	- ARKANSAS		
LOCATION	- LONOKE, COUNTY	DATE SAMPLED	- 03/06/18
SAMPLED BY	- THORNTON/FRAZIER	DATE RECEIVED	- 03/15/18
SAMPLE FROM	- TEST HOLE	DATE TESTED	- 04/10/18
MATERIAL DESC.	- SOIL SURVEY - RESISTANCE R-VALUE ACTUAL RESULTS		

LAB NUMBER	- 20180529	- 20180530	- 20180531
SAMPLE ID	- RV134	- RV135	- RV136
TEST STATUS	- INFORMATION ONLY	- INFORMATION ONLY	- INFORMATION ONLY
STATION	- 230+00	- 255+00	- 288+00
LOCATION	- 27 RT	- 27 LT	- 27 RT
DEPTH IN FEET	- 0-5	- 0-5	- 0-5
MAT'L COLOR	- BR/GR	- BR/GR	- BR/GR
MAT'L TYPE	-	-	-
LATITUDE DEG-MIN-SEC	- 34 56 35.40	- 34 56 32.00	- 34 56 20.40
LONGITUDE DEG-MIN-SEC	- 92 03 16.90	- 92 02 47.60	- 92 02 10.70
% PASSING			
2 IN.	-	-	-
1 1/2 IN.	-	-	-
3/4 IN.	- 100	- 100	- 100
3/8 IN.	- 98	- 93	- 89
NO. 4	- 95	- 90	- 79
NO. 10	- 94	- 88	- 72
NO. 40	- 92	- 86	- 65
NO. 80	- 90	- 84	- 59
NO. 200	- 87	- 78	- 38
LIQUID LIMIT	- 24	- 24	- 37
PLASTICITY INDEX	- 07	- 09	- 24
AASHTO SOIL	- A-4 (4)	- A-4 (5)	- A-6 (4)
UNIFIED SOIL	-	-	-
% MOISTURE CONTENT	-	-	-
	-	-	-
	-	-	-
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REMARKS - W=MULTIPLE LAYERS

AASHTO TESTS : T24 T88 T89 T90 T265
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ARKANSAS STATE HIGHWAY AND TRANSPORTATION DEPARTMENT - LITTLE ROCK, ARKANSAS
MATERIALS DIVISION

MICHAEL BENSON, MATERIALS ENGINEER

*** SOIL SURVEY / PAVEMENT SOUNDING TEST REPORT ***

DATE - 04/11/18 SEQUENCE NO. - 2
JOB NUMBER - 061509 MATERIAL CODE - RV
FEDERAL AID NO. - TO BE ASSIGNED SPEC. YEAR - 2014
PURPOSE - SOIL SURVEY SAMPLE SUPPLIER ID. - 1
SPEC. REMARKS - NO SPECIFICATION CHECK COUNTY/STATE - 43
SUPPLIER NAME - STATE DISTRICT NO. - 06
NAME OF PROJECT - HWY. 367 - HWY. 89 (CABOT) (S)
PROJECT ENGINEER - NOT APPLICABLE
PIT/QUARRY - ARKANSAS
LOCATION - LONOKE, COUNTY DATE SAMPLED - 03/06/18
SAMPLED BY - THORNTON/FRAZIER DATE RECEIVED - 03/15/18
SAMPLE FROM - TEST HOLE DATE TESTED - 04/10/18
MATERIAL DESC. - SOIL SURVEY - RESISTANCE R-VALUE ACTUAL RESULTS

LAB NUMBER	-	20180532	-	-
SAMPLE ID	-	RV137	-	-
TEST STATUS	-	INFORMATION ONLY	-	-
STATION	-	346+00	-	-
LOCATION	-	27 LT	-	-
DEPTH IN FEET	-	0-5	-	-
MAT'L COLOR	-	BR/GR	-	-
MAT'L TYPE	-		-	-
LATITUDE DEG-MIN-SEC	-	34 56 18.80	-	-
LONGITUDE DEG-MIN-SEC	-	92 01 1.50	-	-
% PASSING	2	IN.	-	-
	1 1/2	IN.	-	100
	3/4	IN.	-	91
	3/8	IN.	-	84
	NO. 4		-	82
	NO. 10		-	81
	NO. 40		-	79
	NO. 80		-	76
	NO. 200		-	54
LIQUID LIMIT	-	41	-	-
PLASTICITY INDEX	-	28	-	-
AASHTO SOIL	-	A-7-6(11)	-	-
UNIFIED SOIL	-		-	-
% MOISTURE CONTENT	-		-	-
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REMARKS - W=MULTIPLE LAYERS
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AASHTO TESTS : T24 T88 T89 T90 T265
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