ARKANSAS DEPARTMENT OF TRANSPORTATION



SUBSURFACE INVESTIGATION

STATE JOB NO.			012357		
FEDERAL AID PRO	JECT NO.	STPA	C-0076(123)		
A	RKANSAS F	RIVER PIE		ON REPAIRS	S (S)
STATE HIGHWAY	I-30, 9 8	¥ 23	SECTION	6,	7 & 23
IN	CO	NWAY, FR	ANKLIN & PU	LASKI	COUNTY

The information contained herein was obtained by the Department for design and estimating purposes only. It is being furnished with the express understanding that said information does not constitute a part of the Proposal or Contract and represents only the best knowledge of the Department as to the location, character and depth of the materials encountered. The information is only included and made available so that bidders may have access to subsurface information obtained by the Department and is not intended to be a substitute for personal investigation, interpretation and judgment of the bidder. The bidder should be cognizant of the possibility that conditions affecting the cost and/or quantities of work to be performed may differ from those indicated herein.

2021 Lakeshore Dr. Suite 230 New Orleans, Louisiana 70122 Telephone (504) 872-3000 Facsimile (504) 872-3001 www.hntb.com

Date 1/25/2020 То Arkansas DOT Attn: Jim Pool From Brad Wilder, HNTB





Subject Arkansas DOT I-30 Cell Pier Protection Replacement Geotechnical Analysis

1. Project Description

The I-30 Bridge is a critical link in the Central Arkansas Freeway System through Little Rock, AR. As a navigable waterway extensive cargo shipping occurs along the Arkansas River, requiring robust pier protection measures to protect the bridge from boat collisions. To protect the existing bridge bents 40ft diameter sheet pile cells were constructed upstream and downstream of each bent along the navigation channel. Currently, the northwestern most cell has failed and needs to be replaced.

Existing information consisting of as-built plans for the I-30 project (1968) are included in Attachment A. A boring log from a 2018 geotechnical exploration report (Boring B-68; drill date of 2015) is included in Attachment B. A recent ArDOT 2019 topographic river survey is included in Attachment C. Geotechnical calculations which include bearing capacity, eccentricity, and sliding calculations are included in Attachment D.

2. Existing Conditions

The original protection cell is a circular sheetpile structure founded on shale and filled with quarry stone. The failed cell is mostly submerged based on recent photographs Further information on the failure mechanism is not known; however, it is possible that interior stone was lost out of the top of the cell during high water events, at the base of the cell, or after vessel collisions and this loss of stone may have caused the structure to "buckle" or "tip".

The Arkansas River in the vicinity of the I-30 bridge has experienced scouring around the pier protection cells based on recent 2015 borings and recent survey. The river bed is comprised of silty non-cohesive material ("muck") which is erodible. At the time of construction, the height of overburden above the shale was approximately 33 ft while the 2015 boring log B-68 illustrates only 3 ft of overburden. Below the silty material, a shale layer is encountered at approximate elevation 190 ft in boring B-68. The shale is part of the Jackfork Group. The shale is gray, slightly fractured, moderately weathered, with moderately fractured quartz seams. The shale encountered at boring B-68 is interbedded with sandstone from approximate elevation 179 ft to 169 ft and then the shale continues to the termination depth of the boring at elevation 72 ft (boring depth of 161.5 ft).

3. Geotechnical Analysis

3.1 Design Conditions

Design criteria for this project is AASHTO LRFD Bridge Design Specifications, 7th Edition, with 2016 Interim Revisions. All design criteria and assumptions can be found in the Pier Protection Cell Rehabilitation Design Criteria document (revision date of December 9, 2019). Design life is ten years. Hydraulic calculations are provided in a separate hydraulic memorandum (January 2020). Design assumptions included two cases:

- 1. Case 1: Normal water event with normal operation design force
- 2. Case 2: High water event with normal operation design force.

Design Loads:

- Normal Operation Design Force: 2,800 kips
- Drifting Empty Barge Design Force: 1,440 kips
- Normal Operation Design Speed: 5.9 ft/s
- Drifting Empty Barge Speed: 2.9 ft/s
- Normal Operation Design Water Surface: 231.0 ft

Drifting Empty Barge Design Water Surface: 247.2 ft (10-yr event).

3.2 Analysis

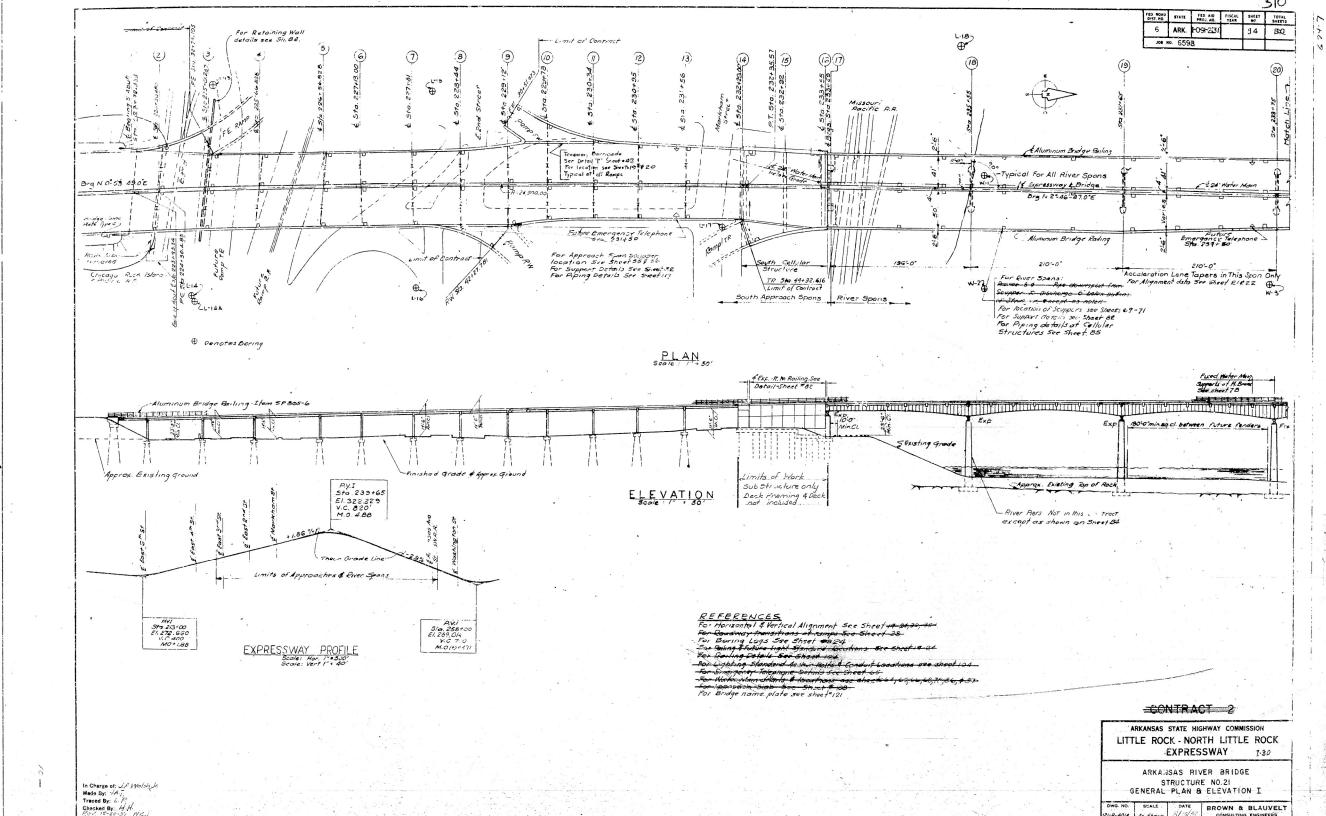
Bearing capacity resistance, sliding resistance, and eccentricity were calculated according to AASHTO LRFD. The design assumes a unit weight of 150 pcf for the cell rockfill material. The factored resistances for both Case 1 and Case 2 exceeded the factored loads. Additionally, the eccentricity is within 9/10 of the cell base as required by AASHTO LRFD. Results are provided in Appendix D.

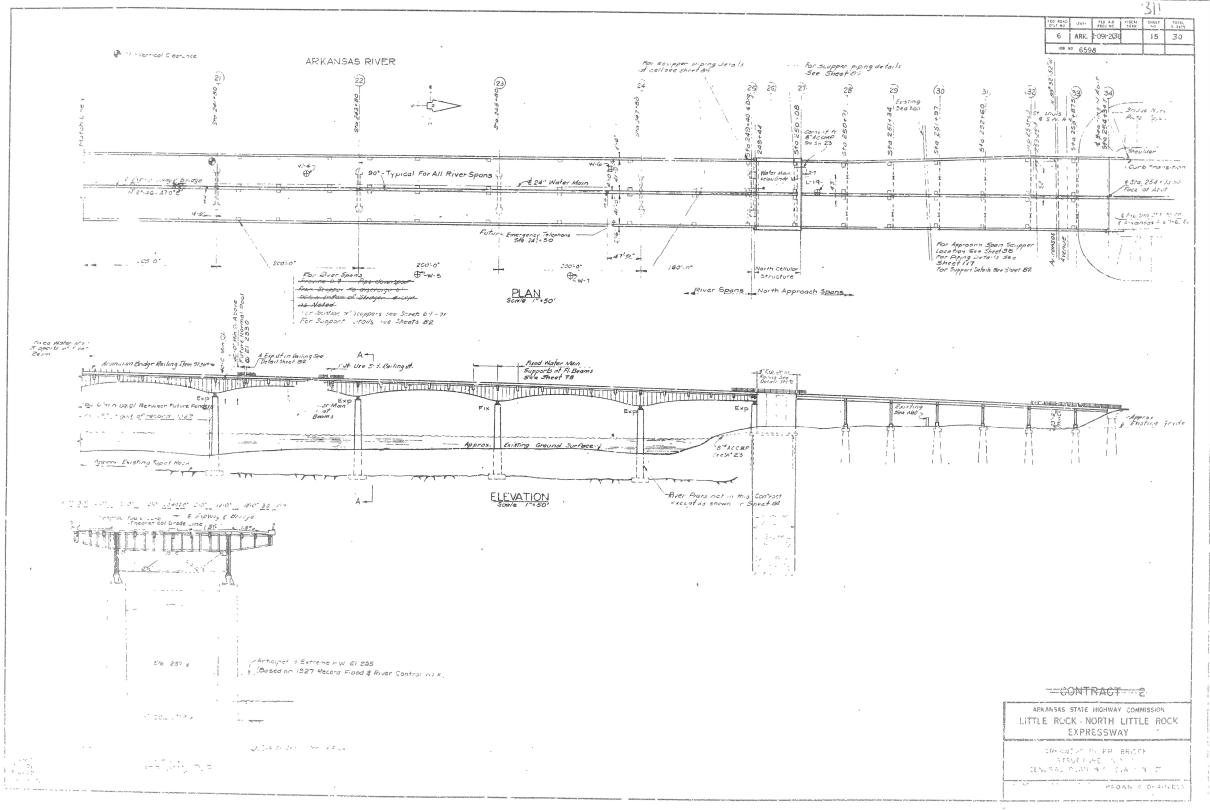
4. Construction Considerations

Sheetpiling is to be driven to refusal in shale with care taken not to damage the piles. Based on the minimal amount of overburden above the shale and depending on the ability to embed the sheetpile into the shale layer, it may be required to use additional bracing methods to support the sheetpile in a vertical position while the cell is constructed.

Preliminary hydraulic calculations for the rock fill gradation for the cell structure determined a d50 of 8 inches or unit weight of 26 lbf and a gradation with 100% passing 20 inches or less than 300 lb stone. The high turbulence set of rock size curves were used for an average velocity equal to 7 ft/s and a rock specific weight equal to 155 lbf/cf. See hydraulic memorandum for additional information.

ATTACHMENT A AS-BUILT PLAN





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INDEX TO SHEETS

SHEET NO.	TITLE	DRAWING NO.
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7	Title Sheet	
2	Index To Sheets, Special Provisions and Quantities	16098
J	Triangulation Control and Soil Information	16000
а	Flan and Elevations	16010
5	Structural Details	16011
6	Navigation Lighting System Details	16012

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REVISIONS Added BP-7-30 Revised BP-8-10 Added Poel Elev.Note, S

ITEM NUMBER	SP	SP	SP & 806	SP	SP	SP	SP & 101	SP	SP
ITEM	STEEL SHEET PILING (MP-102)	QUARRY RUN STONE	STRUCTURAL STEEL IN PIER PRCTECTION CELL (A-36)	CLEARANCE GAGE	EITUMASTIC COATING	WET EXCAVATION	FURNISHING FIELD OFFICE	NAVIGATION LIGHTING SYSTEM	BITUPLASTIC
UNIT	SQ FT.	CU. YD.	LB.	EACH	SQ. FT.	CU. YĐ	SLDG.	COMP. ITEM	SQ. FT.
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CELL 3	7,438	1,851	16,973		10,875				2,500
CELL 4	7,438	1,783	16,973		10,875				2,500
CELI. 5	8,437	1,617	10,218		9,875				2,500
CELL 6	8,437	1,640	10,218		9,875				2,500
TOTALS	45,626	10,176	74,966	2	61,250	300	1	1.00	15,000

PROVISIONS SPECIAL

TITLE

Job 6903	Required Contract Provisions for Work Financed by Corps of Engineers' Funds
4-1	Plant Sites and Equipment Storage A: as
Job 6903	Work Areas
6-3	Sources of Materials
6-6 6-7 7-28	Furnishing Materials from Bars, Pits or Quarries Control –† Materials Plant Pest Control
7-29 7- 30 Uob G903	Protection of Sireams, Lakes and Reservoirs Contractors Responsibility for Damage Claims Requirements of durisdictional Ayencies
8-2	Employment of Labor
Job 6903	Revision of Section 8 - Prosecution and Progress
8-9	Legul Holidays
8-10	Euspansion or Termination of Contract.
8-!/	Limitations of Subletting or Assignment of Cuntract
9-!/	Revision of Article 9.7 - Partial Payments
100-1	Furnishing Field Offices and Laboratories
806-13	Revision of American Welding Society Bridge Sp cifications
Job 6903	Pier Protestion Cells
Job 6903	Ciearance Gages
Uot 6903	Permanent Navigation Lights

SPECIAL PROVISION NO.

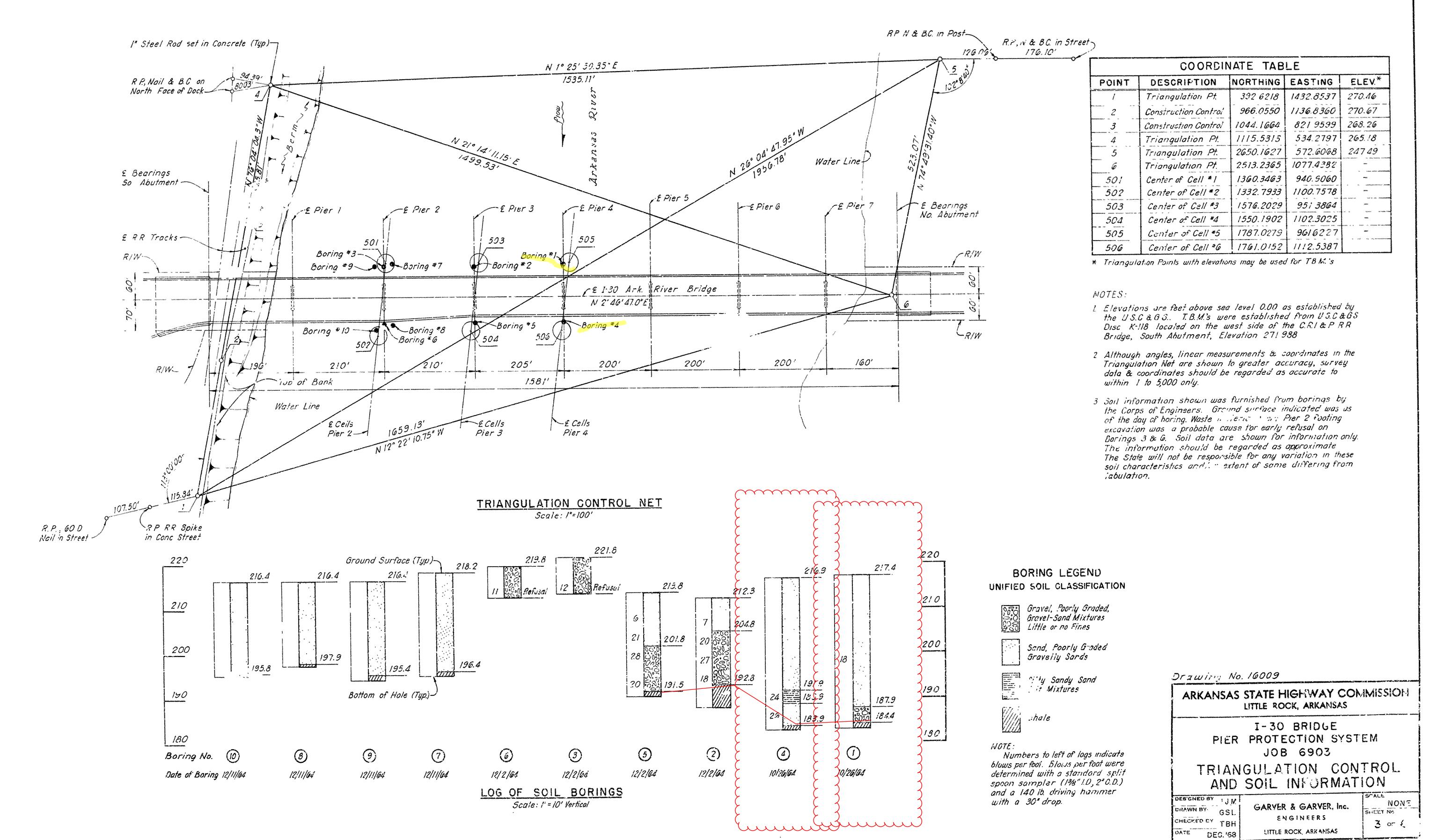
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SUMMARY OF QUANTITIES

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I-30 BRIDGE PIER PROTECTION SYST JUB 6903 INDEX TO SHEETS, SP FROVISIONS AND QUAN	ECIAL
DRAWN BY GSL CHECKED BY TBH DATE DEC. '68 LITTLE ROCK, ARKANSAS	SCALE NONE



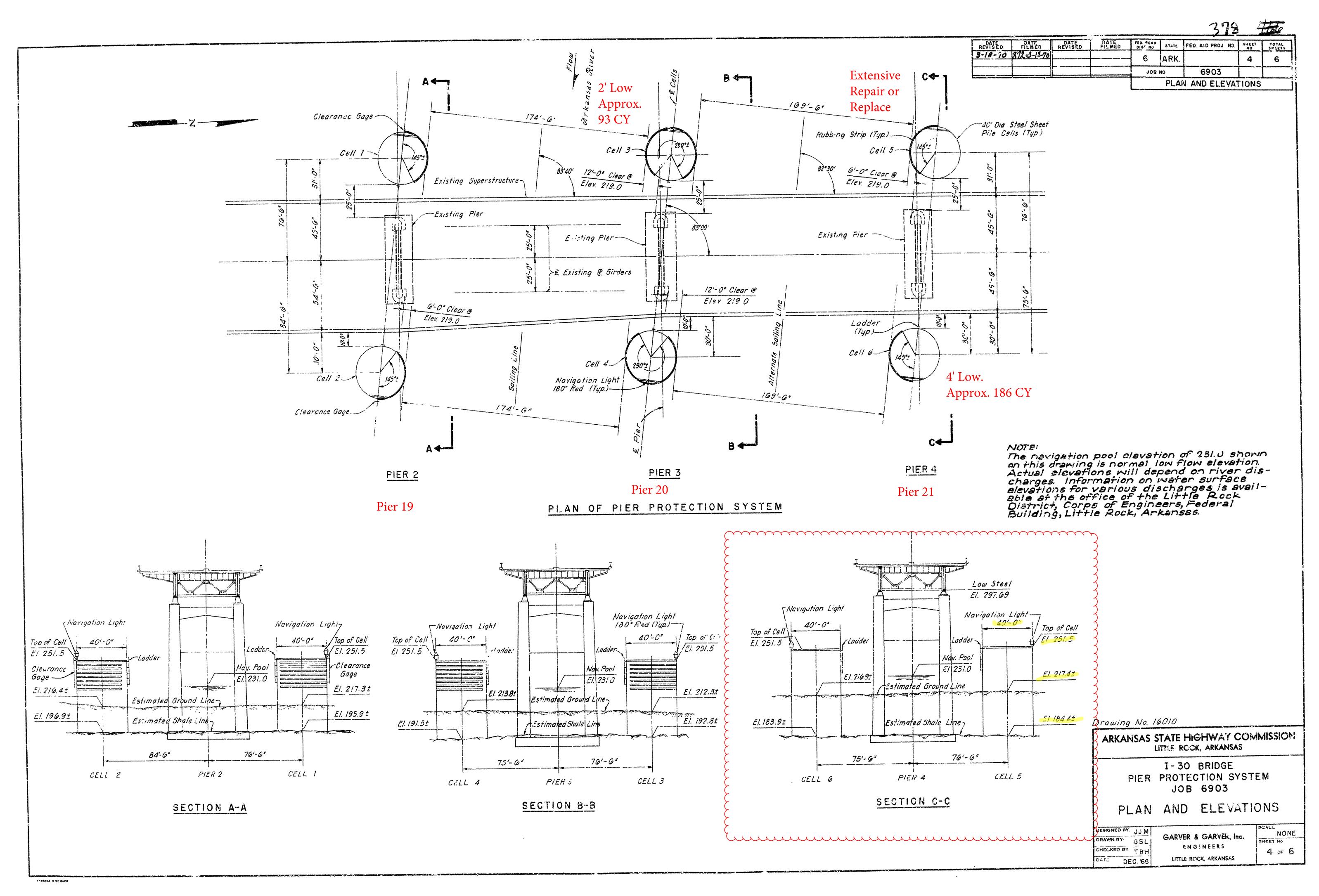
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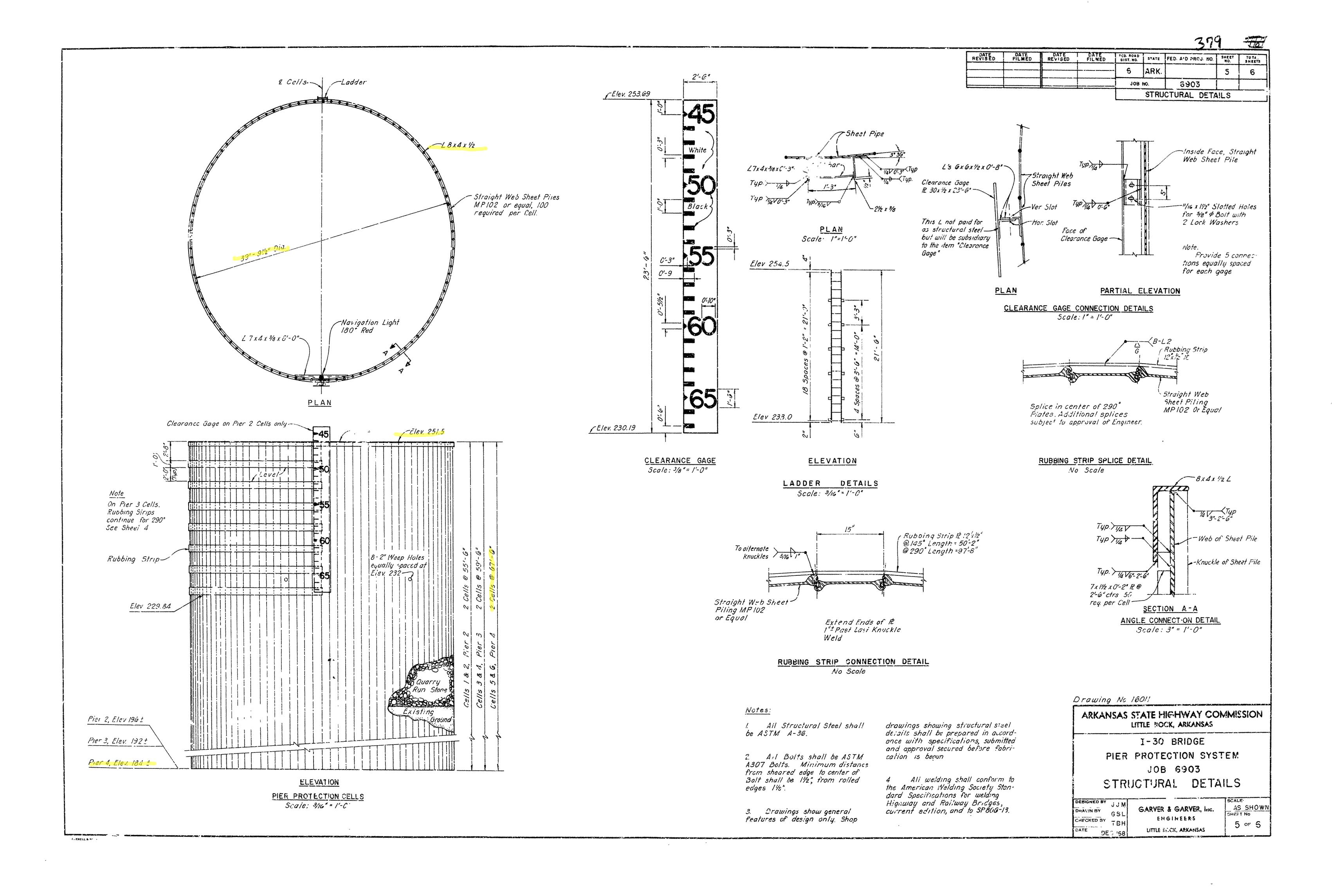
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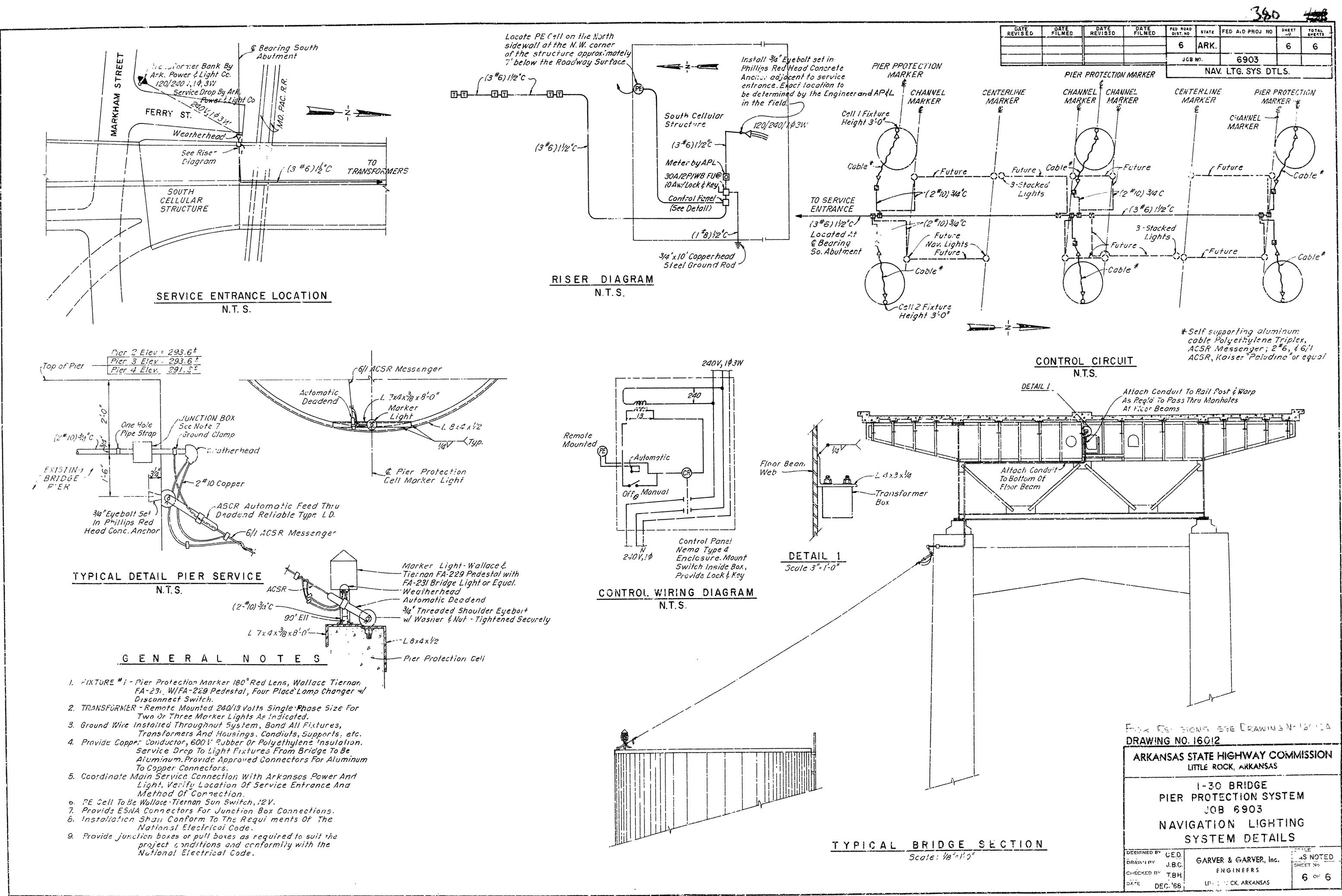
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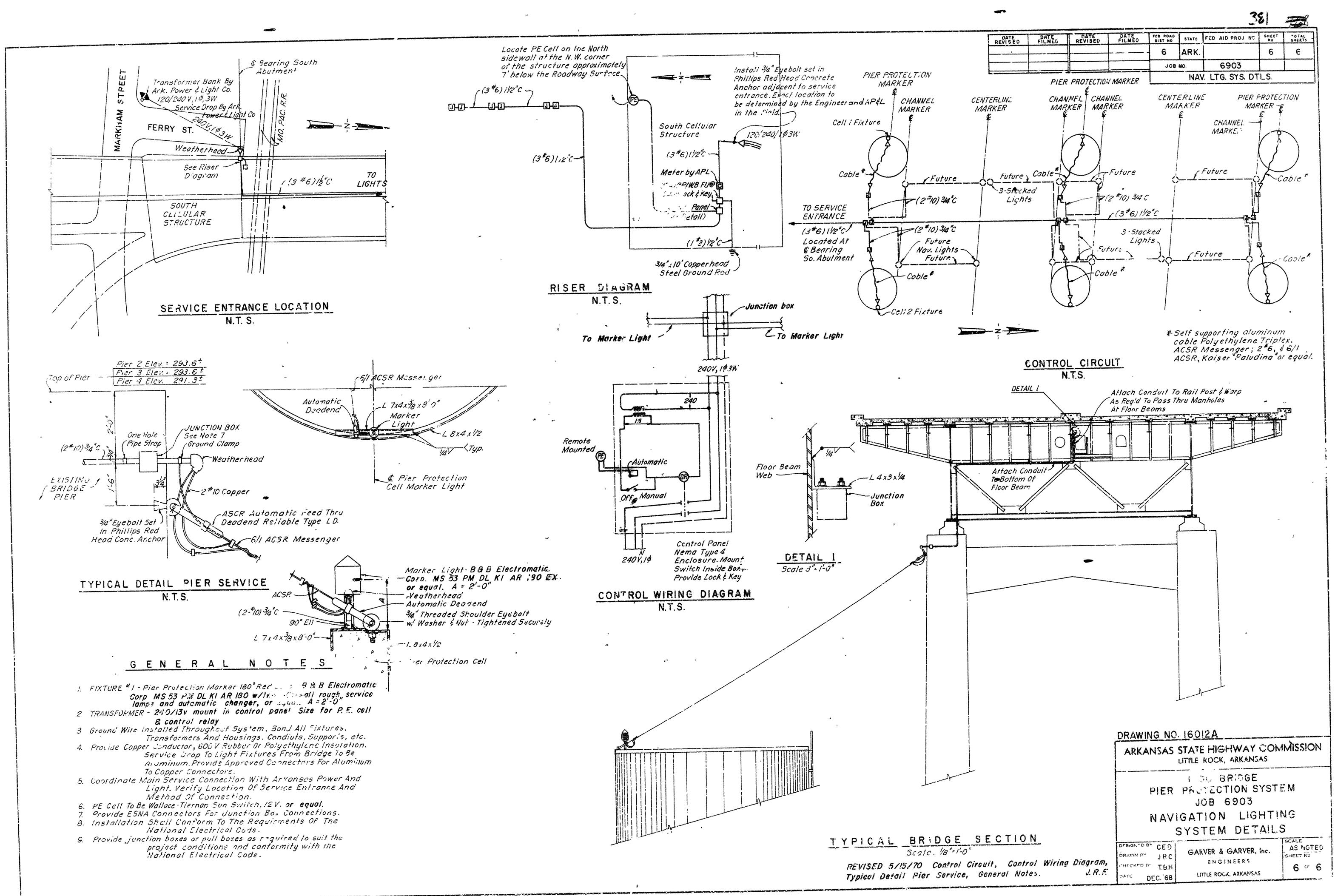
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	COORDIN	NATE TAB	LE	
POINT	DESCRIPTION	NORTHING	EASTING	ELEV.*
<u></u>	Triangulation Pt.	392 6218	1432.8537	270.46
2	Construction Control	966.0550	1136.8360	270.67
	Construction Control	1044.1664	821 9599	268.26
4 A	Triangulation Pt.	1115.5315	534.2797	265.18
5	Triangulation Pt.	26.50.1627	572.6068	24749
6	Triangulation Pt.	2513.2365	1077.4382	geend y y y y a an an an an a geolagica y y y y y y y y y y y y y y y y y y y
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506	Center of Cell *6	1761.0152	1112.5387	



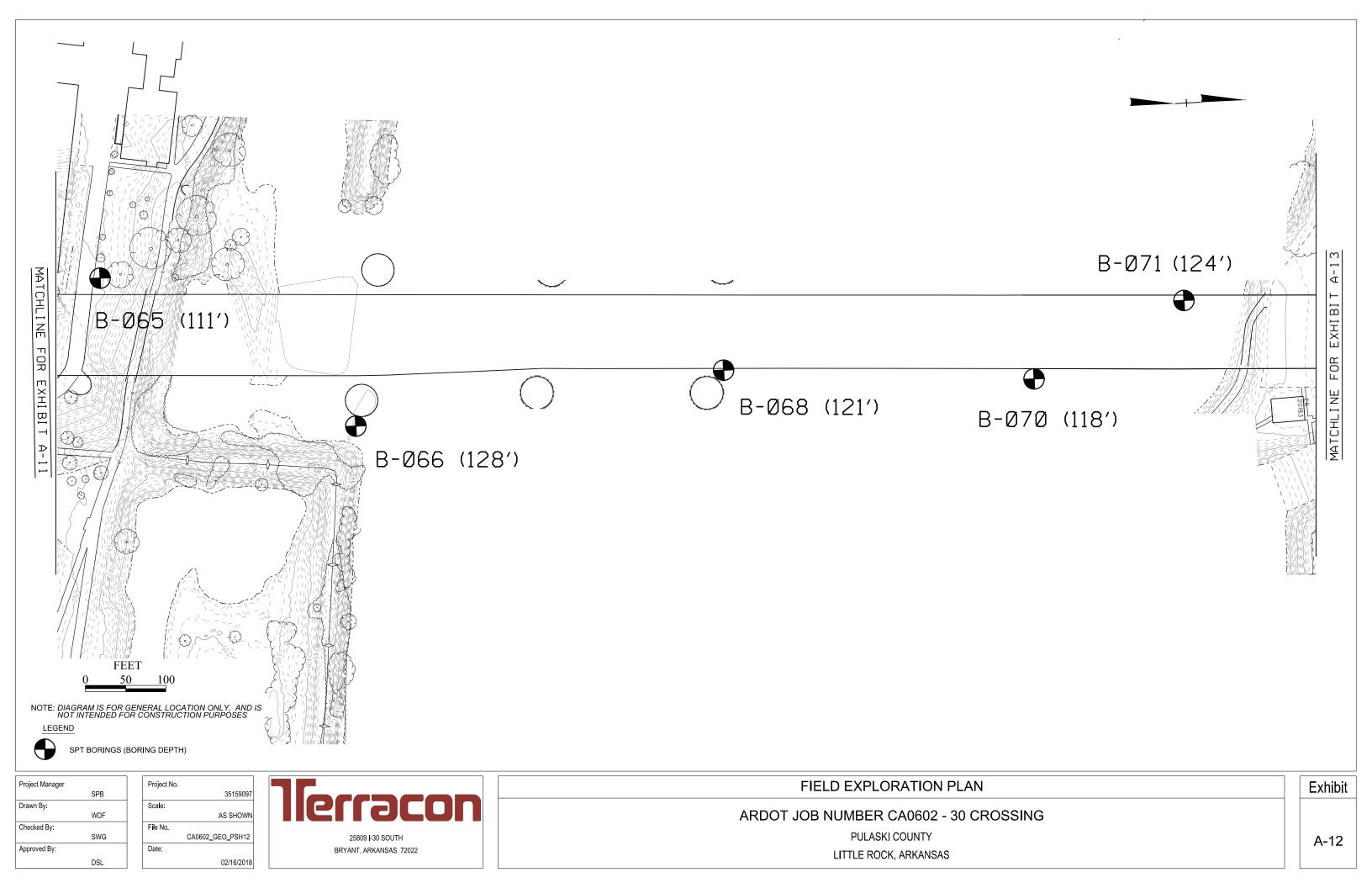






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ATTACHMENT B 2015 BORING LOG B-68



Page 1 of 6

PROJECT: AHTD Job No. CA0602 - 30 Crossing						CLI	CLIENT: Arkansas State Highway and Transportation Dept.										
SIT	E: Little Rock Area Pulaski County, Arkan	sas								•		•					
(1)	LOCATION See Exhibit A-12	1	. v	ш	<u>.</u>						STREN	GTH TES	т			ATTERBERG LIMITS	(0)
GRAPHIC LOG		(H	WATER LEVEL OBSERVATIONS	SAMPLE TYPE	RECOVERY (In.)	щ	EST TS	TOTAL ROCK UNIT WEIGHT (pcf)	LABORATORY HP (tsf)					WATER CONTENT (%)	SOIL DRY UNIT WEIGHT (pcf)	LIWITS	PERCENT FINES
DHIC	Latitude: 34.74983° Longitude: -92.262528°	DEPTH (Ft.)	SVA B	ц	VER	SAMPLE ID	FIELD TEST RESULTS	A H T U H T	P (tst	TEST TYPE	Is50-POINT LOAD INDEX (tsf)	COMPRESSIVE STRENGTH (tsf)	UNDRAINED SHEAR STRENGTH (tsf)	ATE	SOIL ≷Y UN GHT		ЦЦ
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Ŭ	DEPTH ELEVATION (Ft.)		> 8	Ś	R			_		F	≊S	NO CO	5 io	0	_		B
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	Total depth of hole: 161.5 Feet									H	ammer Tv	ne: Auto	matic 140-	lh hamr	ner with	a 30-inch drop	
	Stratification lines are approximate. In-situ, the tran	I.					Er	nergy Trar	sfer Ratio	(ETR) = 7	6.3%			′			
	ement Method:	ntal Da	atum: NA	.D 83 (Y.X L	oczalized)	No	tes:									
0 to 45.5	45.5 Feet: Wash Rotary with 3-7/8" Tricone Roller Bi to 161.5 Feet: NQ Core Barrel	t							•	40.5 feet from water surface to mudline. Advanced 4-inch casing 5 feet past mudline and began rock coring.							
						m: NAVE				Ad	vanced 4-	inch casir	ng 5 teet pa	ast mud	line and	began rock col	ng.
Abandonment Method:					lane Z	Zone: 030	02 Arkansas	s South									
			S	Surveyed by NTB Associates													
	WATER LEVEL OBSERVATIONS						Boring Started: 12-07-2015 Boring Completed: 12-08-						leted: 12-08-20)15			
	40.5' from water surface to mudline					26				Drill Rig: 977, ATV, CME-850 Driller: S. Zeien							
										cked by: F		d). Tennison		
			25809130							Project No.: 35159097 Exhibit: B-29							

Page 2 of 6

PROJECT: AHTD Job No. CA0602 - 30 Crossing							CLI	CLIENT: Arkansas State Highway and Transportation Dept.									
SI	E: Little Rock Area Pulaski County, Arkan	sas															
GRAPHIC LOG	LOCATION See Exhibit A-12 Latitude: 34.74983° Longitude: -92.262528° Northing: 2070540.42 Easting: 1233497.18 Surface Elev:: 233.3 (Ft.)	DEPTH (Ft.)	WATER LEVEL OBSERVATIONS	SAMPLE TYPE	RECOVERY (In.)	SAMPLE ID	FIELD TEST RESULTS	TOTAL ROCK UNIT WEIGHT (pcf)	LABORATORY HP (tsf)	TEST TYPE	Is50-POINT LOAD INDEX (tsf)	COMPRESSIVE DE STRENGTH (tsf) (tsf)	UNDRAINED A SHEAR STRENGTH (tsf)	WATER CONTENT (%)	SOIL DRY UNIT WEIGHT (pcf)	Atterberg Limits	PERCENT FINES
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	gray, slightly fractured, moderately weathered, with quartz seams, moderately fractured 54.5 179 SHALE WITH	45- - - 50- - - - - - - - - - - - -				RUN 1	REC = 88% RQD = 60%			PL /	4.2						
· · · · ·	INTERBEDDED SANDSTONE	-						171 ,		UC /		171.1					
0 to 45.5	Total depth of hole: 161.5 Feet Stratification lines are approximate. In-situ, the tran cement Method: 45.5 Feet: Wash Rotary with 3-7/8" Tricone Roller Bit is to 161.5 Feet: NQ Core Barrel onment Method:		H V S	lorizor 'ertica tate F	ntal Da I Datur Plane Z	m: NAVI	02 Arkansas)	Er	ammer Ty hergy Trar tes:	pe: Auto	matic, 140- o (ETR) = 7	Ib hamr 6.3%	ner with	a 30-inch drop	
	WATER LEVEL OBSERVATIONS 40.5' from water surface to mudline				-	2[]	5809 I 30 Bryant, AR	10	n	Drill Cheo	ng Startec Rig: 977, cked by: F ect No.: 3	ATV, CM P. McClou	E-850	Drille	er: S. Zei led by: D	leted: 12-08-20 en). Tennison 3-29)15

Page 3 of 6

PROJECT: AHTD Job No. CA0602 - 30 Crossing								CLIENT: Arkansas State Highway and Transportation Dept.										
SIT	E: Little Rock Area Pulaski County, Arkan	sas																
g	LOCATION See Exhibit A-12		NS EL	ЫП	ln.)			⊥ (j;	۲۲		STREN	GTH TES	т	(%		ATTERBERG LIMITS	6	
GRAPHIC LOG	Latitude: 34.74983° Longitude: -92.262528°	DEPTH (Ft.)	WATER LEVEL OBSERVATIONS	SAMPLE TYPE	RECOVERY (In.)	SAMPLE ID	FIELD TEST RESULTS	TOTAL ROCK UNIT WEIGHT (pcf)	LABORATORY HP (tsf)	Ы	μŇ	COMPRESSIVE STRENGTH (tsf)	⊟~E	WATER CONTENT (%)	SOIL DRY UNIT WEIGHT (pcf)			
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	DEPTH ELEVATION (Ft.) SHALE WITH			•,	_					PL /	4.2	Ŭ					<u> </u>	
	INTERBEDDED	_	1				DEO											
	SANDSTONE (continued) gray, slightly fractured,	-	1			RUN	REC = 96%											
• • •	moderately weathered	60-	-			2	RQD = 85%											
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	SHALE	65-	-															
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	gray, slightly fractured,							170		UC	0.4	40						
	moderately weathered	-	1							PL	8.4							
		-	-			RUN	REC = 97%											
		70-	-			3	RQD =											
		_	1				91%											
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		_	1															
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	gray, slightly fractured, unweathered	-	-				DE0 -	171		UC,	4.2	107.8						
	unweathered	80-	-			RUN 4	REC = 94%			PL /	<u> </u>	1						
		_					RQD = 93%											
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		_	1					171		UC PL	4.2	44.9						
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	ement Method: 45.5 Feet: Wash Rotary with 3-7/8" Tricone Roller Bil	ŀ	F	lorizo	ntal Da	tum: NA	D 83 (Y.X L	oczalized)	Not	tes:							
	to 161.5 Feet: NQ Core Barrel		v	'ertica	al Datu	m: NAVE	88											
)2 Arkansa	South										
Abando	onment Method:					NTB Ass		SOULI										
	WATER LEVEL OBSERVATIONS		-							Borin	ng Starter	1: 12-07-2	015	Borir	ng Comn	leted: 12-08-20	015	
	40.5' from water surface to mudline						10				-	ATV, CM			er: S. Zei			
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Page 4 of 6

	PROJECT: AHTD Job No. CA0602 - 30 Crossing								CLIENT: Arkansas State Highway and Transportation Dept.										
SI	TE: Little Rock Area Pulaski County, Arkan	isas								-									
Ŋ	LOCATION See Exhibit A-12	-	NS II	Щ	n.)				۲		STREN	GTH TES		(9	l)	ATTERBERG LIMITS	S		
GRAPHIC LOG	Latitude: 34.74983° Longitude: -92.262528° Northing: 2070540.42 Easting: 1233497.18 Surface Elev.: 233.3 (Ft.) DEPTH ELEVATION (Ft.)	DEPTH (Ft.)	WATER LEVEL OBSERVATIONS	SAMPLE TYPE	RECOVERY (In.)	SAMPLE ID	FIELD TEST RESULTS	TOTAL ROCK UNIT WEIGHT (pcf)	LABORATORY HP (tsf)	TEST TYPE	Is50-POINT LOAD INDEX (tsf)	COMPRESSIVE STRENGTH (tsf)	UNDRAINED SHEAR STRENGTH (tsf)	WATER CONTENT (%)	SOIL DRY UNIT WEIGHT (pcf)	LL-PL-PI	PERCENT FINES		
	<u>SHALE</u> (continued)			П															
	gray, slightly fractured, unweathered	- 90 -	-			RUN 5	REC = 93% RQD = 93%			<u>PL</u>	17.7								
9.GD1 Z 10/18		- 95 -	-							UC,		93.5							
		-	-							PL /	12.7								
	gray, slightly fractured, unweathered	- 100-	-			RUN 6	REC = 100% RQD = 99%	, 171		UC		78							
		-	-																
	gray, slightly fractured, unweathered, angled fractures	10 5 - -	-			RUN	REC = 88% RQD = 85%			UC,		159.2							
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		_	1	H				171		UC		83.4	<u> </u>						
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	Total depth of hole: 161.5 Feet Stratification lines are approximate. In-situ, the tran	nsition m	ay be g	radua	al.								omatic, 140- io (ETR) = 7		ner with	a 30-inch drop	Ι,		
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	40.5' from water surface to mudline										977,			-	er: S. Zei				
							5809 30				cked by: F		bu	Logg). Tennison			
							ryant, AR			Proje	ect No.: 3	5159097		Exhib	oit: E	3-29			

Page 5 of 6

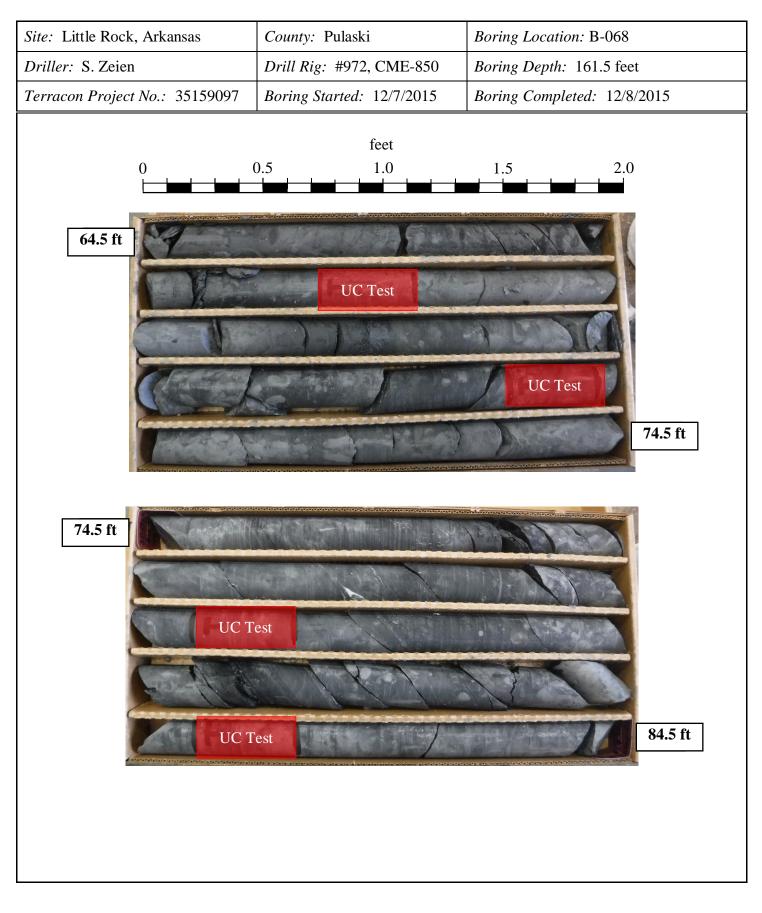
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	SIT	E: Little Rock Area Pulaski County, Arkar	sas								•		•					
ľ	(ľ)	LOCATION See Exhibit A-12		۱ <u>۵</u>	ш	÷				~		STREN	GTH TES	т			ATTERBERG LIMITS	í
	GRAPHIC LOG	Latitude: 34.74983° Longitude: -92.262528° Northing: 2070540.42 Easting: 1233497.18 Surface Elev.: 233.3 (Ft.)	DEPTH (Ft.)	WATER LEVEL OBSERVATIONS	SAMPLE TYPE	RECOVERY (In.)	SAMPLE ID	FIELD TEST RESULTS	TOTAL ROCK UNIT WEIGHT (pcf)	LABORATORY HP (tsf)	тезт түре	Is50-POINT LOAD INDEX (tsf)		UNDRAINED SHEAR STRENGTH (tsf)	WATER CONTENT (%)	SOIL DRY UNIT WEIGHT (pcf)	LL-PL-PI	PERCENT FINES
		DEPTH ELEVATION (Ft.)		- 0	0,		8	REC =			'		8	[©]				ш.
		SHALE (continued) gray, slightly fractured, unweathered, angled fractures	-	-	==			95% RQD = 89%										
			_						172		PL UC	16.9	49.3					
			120-	-														
SDT 2/16/18		gray, wide fracture spacing, unweathered, angled fractures	- - 12 5 -	-			RUN 9	REC = 93% RQD = 93%										
35159097_02-09-18.GPJ_TERRACON2015.GDT			-	-				93%	470		PL /	8.4						
Г			-						172		ΰĊ	0.4	39.8					
-18.G			130-															
02-09			_															
067_0			_															
35159			-															
BORING LOG		gray, wide fracture spacing, unweathered, angled fractures	- 13 5- -	-			RUN 10	REC = 98% RQD = 98%	474				05.0					
35159097			_						171		UC PL	12.7	85.6					
_ E			- 140-	-														
IAL R									173		UC							
THIS BORING LOG IS NOT VALID IF SEPARATED FROM ORIGINAL REPORT.			-	-														
PARATED		Total depth of hole: 161.5 Feet Stratification lines are approximate. In-situ, the tran	nsition ma	ay be g	radua]								matic, 140- o (ETR) = 7		ner with	a 30-inch drop	,
VALID IF SE	0 to	cement Method: 45.5 Feet: Wash Rotary with 3-7/8" Tricone Roller Bi to 161.5 Feet: NQ Core Barrel	t				atum: NA m: NAV[.D 83 (Y.X L D 88	oczalized.)	Not	es:						
DG IS NOT	Abando	onment Method:					Zone: 03 NTB Ass	02 Arkansa: ociates	s South									
10 LC		WATER LEVEL OBSERVATIONS			5						Borin	ig Starteo	: 12-07-2	015	Borir	ng Comp	leted: 12-08-20)15
ORIN		40.5' from water surface to mudline					26	720	20		-		ATV, CM			er: S. Zei		
HIS B							2	5809 I 30					P. McClou	ıd	_		. Tennison	
± 1							E	ryant, AR			Proje	ect No.: 3	5159097		Exhil	טות: Ε	3-29	

Page 6 of 6

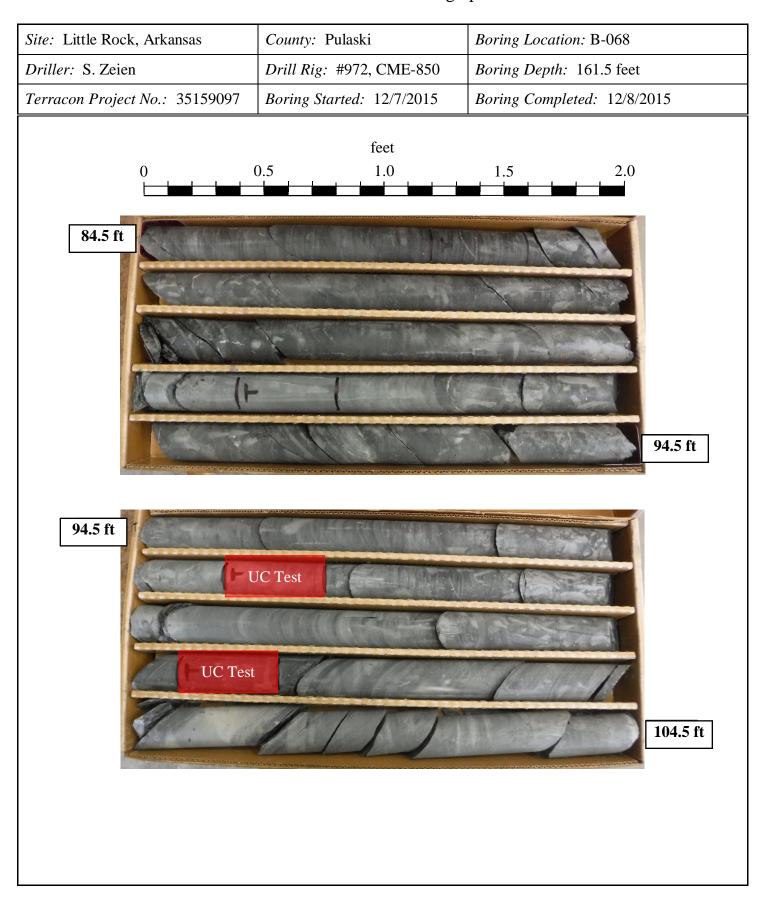
	OJECT: AHTD Job No. CA060	2 - 30) Cro	ossi	ng		CLI	ENT:	Arka Tran	nsa: spo	s Stat rtatio	e Hig n Dep	hway a ot.	and			
SIT	E: Little Rock Area Pulaski County, Arkar	ารลร															
(J	LOCATION See Exhibit A-12		ı م	ш	(;				~		STREN	GTH TES	T	_		ATTERBERG LIMITS	
GRAPHIC LOG	Latitude: 34.74983° Longitude: -92.262528°	(Ft.)	WATER LEVEL OBSERVATIONS	SAMPLE TYPE	RECOVERY (In.)	Щ	EST .TS	TOTAL ROCK UNIT WEIGHT (pcf)	LABORATORY HP (tsf)	ш		Ľ≥⊥		WATER CONTENT (%)	SOIL DRY UNIT WEIGHT (pcf)	LIVITS	PERCENT FINES
PHIC	-	DEPTH (Ft.)	ERL	Ы	OVEF	SAMPLE ID	FIELD TEST RESULTS	GHT C	DRA ⁻ HP (ts	TEST TYPE	Is50-POINT LOAD INDEX (tsf)	COMPRESSIVE STRENGTH (tsf)	UNDRAINED SHEAR STRENGTH (tsf)	VATE	SOII 3Y UI GHT	LL-PL-PI	
GRA	Northing: 2070540.42 Easting: 1233497.18 Surface Elev.: 233.3 (Ft.)	B	WAT	AM	RECO	Ś	빌聪	L OR ME	LAB	TEST	S50-F OAD	TRE (1)	NDR SHI (†	> <u>~</u>	MEI		
	DEPTH ELEVATION (Ft.) SHALE (continued)	_			ш. 					'		8	~,				
	gray, wide fracture spacing,	145				RUN	REC = 98%										
	unweathered, angled	145				11	RQD = 98%										
	fractures	-	1														
		-	1														
			1														
		-	-														
		150	-														-
			-							PL ,	8.4						F
		-	-														
		-															
		_															
		155				RUN	REC = 99%										
		155				12	RQD =										
		-	1				99%										
		-	1														
		-	1														
		160-	-					170				20					
	161.5 72	-	-			RUN 13	REC = 100%			UC PL	14.8	39					
	Boring Terminated at 161.5	1					RQD = 94%										
	Feet						94 /0	/									
	Total depth of hole: 161.5 Feet									<u> </u>	ammer Tu	ne: Auto	matic 140	lh hamr	ner with	a 30-inch drop	
	Stratification lines are approximate. In-situ, the tra	nsition m	ay be g	radua	ıl.					Er	nergy Trar	sfer Rati	o (ETR) = 7	6.3%			-,
	cement Method:	:+	ŀ	lorizo	ntal Da	atum: NA	D 83 (Y.X L	oczalized)	No	tes:						
	45.5 Feet: Wash Rotary with 3-7/8" Tricone Roller B 5 to 161.5 Feet: NQ Core Barrel	il.		/ertica	I Datur	m: NAVE) 88										
\h	anna an Martha al)2 Arkansa	South									
Abando	onment Method:							ຣວບແກ									
				survey	ea by l	NTB Ass	uciates			-							
	40.5' from water surface to mudline		-								ng Starteo			-		leted: 12-08-20	015
	TO.5 ITOITI WALEF SUITACE LO ITIUUIIITE		_					_U			Rig: 977, cked by: F			-	er: S. Zei		
). Tennison	







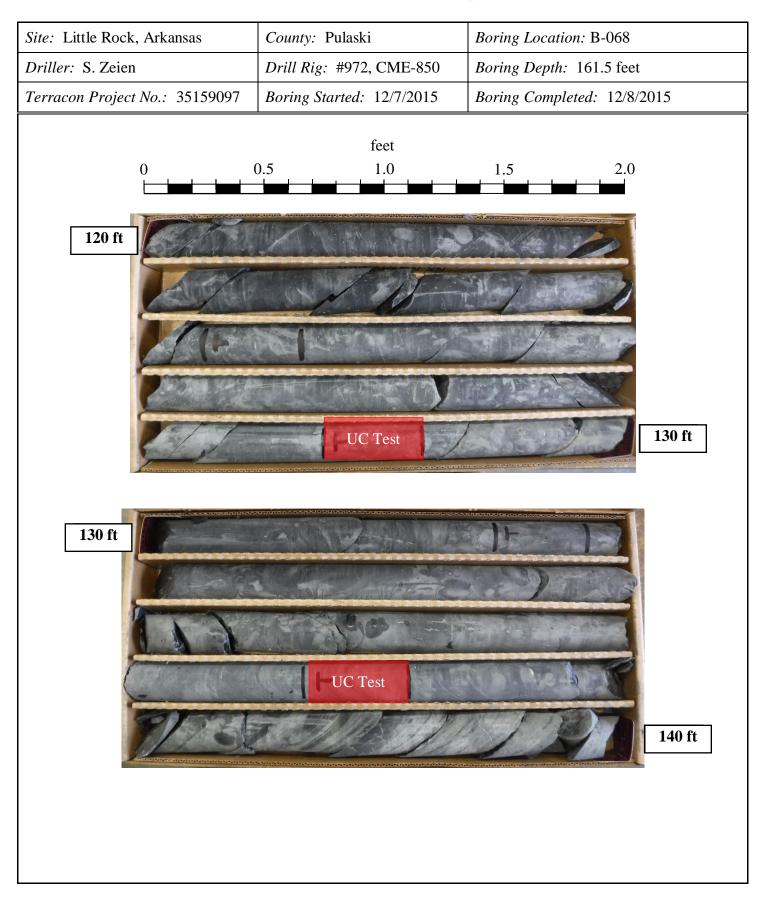




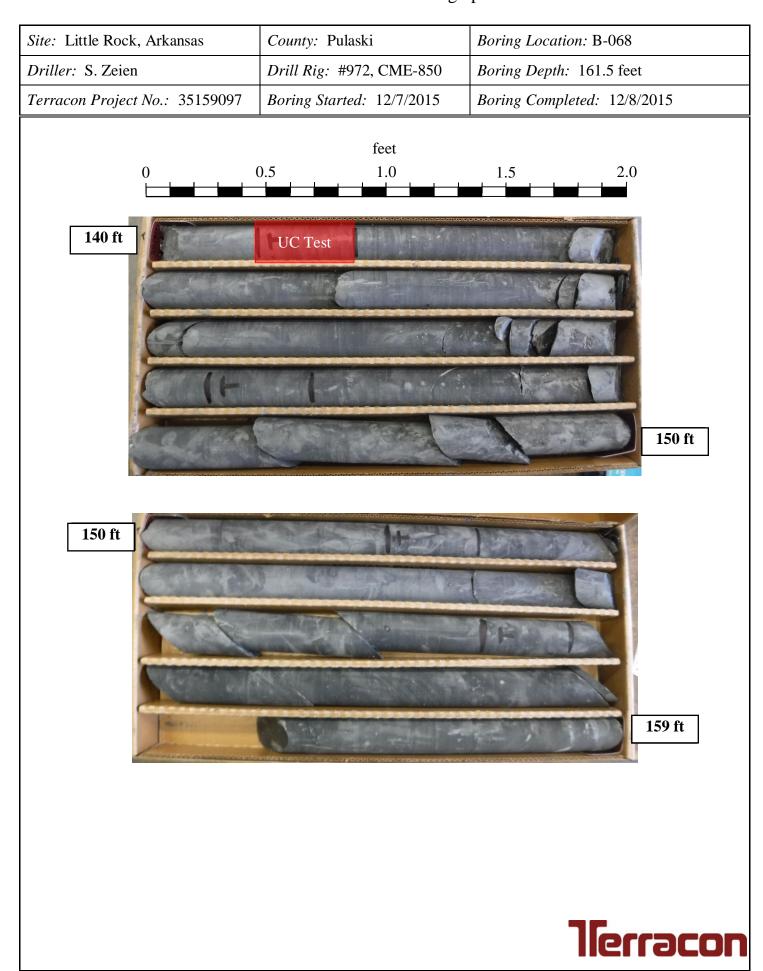


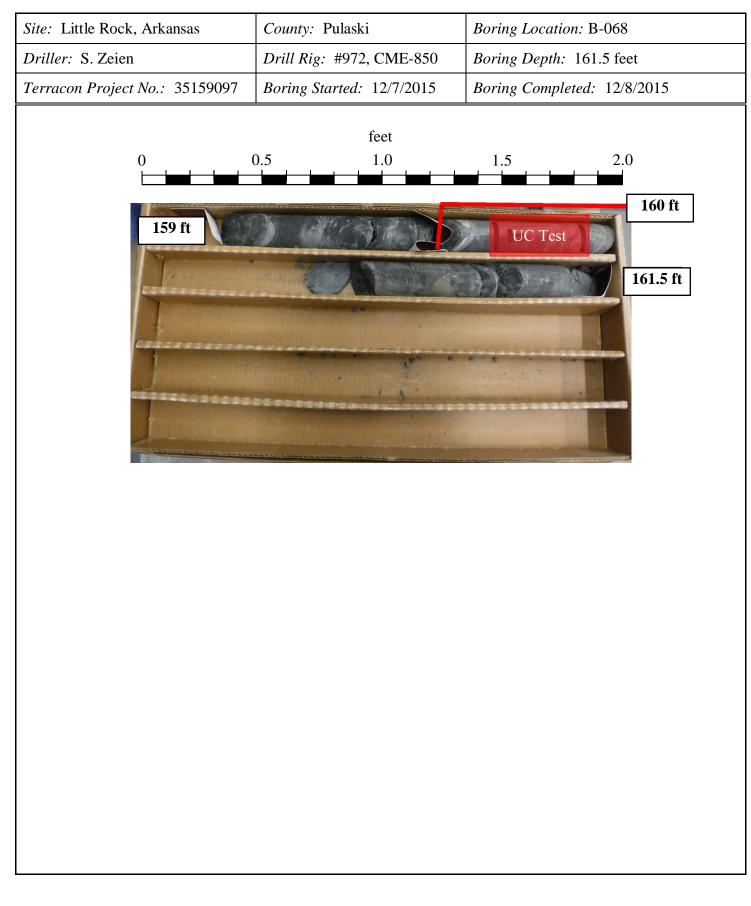








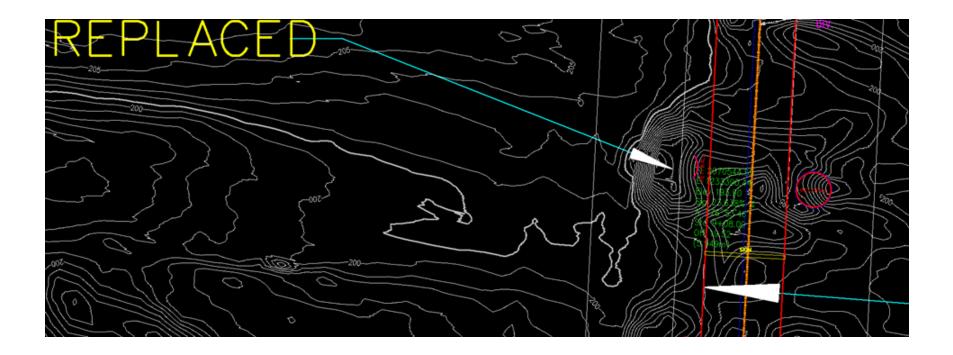






ATTACHMENT C SURVEY CONTOUR PLOT (ArDOT 2019)

Survey Contour Plot (ARDOT 2019)



ATTACHMENT D GEOTECHNICAL ANALYSIS

		ArDOT Pier Protection I	-30 Cell Replacement	Job Number:	63136		
	1ade By:	BDW		Check By: J	MS	BackCheck By: B	BDW
	ate:	1/9/2020		Date: 2	/22/2020	Date: 2	1/23/2020
Top of	PIER O f Cell Elevation	SEOMETRY AND DESIGN 251.5 ft	N CONDITIONS As-Built Plans			Load Factor	AAS s N
•	Shale Elevation	190 ft	Boring B-68 (2015 date)			Load Tactor	
	r Level Surface	231 ft	Case 1: Design Criteria				cv
	t water surface	247.2 ft	Case 2: Design Criteria				WA
Normal Operation	-	2800 kips	Design Criteria; force ap	nlied 5ft above norm	al water level		
Drifting Empty Barg	-	1440 kips	Design Criteria			Resistance F	actors AAS
	-ill Unit Weight	150 pcf	As-Built plans used 150 p	ncf "quarry stone"		Resistance	
Presumptive Shale Bear	Ű,	20 ksf	AASHTO Table C10.6.2.6				
	ter Force (WA)	170 kips	Based on water velocity		in Vessel Collision Stu	ıdv	L
	Cell Diameter	40 ft	As-Built Plans			2 ~ y	
Tr	otal Cell Height	61.5 ft					
	Area	1257 ft ²					
	-						
	merged Height	vent with Normal Opera 41 ft	ation Design Force	(ASE 2: 10 yr High Submerged Height	Water Event with Drifti 57.2 ft	ing Empty Barg
	ht above water	20.5 ft					
					Height above water	$4 \prec TT$	
-	-				Height above water		
Subn	merged Weight	4,513,338 lb			Submerged Weight	6,296,656 lb	
Subn	merged Weight ht above water	4,513,338 lb 3,864,159 lb			Submerged Weight Weight above water	6,296,656 lb 810,531 lb	
Subn	merged Weight	4,513,338 lb			Submerged Weight	6,296,656 lb 810,531 lb	
Subn	merged Weight ht above water	4,513,338 lb 3,864,159 lb	Allowable e = 18 ft		Submerged Weight Weight above water	6,296,656 lb 810,531 lb 7,107,187 lb	
Subn	nerged Weight ht above water Total Weight eccentricity	4,513,338 lb 3,864,159 lb 8,377,497 lb 15.6 ft	Allowable e = 18 ft g Moments) / Total Weight		Submerged Weight Weight above water Total Weight	6,296,656 lb 810,531 lb 7,107,187 lb	
Subn	nerged Weight ht above water Total Weight eccentricity	4,513,338 lb 3,864,159 lb 8,377,497 lb 15.6 ft		Bearing Capacit	Submerged Weight Weight above water Total Weight	6,296,656 lb 810,531 lb 7,107,187 lb	
Subn Weigh	nerged Weight ht above water Total Weight eccentricity at service limit	4,513,338 lb 3,864,159 lb 8,377,497 lb 15.6 ft e = (Sum of Overturning 3.0			Submerged Weight Weight above water Total Weight eccentricity	6,296,656 lb 810,531 lb 7,107,187 lb 12.5 ft 3.5	
Subn Weigh Bearing Capacity FOS LRFD BEARING CAP	nerged Weight ht above water Total Weight eccentricity at service limit	4,513,338 lb 3,864,159 lb 8,377,497 lb 15.6 ft e = (Sum of Overturning 3.0			Submerged Weight Weight above water Total Weight eccentricity y FOS at service limit	6,296,656 lb 810,531 lb 7,107,187 lb 12.5 ft 3.5	
Subn Weigh Bearing Capacity FOS LRFD BEARING CAPA	nerged Weight ht above water Total Weight eccentricity at service limit ACITY AT STREN	4,513,338 lb 3,864,159 lb 8,377,497 lb 15.6 ft e = (Sum of Overturning 3.0 GTH LIMIT STATE			Submerged Weight Weight above water Total Weight eccentricity y FOS at service limit	6,296,656 lb 810,531 lb 7,107,187 lb 12.5 ft 3.5 RENGTH LIMIT STATE	
Subn Weigh Bearing Capacity FOS LRFD BEARING CAP/	nerged Weight ht above water Total Weight eccentricity at service limit ACITY AT STREN Nominal Load	4,513,338 lb 3,864,159 lb 8,377,497 lb 15.6 ft e = (Sum of Overturning 3.0 GTH LIMIT STATE 6666.6 lb/ft2			Submerged Weight Weight above water Total Weight eccentricity ty FOS at service limit RING CAPACITY AT STR Nominal Load	6,296,656 lb 810,531 lb 7,107,187 lb 12.5 ft 3.5 RENGTH LIMIT STATE 5655.72 lb/ft2	
Subn Weigh Bearing Capacity FOS LRFD BEARING CAP/ Nomi	merged Weight ht above water Total Weight eccentricity at service limit ACITY AT STREN Nominal Load Factored Load	4,513,338 lb 3,864,159 lb 8,377,497 lb 15.6 ft e = (Sum of Overturning 3.0 GTH LIMIT STATE 6666.6 lb/ft2 8333.25 lb/ft2			Submerged Weight Weight above water Total Weight eccentricity Y FOS at service limit RING CAPACITY AT STR Nominal Load Factored Load	6,296,656 lb 810,531 lb 7,107,187 lb 12.5 ft 3.5 RENGTH LIMIT STATE 5655.72 lb/ft2 7069.65 lb/ft2 20000 lb/ft2	
Subn Weigh Bearing Capacity FOS LRFD BEARING CAP/ Nomi	merged Weight ht above water Total Weight eccentricity at service limit ACITY AT STREN Nominal Load Factored Load inal Resistance ored Resistance	4,513,338 lb 3,864,159 lb 8,377,497 lb 15.6 ft e = (Sum of Overturning 3.0 GTH LIMIT STATE 6666.6 lb/ft2 8333.25 lb/ft2 20000 lb/ft2			Submerged Weight Weight above water Total Weight eccentricity Y FOS at service limit RING CAPACITY AT STF Nominal Load Factored Load Nominal Resistance	6,296,656 lb 810,531 lb 7,107,187 lb 12.5 ft 3.5 RENGTH LIMIT STATE 5655.72 lb/ft2 7069.65 lb/ft2 20000 lb/ft2	
Subn Weigh Bearing Capacity FOS LRFD BEARING CAPA Nomi Facto	merged Weight ht above water Total Weight eccentricity at service limit ACITY AT STREN Nominal Load Factored Load inal Resistance ored Resistance	4,513,338 lb 3,864,159 lb 8,377,497 lb 15.6 ft e = (Sum of Overturning 3.0 GTH LIMIT STATE 6666.6 lb/ft2 8333.25 lb/ft2 20000 lb/ft2 9000 lb/ft2			Submerged Weight Weight above water Total Weight eccentricity Ty FOS at service limit RING CAPACITY AT STF Nominal Load Factored Load Nominal Resistance Factored Resistance	6,296,656 lb 810,531 lb 7,107,187 lb 12.5 ft 3.5 RENGTH LIMIT STATE 5655.72 lb/ft2 7069.65 lb/ft2 20000 lb/ft2 9000 lb/ft2	
Subn Weigh Bearing Capacity FOS LRFD BEARING CAP/ Nomi Facto	merged Weight ht above water Total Weight eccentricity at service limit ACITY AT STREN Nominal Load Factored Load inal Resistance ored Resistance SLIDING	4,513,338 lb 3,864,159 lb 8,377,497 lb 15.6 ft e = (Sum of Overturning 3.0 GTH LIMIT STATE 6666.6 lb/ft2 8333.25 lb/ft2 20000 lb/ft2 9000 lb/ft2 AASHTO 10.6.3.4			Submerged Weight Weight above water Total Weight eccentricity ty FOS at service limit RING CAPACITY AT STF Nominal Load Factored Load Nominal Resistance Factored Resistance SLIDING	6,296,656 lb 810,531 lb 7,107,187 lb 12.5 ft 3.5 RENGTH LIMIT STATE 5655.72 lb/ft2 7069.65 lb/ft2 20000 lb/ft2 9000 lb/ft2 1,610,410 lb	
Subn Weigh Bearing Capacity FOS LRFD BEARING CAP Nomi Facto	merged Weight ht above water Total Weight eccentricity at service limit ACITY AT STREN Nominal Load Factored Load inal Resistance ored Resistance SLIDING Nominal Load	4,513,338 lb 3,864,159 lb 8,377,497 lb 15.6 ft e = (Sum of Overturning 3.0 GTH LIMIT STATE 6666.6 lb/ft2 8333.25 lb/ft2 20000 lb/ft2 9000 lb/ft2 AASHTO 10.6.3.4 2,970,410 lb			Submerged Weight Weight above water Total Weight eccentricity Ty FOS at service limit Nominal Load Factored Load Nominal Resistance Factored Resistance SLIDING Nominal Load	6,296,656 lb 810,531 lb 7,107,187 lb 12.5 ft 3.5 RENGTH LIMIT STATE 5655.72 lb/ft2 7069.65 lb/ft2 20000 lb/ft2 9000 lb/ft2 1,610,410 lb 1,610,410 lb	

AASHTO 11.6.3.3 Eccentricity limit for foundation on rock, the location of the resultant should be within nine-tenths of the base width. Eccentricity must be less than 18 ft for a 40-ft diameter cell Eccentricity calc is conservative as Water Force (WA) is from 100-yr velocity

Water force assumes uniform water velocity for height of river and is based on 100-yr velocity

ble 3.4.1-1 and 3.4.1-2

Dead Loads Vessel collision force Water load and stream pressure

able 10.5.5.2.2-1 Bearing Resistance Sliding Resistance

n Force