ARKANSAS DEPARTMENT OF TRANSPORTATION



SUBSURFACE INVESTIGATION

STATE JOB NO.		080504						
FEDERAL AID PROJEC	CT NO.	NHPP-0049(16)						
	DEER CR	EEK STR. & APPRS. (S)					
STATE HIGHWAY	27	SECTION	7					
IN	ı	MONTGOMERY		COUNTY				

The information contained herein was obtained by the Department for design and estimating purposes only. It is being furnished with the express understanding that said information does not constitute a part of the Proposal or Contract and represents only the best knowledge of the Department as to the location, character and depth of the materials encountered. The information is only included and made available so that bidders may have access to subsurface information obtained by the Department and is not intended to be a substitute for personal investigation, interpretation and judgment of the bidder. The bidder should be cognizant of the possibility that conditions affecting the cost and/or quantities of work to be performed may differ from those indicated herein.



ARKANSAS DEPARTMENT OF TRANSPORTATION

ARDOT.gov | IDriveArkansas.com | Scott E. Bennett, P.E., Director

MATERIALS DIVISION

11301 West Baseline Road | P.O. Box 2261 | Little Rock, AR 72203-2261 | Phone: 501.569.2185 | Fax: 501.569.2368

January 17, 2019

TO:

Mr. Rick Ellis, Bridge Engineer

SUBJECT:

Job No. 080504

Deer Creek Str. & Apprs. (S)

Route 27 Section 7 Montgomery County

Transmitted herewith are a brief summary of the geology and site conditions, rock core unconfined compression test summary, RMR, and the logs of borings conducted for the structure and approaches of the above referenced project. The samples obtained by the Standard Penetration Tests were brought to the laboratory and visually classified by experienced lab personnel to confirm the field identifications.

This project consists of replacing the bridge crossing Deer Creek, on Highway 27, at Washita. The new bridge alignment will be to the west, upstream, of the existing. Two of the five requested borings were inaccessible due to steep slopes and high water levels in the channel. The borings that were not obtained are located at: 110+18 C.L. Construction and 110+88 C.L. Construction.

The subsurface investigation revealed that bedrock is less than 5 feet deep at the obtained borings and exposed in channel. Utilizing this site information and correlating the elevation of bedrock between the obtained bridge boring logs, competent bedrock should be encountered less than 15 feet below ground level. Based on this information and correspondence with Bridge Division, it is anticipated that all intermediate bents will be founded on drilled shafts and end bents will be founded on piling. Preboring may be necessary in order to achieve minimum piling penetration requirements. Drilled shafts socketed into competent slightly weathered to unweathered Shale should be designed based on the values provided in Table 1.

TABLE 1 – Bearing Capacity Recommendations for Drilled Shafts

Nominal Shaft Side	Factored Shaft Side	Nominal Shaft Tip	Factored Shaft Tip
Resistance (ksf)	Resistance (ksf)	Resistance (ksf)	Resistance (ksf)
32.7	18	137	68.5

The Bigfork Chert formation is exposed in a road cut just south of the existing bridge. Although all obtained borings consisted primarily of shale, it is anticipated that Bigfork Chert will be encountered in the channel and/or to the south, based on geologic maps and visual geologic investigations. Geologic maps show the presence of a contact between the Polk Creek Shale and Bigfork Chert within the limits of this project. Drilling shafts into Bigfork Chert will require considerably more effort than Polk Creek Shale.

If you have any questions concerning these recommendations, please contact the Geotechnical Section.

Michael C. Benson Materials Engineer

MCB:rpt:mlg

CC:

State Construction Engineer - Master File Copy

District 8 Engineer G.C. File

GEOLOGY AND SITE CONDITIONS Job No. 080504

Deer Creek Str. & Apprs. (S) Montgomery County Route 27 Section 7

Site Conditions

The existing Deer Creek Bridge, on Highway 27, is a five span bridge. It is constructed of concrete decking, end walls, and spread footings with four steel I-beams supporting the deck. The guardrail is composed of steel, supported by concrete posts on the bridge and steel posts leading up to the bridge. Overhead power lines parallel the roadway on the east. Highway 88 intersects Highway 27 on the west, approximately 400 feet north of the existing bridge. On the opposite side of the intersection, a gravel road, with a paved intersection, leads to a lake recreation area.

Deer Creek flows to the east at the bridge and turns sharply south a short distance east of the bridge. Water level in the creek is strongly influenced by the water elevation of Lake Ouachita. The location of the bridge is within the detention basin for Lake Ouachita. Bedrock is exposed extensively along the channel. The rock exposed consists of moderately to steeply dipping shale with chert lenses along the north side of the channel and chert along the south side of the channel. Rock is exposed in the road cut on both sides of the roadway, south of the bridge, and consists of near vertical beds of chert. The area around the bridge is moderately to heavily wooded except to the southeast, which consists of a gravel bar with grasses. There is a residence on the hill immediately above the proposed bridge site on the southwest.

Site Geology

The rocks at the proposed job site are moderately to steeply dipping to the north-northwest. The units exposed consist of the Bigfork Chert on the south and the Polk Creek Shale on the north. Deer Creek flows along the contact between these two formations. The Bigfork Chert consists of thin-bedded, dark-gray, cryptocrystalline chert interbedded with varying amounts of black siliceous shale, calcareous siltstone, and dense, bluish-gray limestone. The cherts normally occur in thin to medium beds and are usually highly fractured. The interbedded siliceous shales occur in thin to thick sequences and are often pyritic. Limestones occur mostly as interbeds in the chert and typically weather to soft brown layers. The limestones are more common in the northwestern exposures. The Bigfork Chert in Arkansas ranges in thickness from about 450 feet in the northern Ouachitas to about 750 feet in the southern Ouachitas.

The Bigfork Chert is overlain by the Polk Creek Shale. The Polk Creek rocks are black, sooty, fissile shale with minor black chert and traces of gray sandstone and limestone. The Polk Creek Shale rests conformably on the Bigfork Chert. Its thickness ranges from about 50 to 225 feet.

Subsurface Conditions

Based on the results of the borings made from Station 111+58 to 112+98, the subsurface stratigraphy may be generalized as follows:

0 to 4.5 Feet:

Varies from **clay** with **gravel** (**shale fragments**) to highly weathered to slightly weathered, soft to hard, dark gray **shale**.

4.5 to 33.9 Feet:

Consists of weathered to unweathered, medium hard to hard with very hard layers, occasionally to frequently fractured and slickensided, steeply dipping dark gray **shale** with frequent to occasional seams of quartz and calcite.

33.9 to 38.9 Feet:

Varies from unweathered, hard to very hard, steeply dipping, frequently slickensided, dark gray **shale** with frequent to occasional layers and seams of calcite and guartz to unweathered, hard, steeply dipping gray **chert**.

^{*}No borings were made down-station of 111+55 due to high water levels. Based on current Geologic maps and visual geologic analysis it is likely that the Bigfork Chert, as described in the Site Geology, will be encountered on the south bank of Deer Creek.

Rock Core Unconfined Compression Test Summary

Project Number:

080504

Project Name:

Deer Creek Str. & Apprs. (S)

Date Tested:

7/31/2018

Station	Location	Sample No.	Depth (ft)	Diameter (in)	Height (in)	Total Load (lbs)	Correction Factor	Stress (psi)	Remarks
111+58	C.L.	1	3.1	1.75	3.60	13,550	1.00	5,633	,
111+58	C.L.	2	11.3	1.75	2.85	4,380	1.00	1,821	Not a 2:1 ratio
111+58	C.L.	3	16.0	1.75	4.05	14,110	1.00	5,866	
111+58	C.L.	4	20.3	1.75	4.00	10,230	1.00	4,253	
111+58	C.L.	5	26.3	1.75	3.75	6,390	1.00	2,656	
111+58	C.L.	6	29.3	1.75	3.35	17,050	1.00	7,089	
112+28	C.L.	7	14.3	1.75	3.85	5,560	1.00	2,311	
112+28	C.L.	8	17.0	1.75	3.65	8,090	1.00	3,363	
112+28	C.L.	9	22.6	-	_	-	_	-	Dropped & Broke
112+28	C.L.	10	24.9	1.75	3.85	11,940	1.00	4,964	
112+28	C.L.	11	28.3	1.75	3.55	7,270	1.00	3,022	
112+28	C.L.	12	32.6	1.75	3.60	4,000	1.00	1,663	

^{*} Please note any broken samples, fractures or other characteristics of sample in Remarks.

ROCK MASS RATING SUMMARY JOB # 080504

SAMPLE #1

111+58 C.L.

Uniaxial Compressive Strength RQD Spacing of Joints

Station/Location

Depth (ft)

Condition of Joints Groundwater Conditions Sum

> Class Number Description

III FAIR ROCK

SAMPLE #3

Station/Location
Depth (ft)

Relative Rating
Uniaxial Compressive Strength
RQD
Spacing of Joints
Condition of Joints
Groundwater Conditions
Sum

111+58 C.L.
20.5

Relative Rating
4
17
20
20
20
7
68

Class Number Description

SAMPLE #5

Station/Location 111+58 C.L. Depth (ft) 29.5

Uniaxial Compressive Strength RQD Spacing of Joints Condition of Joints Groundwater Conditions Sum

> Class Number Description

	Relative Rating
	4
	17
	20
10	20
	7
	68

GOOD ROCK

II GOOD ROCK

SAMPLE #7

Station/Location 112+28 C.L. Depth (ft) Relative Rating Uniaxial Compressive Strength RQD Spacing of Joints 25 Condition of Joints 20 **Groundwater Conditions** Sum 67 Class Number GOOD ROCK Description

SAMPLE #1

Station/Location
Depth (ft)

Relative Rating

Uniaxial Compressive Strength
RQD
Spacing of Joints
Condition of Joints
Groundwater Conditions
Sum

111+58 C.L.
16

Relative Rating

4

4

13

50

10

Condition of Joints
20

Groundwater Conditions
7

54

SAMPLE #4

Ш

FAIR ROCK

Class Number

Description

Station/Location Depth (ft)	111+58 C.L. 26.5
	Relative Rating
Uniaxial Compressive Strength	2
RQD	17
Spacing of Joints	20
Condition of Joints	25
Groundwater Conditions	7
Sum	71
Class Number	ll l
Description	GOOD ROCK

SAMPLE #6

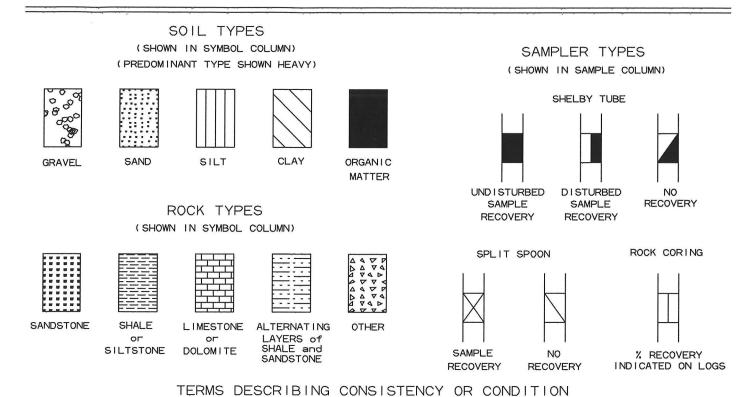
Station/Location Depth (ft)	112+28 C.L. 14.5
	Relative Rating
Uniaxial Compressive Strength	2
RQD	13
Spacing of Joints	25
Condition of Joints	20
Groundwater Conditions	7
Sum	67
	100
Class Number	II
Description	GOOD ROCK

SAMPLE #8

SAW	PLE #8
Station/Location Depth (ft)	112+28 C.L. 25
	Relative Rating
Uniaxial Compressive Strength	4
RQD	13
Spacing of Joints	25
Condition of Joints	20
Groundwater Conditions	7
Sum	69
1	
Class Number	II
Description	GOOD ROCK

SAN	IPLE #9	SAMPLE #10
Station/Location Depth (ft)	112+28 C.L. 28.5	Station/Location 112+28 C.L. Depth (ft) 32.5
Uniaxial Compressive Strength RQD Spacing of Joints Condition of Joints Groundwater Conditions Sum	Relative Rating 2 13 20 20 7 62	Uniaxial Compressive Strength RQD 13 Spacing of Joints 20 Condition of Joints 20 Groundwater Conditions 7 Sum 62
Class Number Description	II GOOD ROCK	Class Number II Description GOOD ROCK
SAM	PLE #11	SAMPLE #12
Station/Location Depth (ft)		Station/Location Depth (ft)
Uniaxial Compressive Strength RQD Spacing of Joints Condition of Joints Groundwater Conditions Sum		Uniaxial Compressive Strength RQD Spacing of Joints Condition of Joints Groundwater Conditions Sum
Class Number Description		Class Number Description
SAM	PLE #13	SAMPLE #14
Station/Location Depth (ft)		Station/Location Depth (ft)
Uniaxial Compressive Strength RQD Spacing of Joints Condition of Joints Groundwater Conditions Sum		Uniaxial Compressive Strength RQD Spacing of Joints Condition of Joints Groundwater Conditions Sum
Class Number Description		Class Number Description
SAM	PLE #15	SAMPLE #16
Station/Location Depth (ft)		Station/Location Depth (ft)
Uniaxial Compressive Strength		Uniaxial Compressive Strength RQD
Spacing of Joints Condition of Joints Groundwater Conditions Sum		Spacing of Joints Condition of Joints Groundwater Conditions Sum

LEGEND



GRANL	LAR SOIL		CLAY	CL	AY-SHALE	SHALE			
"N" Value	Density	"N" Value	Consistency	"N" Value	Consistency	"N" Value	Consistency		
0-4	Very Loose	0-1	Very Soft	0-1	Very Soft				
5-10	Loose	2-4	Soft	2-4	Soft	31-60	Soft		
11-30	Medium Dense	5-8	Medium Stiff	5-8	Medium Stiff	Over 60			
31-50	Dense	9-15	Stiff	9-15	Stiff	More than	2'		
0ver 50	Very Dense	16-30	Very Stiff	16-30	Very Stiff	Penetrati	on		
		31-60	Hard	31-60	Hard	in 60 Blov	vs: Medium Hard		
18		0ver 60	Very Hard	0ver 60	Very Hard	Less than	2'		
			F-127			Penetrati	on		
						in 60 Blow	vs: Hard		

- 1. Ground water elevations indicated on boring logs represent ground water elevations at date or time shown on boring log. Absence of water surface implies that no ground water data is available but does not necessarily mean that ground water will not be encountered at locations or within the vertical reaches of these borings.
- 2. Borings represent subsurface conditions at their respective locations for their respective depths. Variations in conditions between or adjacent to boring locations may be encountered.
- 3. Terms used for describing soils according to their texture or grain size distribution are in accordance with the Unified Soil Classification System.

Standard Penetration Test – Driving a 2.0" O.D., 1-3/8" I.D. sampler a distance of 1.0 foot into undisturbed soil with a 140 pound hammer free falling a distance of 30 inches. It is customary to drive the spoon 6.0 inches to seat into undisturbed soil, then perform the test. The number of hammer blows for seating the spoon and performing the test are recorded for each 6 inches of penetration on the drill log. The field "N" Value (N_f) can be obtained by

adding the bottom two numbers for example: $\frac{6}{8-9} \Rightarrow 8+9=17 blows/ft$. The "N" Value corrected to 60% efficiency (N₆₀) can be obtained by multiplying N_f by the hammer correction factor published on the boring log.

ARKANSAS DEPARTMENT OF TRANSPORTATION MATERIALS DIVISION - GEOTECHNICAL SEC.). 1	. 0					
		LS			PAGE 1 OF 2								
JOB N	NO. NAME:		080504 Montgomery County		DATE: January 31, 2018 TYPE OF DRILLING:								
JOB N	NAME:		Deer Creek Str. & Apprs. (S) Route 27 Section 7						D	:		7	
STAT	ION.		111+58				tem A					ore	
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			Steve Faulkner			D 00	DDECT	NON F	OT	OD		NT/A	
-	2000 1 St. O-		N DEPTH: 34.3		HAMME	R CO	RRECT	ION F	ACT	OR:		N/A	
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			Recovery)										
			SHALE - Slightly Weathered, Hard, Steeply									72	16
			Dipping, Dark Gray										
5		Т											
			SHALE - Unweathered, Hard, Steeply Dipping,										
-			Frequent Fractures, Frequent Quartz Seams,									84	14
			Dark Gray										
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15													
			SHALE - Unweathered, Hard with Very Hard										
			Layers, Steeply Dipping, Frequent Fractures,										
\vdash \vdash			Frequent Slickensides, Frequent Calcite and									94	56
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			Layers, Steeply Dipping, Frequent Slickensides,									98	72
			Frequent Calcite and Quartz Seams and Layers,									00	12
			Dark Gray										
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MATERIALS DIVISION - GEOTECHNICAL SEC. JOB NO. 080504 Montgomery County						2		2	21	2010	_		
			080504 Montgomery County		DATE: January 31, 2018								
JOB N	AME	AME: Deer Creek Str. & Apprs. (S) Route 27 Section 7					TYPE OF DRILLING: Hollow Stem Auger - Diamond Core						
STATION: 111+58					EQUIPM			_		nona (1779	Jore	;	
LOCATION: C.L. Construction						ENI:		A	sker	1//9			
			Steve Faulkner		НАММЕ	R CO	RRECT	ION FA	TOR		N/A		
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MATERIALS DIVISION - GEOTECHINGAL SEC. PAGE 1 OF 2	ARKANSAS DEPARTMENT OF TRANSPORTATION MATERIALS DIVISION - GEOTECHNICAL SEC.). 2						
JOB NAME: Deer Creek Str. & Apprs. (S) Route 27 Section 7 112+28 C.L. Construction COMPLETION: DEPTH: 38.9 Description of MATERIAL F.T. L SURFACE ELEVATION: 572.9 Clay with Gravel (Rock Fragments) SHALE - Highly Weathered, Hard, Steeply Dipping, Occasional Fractures, Frequent Quartz Seams, Dark Gray SHALE - Unweathered, Hard, Steeply Dipping, Frequent Fractures, Frequent Quartz Seams, Dark Gray SHALE - Unweathered, Hard, Steeply Dipping, Frequent Fractures, Frequent Quartz Seams, Dark Gray SHALE - Unweathered, Hard, Steeply Dipping, Occasional Fractures, Frequent Quartz Seams, Dark Gray SHALE - Unweathered, Hard, Steeply Dipping, Frequent Fractures, Frequent Quartz Seams, Dark Gray SHALE - Unweathered, Hard, Steeply Dipping, Frequent Fractures, Frequent Quartz Seams, Dark Gray SHALE - Unweathered, Hard, Steeply Dipping, Frequent Fractures, Frequent Quartz Seams, Dark Gray SHALE - Unweathered, Hard, Steeply Dipping, Occasional Fractures, Frequent Quartz Seams, Dark Gray SHALE - Unweathered, Hard, Steeply Dipping, Frequent Fractures, Frequent Quartz Seams, Dark Gray SHALE - Unweathered, Hard, Steeply Dipping, Occasional Fractures, Frequent Quartz Seams, Dark Gray SHALE - Unweathered, Hard, Steeply Dipping, Occasional Fractures, Frequent Quartz Seams, Dark Gray	-		LS			PAGE	1		400000000000000000000000000000000000000	··· 2	0.20	10		
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			DEPARTMENT OF TRANSPORTATION		BORING								
JOB N		LS.	DIVISION - GEOTECHNICAL SEC. 080504 Montgomery County		PAGE	2		2	- 24	0.20	1.0		
	NO. NAME:		080504 Montgomery County Deer Creek Str. & Apprs. (S)		DATE: January 30, 2018 TYPE OF DRILLING:								
JOBA	MANIE.		Route 27 Section 7				tem A		D	iomo	nd (7000	
STAT	ION:		112+28					-		er 17		ore	
LOCA			C.L. Construction		EQUIPM	ENI:		F	ACK	er 17	19		
5			Steve Faulkner		НАММЕ	R CO	PPECT	ION E	СТ	OP:	,	N/A	
-		-11/2	N DEPTH: 38.9		TAMMINI	K CO	KKECI	IONTA	ic i	OK.		N/A	
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E	S	A											
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FT.	L	E S	SURFACE ELEVATION: 572.9		PLASTIC LIMIT	% MOIST.	LIQUID	DRY WEIGHT	LBS PER CU.FT	NO. OF BLOWS	PER 6-IN.		
		$\ddot{-}$	SORFACE ELEVATION. 572.9		P	%	77	Ω	L	Z	P		
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JOB 1		10	080504 Montgomery County		DATE:			anua	rv 2	4 20	118		
1	NAME:		Deer Creek Str. & Apprs. (S)		TYPE O	E DDI			.1 y 2	4, 20	110		
l vob i	17 11111	•10	Route 27 Section 7				tem A		- D	iamo	and (ore	
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			Stanley Bates		HAMMI	D CO	DDECT	LIONI	EACT	OD.	,	N/A	
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Н	В	L		GROUI	\square	IST	Q	VEI	ER	F B	Ż	C R	Q D
	L	E			PLASTIC LIMIT	% MOIST.	LIQUID	DRY WEIGHT	LBS PER CU.FT	NO. OF BLOWS	PER 6-IN.		
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			Steeply Dippling, Dark Gray										
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15													
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25		Ш	Shale - Unweathered, Hard, Steeply Dipping,										
			Occasional Fractures, Frequent Calcite and									96	66
			Quartz Seams and Layers, Dark Gray										
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35													
	ARKS	*	Water was encountered at 3.9' below ground level. I	l at/l or	ia: 34	6516	344	93 1	5331	241			
		100	mas should at old bolow ground level. I	_~~	g. 07.	5510		50.0		- 16 J			

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ARKANSAS DEPARTMENT OF TRANSPORTATION

ARDOT.gov | IDriveArkansas.com | Scott E. Bennett, P.E., Director

MATERIALS DIVISION

11301 West Baseline Road | P.O. Box 2261 | Little Rock, AR 72203-2261 | Phone: 501.569.2185 | Fax: 501.569.2368

August 24, 2017

TO:

Mr. Trinity Smith, Engineer of Roadway Design

SUBJECT:

Job No. 080504

Deer Creek Str. & Apprs. (S)

Route 27 Section 7 Montgomery County

Transmitted herewith is the requested Soil Survey, strength data and Resilient Modulus test results for the above referenced job. The project consists of replacing the bridge crossing Deer Creek on new location. Samples were obtained in the existing travel lanes and ditch line. There were no paved shoulders within the project limits.

Based on laboratory results of samples obtained, the subgrade soils consist primarily of clayey sands with varying amounts of shale. Cross-sections are not currently available, but it is assumed the construction grade line will closely match that of the existing roadway. The subgrade soils are expected to provide a stable working platform with normal drying and compactive effort, if the weather is favorable during construction. Rock was encountered at station 102+00, 6 feet right of centerline at a depth of 3.0 feet.

Additional earthwork recommendations will be made upon request when plans are further developed and cross sections are available.

Listed below is the additional information requested for use in developing the plans:

- 1. The Qualified Products List (QPL) indicates that Aggregate Base Course (Class CL-7) is available from commercial producers located in the vicinity Bismarck.
- 2. Asphalt Concrete Hot Mix

	PG 64-22	
Type	Asphalt Cement %	Mineral Aggregate %
Surface Course	5.5	94.5
Binder Course	4.4	95.6
Base Course	4.0	96.0

	PG 70-22	14
Type	Asphalt Cement %	Mineral Aggregate %
Surface Course	5.5	94.5
Binder Course	4.5	95.5
Base Course	4.0	96.0

Job 080504 August 25, 2017

	PG 76-22	
Туре	Asphalt Cement %	Mineral Aggregate %
Surface Course	5.4	94.6
Binder Course	4.3	95.7
Base Course	3.8	96.2

MCB:pt:bjj Attachment

CC:

State Constr. Eng. - Master File Copy

District 8 Engineer

System Information and Research Div.

G. C. File

Michael C. Benson Materials Engineer

MICHAEL BENSON, MATERIALS ENGINEER

*** SOIL SURVEY STRENGTH TEST REPORT ***

DATE - 08/18/2017 SEQUENCE NO. - 1

JOB NUMBER - 080504 MATERIAL CODE - SSRV

SPEC. YEAR - 2014

SUPPLIER ID. = 1

COUNTY/STATE - 49 DISTRICT NO. - 06

JOB NAME - DEER CREEK STR. & APPRS.(S)

BEGIN JOB - END JOB 14

RESILIENT MODULUS

STA. 102+00 11281

REMARKS =

AASHTO TESTS : T190

ARKANSAS STATE HIGHWAY AND TRANSPORTATION DEPARTMENT MATERIALS DIVISION

AASHTO T 307-99 - RESILIENT MODULUS OF SUBGRADE SOILS RECOMPACTED SAMPLES

Job No.	080504	Material Code	SSRVPS
Date Sampled:	7/18/17	Station No.:	102+00
Date Tested:	August 11, 2017	Location:	30'RT
Name of Project:	DEER CREEK STR. & APPRS. (S)		
County:	Code: 49 Name: MONTGOMERY		
Sampled By:	BATES/JORDAN	Depth:	0-5
Lab No.:	20172450	AASHTO Class:	A-6(32)
Sample ID:	RV486	Material Type (1 or 2):	2
LATITUDE:		LONGITUDE:	
1. Testing Inform	nation:		
_	Preconditioning - Permanent Strain > 5% (Y=)	es or N= No)	N
	Testing - Permanent Strain > 5% (Y=Yes or N=		N
	Number of Load Sequences Completed (0-15)	,	15
2 Specimen Info	-motion:		
2. Specimen Info	Specimen Diameter (in):		
	Тор		3.95
	Middle		3.95
	Bottom		3.95
	Average		3.95
	Membrane Thickness (in):		0.01
	Height of Specimen, Cap and Base (in):		8.03
	Height of Cap and Base (in):		0.00
	Initial Length, Lo (in):		8.03
	Initial Area, Ao (sq. in):		12.18
	Initial Volume, AoLo (cu. in):		97.80
3. Soil Specimen	Weight:		
	Weight of Wet Soil Used (g):		3203.30
4. Soil Properties	s:		
•	Optimum Moisture Content (%):		16.3
	Maximum Dry Density (pcf):		111.3
	95% of MDD (pcf):		105.7
	In-Situ Moisture Content (%):		N/A
5. Specimen Pro	nartias:		
o. opcomici i io	Wet Weight (g):		3203.30
	Compaction Moisture content (%):		16.6
	Compaction Wet Density (pcf):	5.	124.79
	Compaction Dry Density (pcf):		107.03
	Moisture Content After Mr Test (%):		16.4
	Moisture Content After Wil Test (%),		10.4
6. Quick Shear T	est (Y=Yes, N=No, N/A=Not Applicable):		#VALUE!
7. Resilient Mod	ulus, Mr:	17790(S	c)^-0.27240(S3)^0.21340
8. Comments			
9. Tested By:	GW D	ate: August 11, 2017	
J. Toolog Dy.	<u> </u>	Aco. Magast 11, 2017	

ARKANSAS STATE HIGHWAY AND TRANSPORTATION DEPARTMENT MATERIALS DIVISION

AASHTO T 307-99 - RESILIENT MODULUS OF SUBGRADE SOILS RECOMPACTED SAMPLES

SSRVPS 102+00 30'RT

Material Code Station No.:

Location:

Job No. 080504

Date Sampled: 7/18/17

Date Tested: August 11, 2017

Name of Project: DEER CREEK STR. & APPRS. (S)

County: Code: 49 Name: MONTGOMERY

Sampled By: BATES/JORDAN

Lab No.: 20172450

Sample ID: RV486

LATITUDE:

Material Type (1 or 2): 2 LONGITUDE:

A-6(32)

AASHTO Class:

Depth:

0-5

	Chamber	Nominal	Actual	Actual	Actual	Actual	Actual	Actual	Average	Resilient	Resilient
	Confining	Maximum	Applied	Applied	Applied	Applied	Applied	Applied	Recov Def.	Strain	Modulus
PARAMETER	Pressure	Axial	Max. Axial	Cyclic Load	Contact	Мах.	Cyclic	Contact	LVDT 1		
		Stress	Load		Load	Axial	Stress	Stress	and 2		
						Stress					
DESIGNATION	ၓၲ	Scyclic	Р _{шах}	P _{cyclic}	Pcontact	S _{max}	Scyclic	Scontact	H _{avg}	کٰ	M
UNIT	psi	psi	sql	sql	sql	psi	psi	isd	in	in/in	psi
Sequence 1	0.9	2.0	25.3	22.5	2.8	2.1	1.9	0.2	0.00070	0.0000	21,314
Sequence 2	0.9	4.0	47.6	44.8	2.8	3.9	3.7	0.2	0.00152	0.00019	19,413
Sequence 3	6.0	6.0	70.4	8.99	3.6	5.8	5.5	0.3	0.00254	0.00032	17,355
Sequence 4	6.0	8.0	94.0	87.9	6.0	7.7	7.2	C.5	0.00384	0.00048	15,115
Sequence 5	0.9	10.0	116.7	108.3	8.5	9.6	8.9	C.7	0.00524	0.00065	13,617
Sequence 6	4.0	2.0	25.3	22.5	2.8	2.1	1.8	0.2	0.00076	0.00010	19,433
Sequence 7	4.0	4.0	47.4	44.6	2.8	3.9	3.7	0.2	0.00169	0.00021	17,408
Sequence 8	4.0	6.0	0.69	66.3	2.8	5.7	5.4	0.2	0.00282	0.00035	15,481
Sequence 9	4.0	8.0	92.2	87.0	5.1	7.6	7.1	C.4	0.00417	0.00052	13,770
Sequence 10	4.0	10.0	115.6	108.0	9.7	9.5	8.9	0.0	0.00562	0.00070	12,680
Sequence 11	2.0	2.0	25.2	22.4	2.7	2.1	1.8	0.2	0.00087	0.00011	17,028
Sequence 12	2.0	4.0	47.0	44.3	2.8	3.9	3.6	C.2	0.00196	0.00024	14,921
Sequence 13	2.0	0.9	68.3	65.5	2.8	5.6	5.4	0.2	0.00324	0.00040	13,330
Sequence 14	2.0	8.0	90.4	86.2	4.2	7.4	7.1	0.3	0.00469	0.00058	12,117
Sequence 15	2.0	10.0	112.9	106.3	9.9	9.3	8.7	0.5	0.00621	0.00077	11,281

DATE August 11, 20	DATE
GW	
FESTED BY	REVIEWED BY

ARKANSAS STATE HIGHWAY AND TRANSPORTATION DEPARTMENT **MATERIALS DIVISION**

AASHTO T 307-99 - RESILIENT MODULUS OF SUBGRADE SOILS RECOMPACTED / THINWALL TUBE SAMPLES

Job No.

080504

Material Code SSRVPS

Date Sampled:

7/18/17

Station No.: 102+00

Date Tested:

Name of Project: DEER CREEK STR. & APPRS. (S)

August 11, 2017

Location: 30'RT

County:

Code: 49

Name: MONTGOMERY

Sampled By:

BATES/JORDAN

Depth: 0-5

Lab No.:

20172450

AASHTO Class: A-6(32)

Sample ID:

RV486

Material Type (1 or 2): 2

LATITUDE:

LONGITUDE:

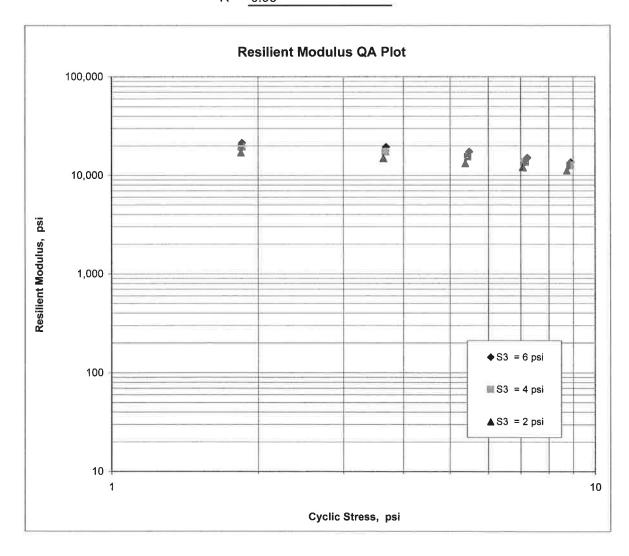
$$M_R = K1 (S_C)^{K2} (S_3)^{K5}$$

K1 = 17,790

K2 = -0.27240

K5 = 0.21340

 $R^2 = 0.96$



JOB: 080504

Arkansas State Highway Transporation Department

· JOB NAME: DEER CREEK STR. & APPRS.(S)

Materials Division

COUNTY NO. 49 **DATE TESTED** 8/1/2017

Michael Benson, Materials Engineer

STA.#	LOC.	DEPTH	COLOR	#4	#10	#40	#80	#200	L.L.	P.I.	SOIL CLASS	<i>LAB</i> #:	%MOISTURE
102+00	30 RT	0-5	RD/BR	83	71	56	49	43	38	16	A-6(32)	RV486	
102+00	06 RT	0-3Z	RD/BR	82	66	54	48	40	32	10	A-4(0)	S482	13.1
102+00	21 RT	0-5	RD/BR	78	68	54	48	44	33	14	A-6(2)	S483	12.1
119+00	06 LT	0-5	BR/GR	91	81	64	59	55	30	10	A-4(3)	S484	22.8
119+00	19 LT	0-5	BROWN	83	69	53	47	43	31	12	A-6(2)	S485	12.9

JOB: JOB NAME: DEER CREEK STR. & APPRS.(S) 080504

Arkansas State Highway Transporation Department Materials Division

DATE TESTED 8/1/2017

COUNTY NO. 49

Michael Benson, Materials Engineer

	119+00 06 LT	102+00		102+00	STA.# LOC.
	06 LT	21 RT		06 RT	OC.
2.5	ACHMSC	102+00 21 RT ACHMSC	3.0W	102+00 06 RT ACHMSC	
0.5X	ACHMSC	ACHMSC	1.0X	ACHMSC	
4.0W	ACHMSC	ACHMSC	2.0	ACHMSC	
1	ACHMSC	ACHMSC	1.0X	ACHMSC	PAVEMENT SOUNDINGS
1	ACHMSC	ACHMSC	1.5	ACHMSC	DUNDINGS
3.0W	ACHMBC	ACHMBC	1	ACHMBC	
6.0	AGG.BASE CRS C	AGG.BASE CRS C	7.0	AGG.BASE CRS C	

comments: W=MULTIPLE LAYERS, X=STRIPPED, Z=AUGER REFUSAL

Monday, August 21, 2017

MICHAEL BENSON, MATERIALS ENGINEER

*** SOIL SURVEY / PAVEMENT SOUNDING TEST REPORT ***

DATE - 08/ JOB NUMBER - 080 FEDERAL AID NO TO : PURPOSE - SOI: SPEC. REMARKS - NO : SUPPLIER NAME - STA' NAME OF PROJECT - D PROJECT ENGINEER - N PIT/QUARRY - ARKAN LOCATION - MONTG SAMPLED BY - BATES/ SAMPLE FROM - TEST : MATERIAL DESC SOI	504 BE ASSI L SURVE SPECIFI TE EER CRE OT APPI SAS OMERY, JORDAN HOLE	EY SAMPLE CATION CHECK EEK STR. & APPRS.(S LICABLE COUNTY			MATERI SPEC. SUPPLI COUNTY DISTRI DATE S DATE R DATE I	AL YEA ER /ST CT SAM	NO 1 CODE - SS AR - 20 ID 1 TATE - 49 NO 06 PLED - 07 TED - 08	14 /18/17 /25/17
LAB NUMBER	=3	20172446	_	20172447		-	20172448	
SAMPLE ID				S483			201/2448 S484	
TEST STATUS	-				N ONLY			ON ONLV
STATION				102+00			119+00	ON ONLI
LOCATION	4		-	21 RT		77	06 LT	
DEPTH IN FEET	=	0-3Z	-	0-5		-	0-5	
MAT'L COLOR	:	RD/BR	-	RD/BR		*	BR/GR	
MAT'L TYPE	= = =	·	_	·		2	•	
LATITUDE DEG-MIN-S	SEC =	34 38 55.20	_	34 38 5	55.20	4	34 39	11.70
LONGITUDE DEG-MIN-	SEC -	93 32 1.30		93 32	1.20		93 32	1.30
% PASSING 2	IN.		-			_		
	IN		-			_		
•	IN	100	-	100		-	100	
	IN. =			90		-	96	
NO.	4 =	82	*	78		-	91	
NO.	10 -	66	-	68		_	81	
NO.	40 -	54	-	54		_	64	
NO.	80 = 0	48	-	48		-	59	
NO.	200 -	40		44			55	
LIQUID LIMIT	2	32	_	33		_	30	
PLASTICITY INDEX			_	14		-	10	
	30		-			*		
UNIFIED SOIL	5		-			***	, , ,	
% MOISTURE CONTENT		13.1	-	12.1		-	22.8	
ACHMSC	(IN) -	3.0W	2			_	2.5	
ACHMSC	(IN) -	1.0X	4			_	0.5X	
ACHMSC	(IN) -	2.0	=			-	4.0W	
ACHMSC	(IN)	1.0X	3.5			-	# #	
ACHMSC	(IN)	1.5	12	-		_		
ACHMBC	(IN) _					_	3.0W	
AGG.BASE CRS CL-7	(IN) -	7.0		44		_	6.0	
	=7.		1,5			-		
	=0		_			-		

REMARKS - W=MULTIPLE LAYERS, X=STRIPPED, Z=AUGER REFUSAL

AASHTO TESTS : T24 T88 T89 T90 T265

MICHAEL BENSON, MATERIALS ENGINEER

*** SOIL SURVEY / PAVEMENT SOUNDING TEST REPORT ***

SUPPLIER NAME - STATE NAME OF PROJECT - DEER PROJECT ENGINEER - NOT PIT/QUARRY - ARKANSAS LOCATION - MONTGOME SAMPLED BY - BATES/JOR SAMPLE FROM - TEST HOL	ASSIGNED URVEY SAMPLE CIFICATION CHECK CREEK STR. & APPRS.(S) APPLICABLE RY, COUNTY	SEQUENCE NO 2 MATERIAL CODE - SSRVPS SPEC. YEAR - 2014 SUPPLIER ID 1 COUNTY/STATE - 49 DISTRICT NO 06 DATE SAMPLED - 07/18/17 DATE RECEIVED - 07/25/17 DATE TESTED - 08/01/17
LAB NUMBER	- 20172449 -	
SAMPLE ID	- S485 -	56
	- INFORMATION ONLY -	=
STATION	- 119+00 -	-
LOCATION	- 19 LT	=
DEPTH IN FEET		젊
	- U-5 - BROWN	<u> </u>
IIII D CODOR	- BROWN _	-
MAT'L TYPE		Ē.
LATITUDE DEG-MIN-SEC		₹
LONGITUDE DEG-MIN-SEC	- 93 32 1.30	
% PASSING 2 IN	<u> </u>	8=
1 1/2 IN	ະ ເ_ = — — — — — — — — — — — — — — — — — —	.:=
3/4 IN	100	15 T
3/8 IN	97	Y#
NO. 4	- 83	-
NO. 10	- 69	
NO. 40	- 53	<u> </u>
NO. 80		18
	- 43	
LIQUID LIMIT	- 31	<u></u>
	- 12	-
AASHTO SOIL	- A-6(2)	-
UNIFIED SOIL	- ·	-
% MOISTURE CONTENT	- 12.9	
		; = ;
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REMARKS - W=MULTIPLE LAYERS, X=STRIPPED, Z=AUGER REFUSAL

AASHTO TESTS : T24 T88 T89 T90 T265

MICHAEL BENSON, MATERIALS ENGINEER

*** SOIL SURVEY / PAVEMENT SOUNDING TEST REPORT ***

SPEC. REMARKS - NO SPECIE SUPPLIER NAME - STATE NAME OF PROJECT - DEER CE PROJECT ENGINEER - NOT APE PIT/QUARRY - ARKANSAS LOCATION - MONTGOMERY, SAMPLED BY - BATES/JORDAN SAMPLE FROM - TEST HOLE	YEY SAMPLE FICATION CHECK REEK STR. & APPRS.(S PLICABLE COUNTY	3)	COUNTY/STATE - DISTRICT NO DATE SAMPLED - DATE RECEIVED - DATE TESTED -	RV 2014 1 49 06
MATERIAL DESC SOIL SURVEY - RESISTANCE R-VALUE ACTUAL RESULTS				
LAB NUMBER -	20172450	-	-	
	RV486	-	¥	
TEST STATUS -		-	=	
STATION -		_	2 2	
	30 RT	_	ā ≧	
	0-5	_	¥	
	RD/BR	-	-	
MAT'L TYPE -		-	7	
LATITUDE DEG-MIN-SEC -		-	=	
LONGITUDE DEG-MIN-SEC - 93 32 1.10				
% PASSING 2 IN		# :	::=	
1 1/2 IN		77	. 	
3/4 IN	100		16	
3/8 IN	94	₩ ;	12	
NO. 4 -	83	2	.=	
NO. 10 -			<u> </u>	
NO. 40 -	56	₩ 3	:#	
NO. 80 -			i e	
NO. 200 -	43			
LIQUID LIMIT -	38		2	
PLASTICITY INDEX -	16	2	2	
AASHTO SOIL -	A-6(32)	W	-	
UNIFIED SOIL -		•	≅ 	
% MOISTURE CONTENT -			<u> </u>	
_		=		
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REMARKS - W=MULTIPLE LAYERS, X=STRIPPED, Z=AUGER REFUSAL

AASHTO TESTS : T24 T88 T89 T90 T265

105