

"A FULLY CONTROLLED ACCESS FACILITY"
ARKANSAS STATE HIGHWAY AND TRANSPORTATION DEPARTMENT
CONSTRUCTION PLANS FOR STATE HIGHWAY

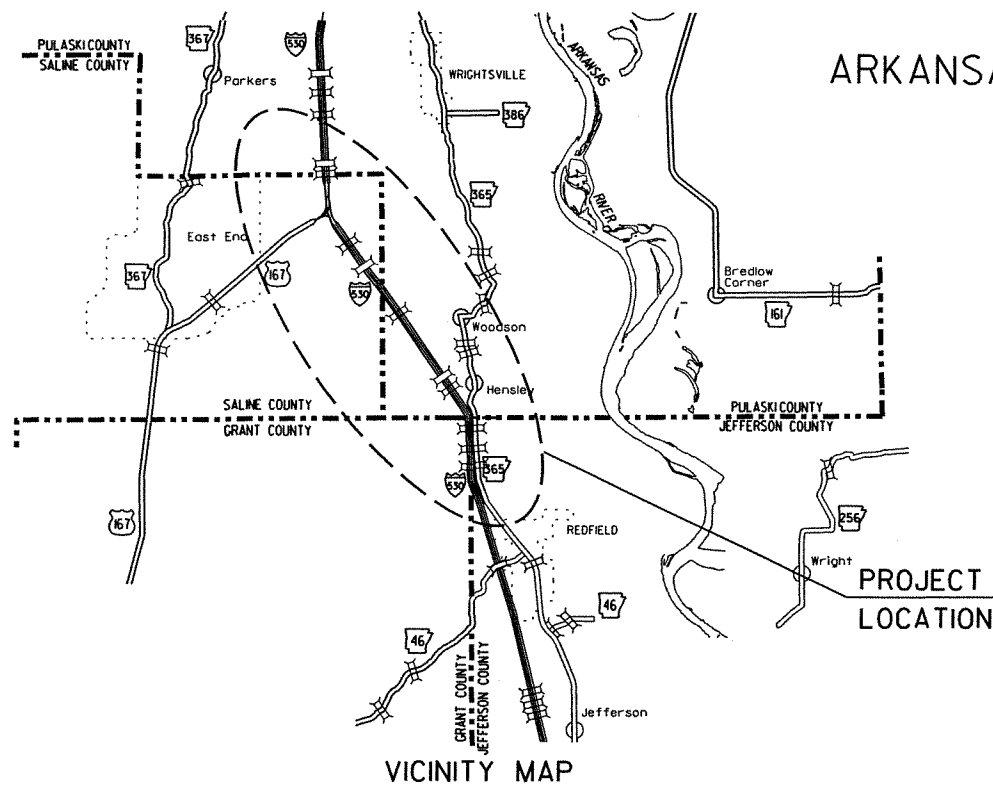
BINGHAM RD. - GRANT
CO. LINE (S)

PULASKI & SALINE COUNTIES
ROUTE 530 SECTION 1, 2, & 3

JOB BB0614

FEDERAL AID PROJ.
BIM-B530(204) & HSIP-530-I(5)9

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB NO.		BB0614	1	219
				BINGHAM RD. - GRANT CO. LINE (S)				



VICINITY MAP
BRIDGE DATA

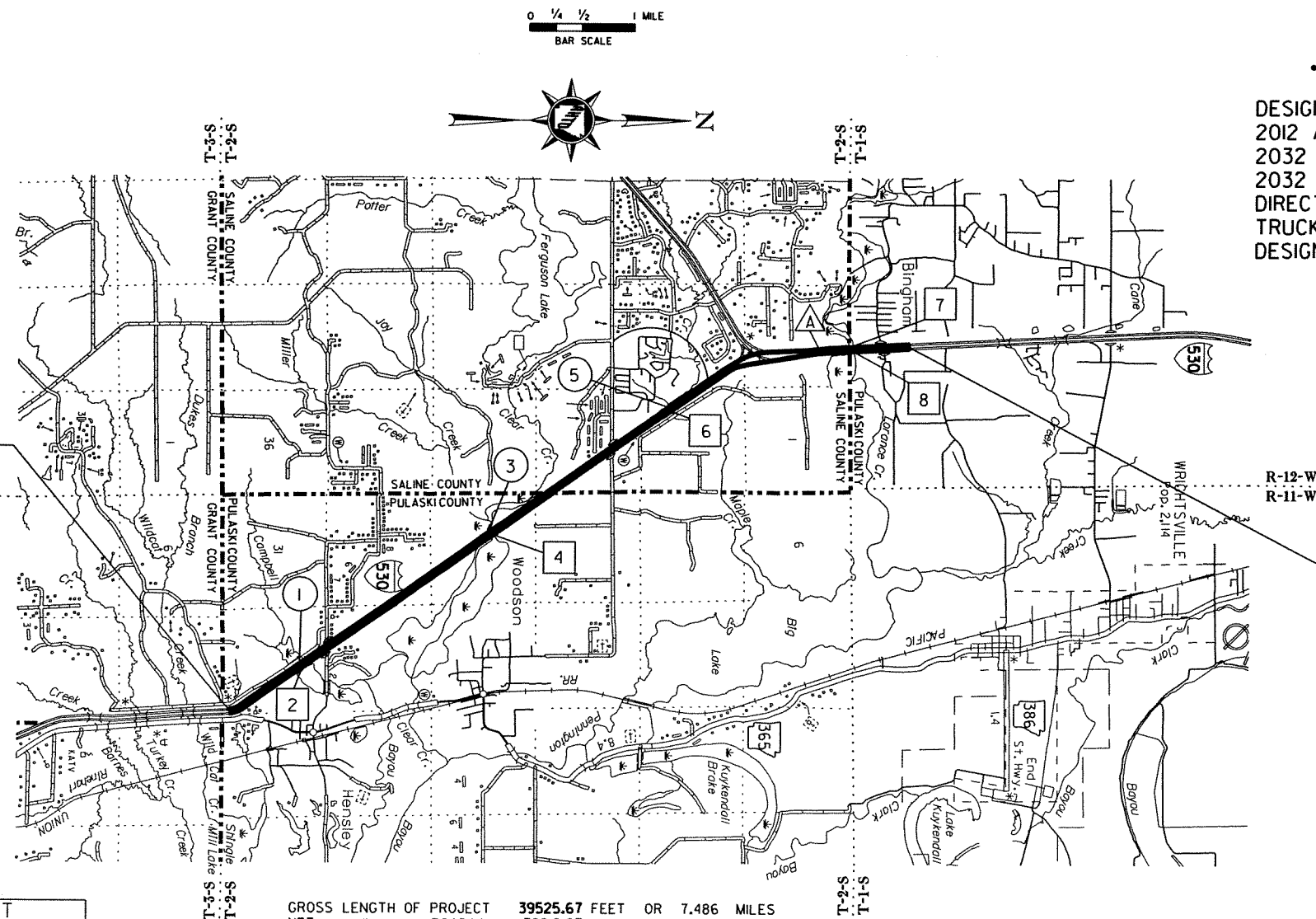
- | | |
|--|---|
| <p>2 STA. 58+55.00 BR. END
90' BRIDGE NO. B3323
40'-0" CLEAR ROADWAY
STA. 59+45.00 BR. END
REHABILITATE BRIDGE DECK-
HYDRODEMOLITION</p> <p>4 STA. 175+05.97 BR. END
151'-6" CONTINUOUS R.C.
SLAB UNIT (30.75' -
3 @ 30' - 30.75')
40'-0" CLEAR ROADWAY
BRIDGE NO. B7266
STA. 176+57.47 BR. END</p> <p>6 STA. 285+69.95 BR. END
90' BRIDGE NO. B3326
40'-0" CLEAR ROADWAY
STA. 286+59.95 BR. END
REHABILITATE BRIDGE DECK-
HYDRODEMOLITION</p> | <p>7 STA. 385+67.12 BR. END
181'-6" CONTINUOUS R.C.
SLAB UNIT (30.75' -
4 @ 30' - 30.75')
48'-0" CLEAR ROADWAY
BRIDGE NO. A7267
STA. 387+48.62 BR. END</p> <p>8 STA. 385+67.12 BR. END
181'-6" CONTINUOUS R.C.
SLAB UNIT (30.75' -
4 @ 30' - 30.75')
40'-0" CLEAR ROADWAY
BRIDGE NO. B7267
STA. 387+48.62 BR. END</p> |
|--|---|

TOTAL LENGTH OF BRIDGES
513.00' MEASURED ALONG C
EXCEPTIONS TO JOB BB0614
(BRIDGES)

- | |
|---|
| <p>1 STA. 58+55.00 BR. END
90' BRIDGE NO. A3323
40'-0" CLEAR ROADWAY
STA. 59+45.00 BR. END
RETAIN</p> <p>3 STA. 175+16.67 BR. END
150.00' BRIDGE NO. A3324
40'-0" CLEAR ROADWAY
STA. 176+66.67 BR. END
RETAIN</p> <p>5 STA. 285+66.25 BR. END
90' BRIDGE NO. A3326
40'-0" CLEAR ROADWAY
STA. 286+56.25 BR. END
RETAIN</p> |
|---|

STA. 11+48.68-BEGIN
JOB BB0614
LOG MILE 16.28

EQUATIONS
A STA. 349+84.42 BACK=
STA. 350+00.00 AHEAD



• DESIGN TRAFFIC DATA •

DESIGN YEAR	2012
2012 ADT	20,000
2032 ADT	26,000
2032 DHV	2,860
DIRECTIONAL DISTRIBUTION	60%
TRUCKS	12%
DESIGN SPEED	70 MPH

STA. 406+89.93-END
JOB BB0614
LOG MILE 8.80

APPROVED

STATE OF
ARKANSAS
REGISTERED
PROFESSIONAL
ENGINEER
No. 3917
FRANK VOZEL
10/23/12
DEPUTY DIRECTOR
AND CHIEF ENGINEER

GROSS LENGTH OF	PROJECT	39525.67 FEET	OR	7.486 MILES
NET	ROADWAY	39012.67		7.389
NET	BRIDGES	513.00		0.097
NET	PROJECT	39525.67		7.486

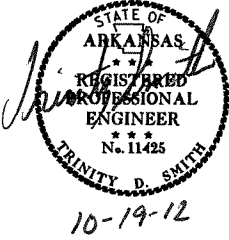
P.E. JOB BB0614
NON-PART.

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9/20/2012

RB0614.DGN

GOVERNING SPECIFICATIONS

ARKANSAS STATE HIGHWAY COMMISSION STANDARD SPECIFICATIONS FOR HIGHWAY CONSTRUCTION, EDITION OF 2003, AND THE FOLLOWING SPECIAL PROVISIONS AND SUPPLEMENTAL SPECIFICATIONS:

NUMBER	TITLE
ERRATA_____	ERRATA FOR THE BOOK OF STANDARD SPECIFICATIONS
FHWA-1273__	REQUIRED CONTRACT PROVISIONS FEDERAL-AID CONSTRUCTION CONTRACTS
FHWA-1273__	SUPPLEMENT - EQUAL EMPLOYMENT OPPORTUNITY - NOTICE TO CONTRACTORS
FHWA-1273__	SUPPLEMENT - SPECIFIC EQUAL EMPLOYMENT OPPORTUNITY RESPONSIBILITIES (23 U.S.C. 140)
FHWA-1273__	SUPPLEMENT - EQUAL EMPLOYMENT OPPORTUNITY - GOALS AND TIMETABLES
FHWA-1273__	SUPPLEMENT - EQUAL EMPLOYMENT OPPORTUNITY - FEDERAL STANDARDS
FHWA-1273__	SUPPLEMENT - TRAINING PROGRAM - JOB BB0614
FHWA-1273__	SUPPLEMENT - POSTERS AND NOTICES REQUIRED FOR FEDERAL-AID PROJECTS
FHWA-1273__	SUPPLEMENT - WAGE RATE DETERMINATION
100-2_____	MANUAL FOR ASSESSING SAFETY HARDWARE (MASH)
102-1_____	BIDDING REQUIREMENTS AND CONDITIONS
103-1_____	DETERMINATION OF DBE PARTICIPATION
105-1_____	CONSTRUCTION CONTROL MARKINGS
105-2_____	EQUIPMENT AND MATERIAL STORAGE ON BRIDGE STRUCTURES
105-3_____	CONTROL OF WORK
107-1_____	WORKER VISIBILITY
108-1_____	LIQUIDATED DAMAGES
110-1_____	PROTECTION OF WATER QUALITY AND WETLANDS
303-1_____	AGGREGATE BASE COURSE
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JOB BB0614_	EXTENSION FOR PIPE CULVERTS
JOB BB0614_	GOALS FOR DISADVANTAGED BUSINESS ENTERPRISE PARTICIPATION
JOB BB0614_	HIGH PERFORMANCE PAVEMENT MARKING
JOB BB0614_	HYDRODEMOLITION
JOB BB0614_	INTERNET BIDDING
JOB BB0614_	LATEX MODIFIED CONCRETE OVERLAY
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JOB BB0614_	WIRE ROPE SAFETY FENCE MAINTENANCE MATERIALS
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JOB BB0614_	WRSF TRAINING WORKSHOP

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2 GOVERNING SPECIFICATIONS AND GENERAL NOTES

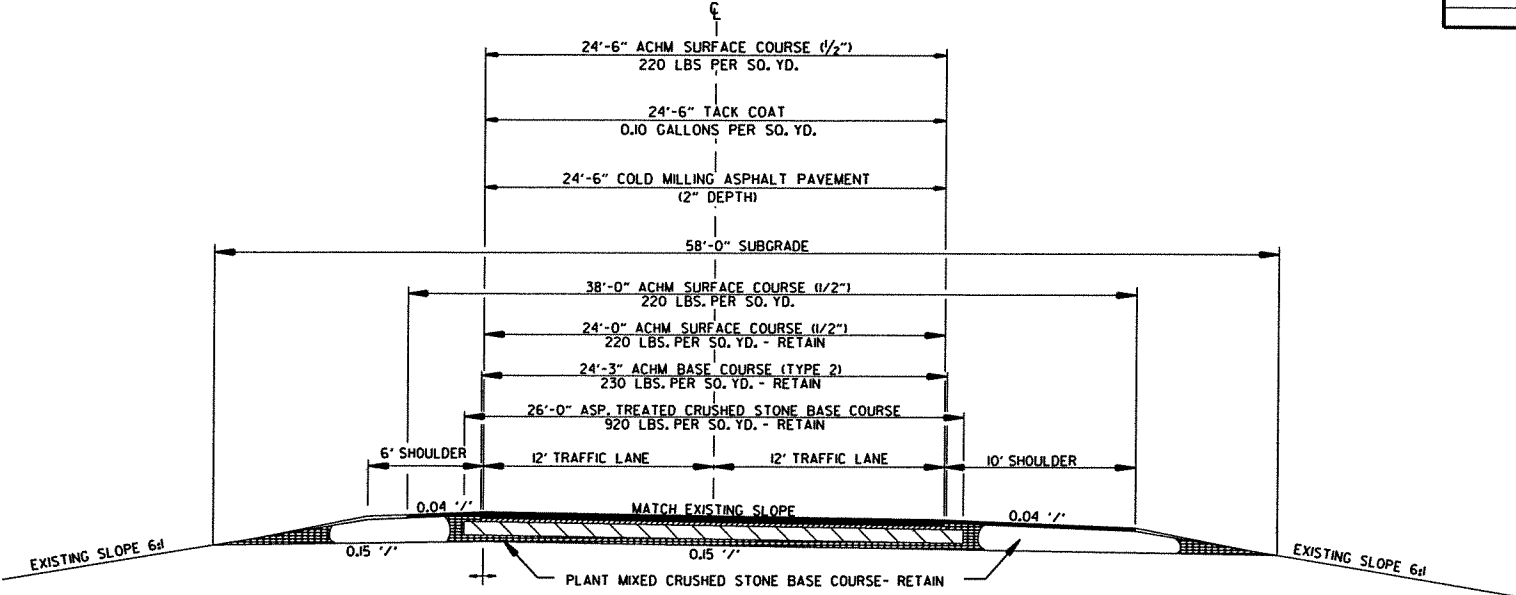
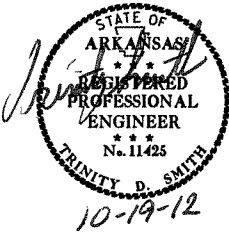


GENERAL NOTES

- GRADE LINE DENOTES FINISHED GRADE WHERE SHOWN ON PLANS.
- ALL PIPE LINES, POWER, TELEPHONE AND TELEGRAPH LINES TO BE MOVED OR LOWERED BY THE RESPECTIVE OWNERS AS PER AGREEMENT WITH SUCH OWNERS.
- ANY EQUIPMENT OR APPURTENANCE THAT INTERFERES WITH THE PROPOSED CONSTRUCTION AND WHICH MAY BE THE PROPERTY OF UTILITY SERVICE ORGANIZATIONS SHALL BE MOVED BY THE OWNERS UNLESS OTHERWISE PROVIDED.
- ALL LAND MONUMENTS LOCATED WITHIN THE CONSTRUCTION AREA SHALL BE PROTECTED IN ACCORDANCE WITH SECTION 107.12 OF THE STANDARD SPECIFICATIONS.
- ALL TREES THAT DO NOT DIRECTLY INTERFERE WITH THE PROPOSED CONSTRUCTION SHALL BE SPARED AS DIRECTED BY THE ENGINEER. CARE AND DISCRETION SHALL BE USED TO INSURE THAT ALL TREES NOT TO BE REMOVED SHALL BE HARMED AS LITTLE AS POSSIBLE DURING THE CONSTRUCTION OPERATIONS.
- ALL FLEXIBLE BASE AND ASPHALTIC PAVEMENTS REMOVED SHALL BE PAID FOR UNDER THE ITEM NO. 210 - UNCLASSIFIED EXCAVATION.
- THE EXISTING ASPHALT PAVEMENT TO BE REMOVED FROM THE REMAINING PAVEMENT SHALL BE SEPARATED BY SAWING ALONG A NEAT LINE. AFTER SAWING, THE PAVEMENT TO BE REMOVED SHALL BE CAREFULLY REMOVED IN A MANNER THAT WILL NOT DAMAGE THE PAVEMENT THAT IS TO REMAIN. ANY DAMAGE OF THE ASPHALT PAVEMENT THAT IS TO REMAIN IN PLACE SHALL BE REPAIRED AT THE CONTRACTOR'S EXPENSE.

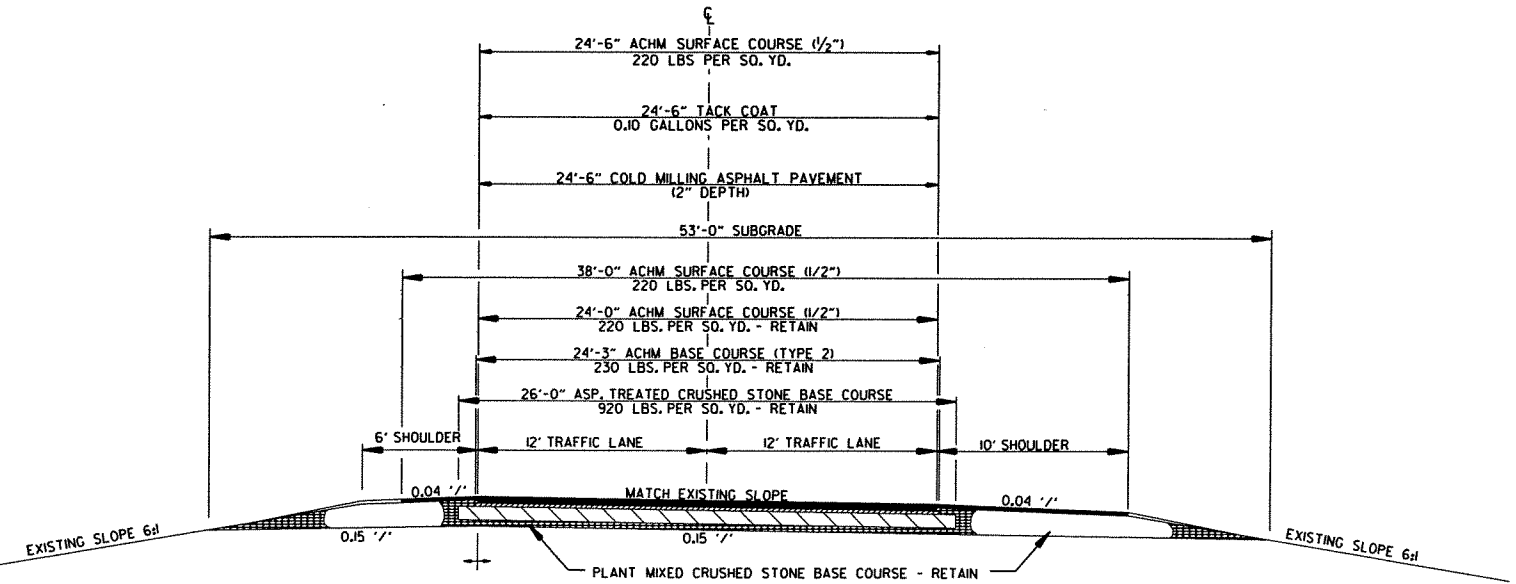
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2 TYPICAL SECTIONS OF IMPROVEMENT



INTERSTATE 530 - MILL & INLAY
(SHOWN IN DIRECTION OF TRAFFIC)

LEFT MAIN LANES	RIGHT MAIN LANES
STA. 22+42.76 TO STA. 58+55.00	STA. 22+42.76 TO STA. 58+55.00
STA. 59+44.00 TO STA. 80+00.00	STA. 59+45.00 TO STA. 80+00.00
STA. 120+00.00 TO STA. 175+16.67	STA. 120+00.00 TO STA. 175+14.97
STA. 176+66.67 TO STA. 285+66.25	STA. 176+66.67 TO STA. 285+69.95
STA. 286+56.25 TO STA. 311+59.80	STA. 286+59.95 TO STA. 311+59.80



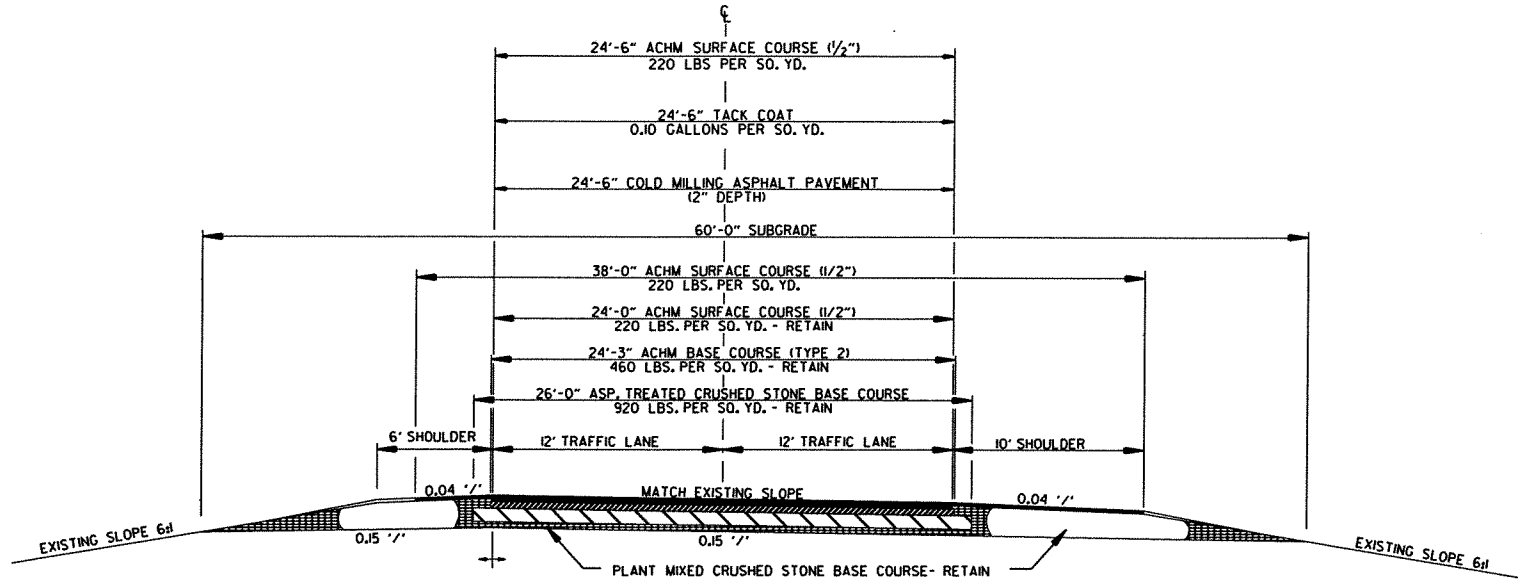
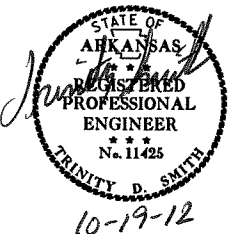
INTERSTATE 530 - MILL & INLAY
(SHOWN IN DIRECTION OF TRAFFIC)

LEFT MAIN LANES	RIGHT MAIN LANES
STA. 311+59.80 TO 406+89.93	STA. 311+59.80 TO STA. 406+89.93

TYPICAL SECTIONS OF IMPROVEMENT

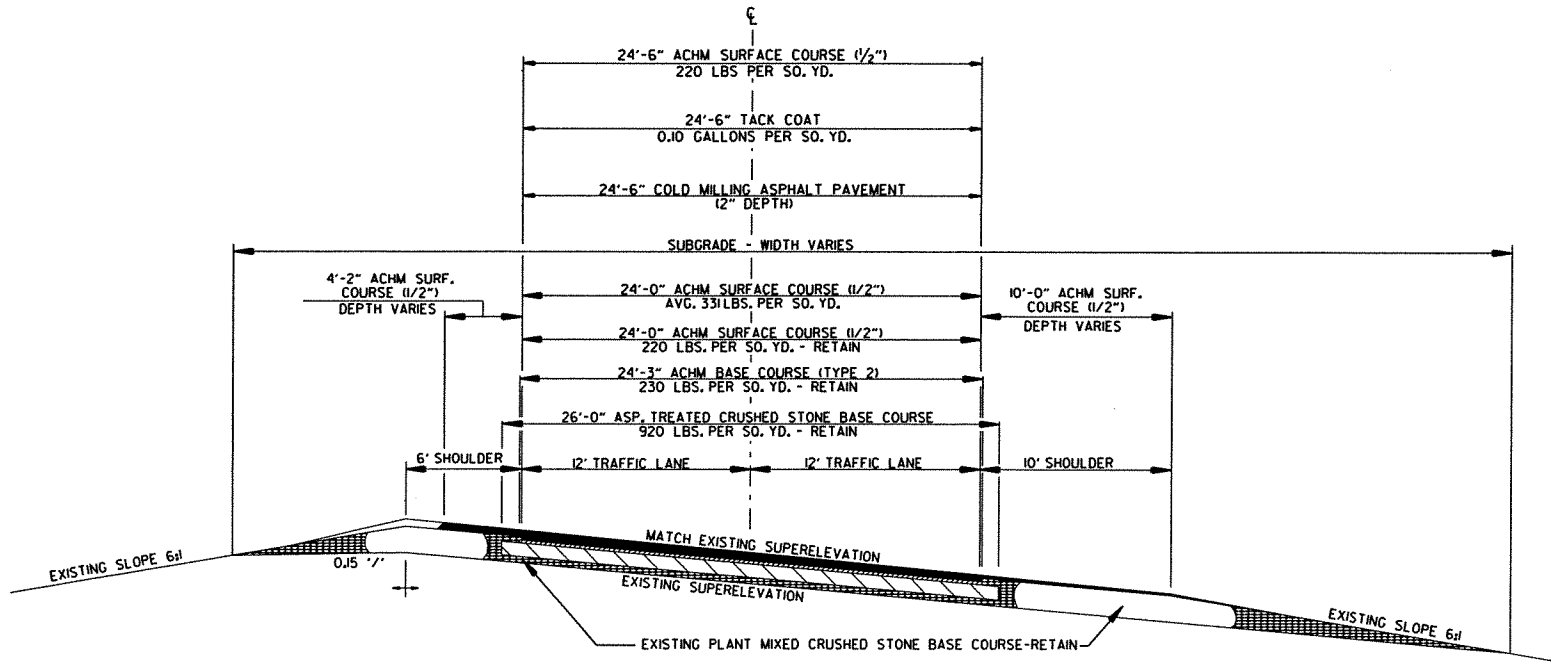
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2 TYPICAL SECTIONS OF IMPROVEMENT



INTERSTATE 530 - MILL & INLAY
(SHOWN IN DIRECTION OF TRAFFIC)

LEFT MAIN LANES STA. 80+00.00 TO STA. 120+00.00
RIGHT MAIN LANES STA. 80+00.00 TO STA. 120+00.00



INTERSTATE 530 WITH SUPERELEVATION - MILL & INLAY

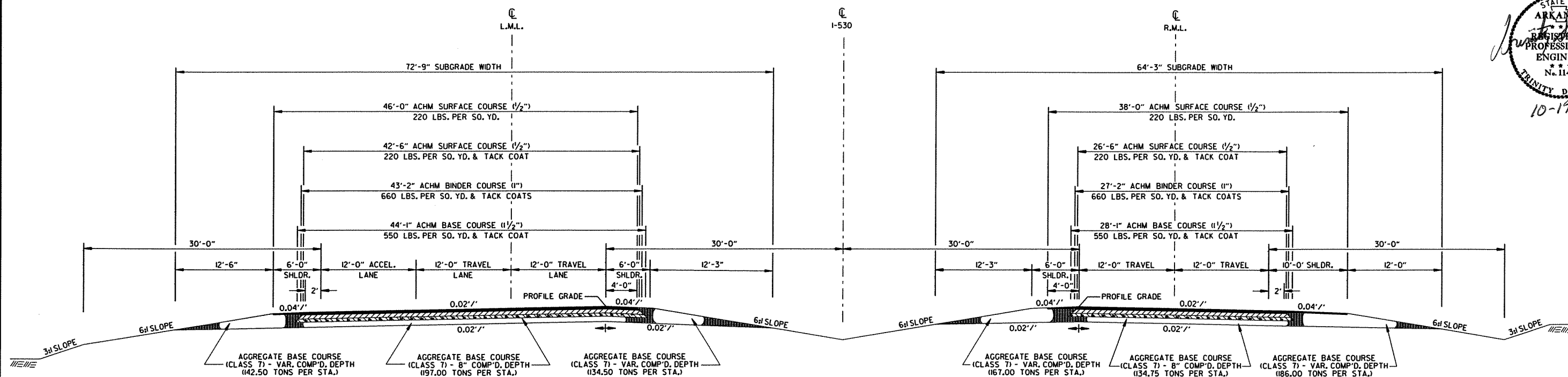
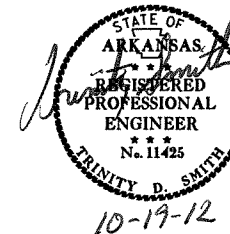
LEFT MAIN LANES STA. 11+48.68 TO STA. 22+42.76
RIGHT MAIN LANES STA. 11+48.68 TO STA. 22+42.76

TYPICAL SECTIONS OF IMPROVEMENT

10/19/2012
RBB0614.DGN

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2 TYPICAL SECTIONS OF IMPROVEMENT



INTERSTATE 530 - FULL DEPTH
(APPROACH RECONSTRUCTION OVER LORANCE CREEK)
STA. 381+90.00 TO STA. 389+52.49

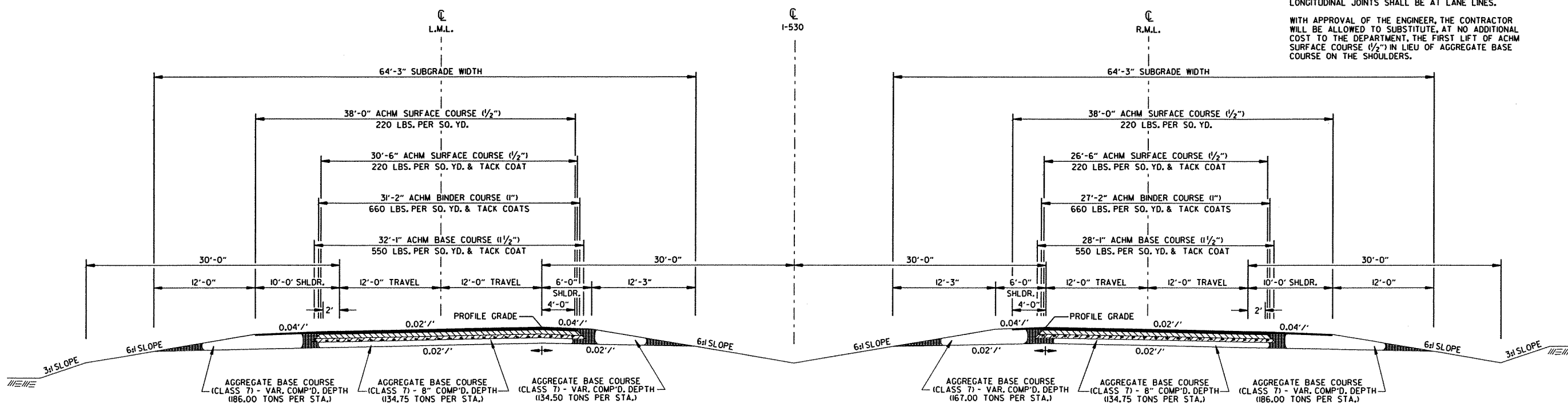
NOTES:

REFER TO CROSS SECTIONS FOR DEVIATION FROM THE NORMAL SLOPES. NO CHANGES SHALL BE MADE FROM THE PLANNED SLOPES WITHOUT THE APPROVAL OF THE ENGINEER.

THE THICKNESS OF AGGREGATE BASE COURSE SHALL BE WITHIN PLUS OR MINUS ONE INCH OF THE PLAN THICKNESS SHOWN. THE CONTRACTOR WILL CORRECT ANY DEFICIENT THICKNESS THAT DOES NOT MEET TOLERANCE INDICATED. PAYMENT WILL NOT BE MADE FOR MATERIAL PLACED IN EXCESS OF THE TOLERANCE INDICATED.

THE FINAL 2" OF SURFACE COURSE IS TO BE PLACED AFTER ALL OTHER COURSES HAVE BEEN LAID. LONGITUDINAL JOINTS SHALL BE AT LANE LINES.

WITH APPROVAL OF THE ENGINEER, THE CONTRACTOR WILL BE ALLOWED TO SUBSTITUTE, AT NO ADDITIONAL COST TO THE DEPARTMENT, THE FIRST LIFT OF ACHM SURFACE COURSE (1/2") IN LIEU OF AGGREGATE BASE COURSE ON THE SHOULDERS.



INTERSTATE 530 - FULL DEPTH
(APPROACH RECONSTRUCTION OVER LORANCE CREEK)
STA. 389+52.49 TO STA. 392+80.00

TYPICAL SECTIONS OF IMPROVEMENT

10/19/2012

RB0614.DGN

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
11-16-12				6	ARK.			
						JOB NO. BB0614	7	219

2 TYPICAL SECTIONS OF IMPROVEMENT



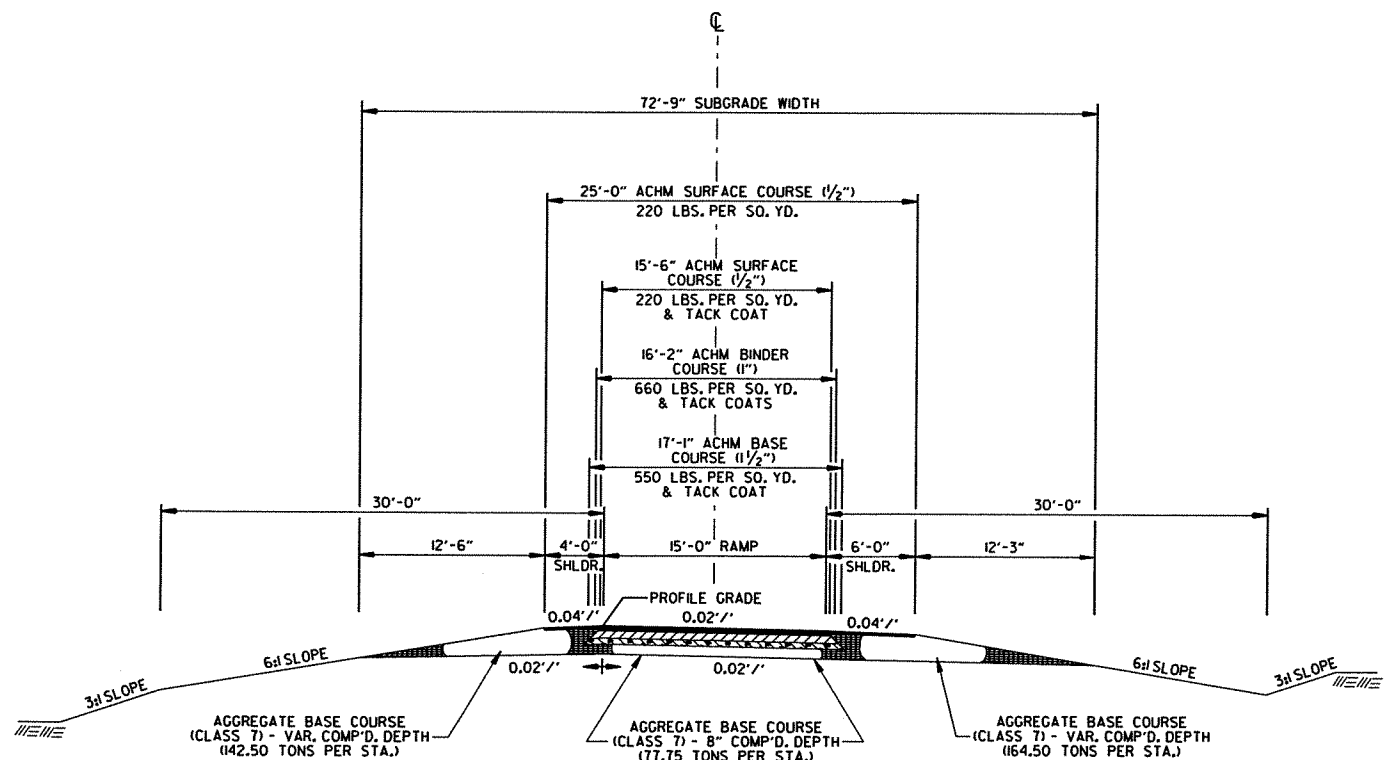
NOTES:

REFER TO CROSS SECTIONS FOR DEVIATION FROM THE NORMAL SLOPES. NO CHANGES SHALL BE MADE FROM THE PLANNED SLOPES WITHOUT THE APPROVAL OF THE ENGINEER.

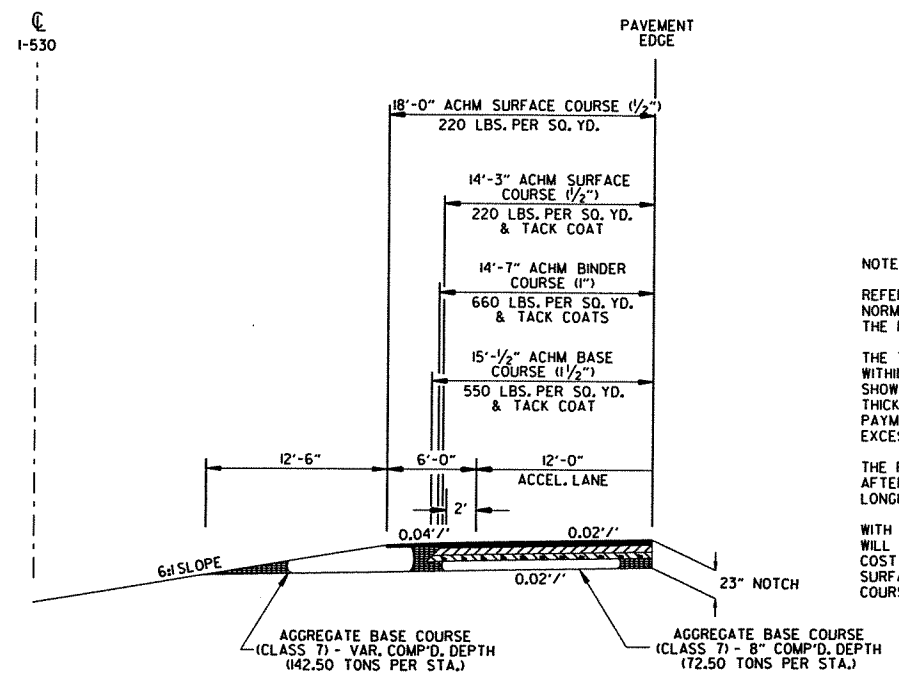
THE THICKNESS OF AGGREGATE BASE COURSE SHALL BE WITHIN PLUS OR MINUS ONE INCH OF THE PLAN THICKNESS SHOWN. THE CONTRACTOR WILL CORRECT ANY DEFICIENT THICKNESS THAT DOES NOT MEET TOLERANCE INDICATED. PAYMENT WILL NOT BE MADE FOR MATERIAL PLACED IN EXCESS OF THE TOLERANCE INDICATED.

THE FINAL 2" OF SURFACE COURSE IS TO BE PLACED AFTER ALL OTHER COURSES HAVE BEEN LAID. LONGITUDINAL JOINTS SHALL BE AT LANE LINES.

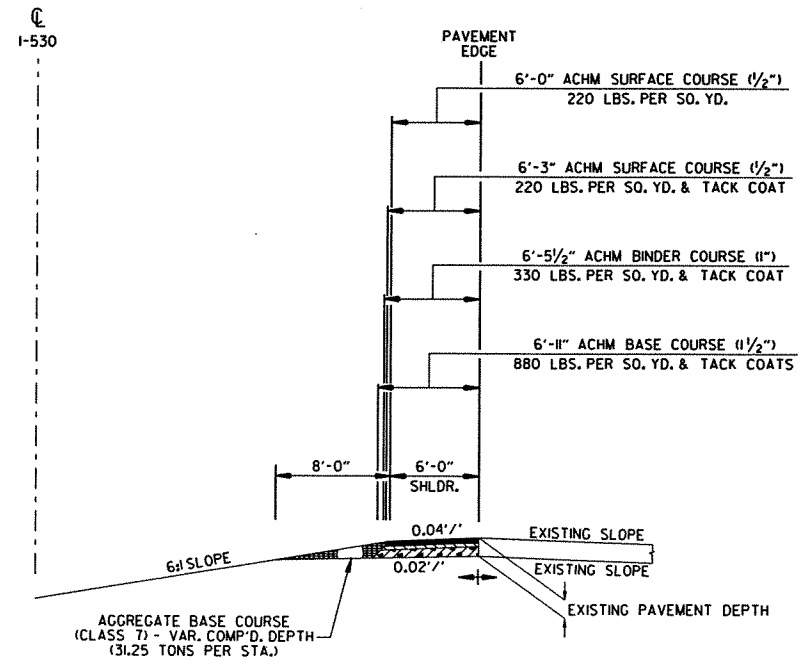
WITH APPROVAL OF THE ENGINEER, THE CONTRACTOR WILL BE ALLOWED TO SUBSTITUTE, AT NO ADDITIONAL COST TO THE DEPARTMENT, THE FIRST LIFT OF ACHM SURFACE COURSE (1/2") IN LIEU OF AGGREGATE BASE COURSE ON THE SHOULDERS.



INTERSTATE 530 ENTRANCE RAMP
AT BINGHAM RD. INTERCHANGE - RAMP 4
(SHOWN IN DIRECTION OF TRAFFIC)
STA. 389+52.49 TO STA. 397+55.95



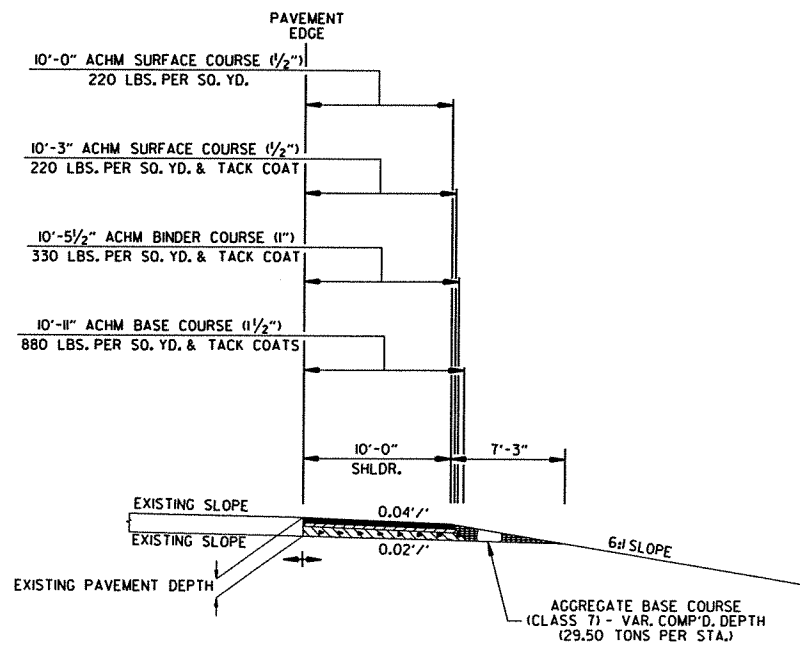
INTERSTATE 530 N.B. ACCELERATION LANE
AT HWY. 167 INTERCHANGE
(SHOWN IN DIRECTION OF TRAFFIC)
STA. 353+32.03 TO STA. 362+84.43



TEMPORARY SHOULDER RECONSTRUCTION FOR M.O.T.
(SHOWN IN DIRECTION OF TRAFFIC)

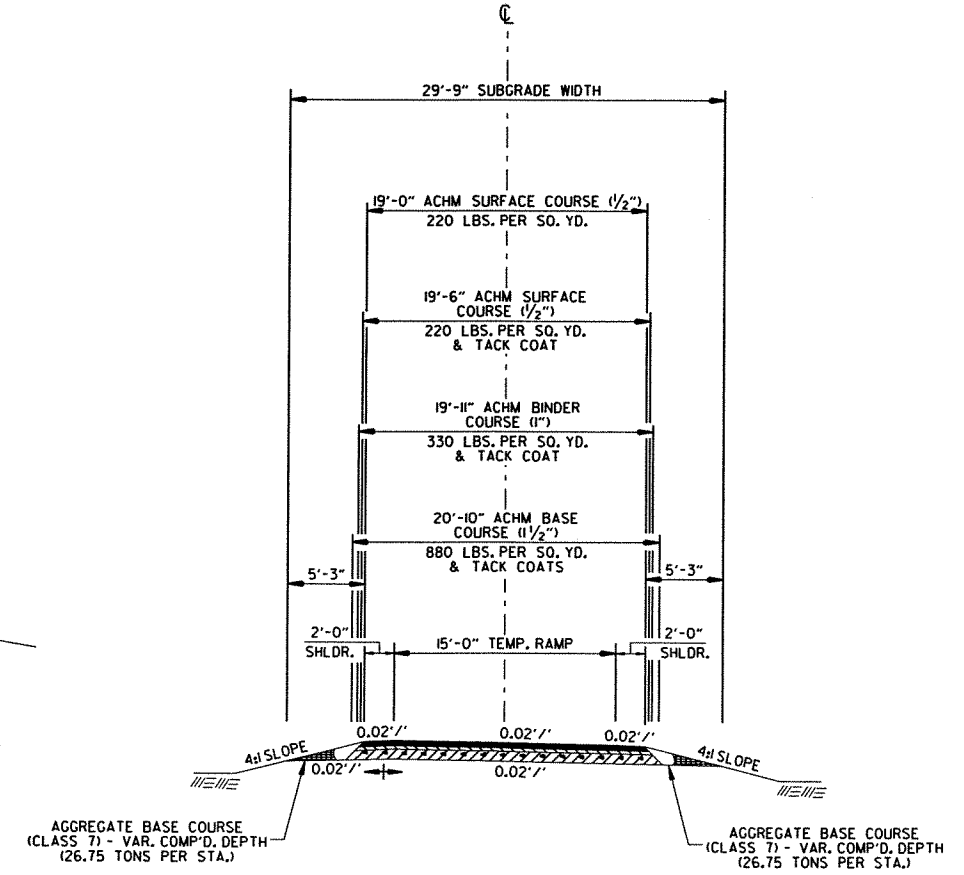
LEFT MAIN LANES
STA. 168+00.00 TO STA. 175+22.96
STA. 176+71.91 TO STA. 180+00.00

RIGHT MAIN LANES
STA. 56+20.00 TO STA. 58+20.00
STA. 59+80.00 TO STA. 60+60.00
STA. 283+18.00 TO STA. 285+18.00
STA. 286+80.00 TO STA. 287+60.00
STA. 380+00.00 TO STA. 385+56.72
STA. 394+00.00 TO STA. 402+00.00



TEMPORARY SHOULDER RECONSTRUCTION FOR M.O.T.
(SHOWN IN DIRECTION OF TRAFFIC)

RIGHT MAIN LANES
STA. 56+20.00 TO STA. 58+20.00
STA. 59+80.00 TO STA. 60+60.00
STA. 283+18.00 TO STA. 285+18.00
STA. 286+80.00 TO STA. 287+60.00



TEMPORARY RAMPS AND CROSSOVERS

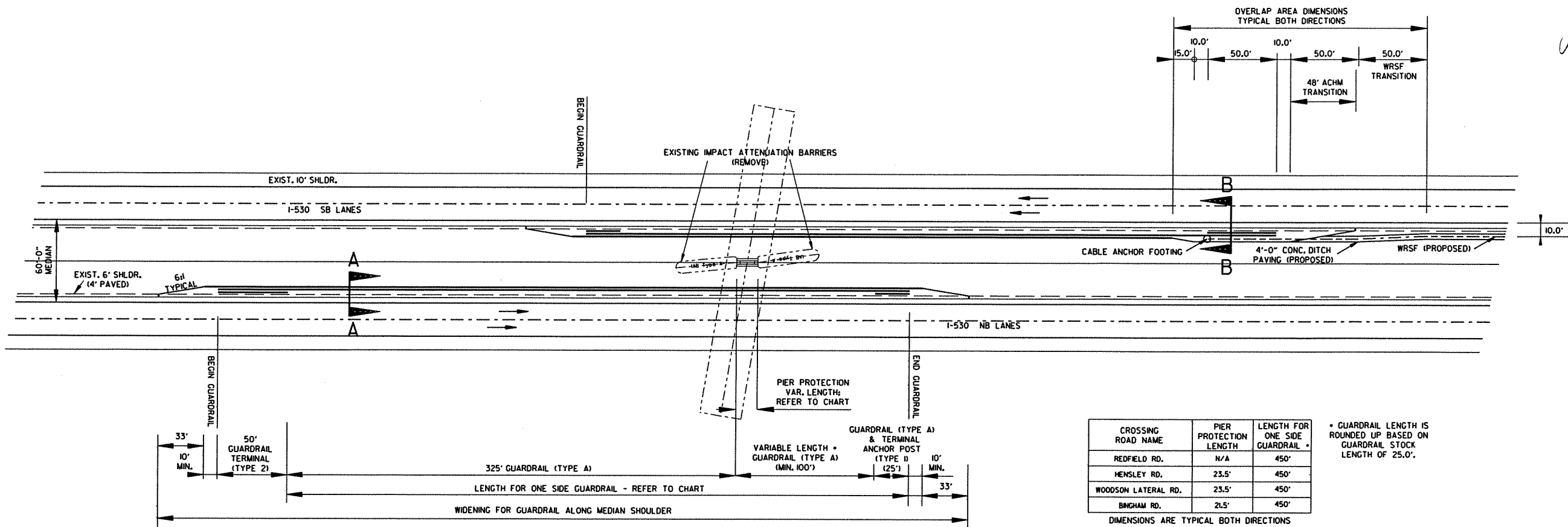
TYPICAL SECTIONS OF IMPROVEMENT

11/16/2012

RBB0614.DGN

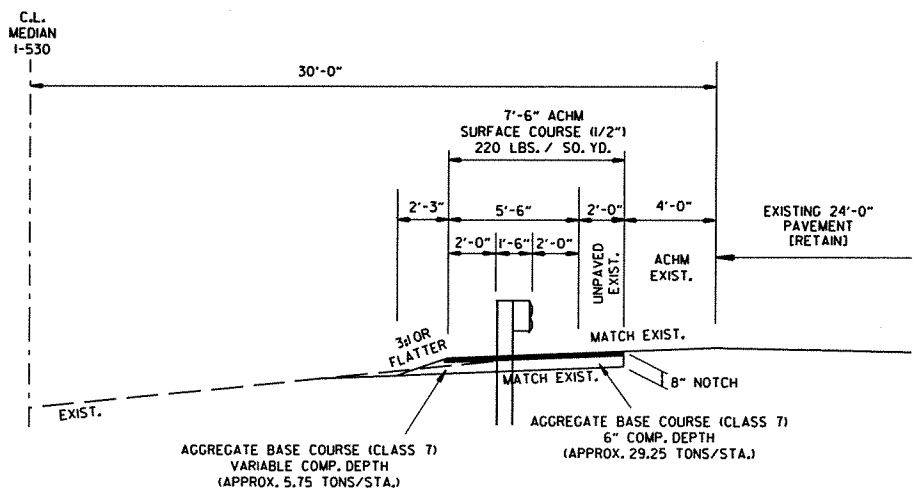
DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB NO.	BB0614		8	219

2 SPECIAL DETAILS

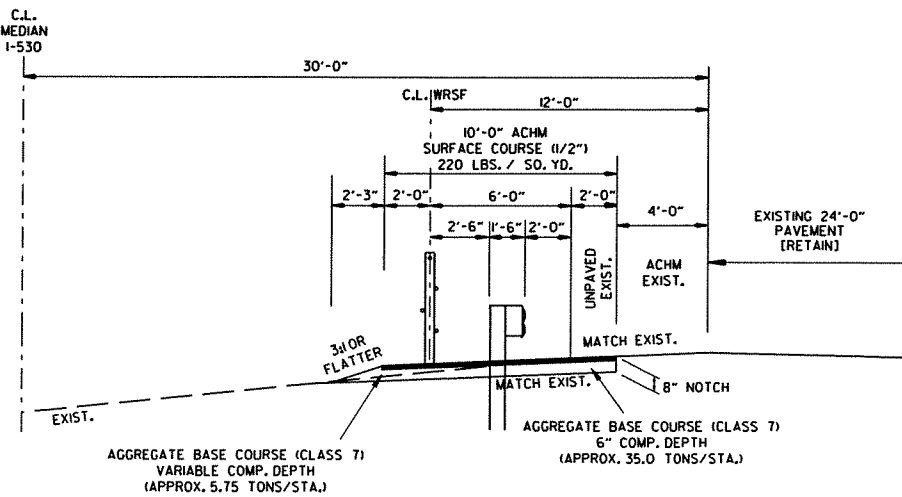


DETAIL AT OVERPASSES

NOTE: REFER TO PLAN SHEETS FOR PLACEMENT OF WIRE ROPE SAFETY FENCE ON NORTHBOUND OR SOUTHBOUND FORESLOPES.



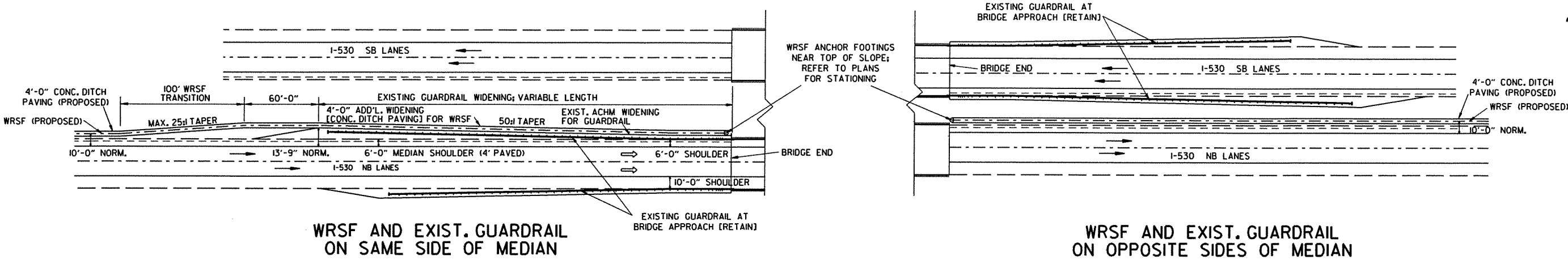
SECTION A-A



SECTION B-B

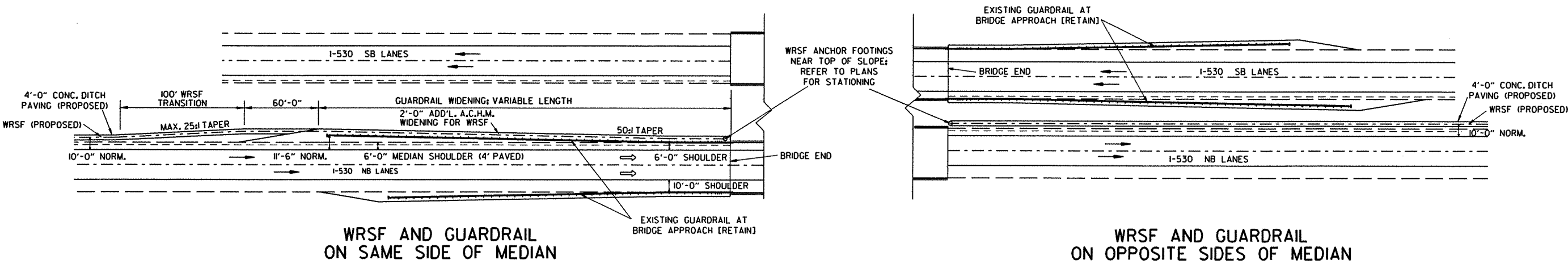
DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB NO.	BB0614		9	219

2 SPECIAL DETAILS



DETAIL OF WIRE ROPE SAFETY FENCE AT EXISTING BRIDGE ENDS

REFER TO PLANS FOR RELATIVE PLACEMENT
OF GUARDRAIL AND WIRE ROPE SAFETY FENCE
AT EACH BRIDGE END

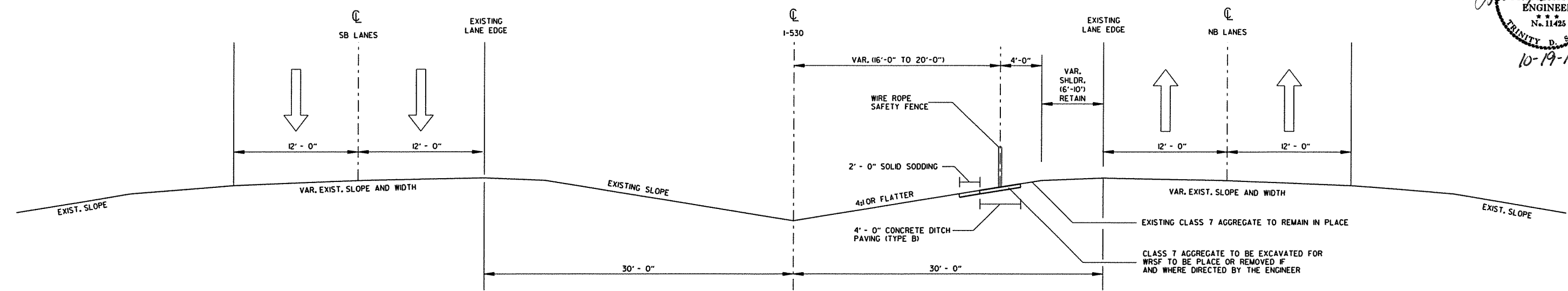


DETAIL OF WIRE ROPE SAFETY FENCE AT NEW AND REHABILITATED BRIDGE ENDS

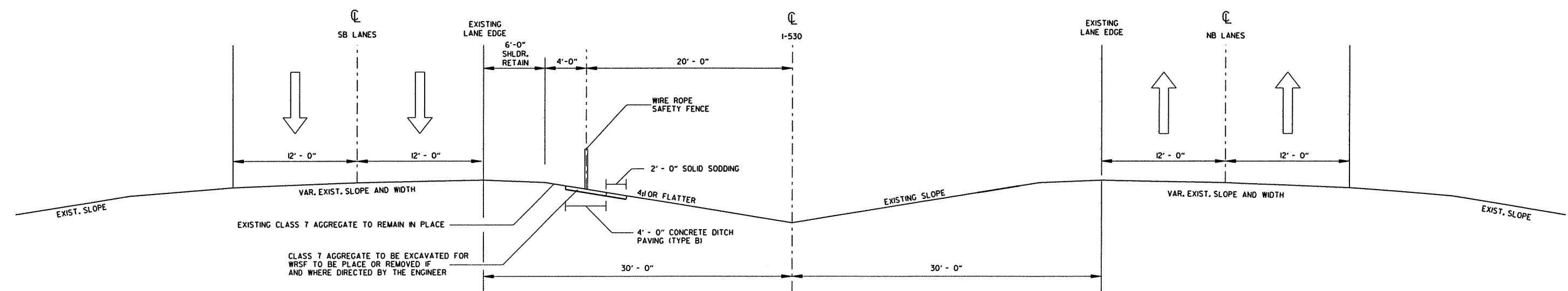
REFER TO PLANS FOR RELATIVE PLACEMENT
OF GUARDRAIL AND WIRE ROPE SAFETY FENCE
AT EACH BRIDGE END

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB NO.		BB0614	10	219

2 SPECIAL DETAILS



TYPICAL SECTION OF IMPROVEMENT
FOR WIRE ROPE SAFETY FENCE RIGHT OF CENTERLINE

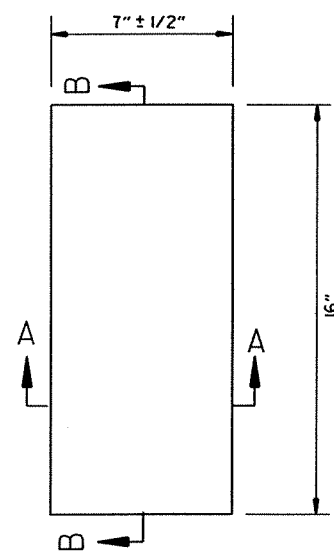


TYPICAL SECTION OF IMPROVEMENT
FOR WIRE ROPE SAFETY FENCE LEFT OF CENTERLINE

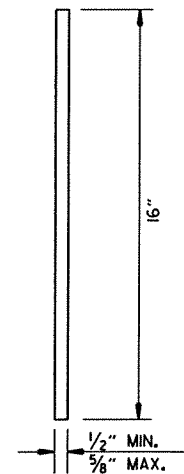
SPECIAL DETAILS

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
						JOB NO. BB0614	11	219

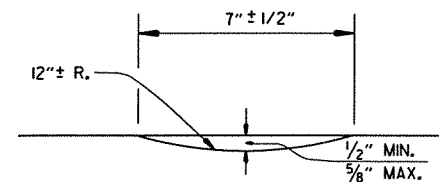
2 SPECIAL DETAILS



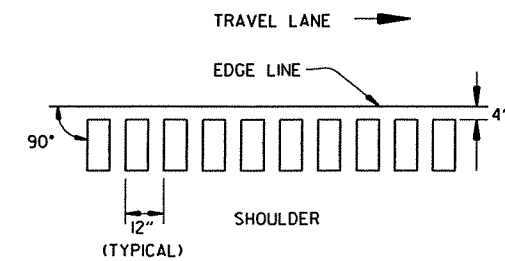
PLAN



SECTION B-B

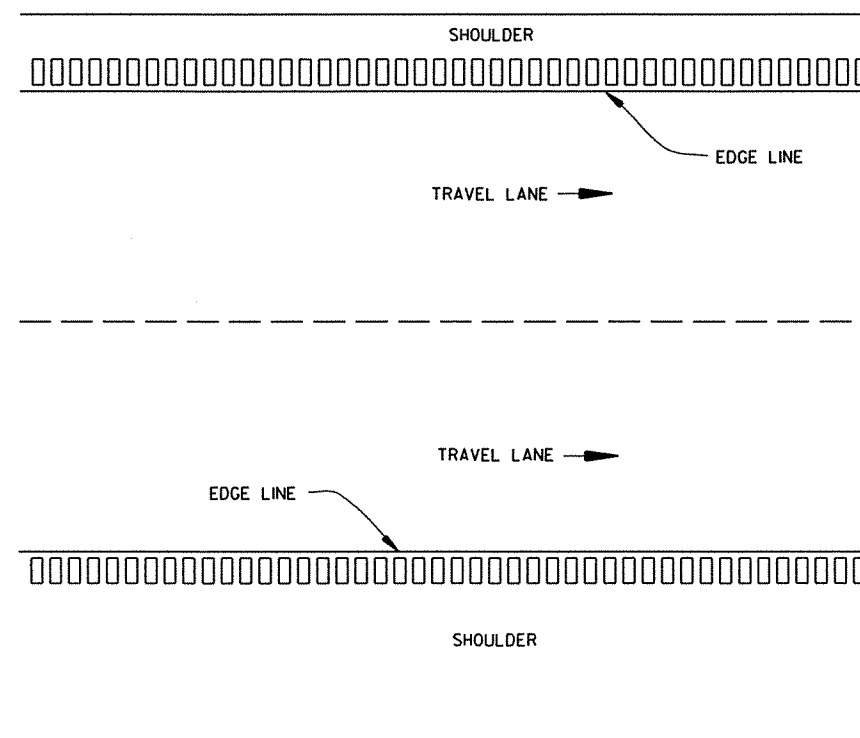


SECTION A-A



LOCATION PLAN OF RUMBLE STRIPS
LEFT OR RIGHT SHOULDER

DETAILS OF RUMBLE STRIPS



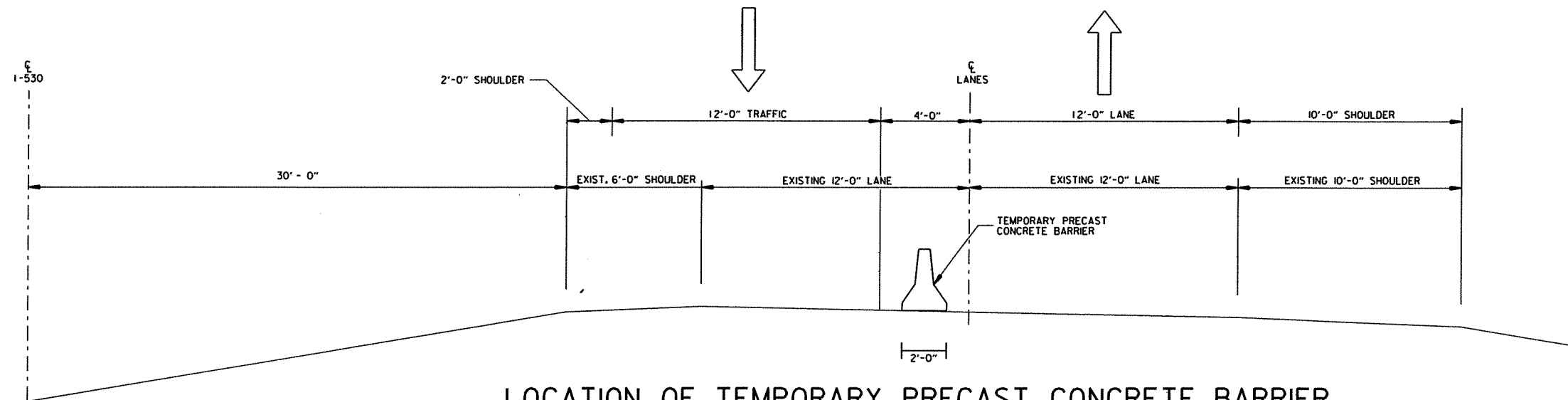
PLAN VIEW

NOTES:

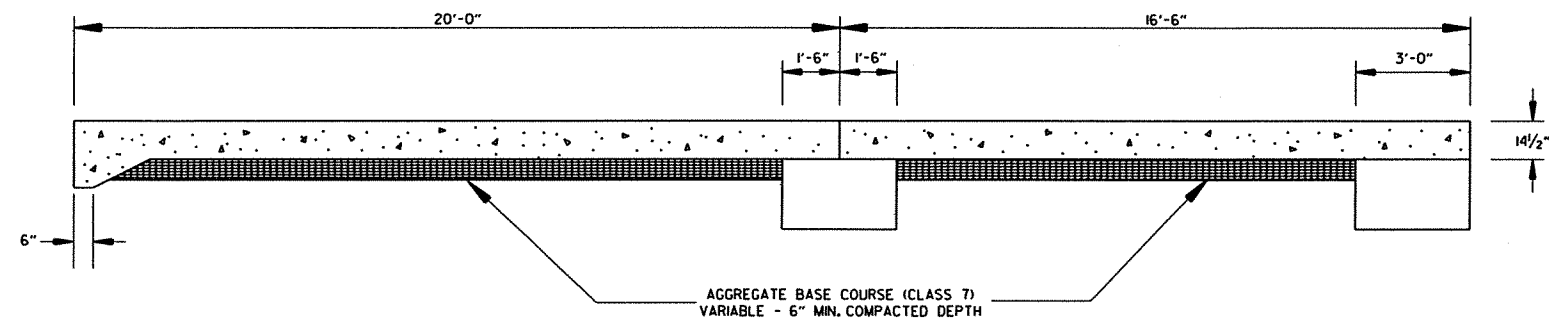
1. ALIGNMENT OF RUMBLE STRIPS SHALL GENERALLY BE STRAIGHT AND OFFSET APPROXIMATELY 4" FROM THE OUTER EDGE OF THE EDGE LINE. THIS OFFSET MAY BE ADJUSTED TO ACCOMMODATE VARIATIONS IN THE EDGE LINE.
2. THE 1/2" DEPTH SHALL GENERALLY APPLY FOR THE ENTIRE 16" LENGTH. SOME VARIATION TO SUIT SHOULDER SLOPE BREAKS MAY BE NECESSARY.
3. RUMBLE STRIPS SHALL NOT BE INSTALLED ON BRIDGE DECKS, APPROACH SLABS, OR ACROSS TRANSVERSE JOINTS OF CONCRETE SHOULDER.

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
						JOB NO. 880614	12	219

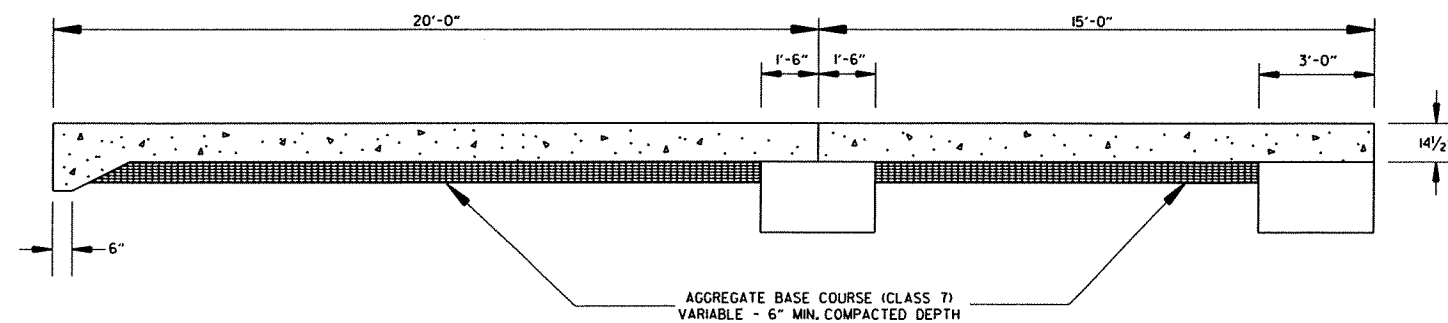
2 SPECIAL DETAILS



LOCATION OF TEMPORARY PRECAST CONCRETE BARRIER
FOR MAINTENANCE OF TRAFFIC



DETAIL OF APPROACH SLAB TYPE SPECIAL 2 & TYPE SPECIAL 3

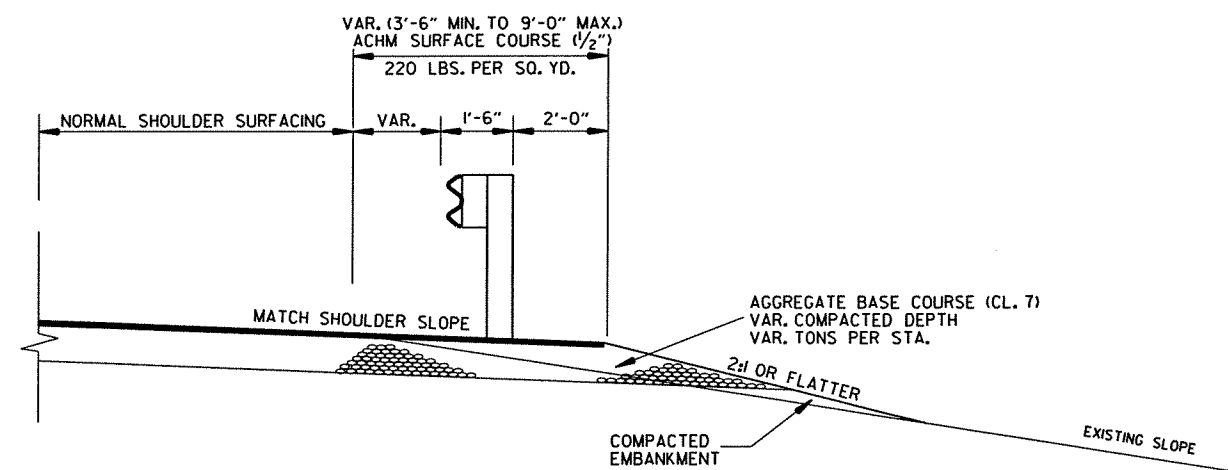


DETAIL OF APPROACH SLAB TYPE SPECIAL 1

SPECIAL DETAILS

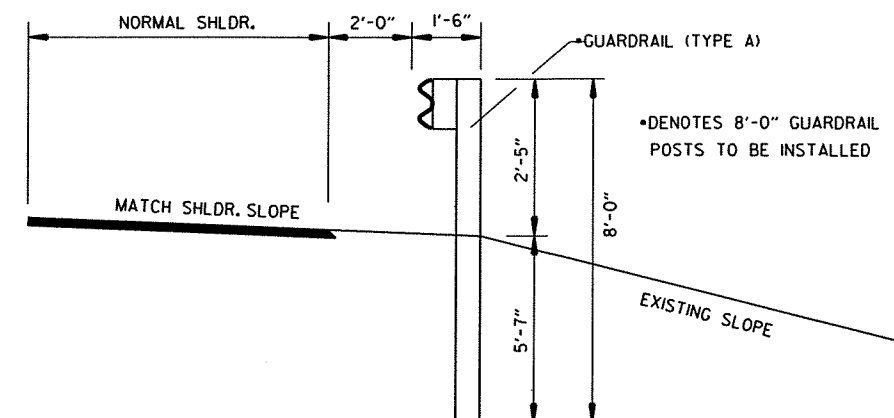
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				6	ARK.			
				JOB NO.	BB0614		13	219

2 SPECIAL DETAILS



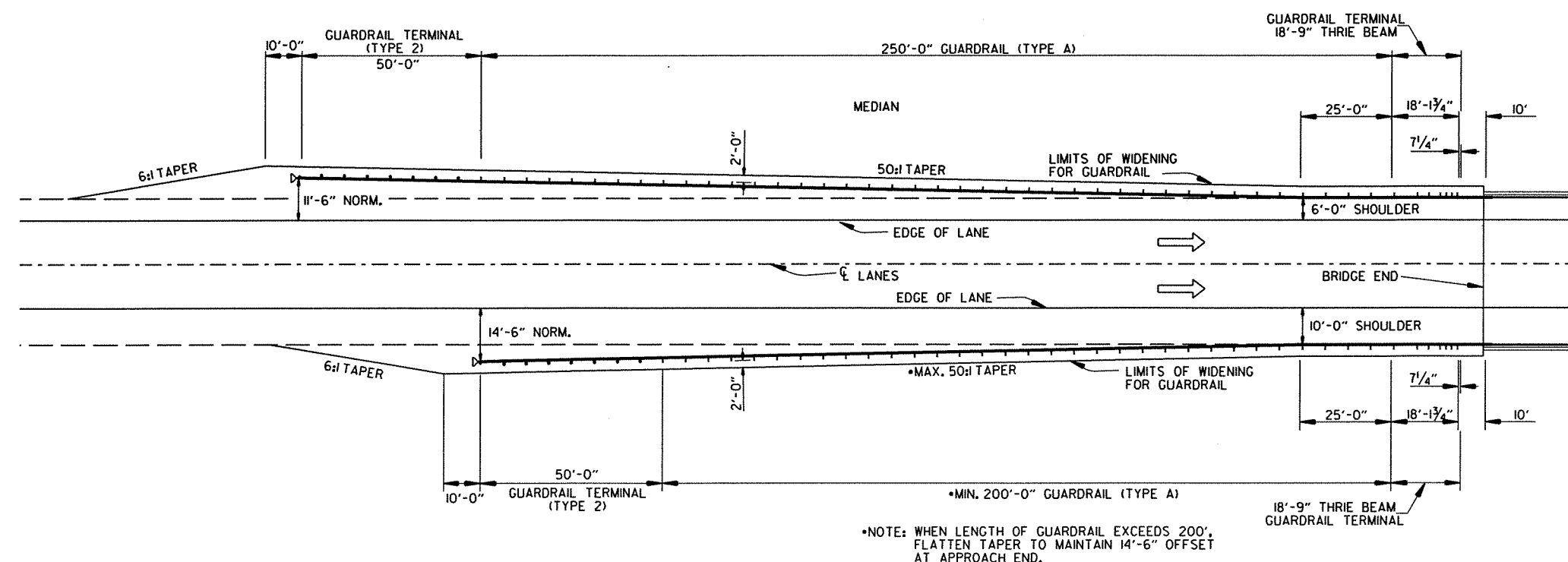
SECTION DETAIL OF WIDENING FOR GUARDRAIL

NOTE: REFER TO STANDARD DRAWINGS, GR-8, GR-9, GR-9A, GR-10, GR-10A, & GRT-1 FOR ADDITIONAL INFORMATION.



SECTION DETAIL OF WIDENING FOR GUARDRAIL FOR HWY. 167 OVERPASS

NOTE: REFER TO STANDARD DRAWINGS, GR-8, GR-9, GR-9A, GR-10 & GR-10A FOR ADDITIONAL INFORMATION.

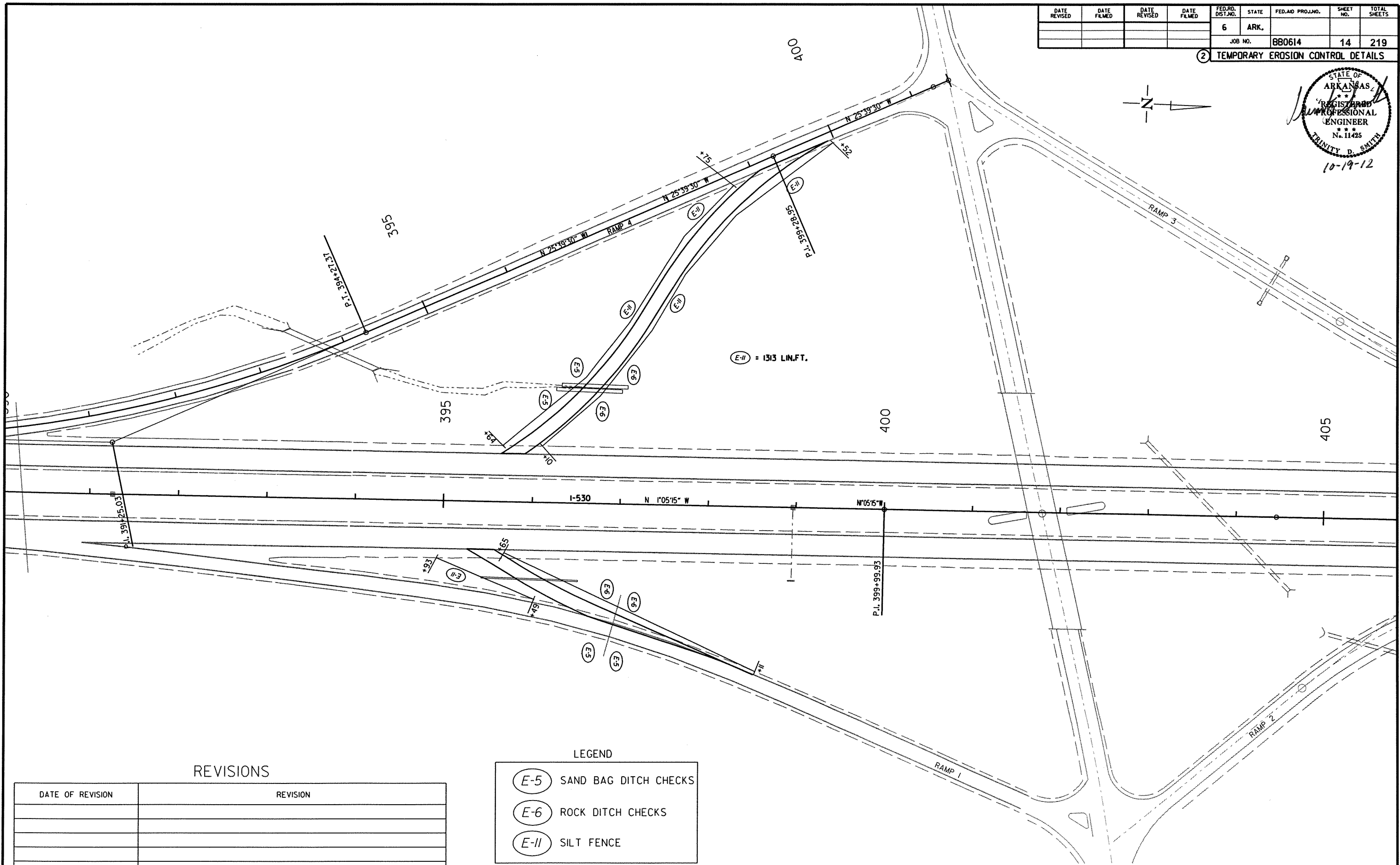
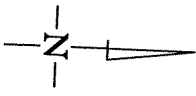
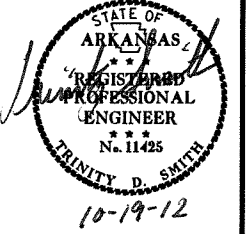


TYPICAL LAYOUT OF GUARDRAIL AT BRIDGE ENDS

SPECIAL DETAILS

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
						JOB NO. BB0614	14	219

2 TEMPORARY EROSION CONTROL DETAILS



LEGEND

- E-5 SAND BAG DITCH CHECKS
- E-6 ROCK DITCH CHECKS
- E-II SILT FENCE

NOTE: RETAIN ALL EROSION CONTROL DEVICES UNTIL END OF CONSTRUCTION UNLESS OTHERWISE NOTED.

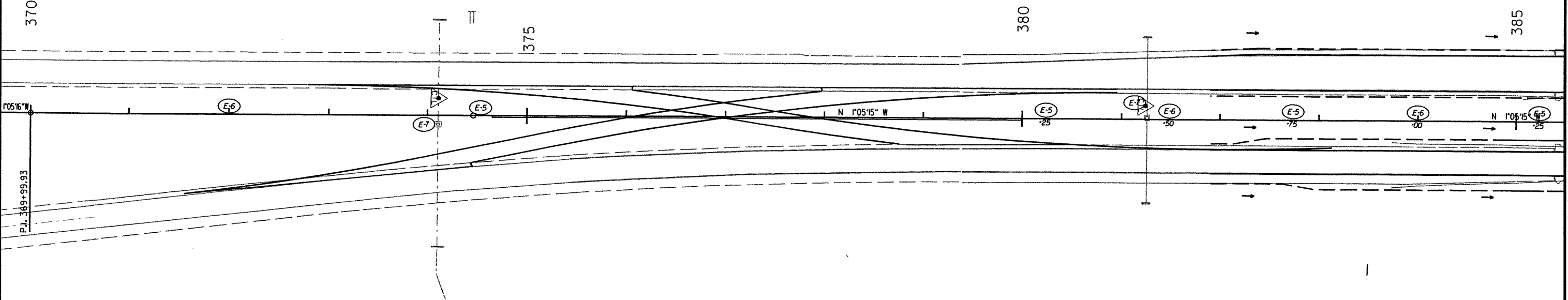
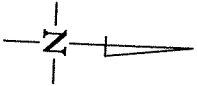
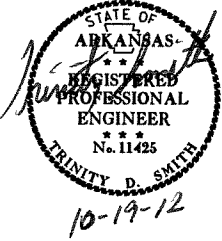
REVISIONS

DATE OF REVISION	REVISION

TEMPORARY EROSION CONTROL DETAILS
STAGE IA

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB NO.	BB0614		15	219

2 TEMPORARY EROSION CONTROL DETAILS



REVISIONS

DATE OF REVISION	REVISION

LEGEND

- (E-5) SAND BAG DITCH CHECKS
- (E-6) ROCK DITCH CHECKS
- (E-7) DROP INLET SILT FENCE
- (E-11) SILT FENCE

NOTE: RETAIN ALL EROSION CONTROL DEVICES UNTIL END OF CONSTRUCTION UNLESS OTHERWISE NOTED.

TEMPORARY EROSION CONTROL DETAILS
STAGE 1B

10/19/2012 RBB0614.DGN

REVISIONS

DATE OF REVISION	REVISION

LEGEND

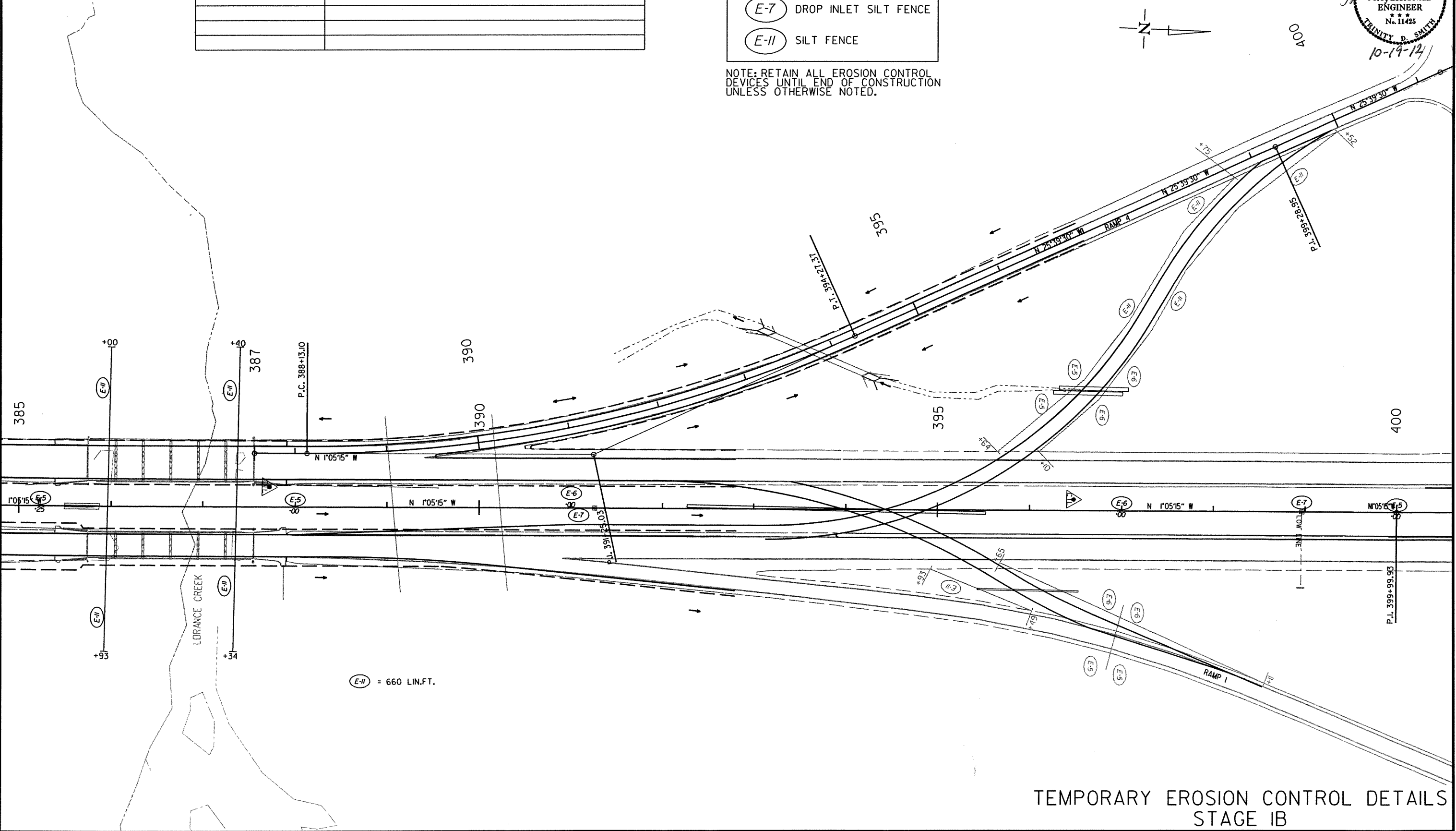
- E-5 SAND BAG DITCH CHECKS
- E-6 ROCK DITCH CHECKS
- E-7 DROP INLET SILT FENCE
- E-11 SILT FENCE

NOTE: RETAIN ALL EROSION CONTROL DEVICES UNTIL END OF CONSTRUCTION UNLESS OTHERWISE NOTED.

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB NO.	880614		16	219

2 TEMPORARY EROSION CONTROL DETAILS

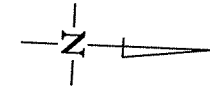
STATE OF ARKANSAS
REGISTERED PROFESSIONAL ENGINEER
No. 11425
TRINITY D. SMITH
10-19-12



TEMPORARY EROSION CONTROL DETAILS
STAGE 1B

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB NO.		BB0614	17	219

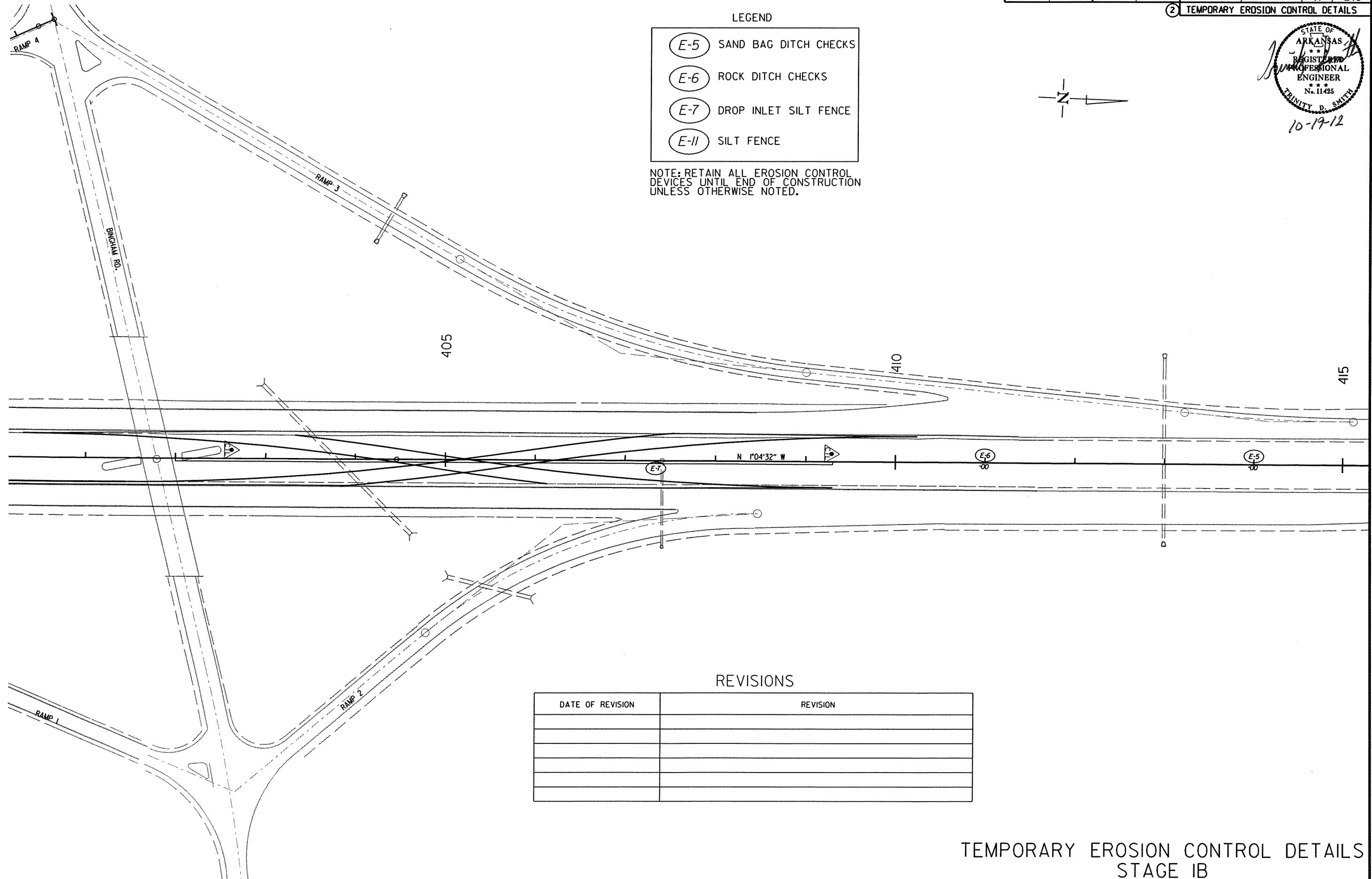
2 TEMPORARY EROSION CONTROL DETAILS



LEGEND

- (E-5) SAND BAG DITCH CHECKS
- (E-6) ROCK DITCH CHECKS
- (E-7) DROP INLET SILT FENCE
- (E-11) SILT FENCE

NOTE: RETAIN ALL EROSION CONTROL DEVICES UNTIL END OF CONSTRUCTION UNLESS OTHERWISE NOTED.



REVISIONS

DATE OF REVISION	REVISION

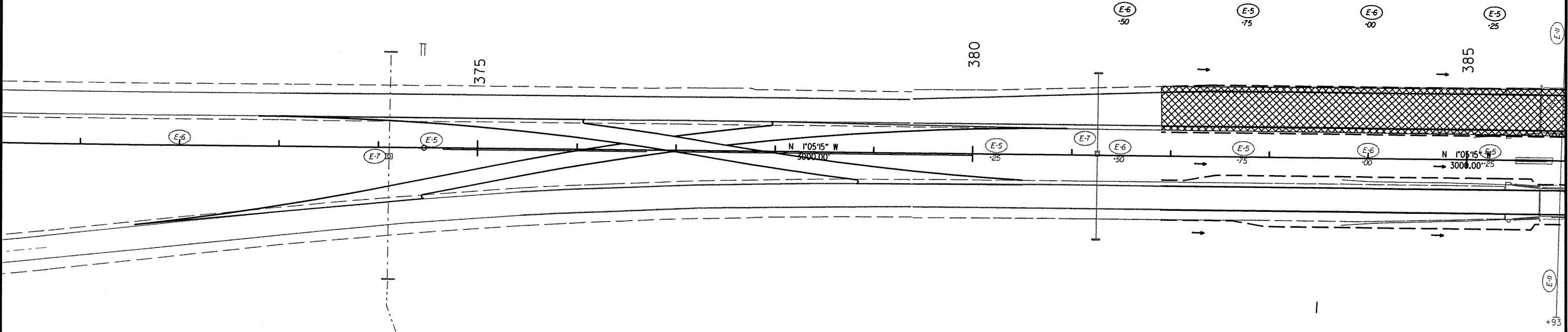
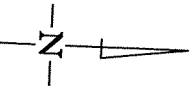
TEMPORARY EROSION CONTROL DETAILS
STAGE 1B

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB NO. BB0614		18	219	

2 TEMPORARY EROSION CONTROL DETAILS



10-19-12



LEGEND

-  SAND BAG DITCH CHECKS
-  ROCK DITCH CHECKS
-  DROP INLET SILT FENCE
-  SILT FENCE

NOTE: RETAIN ALL EROSION CONTROL DEVICES UNTIL END OF CONSTRUCTION UNLESS OTHERWISE NOTED.

REVISIONS

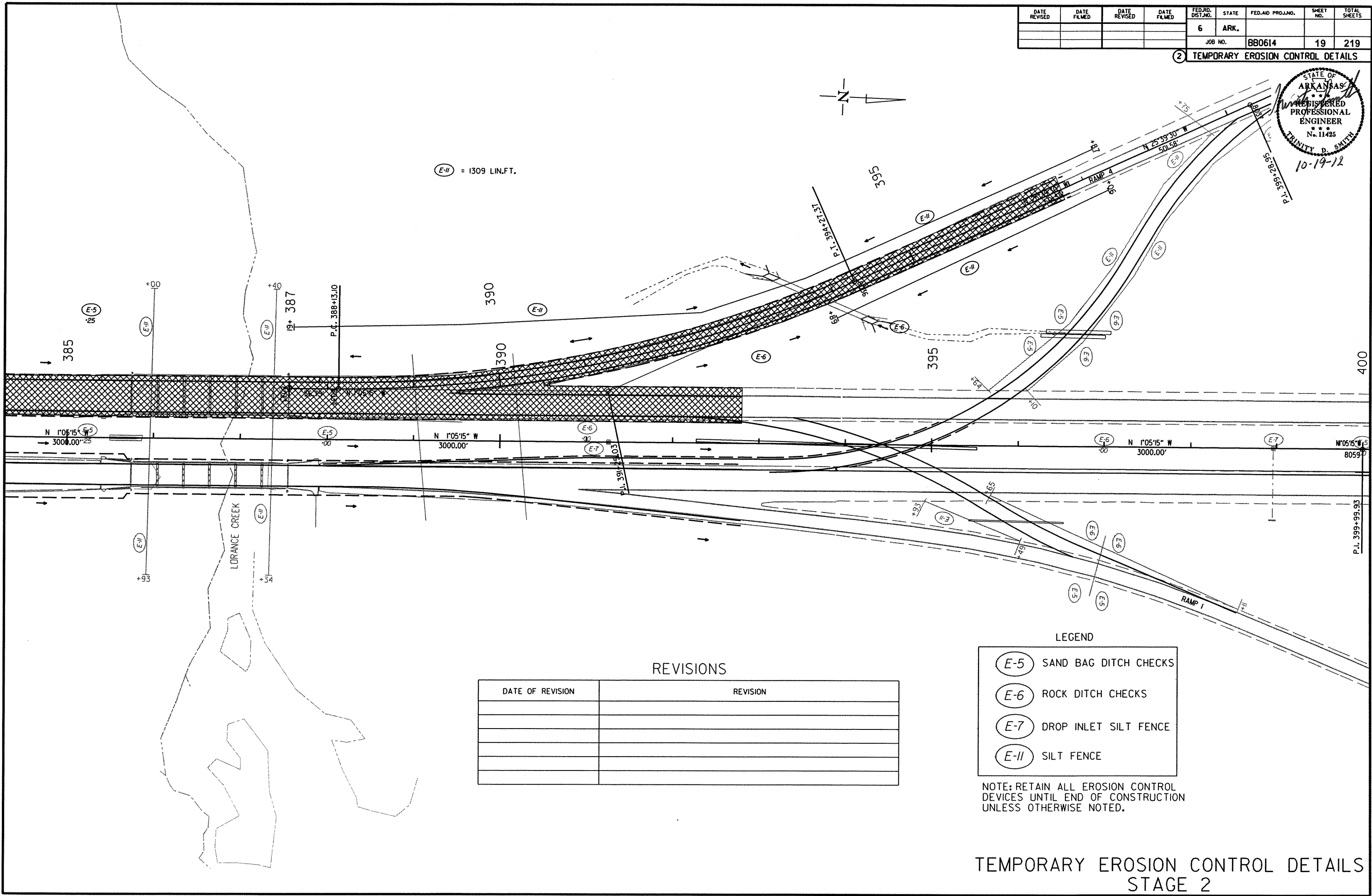
DATE OF REVISION	REVISION

TEMPORARY EROSION CONTROL DETAILS
STAGE 2

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
						JOB NO. BB0614	19	219

2 TEMPORARY EROSION CONTROL DETAILS

STATE OF ARKANSAS
REGISTERED PROFESSIONAL ENGINEER
No. 11425
TRINITY D. SMITH
10-19-12



REVISIONS	
DATE OF REVISION	REVISION

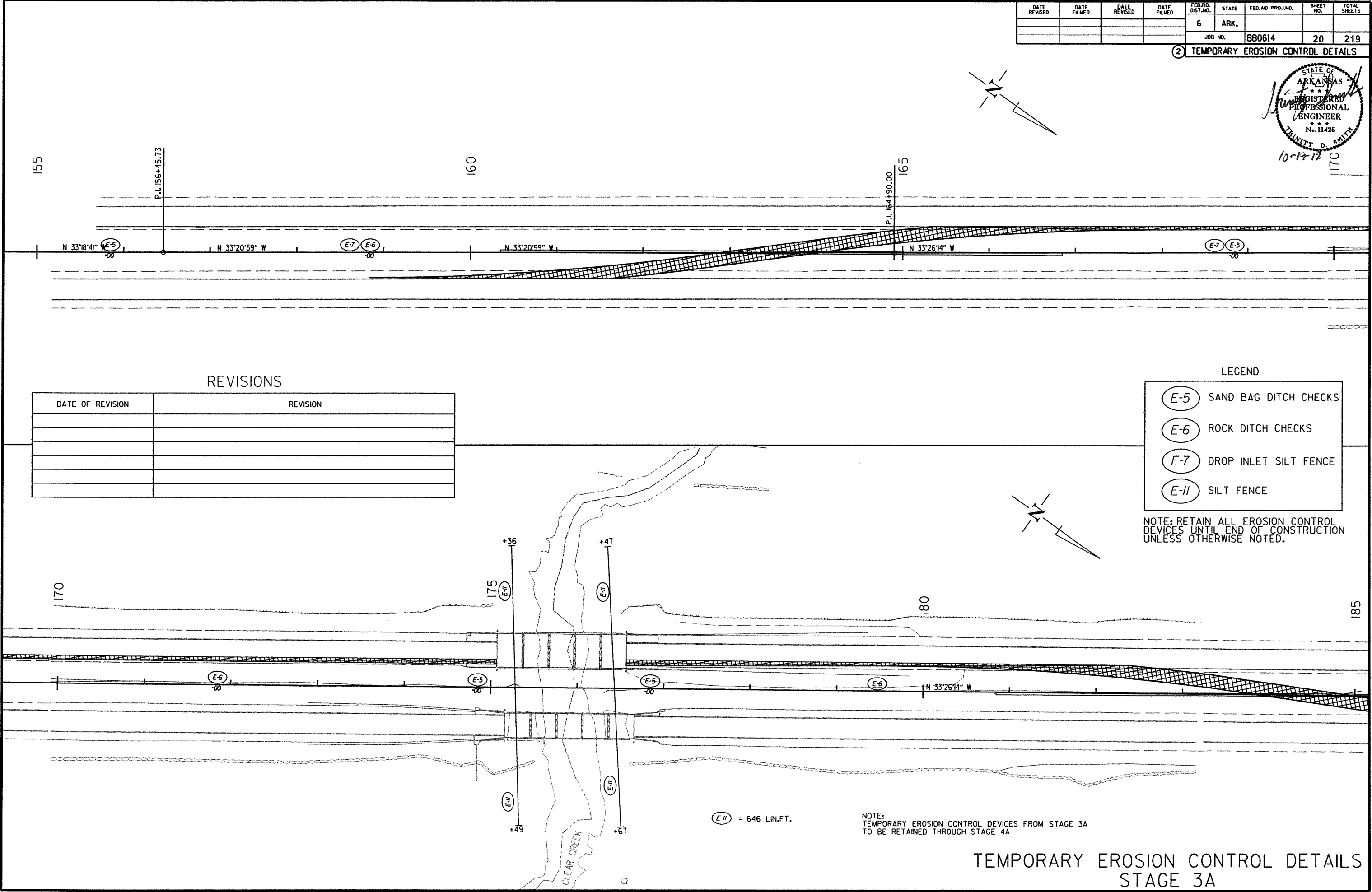
E-5	SAND BAG DITCH CHECKS
E-6	ROCK DITCH CHECKS
E-7	DROP INLET SILT FENCE
E-II	SILT FENCE

NOTE: RETAIN ALL EROSION CONTROL DEVICES UNTIL END OF CONSTRUCTION UNLESS OTHERWISE NOTED.

TEMPORARY EROSION CONTROL DETAILS
STAGE 2

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB NO.	BB0614		20	219

2 TEMPORARY EROSION CONTROL DETAILS



REVISIONS

DATE OF REVISION	REVISION

LEGEND

- (E-5) SAND BAG DITCH CHECKS
- (E-6) ROCK DITCH CHECKS
- (E-7) DROP INLET SILT FENCE
- (E-11) SILT FENCE

NOTE: RETAIN ALL EROSION CONTROL DEVICES UNTIL END OF CONSTRUCTION UNLESS OTHERWISE NOTED.

NOTE: TEMPORARY EROSION CONTROL DEVICES FROM STAGE 3A TO BE RETAINED THROUGH STAGE 4A

TEMPORARY EROSION CONTROL DETAILS
STAGE 3A

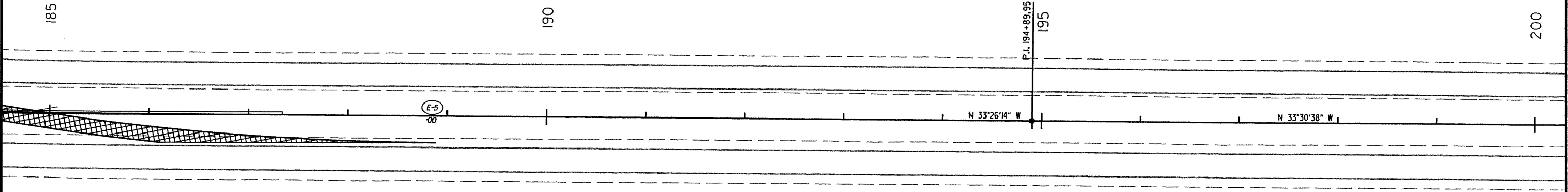
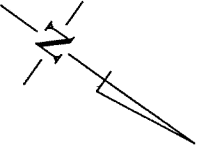
10/19/2012

RB0614.DGN

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB NO.		880614	21	219
② TEMPORARY EROSION CONTROL DETAILS								



10-19-12



NOTE:
TEMPORARY EROSION CONTROL DEVICES FROM STAGE 3A
TO BE RETAINED THROUGH STAGE 4A

REVISIONS

DATE OF REVISION	REVISION

LEGEND

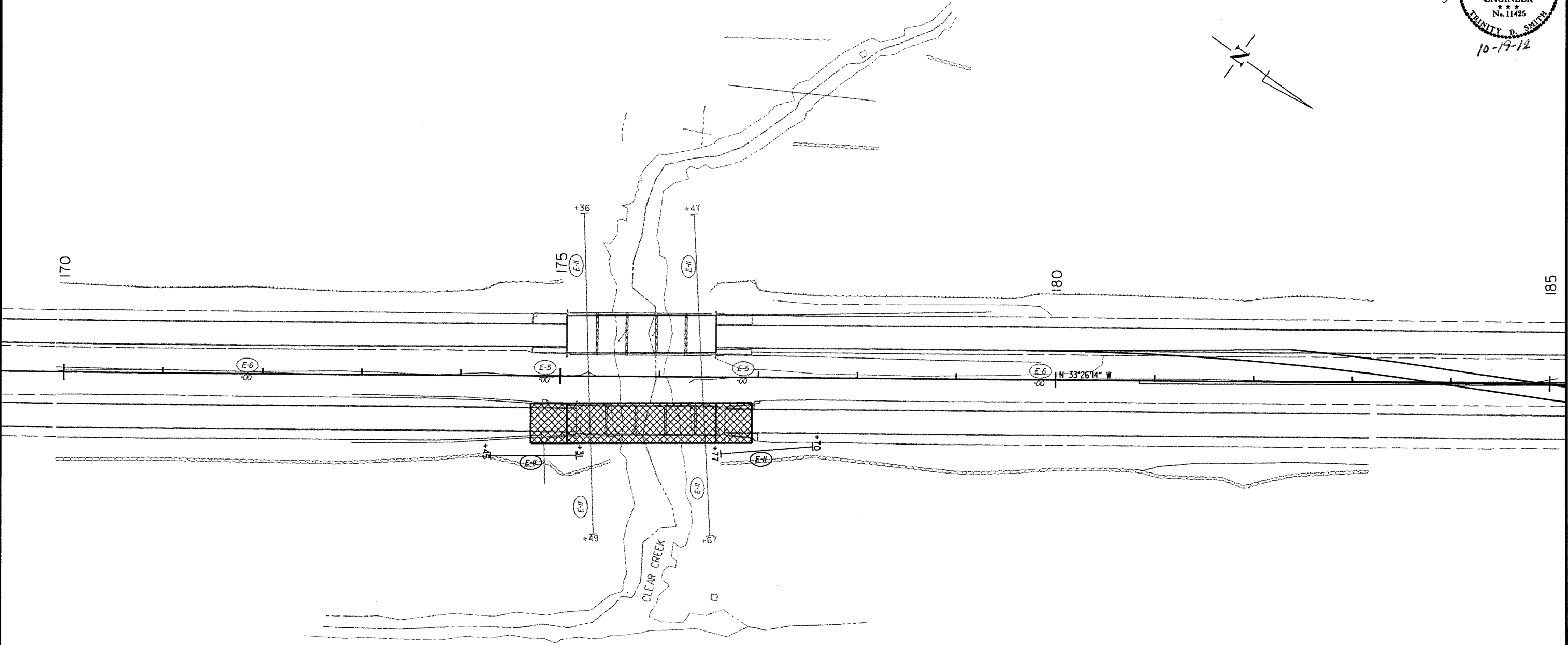
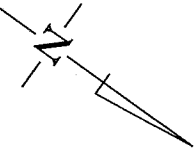
- (E-5) SAND BAG DITCH CHECKS
- (E-6) ROCK DITCH CHECKS
- (E-7) DROP INLET SILT FENCE
- (E-11) SILT FENCE

NOTE: RETAIN ALL EROSION CONTROL
DEVICES UNTIL END OF CONSTRUCTION
UNLESS OTHERWISE NOTED.

TEMPORARY EROSION CONTROL DETAILS
STAGE 3A

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB NO. BB0614			22	219

2 TEMPORARY EROSION CONTROL DETAILS



REVISIONS

DATE OF REVISION	REVISION

LEGEND

-  SAND BAG DITCH CHECKS
-  ROCK DITCH CHECKS
-  DROP INLET SILT FENCE
-  SILT FENCE

NOTE: RETAIN ALL EROSION CONTROL DEVICES UNTIL END OF CONSTRUCTION UNLESS OTHERWISE NOTED.

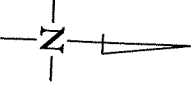
TEMPORARY EROSION CONTROL DETAILS
STAGE 3B

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB NO.	BB0614		23	219

2 TEMPORARY EROSION CONTROL DETAILS



10-19-12



REVISIONS

DATE OF REVISION	REVISION

LEGEND

E-5

SAND BAG DITCH CHECKS

E-6

ROCK DITCH CHECKS

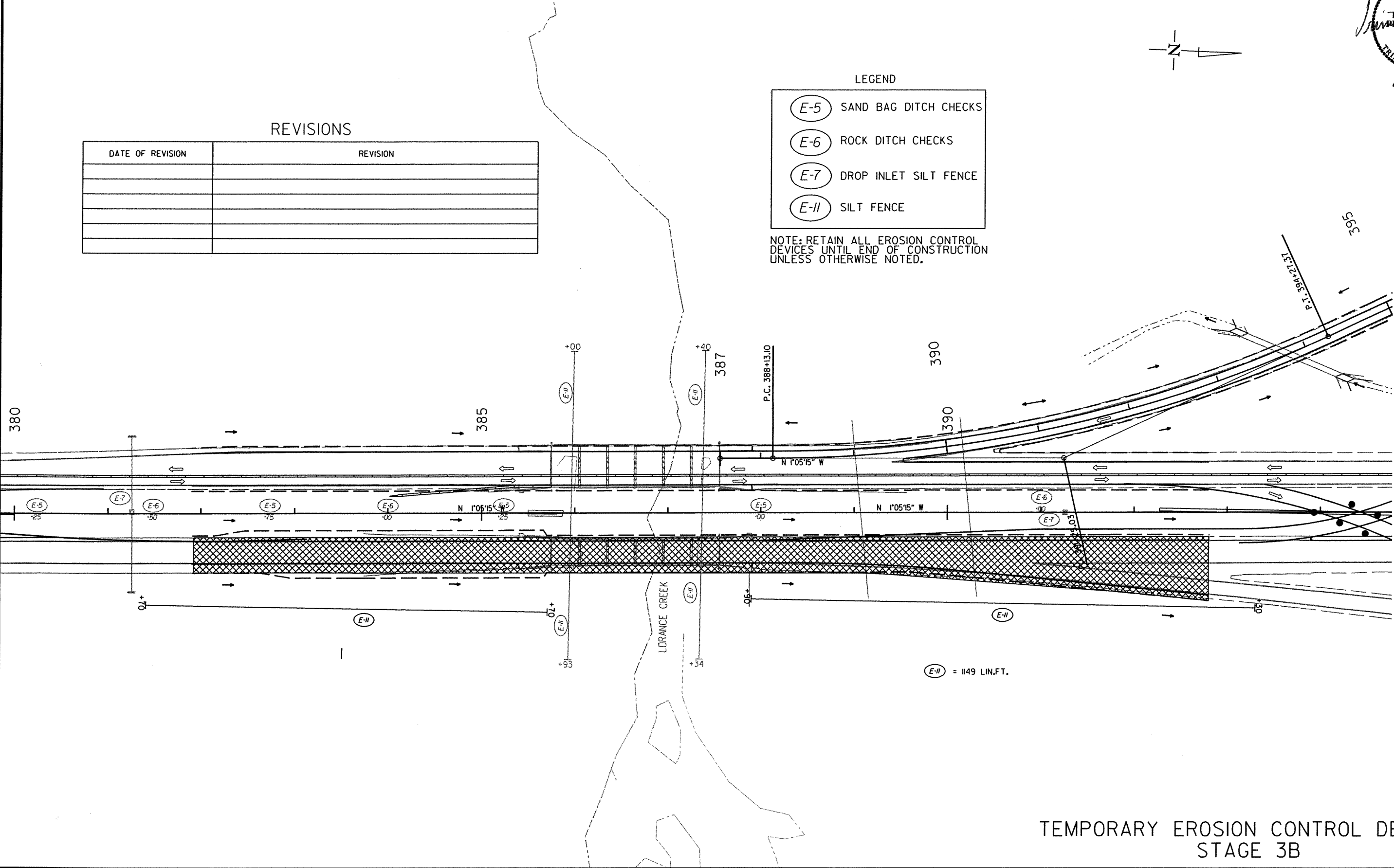
E-7

DROP INLET SILT FENCE

E-11

SILT FENCE

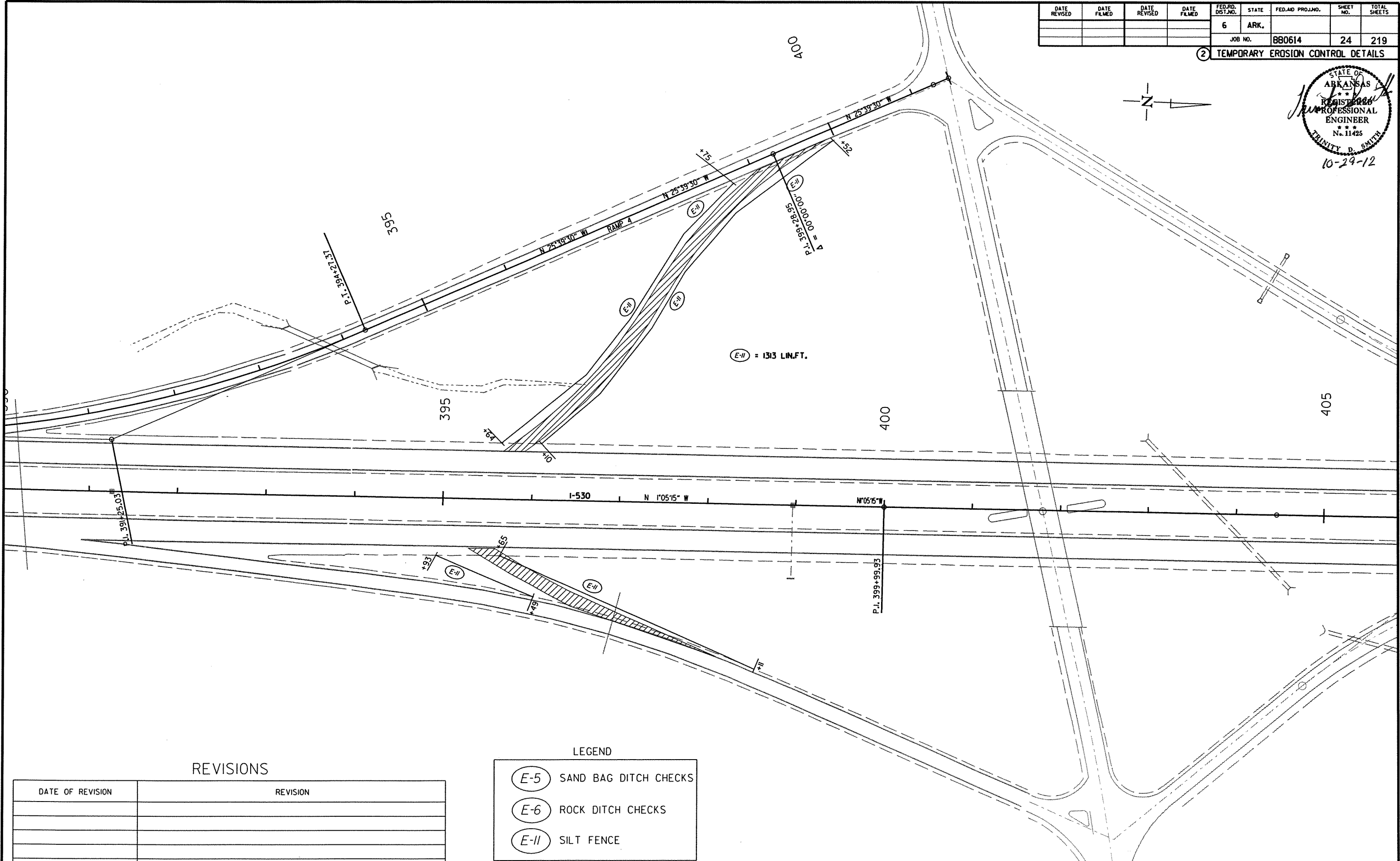
NOTE: RETAIN ALL EROSION CONTROL DEVICES UNTIL END OF CONSTRUCTION UNLESS OTHERWISE NOTED.



TEMPORARY EROSION CONTROL DETAILS
STAGE 3B

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB NO.		880614	24	219
② TEMPORARY EROSION CONTROL DETAILS								

STATE OF ARKANSAS
 REGISTERED PROFESSIONAL ENGINEER
 No. 11425
 TRINITY D. SMITH
 10-29-12



REVISIONS

DATE OF REVISION	REVISION

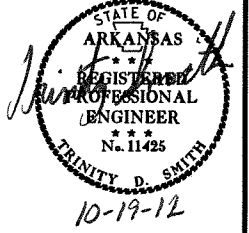
LEGEND

- (E-5) SAND BAG DITCH CHECKS
- (E-6) ROCK DITCH CHECKS
- (E-11) SILT FENCE

NOTE: RETAIN ALL EROSION CONTROL DEVICES UNTIL END OF CONSTRUCTION UNLESS OTHERWISE NOTED.

TEMPORARY EROSION CONTROL DETAILS
 STAGE 4B

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB NO.		BB0614	25	219
② TEMPORARY EROSION CONTROL DETAILS								



LEGEND

E-5

SAND BAG DITCH CHECKS

E-6

ROCK DITCH CHECKS

E-7

DROP INLET SILT FENCE

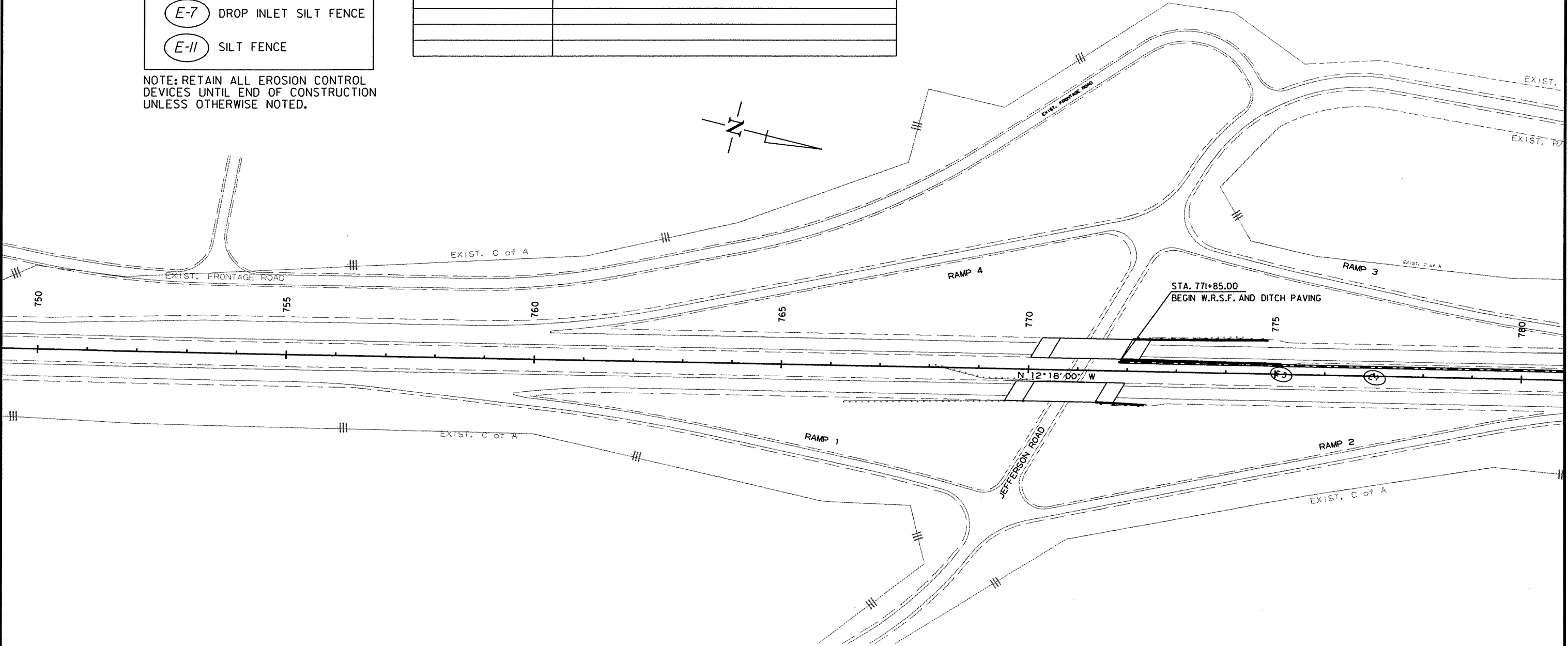
E-II

SILT FENCE

NOTE: RETAIN ALL EROSION CONTROL DEVICES UNTIL END OF CONSTRUCTION UNLESS OTHERWISE NOTED.

REVISIONS

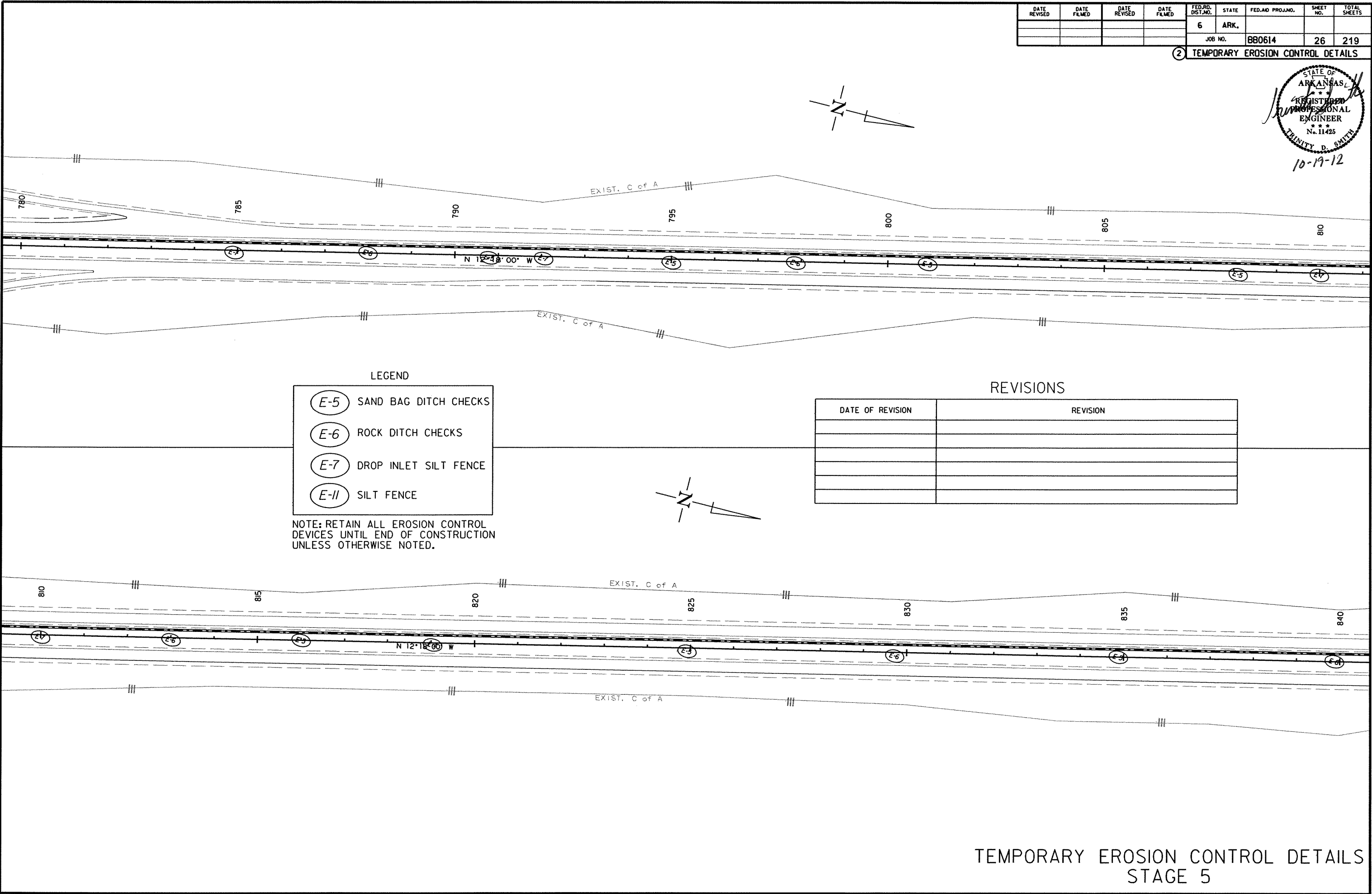
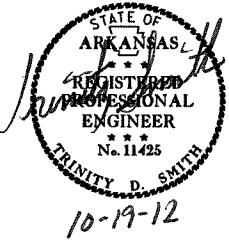
DATE OF REVISION	REVISION



TEMPORARY EROSION CONTROL DETAILS
STAGE 5

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB NO.		BB0614	26	219

2 TEMPORARY EROSION CONTROL DETAILS



LEGEND

E-5

SAND BAG DITCH CHECKS

E-6

ROCK DITCH CHECKS

E-7

DROP INLET SILT FENCE

E-11

SILT FENCE

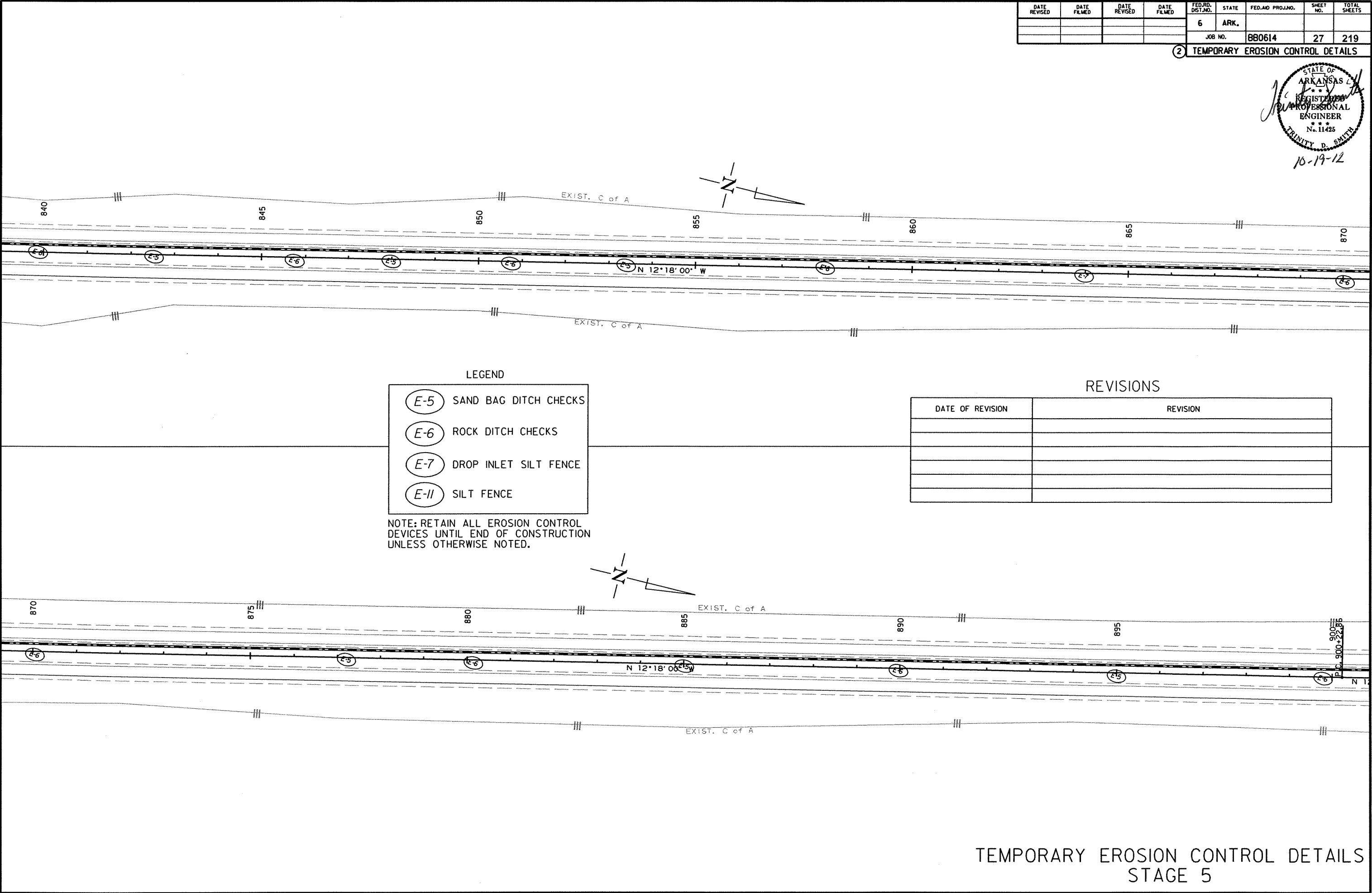
NOTE: RETAIN ALL EROSION CONTROL DEVICES UNTIL END OF CONSTRUCTION UNLESS OTHERWISE NOTED.

REVISIONS

DATE OF REVISION	REVISION

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB NO.		BB0614	27	219

2 TEMPORARY EROSION CONTROL DETAILS

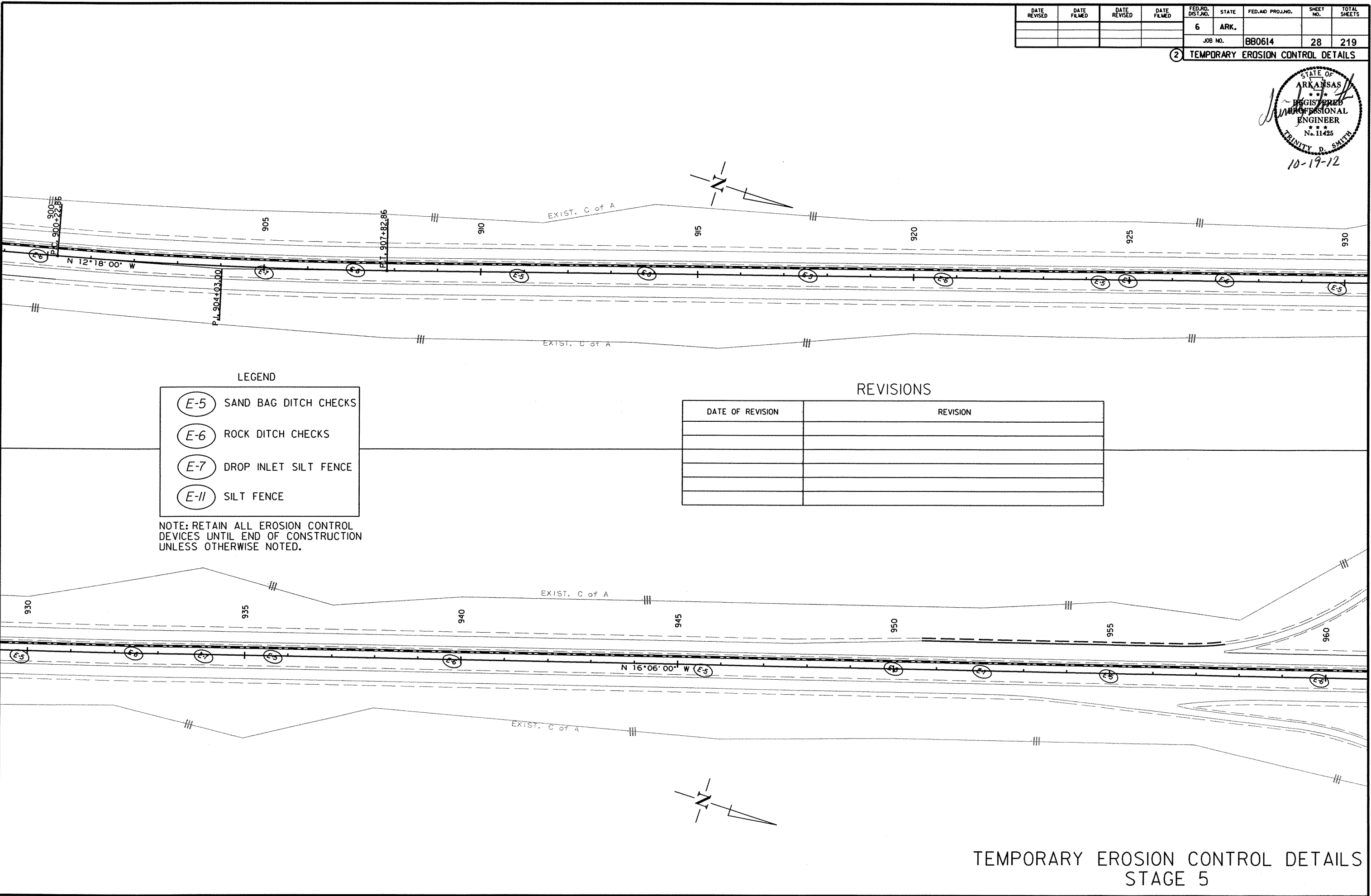


10/18/2012

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DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB NO. 880614			28	219

2 TEMPORARY EROSION CONTROL DETAILS



LEGEND

- (E-5) SAND BAG DITCH CHECKS
- (E-6) ROCK DITCH CHECKS
- (E-7) DROP INLET SILT FENCE
- (E-11) SILT FENCE

NOTE: RETAIN ALL EROSION CONTROL DEVICES UNTIL END OF CONSTRUCTION UNLESS OTHERWISE NOTED.

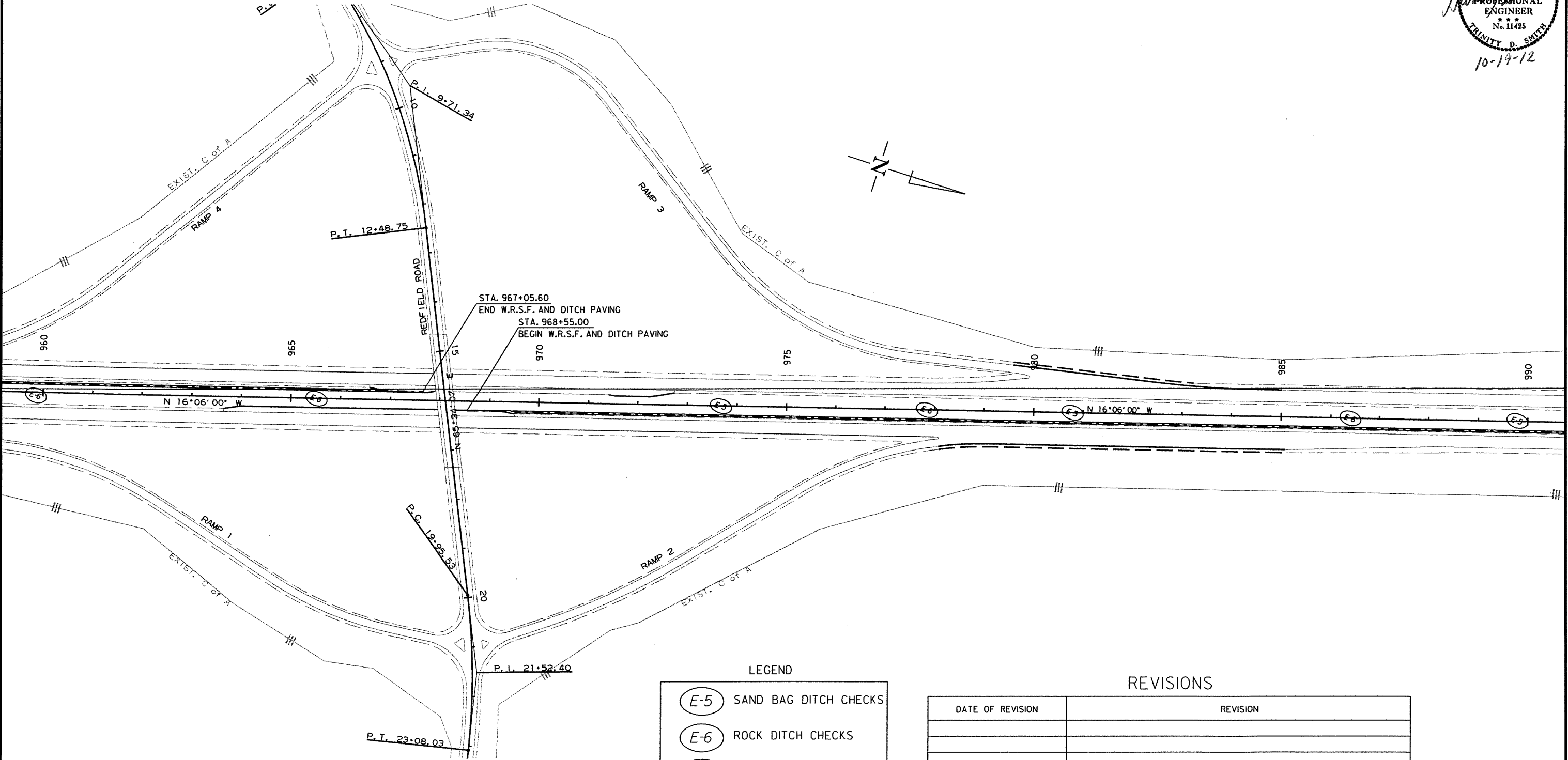
REVISIONS

DATE OF REVISION	REVISION

TEMPORARY EROSION CONTROL DETAILS
STAGE 5

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB NO.		B80614	29	219

2 TEMPORARY EROSION CONTROL DETAILS



LEGEND

- (E-5) SAND BAG DITCH CHECKS
- (E-6) ROCK DITCH CHECKS
- (E-7) DROP INLET SILT FENCE
- (E-11) SILT FENCE

NOTE: RETAIN ALL EROSION CONTROL DEVICES UNTIL END OF CONSTRUCTION UNLESS OTHERWISE NOTED.

REVISIONS	
DATE OF REVISION	REVISION

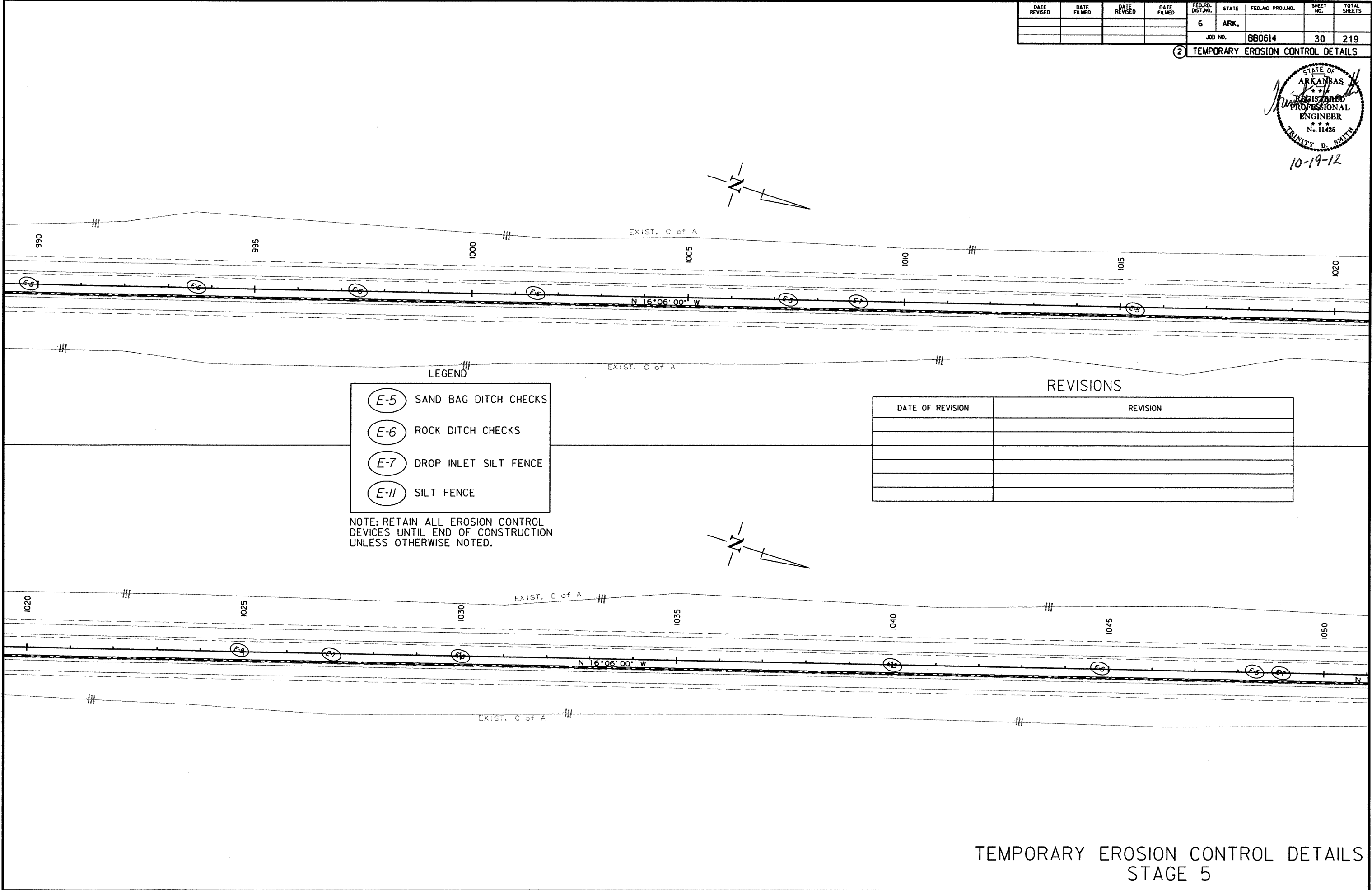
TEMPORARY EROSION CONTROL DETAILS
STAGE 5

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
						JOB NO. BB0614	30	219

2 TEMPORARY EROSION CONTROL DETAILS



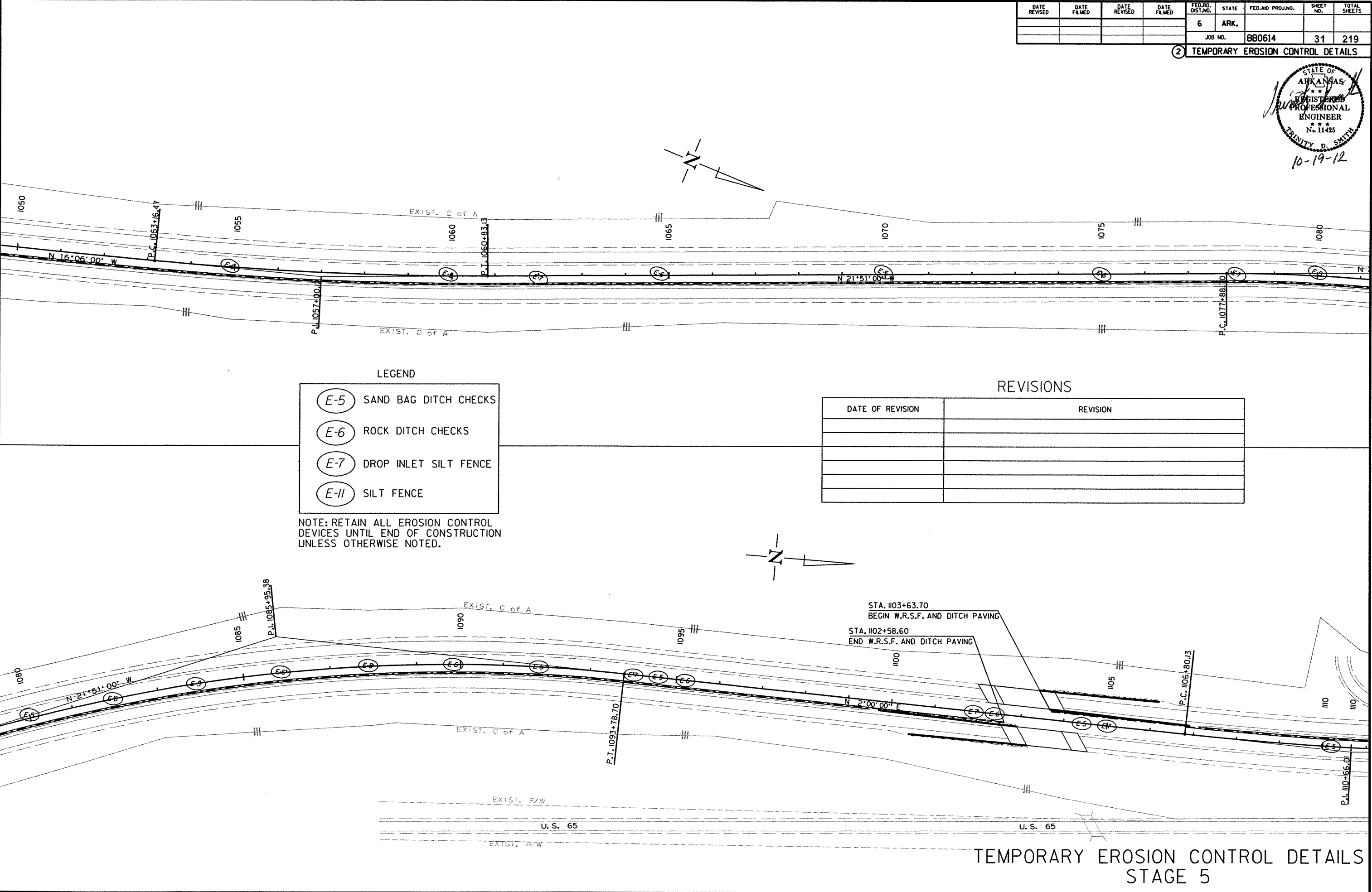
10-19-12



TEMPORARY EROSION CONTROL DETAILS
STAGE 5

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB NO.	880614		31	219

2 TEMPORARY EROSION CONTROL DETAILS



LEGEND

E-5

SAND BAG DITCH CHECKS

E-6

ROCK DITCH CHECKS

E-7

DROP INLET SILT FENCE

E-11

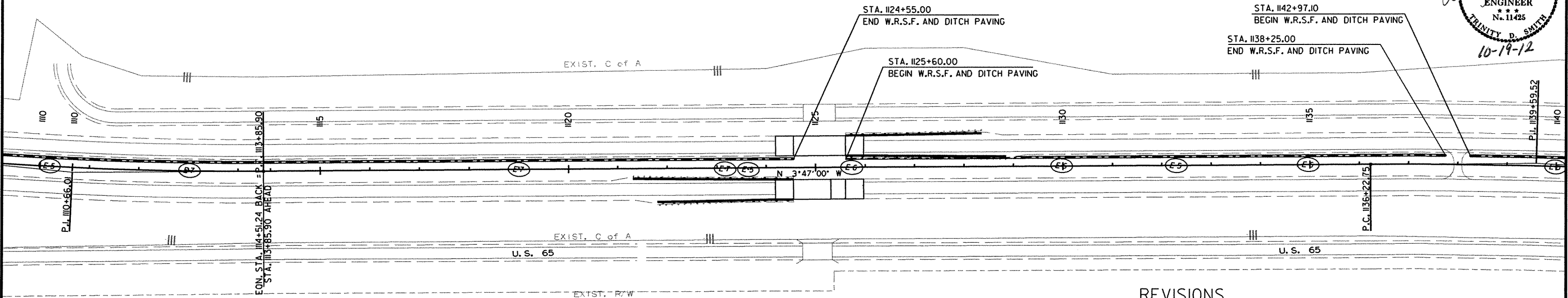
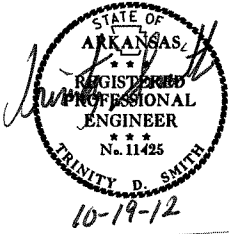
SILT FENCE

NOTE: RETAIN ALL EROSION CONTROL DEVICES UNTIL END OF CONSTRUCTION UNLESS OTHERWISE NOTED.

REVISIONS	
DATE OF REVISION	REVISION

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. AID DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB NO.	BB0614		32	219

2 TEMPORARY EROSION CONTROL DETAILS

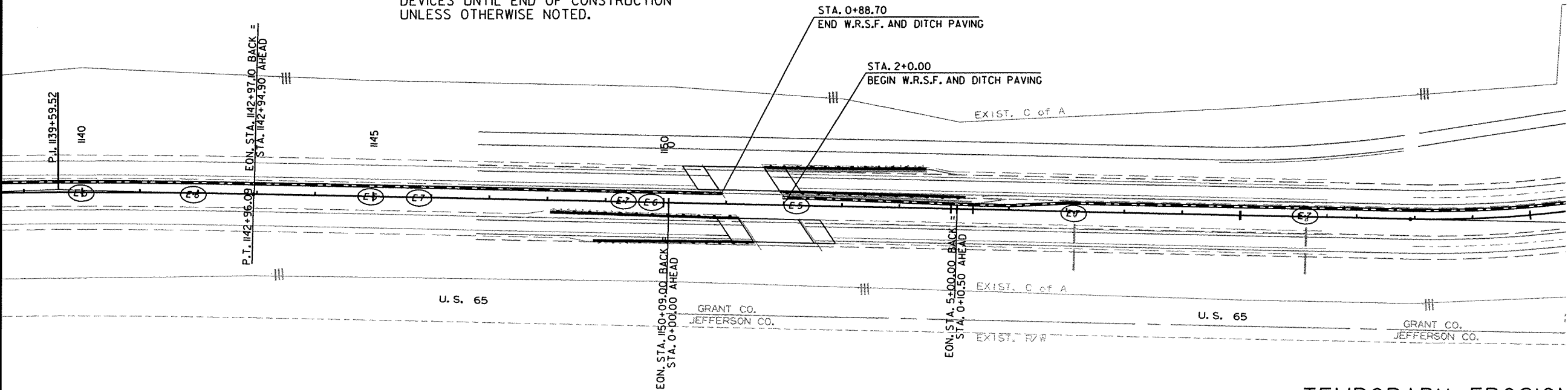
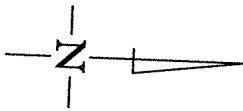


LEGEND

- (E-5) SAND BAG DITCH CHECKS
- (E-6) ROCK DITCH CHECKS
- (E-7) DROP INLET SILT FENCE
- (E-11) SILT FENCE

NOTE: RETAIN ALL EROSION CONTROL DEVICES UNTIL END OF CONSTRUCTION UNLESS OTHERWISE NOTED.

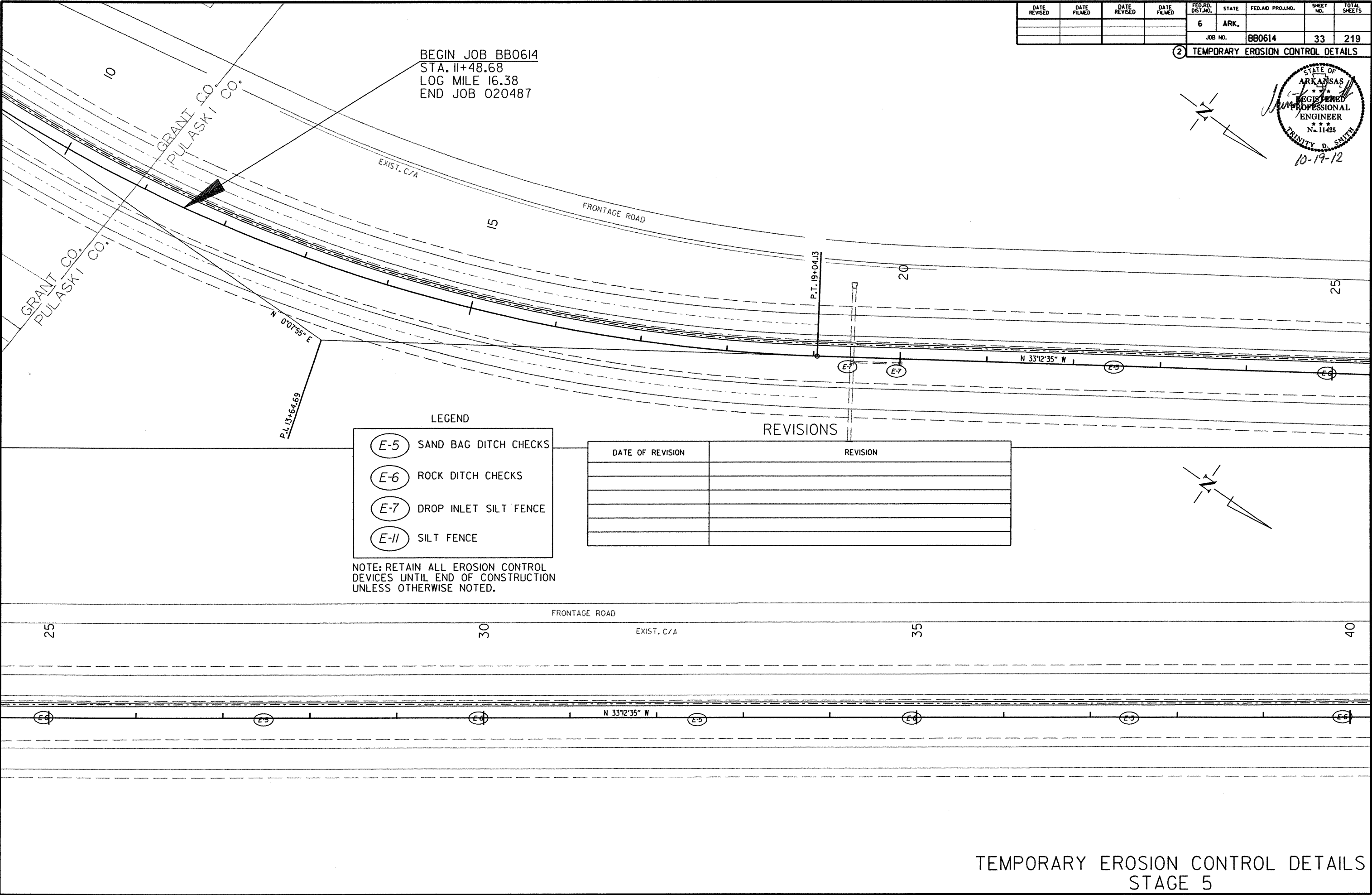
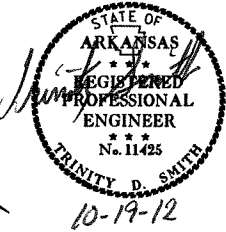
DATE OF REVISION	REVISION



TEMPORARY EROSION CONTROL DETAILS
STAGE 5

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB NO.		BB0614	33	219

② TEMPORARY EROSION CONTROL DETAILS



BEGIN JOB BB0614
STA. 11+48.68
LOG MILE 16.38
END JOB 020487

LEGEND

- (E-5) SAND BAG DITCH CHECKS
- (E-6) ROCK DITCH CHECKS
- (E-7) DROP INLET SILT FENCE
- (E-11) SILT FENCE

NOTE: RETAIN ALL EROSION CONTROL
DEVICES UNTIL END OF CONSTRUCTION
UNLESS OTHERWISE NOTED.

REVISIONS

DATE OF REVISION	REVISION

TEMPORARY EROSION CONTROL DETAILS
STAGE 5

10/18/2012

RB0614.DGN

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB NO.		BB0614	34	219

2 TEMPORARY EROSION CONTROL DETAILS



FRONTAGE ROAD

40

45

50

55

N 33°12'35" W

LEGEND

- (E-5) SAND BAG DITCH CHECKS
- (E-6) ROCK DITCH CHECKS
- (E-7) DROP INLET SILT FENCE
- (E-11) SILT FENCE

NOTE: RETAIN ALL EROSION CONTROL DEVICES UNTIL END OF CONSTRUCTION UNLESS OTHERWISE NOTED.

REVISIONS

DATE OF REVISION	REVISION

STA. 58+45.00
END W.R.S.F. AND DITCH PAVING

55

BR. END 58+55.00

BR. NO.
A3323

BR. END 59+45.00

300' TAPER

EXIST. C/A

FRONTAGE ROAD

700' ACCELERATION LANE

FRONTAGE ROAD

EXIST. C/A

STA. 59+55.00
BEGIN W.R.S.F. AND DITCH PAVING

65

N 33°12'35" W

BR. NO.
B3323

BR. END 58+55.00

BR. END 59+45.00

CAMPBELL CREEK

RAMP 1

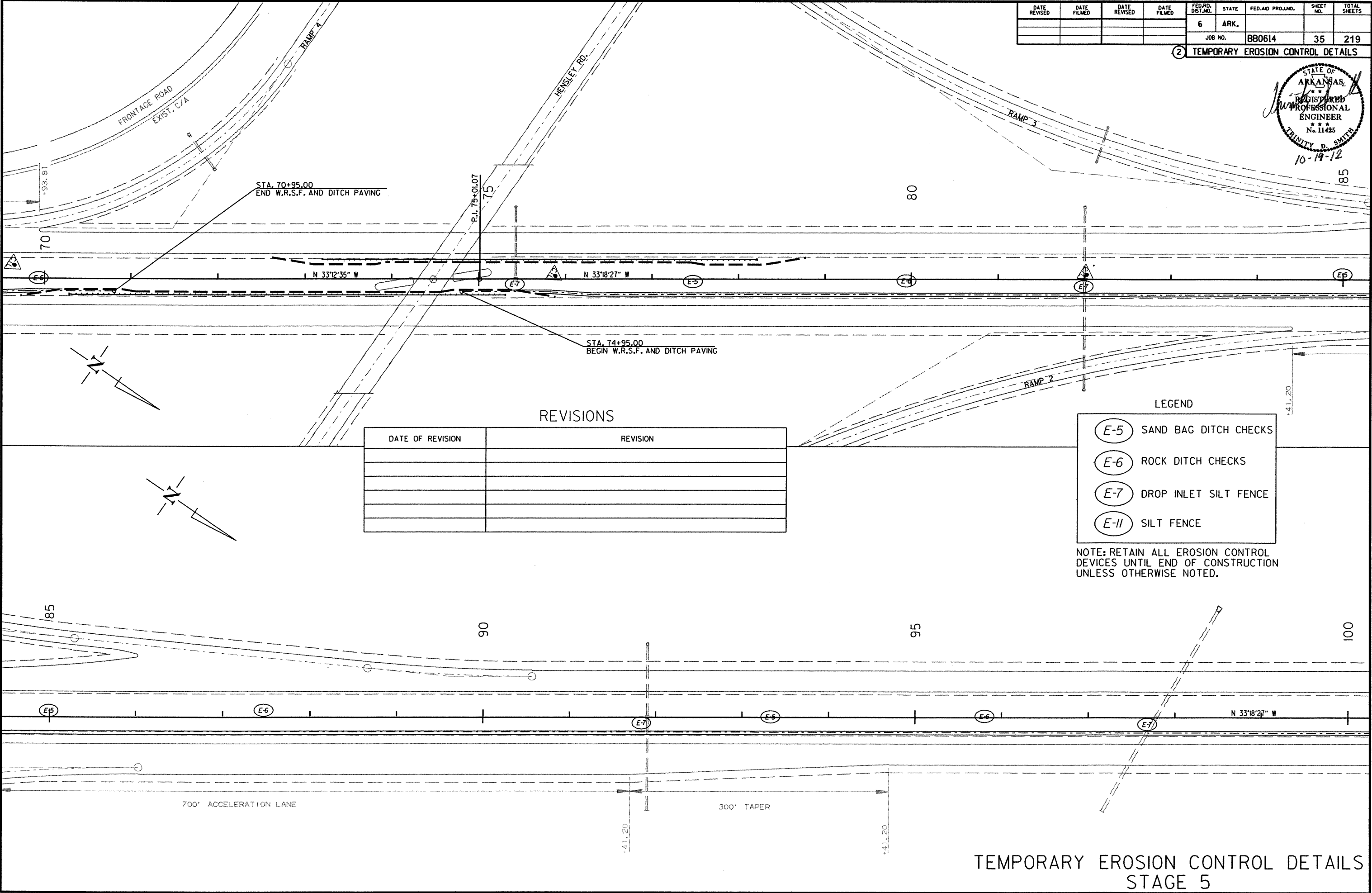
TEMPORARY EROSION CONTROL DETAILS
STAGE 5

10/18/2012

R880614.DGN

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB NO.	BB0614		35	219

2 TEMPORARY EROSION CONTROL DETAILS



REVISIONS

DATE OF REVISION	REVISION

LEGEND

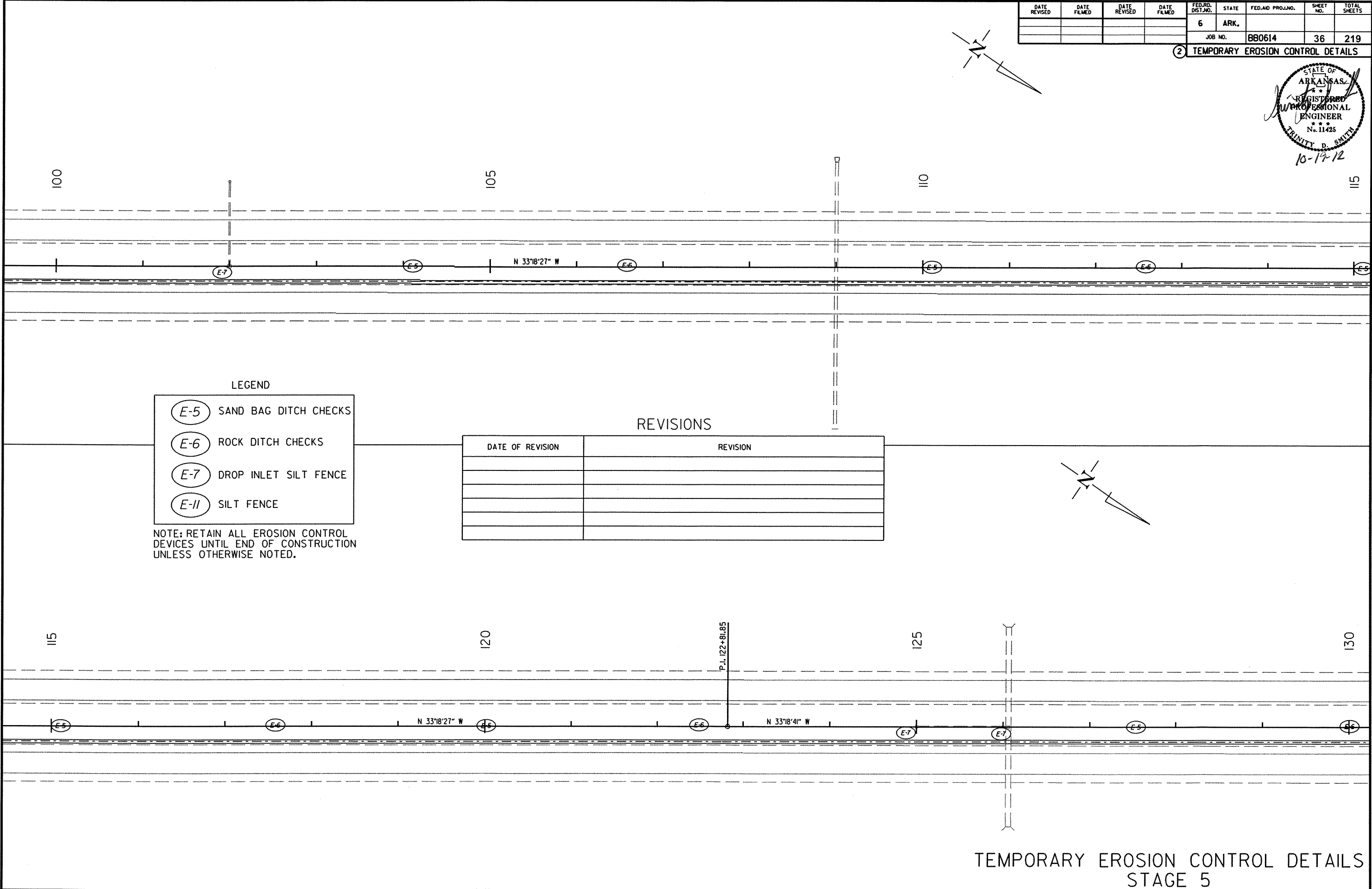
- E-5 SAND BAG DITCH CHECKS
- E-6 ROCK DITCH CHECKS
- E-7 DROP INLET SILT FENCE
- E-11 SILT FENCE

NOTE: RETAIN ALL EROSION CONTROL DEVICES UNTIL END OF CONSTRUCTION UNLESS OTHERWISE NOTED.

TEMPORARY EROSION CONTROL DETAILS
STAGE 5

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
						JOB NO. BB0614	36	219

2 TEMPORARY EROSION CONTROL DETAILS



LEGEND

E-5

SAND BAG DITCH CHECKS

E-6

ROCK DITCH CHECKS

E-7

DROP INLET SILT FENCE

E-11

SILT FENCE

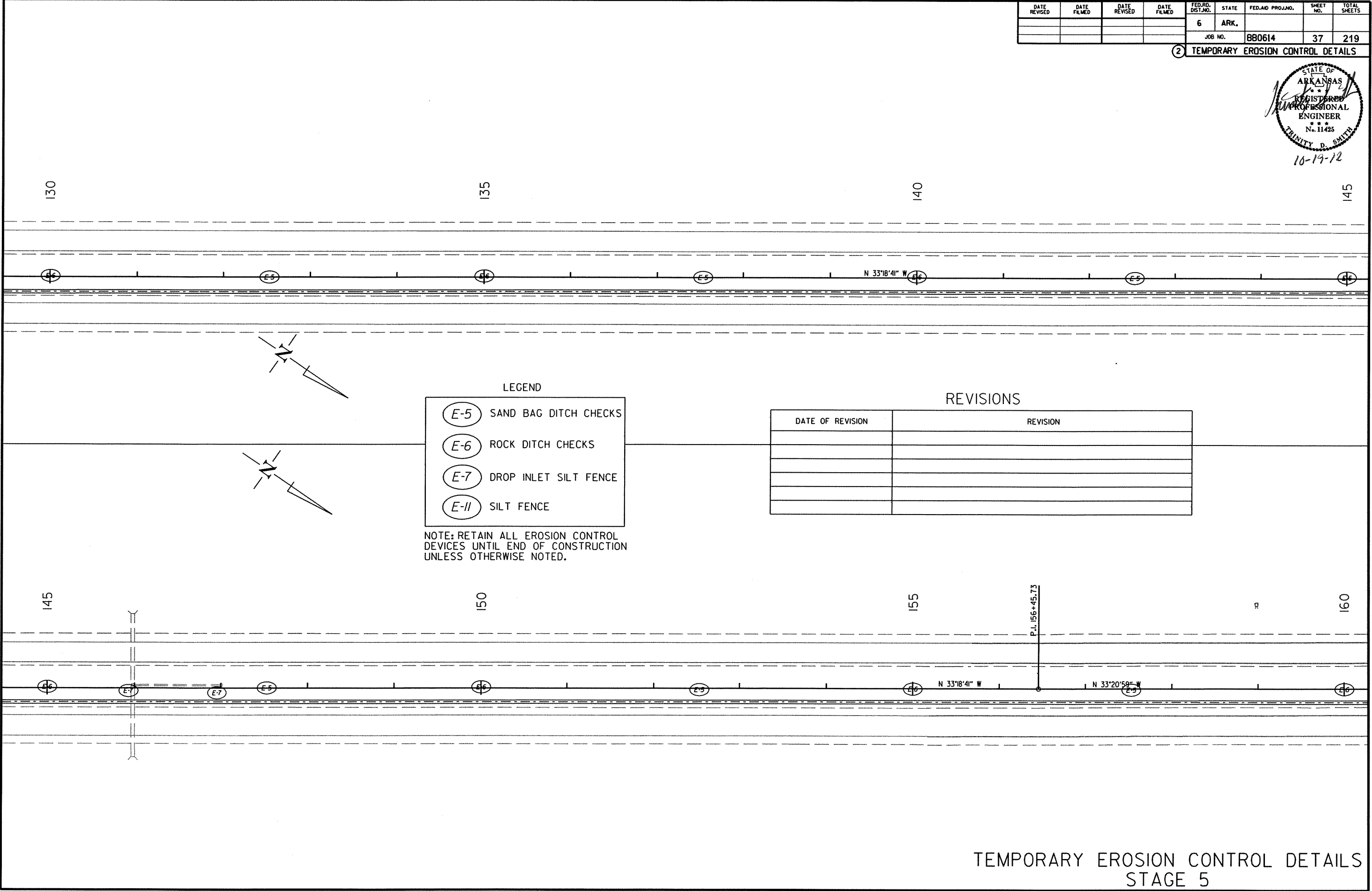
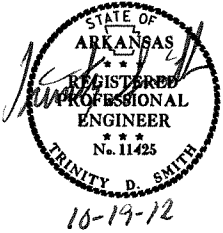
NOTE: RETAIN ALL EROSION CONTROL DEVICES UNTIL END OF CONSTRUCTION UNLESS OTHERWISE NOTED.

REVISIONS	
DATE OF REVISION	REVISION

TEMPORARY EROSION CONTROL DETAILS
STAGE 5

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB NO. 880614		37	219	

2 TEMPORARY EROSION CONTROL DETAILS



LEGEND

E-5

SAND BAG DITCH CHECKS

E-6

ROCK DITCH CHECKS

E-7

DROP INLET SILT FENCE

E-II

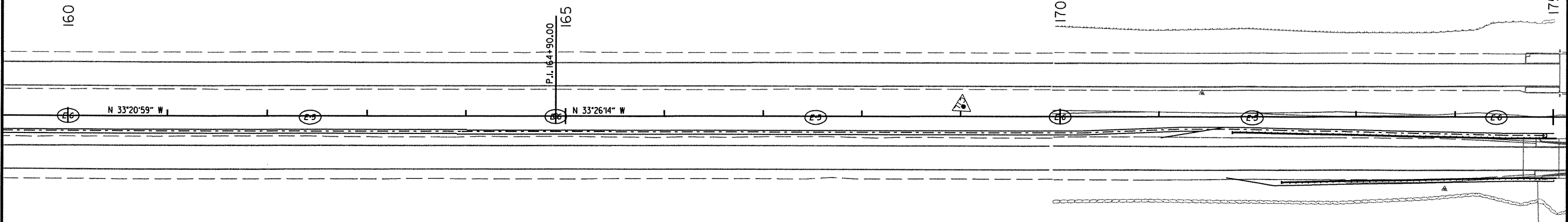
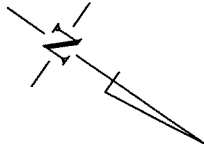
SILT FENCE

NOTE: RETAIN ALL EROSION CONTROL DEVICES UNTIL END OF CONSTRUCTION UNLESS OTHERWISE NOTED.

REVISIONS	
DATE OF REVISION	REVISION

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB NO. BB0614		38	219	

2 TEMPORARY EROSION CONTROL DETAILS



LEGEND

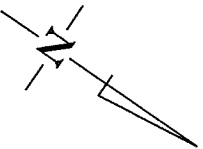
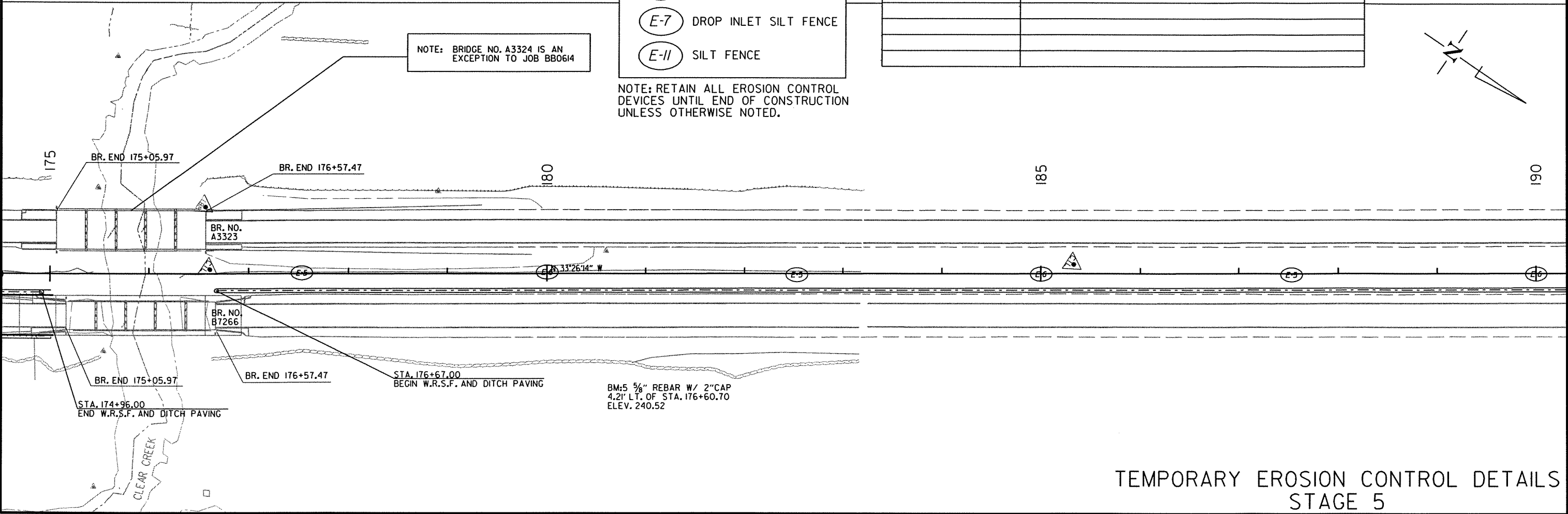
- (E-5) SAND BAG DITCH CHECKS
- (E-6) ROCK DITCH CHECKS
- (E-7) DROP INLET SILT FENCE
- (E-11) SILT FENCE

NOTE: RETAIN ALL EROSION CONTROL DEVICES UNTIL END OF CONSTRUCTION UNLESS OTHERWISE NOTED.

REVISIONS

DATE OF REVISION	REVISION

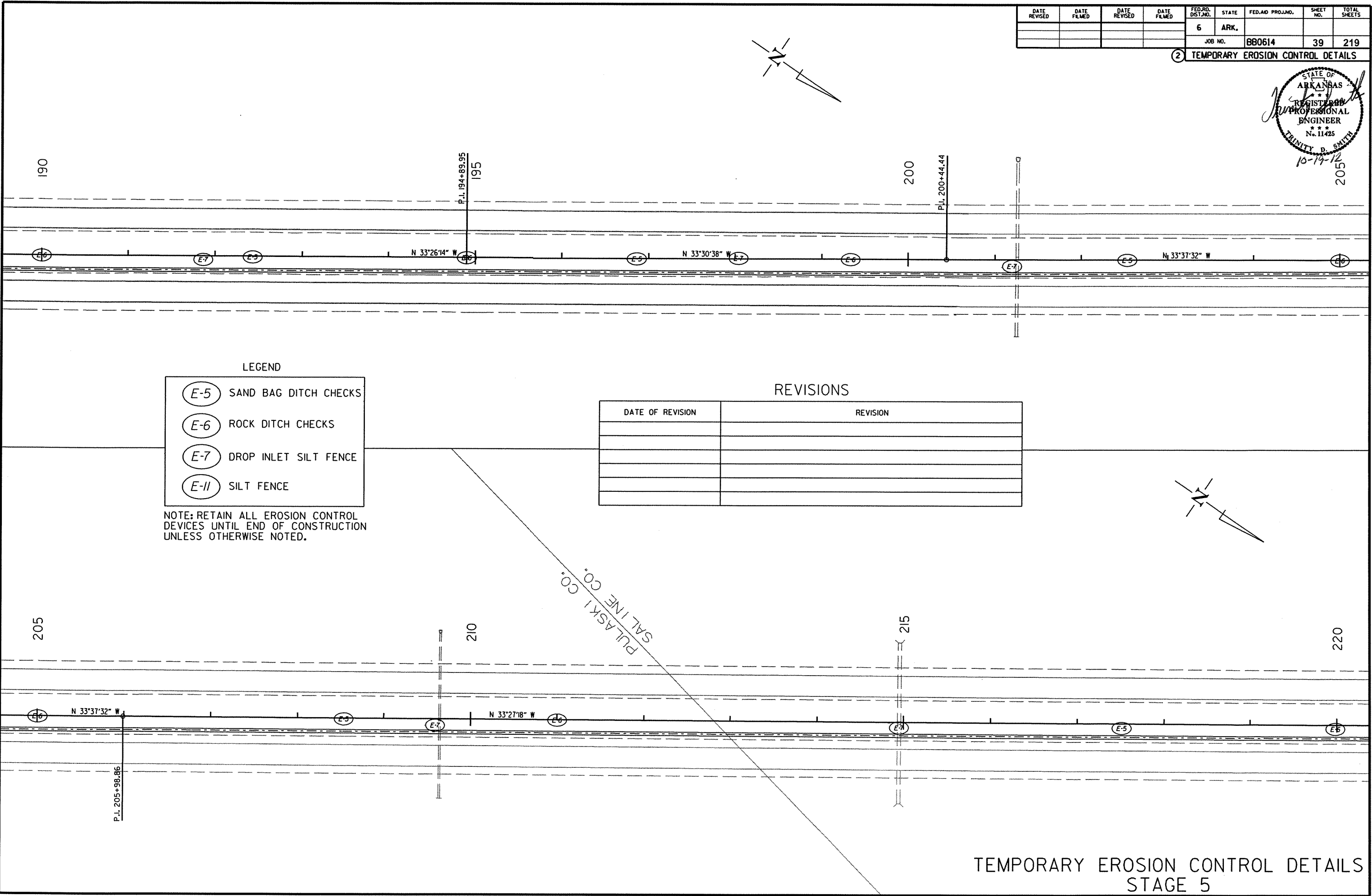
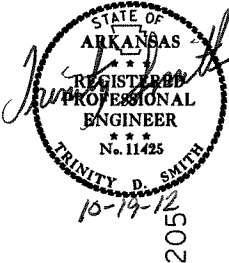
NOTE: BRIDGE NO. A3324 IS AN EXCEPTION TO JOB BB0614



TEMPORARY EROSION CONTROL DETAILS
STAGE 5

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB NO.	BB0614		39	219

② TEMPORARY EROSION CONTROL DETAILS



LEGEND

- (E-5) SAND BAG DITCH CHECKS
- (E-6) ROCK DITCH CHECKS
- (E-7) DROP INLET SILT FENCE
- (E-11) SILT FENCE

NOTE: RETAIN ALL EROSION CONTROL DEVICES UNTIL END OF CONSTRUCTION UNLESS OTHERWISE NOTED.

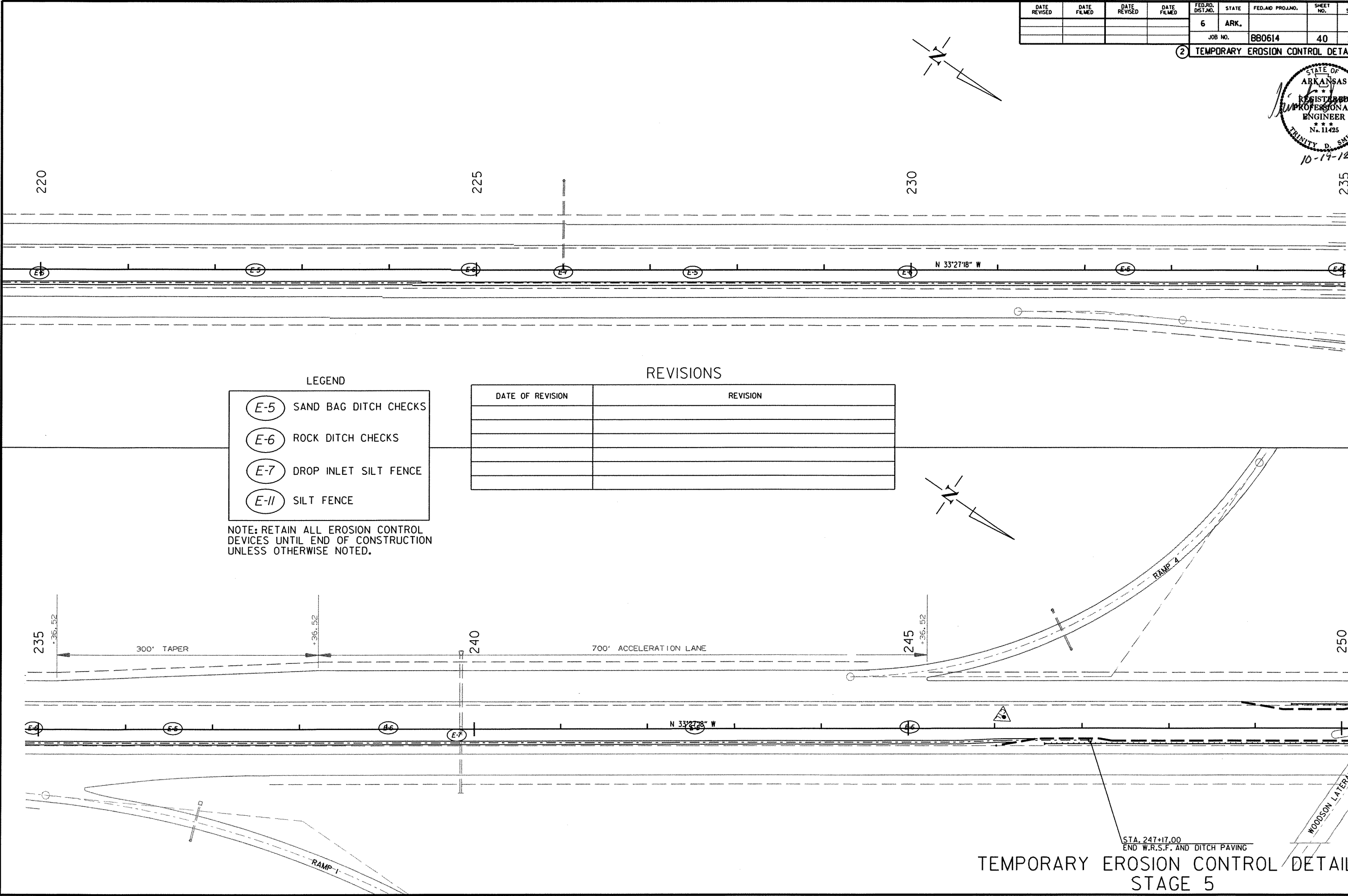
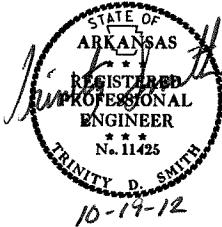
REVISIONS

DATE OF REVISION	REVISION

TEMPORARY EROSION CONTROL DETAILS
STAGE 5

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
						JOB NO. 880614	40	219

2 TEMPORARY EROSION CONTROL DETAILS



LEGEND

- E-5 SAND BAG DITCH CHECKS
- E-6 ROCK DITCH CHECKS
- E-7 DROP INLET SILT FENCE
- E-11 SILT FENCE

NOTE: RETAIN ALL EROSION CONTROL DEVICES UNTIL END OF CONSTRUCTION UNLESS OTHERWISE NOTED.

REVISIONS

DATE OF REVISION	REVISION

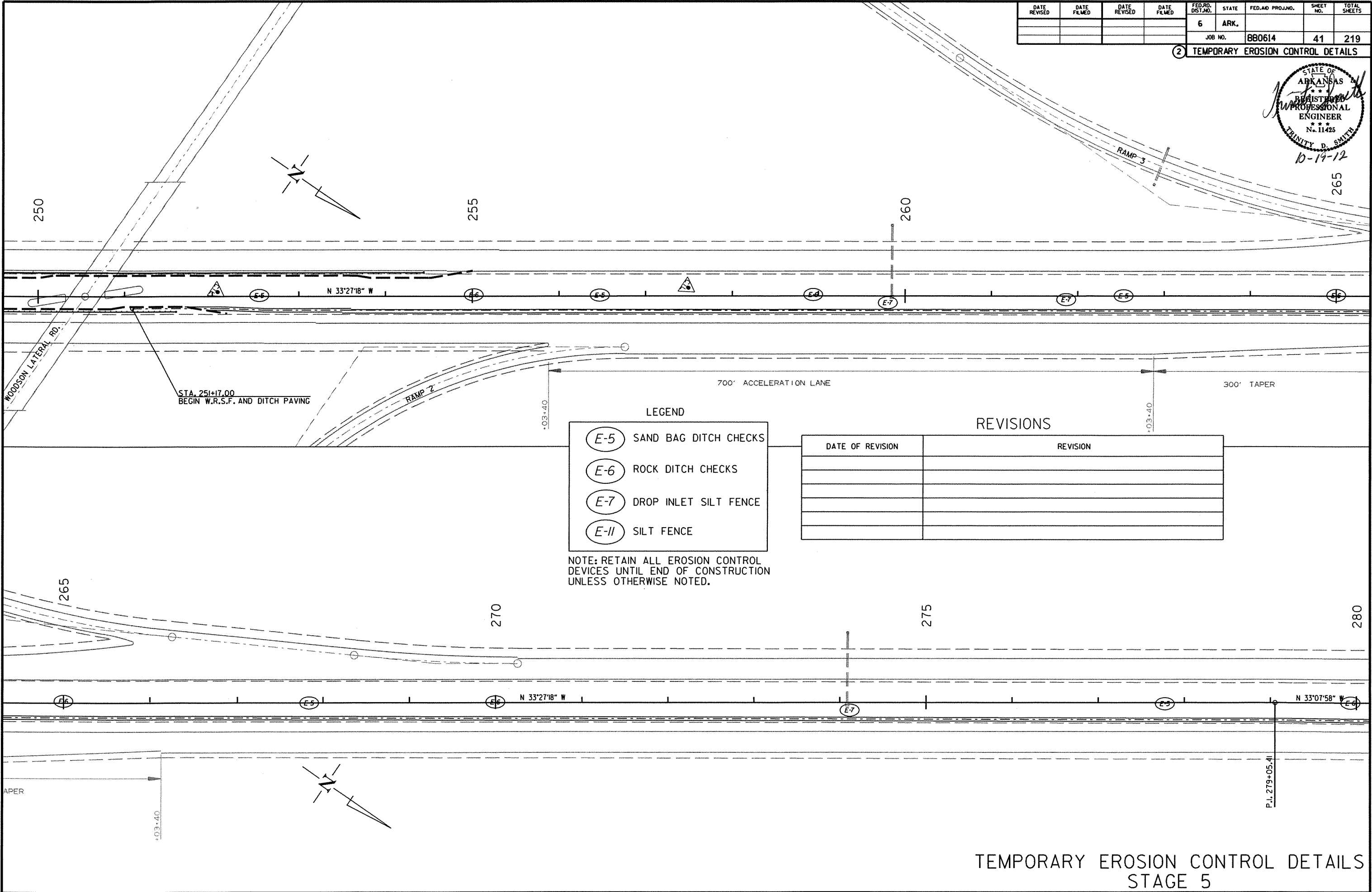
10/18/2012

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TEMPORARY EROSION CONTROL DETAILS
STAGE 5

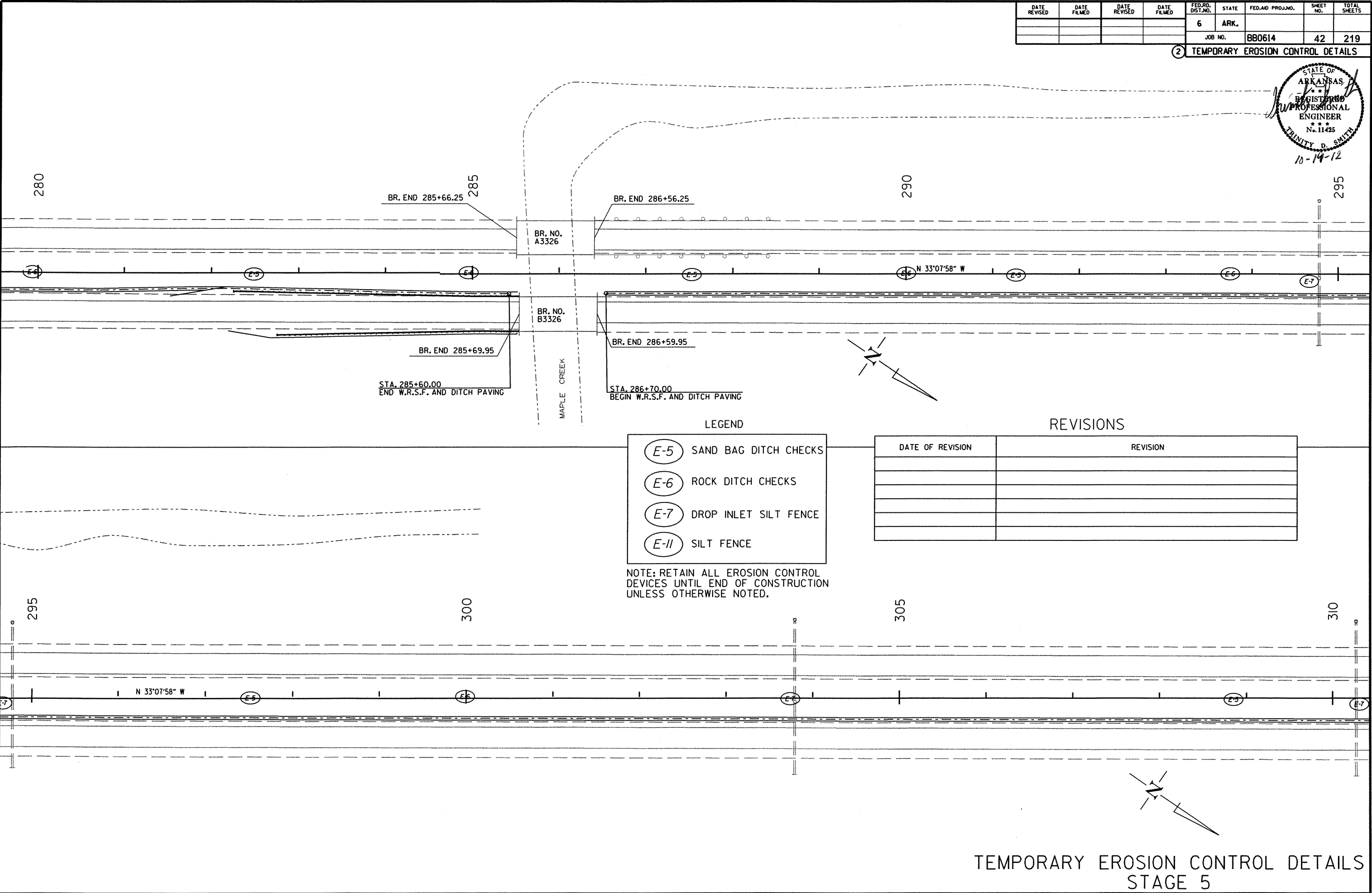
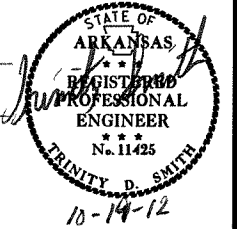
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				6	ARK.			
				JOB NO.		BB0614	41	219

2 TEMPORARY EROSION CONTROL DETAILS



DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB NO.		BB0614	42	219

2 TEMPORARY EROSION CONTROL DETAILS



LEGEND

- E-5 SAND BAG DITCH CHECKS
- E-6 ROCK DITCH CHECKS
- E-7 DROP INLET SILT FENCE
- E-11 SILT FENCE

NOTE: RETAIN ALL EROSION CONTROL DEVICES UNTIL END OF CONSTRUCTION UNLESS OTHERWISE NOTED.

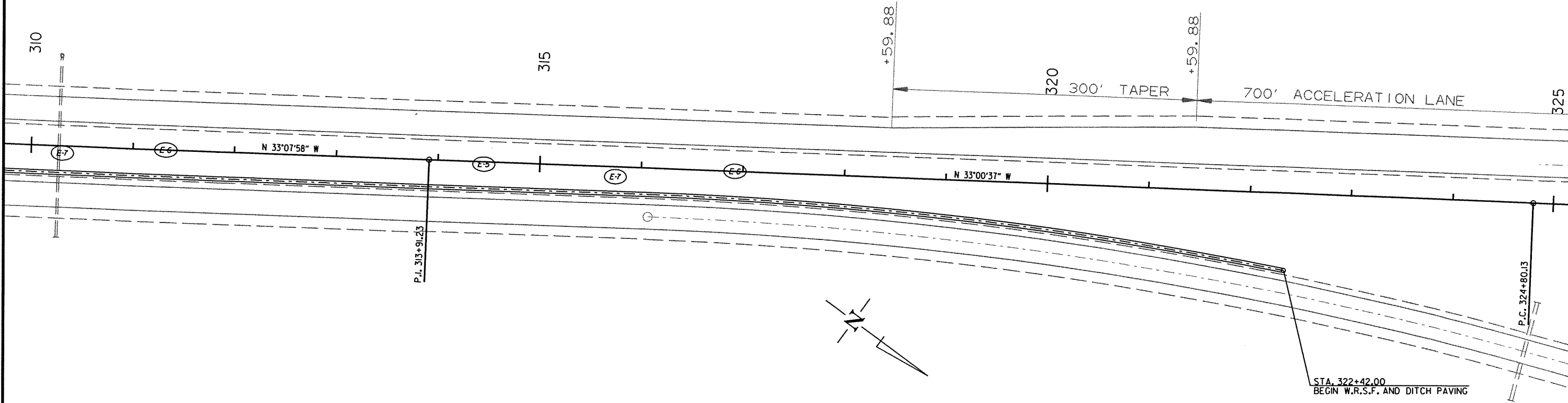
REVISIONS

DATE OF REVISION	REVISION

TEMPORARY EROSION CONTROL DETAILS
STAGE 5

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
						JOB NO. 880614	43	219

② TEMPORARY EROSION CONTROL DETAILS



LEGEND

- E-5 SAND BAG DITCH CHECKS
- E-6 ROCK DITCH CHECKS
- E-7 DROP INLET SILT FENCE
- E-11 SILT FENCE

NOTE: RETAIN ALL EROSION CONTROL DEVICES UNTIL END OF CONSTRUCTION UNLESS OTHERWISE NOTED.

REVISIONS

DATE OF REVISION	REVISION

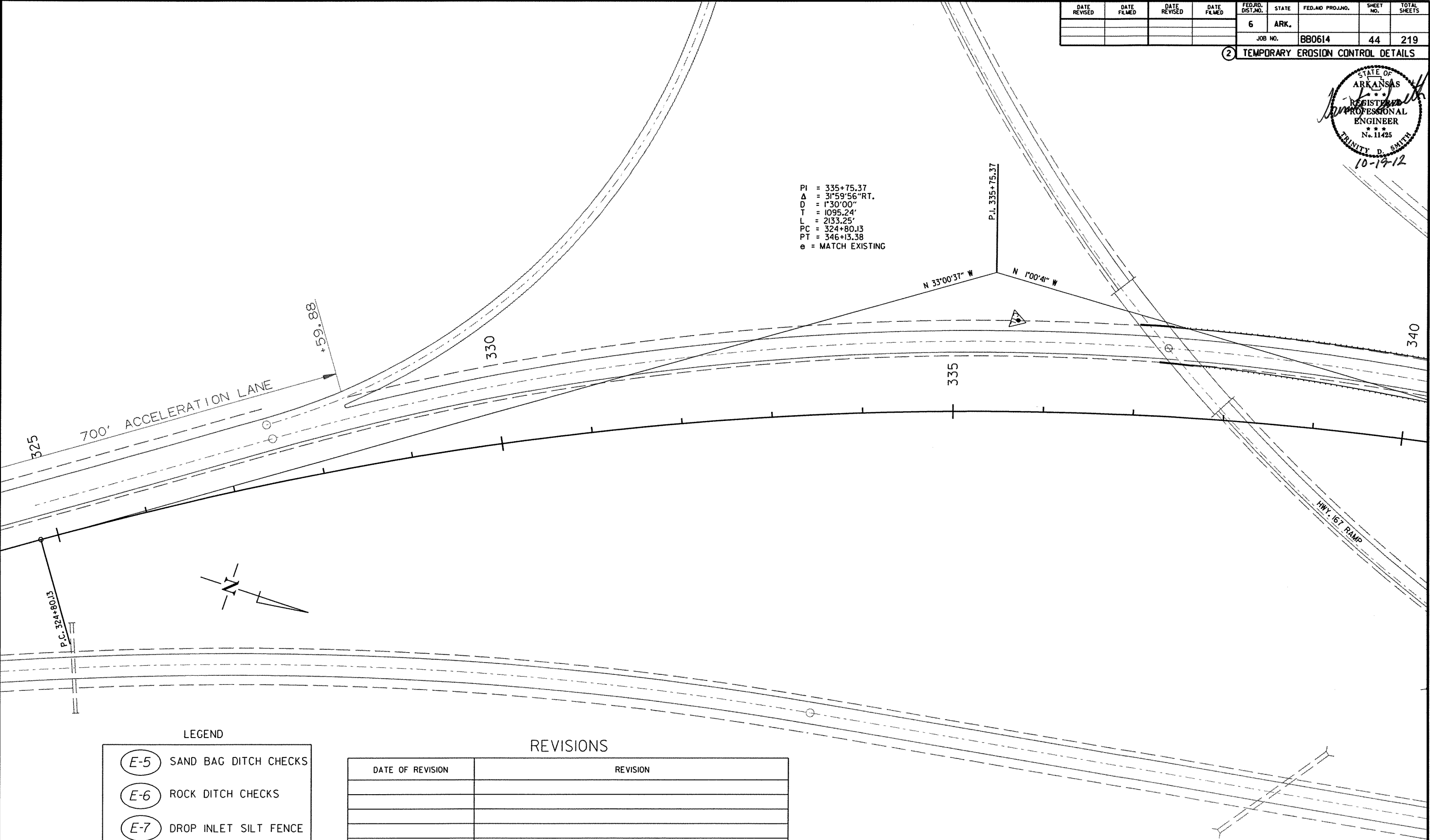
TEMPORARY EROSION CONTROL DETAILS
STAGE 5

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB NO. BB0614		44	219	

2 TEMPORARY EROSION CONTROL DETAILS



PI = 335+75.37
Δ = 31°59'56" RT.
D = 1°30'00"
T = 1095.24'
L = 2133.25'
PC = 324+80.13
PT = 346+13.38
e = MATCH EXISTING



LEGEND

- (E-5) SAND BAG DITCH CHECKS
- (E-6) ROCK DITCH CHECKS
- (E-7) DROP INLET SILT FENCE
- (E-11) SILT FENCE

NOTE: RETAIN ALL EROSION CONTROL DEVICES UNTIL END OF CONSTRUCTION UNLESS OTHERWISE NOTED.

REVISIONS

DATE OF REVISION	REVISION

TEMPORARY EROSION CONTROL DETAILS
STAGE 5

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB NO. BB0614		45	219	

2 TEMPORARY EROSION CONTROL DETAILS



LEGEND

E-5

SAND BAG DITCH CHECKS

E-6

ROCK DITCH CHECKS

E-7

DROP INLET SILT FENCE

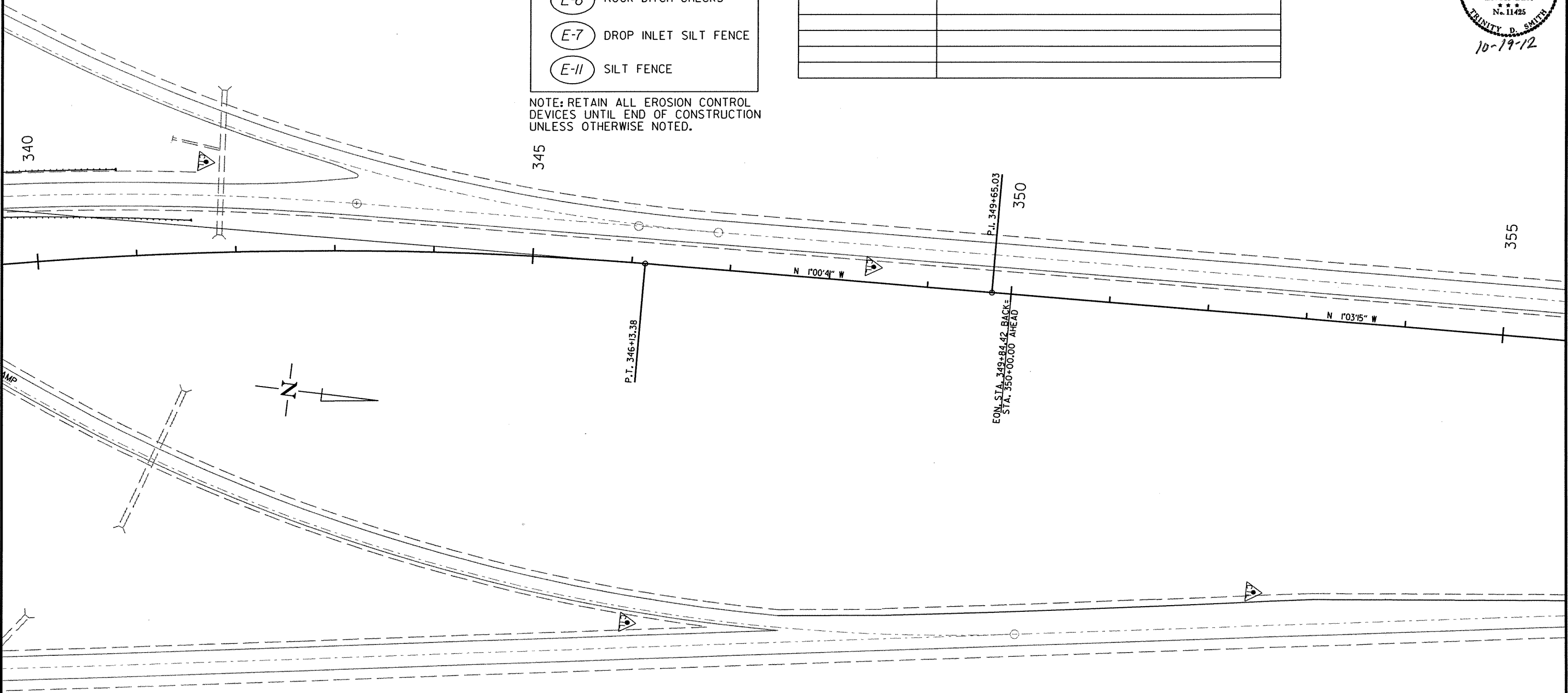
E-11

SILT FENCE

NOTE: RETAIN ALL EROSION CONTROL DEVICES UNTIL END OF CONSTRUCTION UNLESS OTHERWISE NOTED.

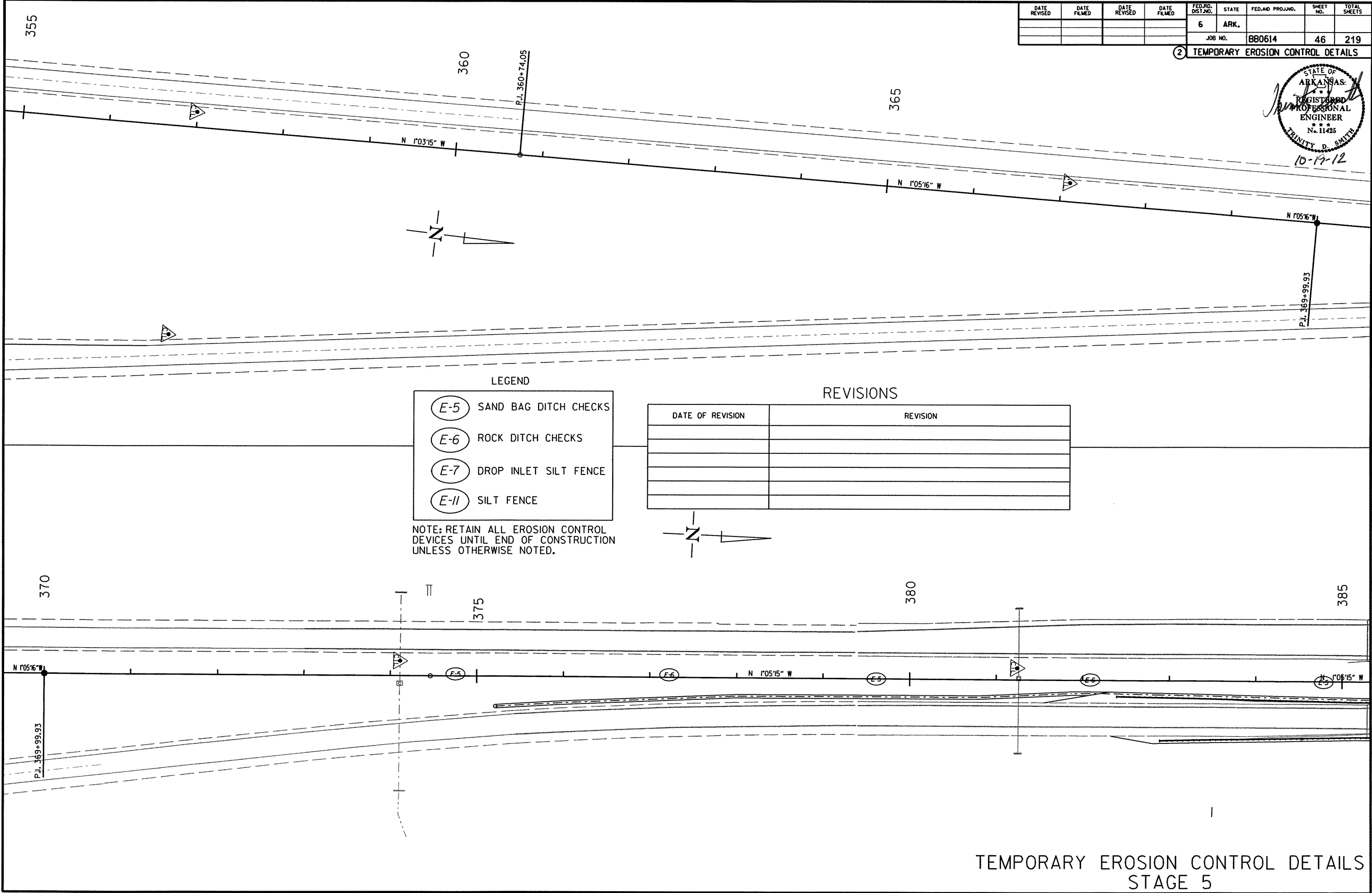
REVISIONS

DATE OF REVISION	REVISION



TEMPORARY EROSION CONTROL DETAILS
STAGE 5

10/19/2012
RB80614.DGN



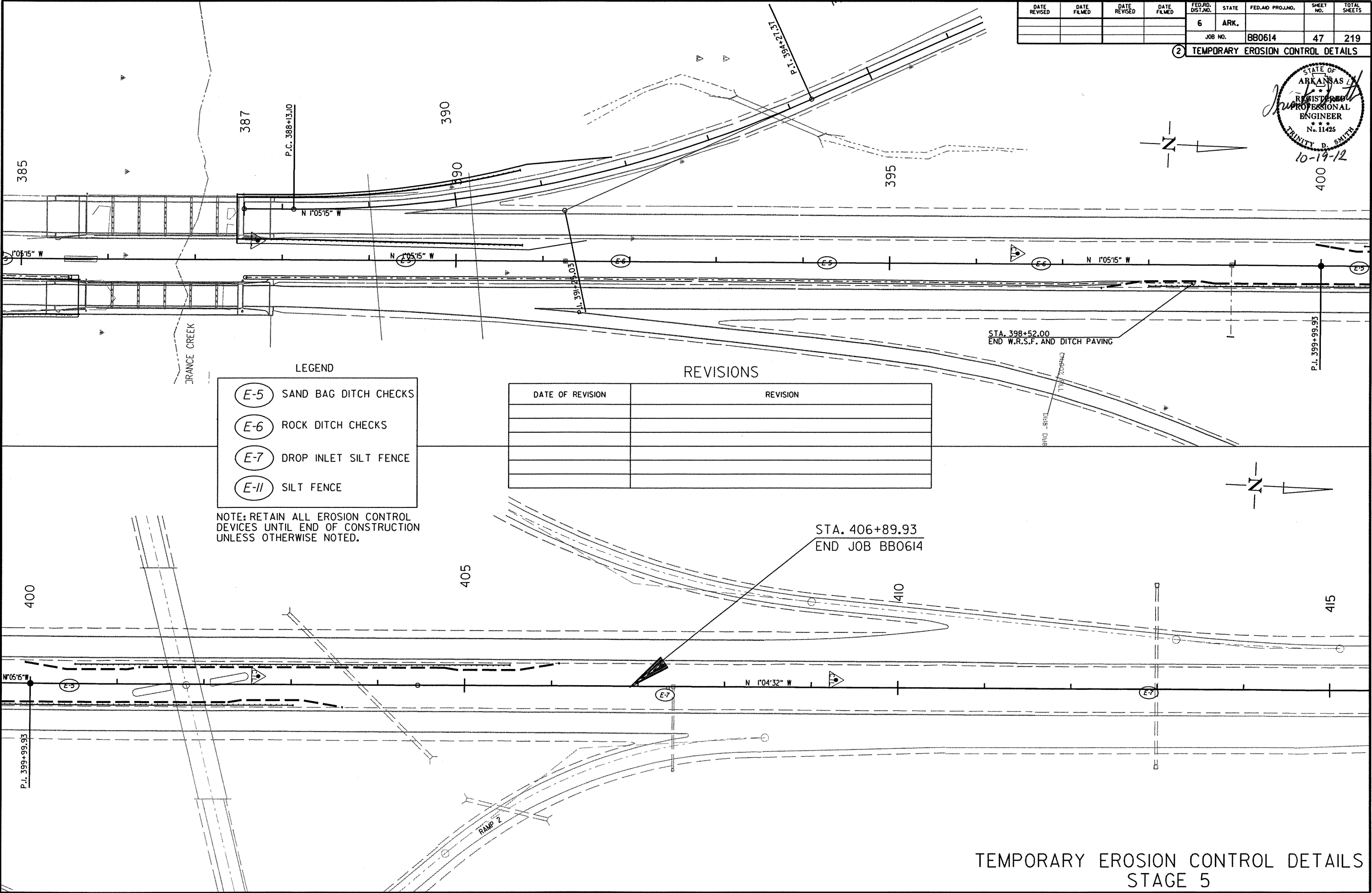
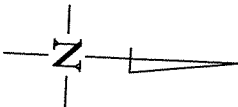
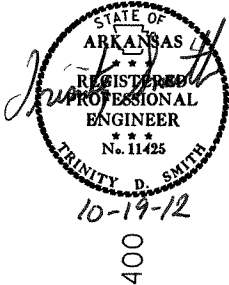
DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB NO.	BB0614		46	219

2 TEMPORARY EROSION CONTROL DETAILS



DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB NO.	BB0614		47	219

2 TEMPORARY EROSION CONTROL DETAILS



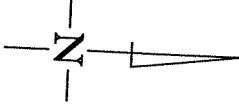
LEGEND

- (E-5) SAND BAG DITCH CHECKS
- (E-6) ROCK DITCH CHECKS
- (E-7) DROP INLET SILT FENCE
- (E-II) SILT FENCE

NOTE: RETAIN ALL EROSION CONTROL DEVICES UNTIL END OF CONSTRUCTION UNLESS OTHERWISE NOTED.

REVISIONS

DATE OF REVISION	REVISION



TEMPORARY EROSION CONTROL DETAILS
STAGE 5

10/18/2012

RBB0614.DGN

SEQUENCE OF CONSTRUCTION
STAGE 1A
SHIFT TRAFFIC TO INSIDE LANES AT BINGHAM RD. INTERCHANGE.
CONSTRUCT TEMPORARY RAMPS BETWEEN MAIN LANES AND RAMPS.

STAGE 1B
SHIFT TRAFFIC TO THE OUTSIDE LANES ON MAIN LANES AT BINGHAM RD. INTERCHANGE.
CONSTRUCT CROSSOVERS, TEMPORARY RAMPS, AND ACCEL. LANES IN MEDIAN AT BINGHAM RD. INTERCHANGE.
RECONSTRUCT INSIDE SHOULDERS WHERE SHOWN IN PLANS.

STAGE 2
INSTALL PRECAST CONCRETE BARRIER ONTO EXISTING NB LANES IN AREAS SHOWN.
ROUTE SB TRAFFIC TO INSIDE LANE OF NB LANES THROUGH CROSSOVERS.
RECONSTRUCT SB BRIDGE, APPROACH GUTTERS, & SLABS OVER LORANCE CREEK.
RECONSTRUCT SB LANES BETWEEN STA. 382+35 TO STA. 392+80.
RECONSTRUCT I-530 SB ENTRANCE RAMP AT BINGHAM RD. INTERCHANGE.

STAGE 3A
ROUTE SB TRAFFIC ONTO OUTSIDE OF RECONSTRUCTED SB LANES.
LEAVE NB TRAFFIC ON OUTSIDE NB LANES.
CONSTRUCT TEMPORARY CROSSOVERS IN AREAS SHOWN.

STAGE 3B
INSTALL PRECAST CONCRETE BARRIER ONTO EXISTING SB LANES IN AREAS SHOWN.
ROUTE NB TRAFFIC ONTO INSIDE OF SB LANES THROUGH CROSSOVERS.
RECONSTRUCT NB BRIDGES, APPROACH GUTTERS, & SLABS OVER LORANCE CREEK.
RECONSTRUCT NB LANES BETWEEN STA. 382+35 TO STA. 392+80.

STAGE 3C
RECONSTRUCT NB BRIDGES, APPROACH GUTTERS, & SLAB OVER CLEAR CREEK.

STAGE 3D
CONDUCT HYDRODEMOLITION/REHABILITATION OPERATIONS AT CAMPBELL CREEK & MAPLE CREEK.

STAGE 4A
ROUTE NB TRAFFIC TO OUTSIDE OF RECONSTRUCTED NB LANES.
RETAIN SB TRAFFIC IN OUTSIDE SB LANE.
REMOVE PCCB AND REPLACE WITH TRAFFIC DRUMS.
OBLITERATE TEMPORARY CROSSOVERS AND TEMPORARY RAMPS IN MEDIAN.
MILL AND INLAY INSIDE LANES.

STAGE 4B
SHIFT NB TRAFFIC TO INSIDE OF NB LANES.
SHIFT SB TRAFFIC TO INSIDE OF SB LANES.
OBLITERATE REMAINING TEMPORARY RAMPS.
OUTSIDE OF PROPOSED MAIN LANES.
MILL AND INLAY OUTSIDE LANES.

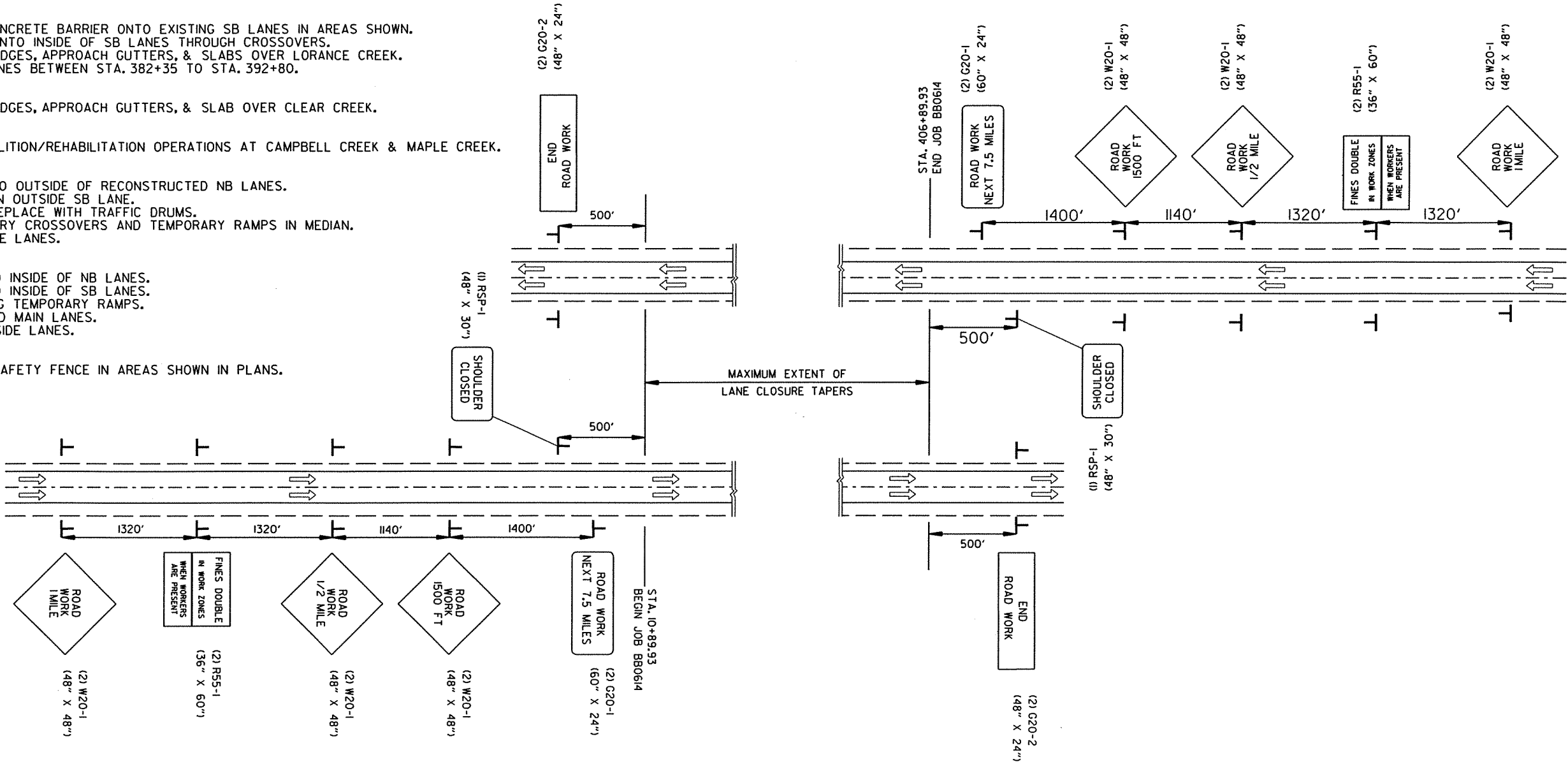
STAGE 5
INSTALL WIRE ROPE SAFETY FENCE IN AREAS SHOWN IN PLANS.

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB NO.	BB0614		48	219

2 MAINTENANCE OF TRAFFIC DETAILS



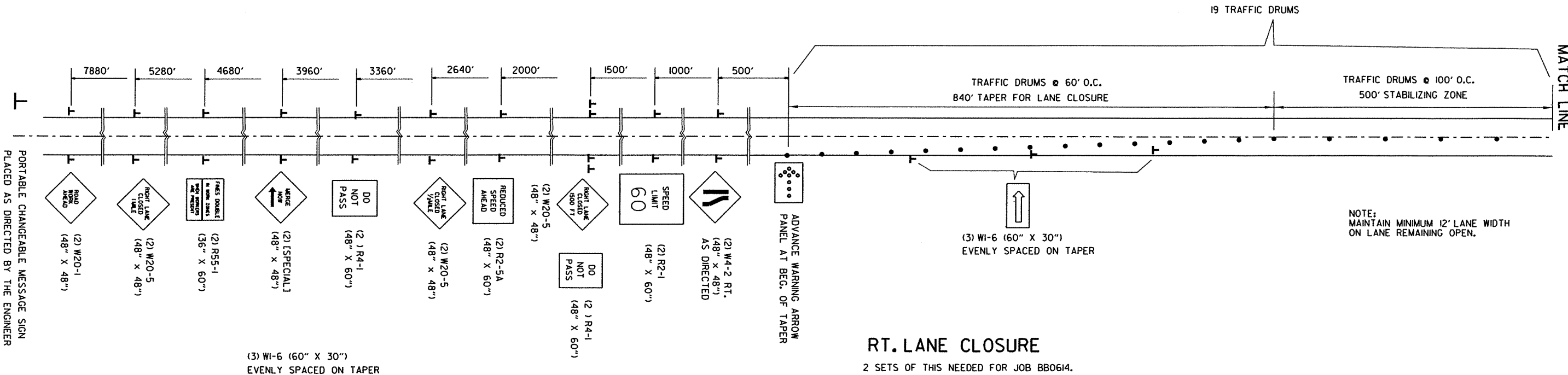
PORTABLE CHANGEABLE MESSAGE SIGN



MAINTENANCE OF TRAFFIC DETAILS
ADVANCE SIGNS AT JOB ENDS

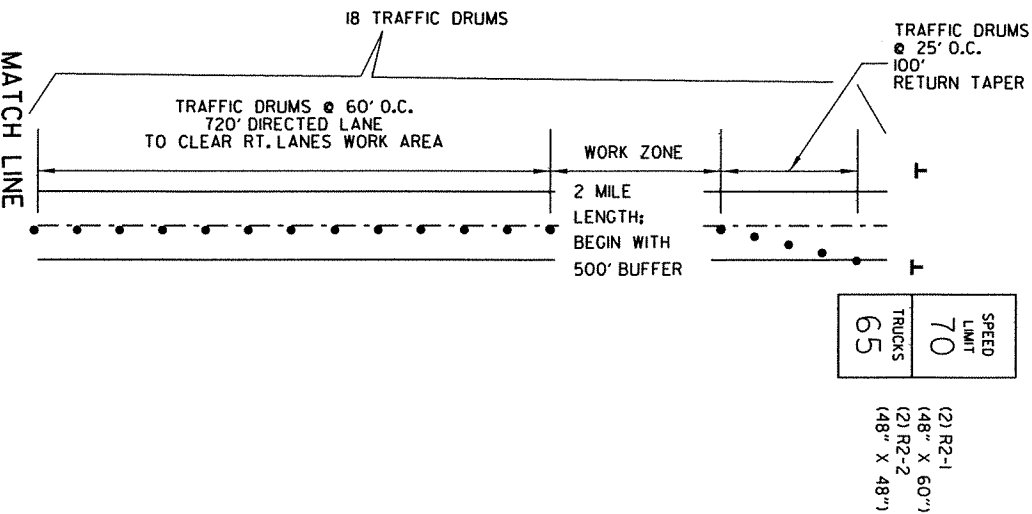
DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB NO.	BB0614		49	219

2 MAINTENANCE OF TRAFFIC DETAILS



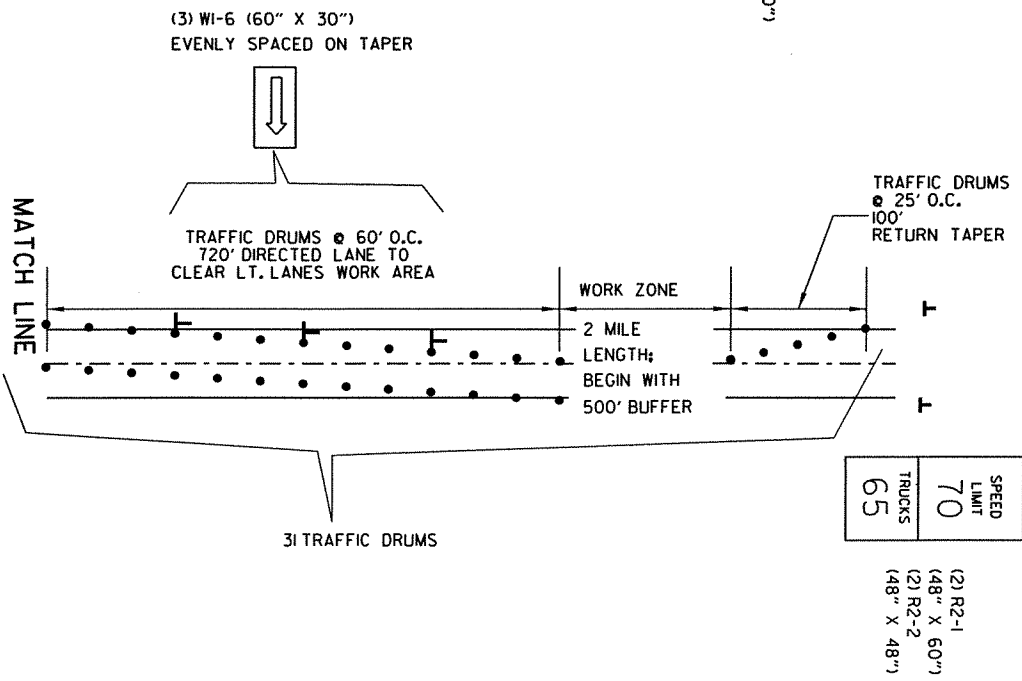
RT. LANE CLOSURE

2 SETS OF THIS NEEDED FOR JOB BB0614.



DIVERSION FOR RT. LANE WORK ZONE

2 SETS OF THIS NEEDED FOR JOB BB0614.



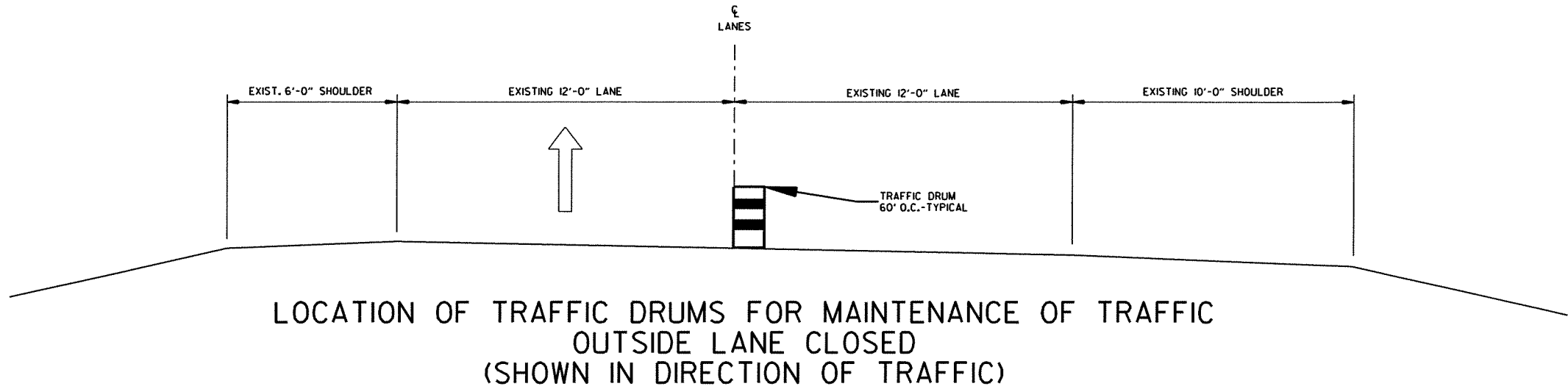
DIVERSION FOR LT. LANE WORK ZONE

2 SETS OF THIS NEEDED FOR JOB BB0614.

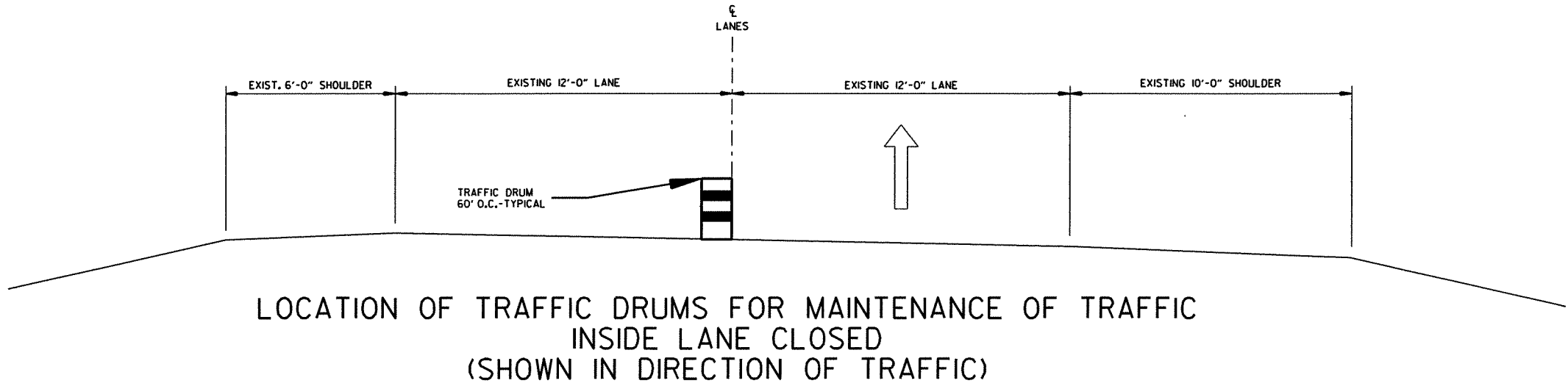
MAINTENANCE OF TRAFFIC DETAILS WORK ZONE - LANE CLOSURE

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB NO.		BB0614	50	219

2 MAINTENANCE OF TRAFFIC DETAILS



LOCATION OF TRAFFIC DRUMS FOR MAINTENANCE OF TRAFFIC
OUTSIDE LANE CLOSED
(SHOWN IN DIRECTION OF TRAFFIC)

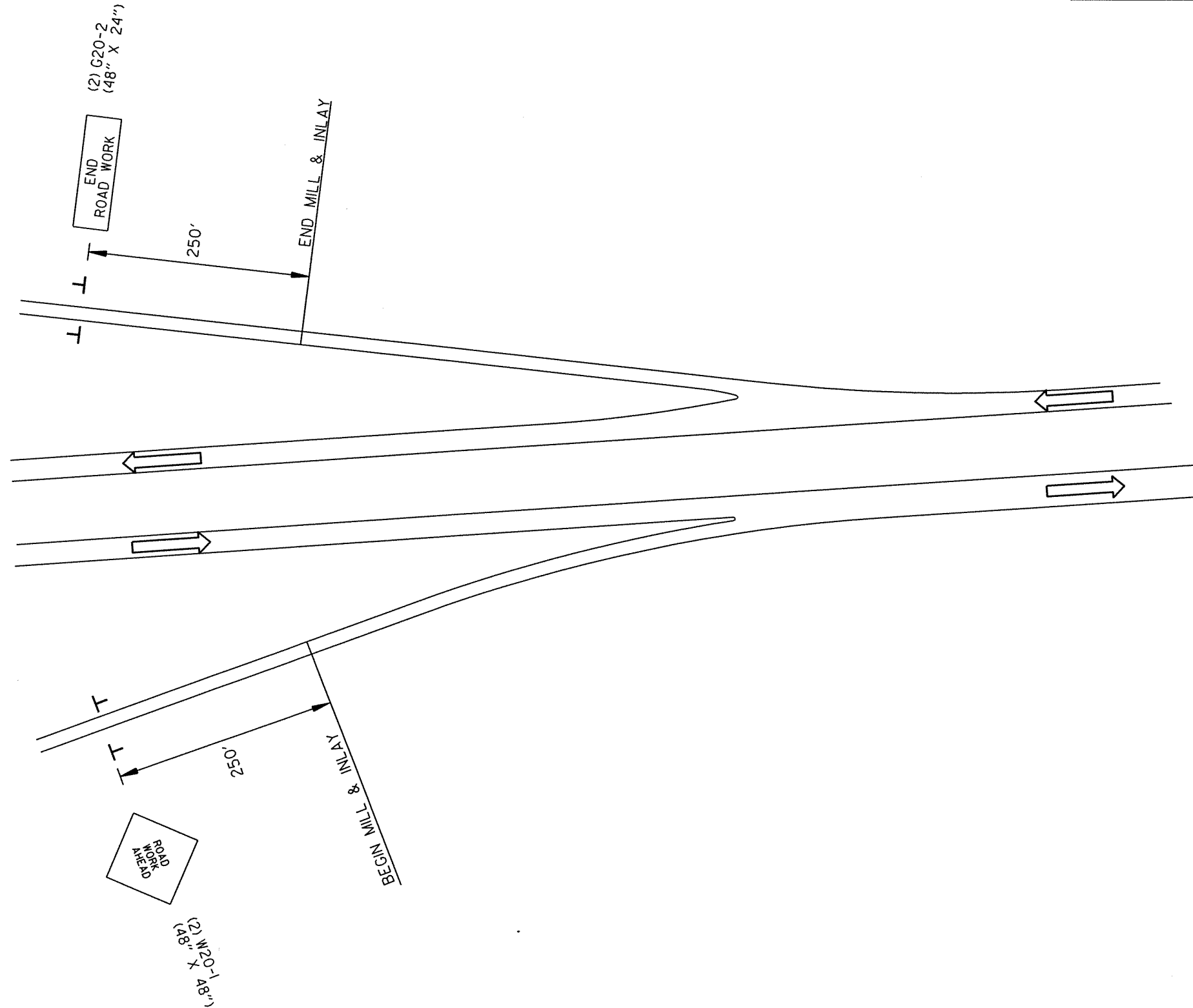
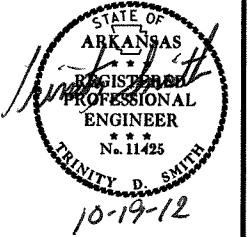


LOCATION OF TRAFFIC DRUMS FOR MAINTENANCE OF TRAFFIC
INSIDE LANE CLOSED
(SHOWN IN DIRECTION OF TRAFFIC)

ADVANCE WARNING SIGNS FOR ENTRANCE AND EXIT RAMP
ROAD WORK AHEAD (16) = 256 SQ. FT.
END ROAD WORK (16) = 128 SQ. FT.

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB NO.		BB0614	51	219

② MAINTENANCE OF TRAFFIC DETAILS



DETAIL OF ENTRANCE AND EXIT RAMP

1. HENSLEY RD. (2 SETS)
2. WOODSON LATERAL RD. (2 SETS)
3. HWY. 167 (1 SET)
4. BINGHAM RD. (2 SETS)

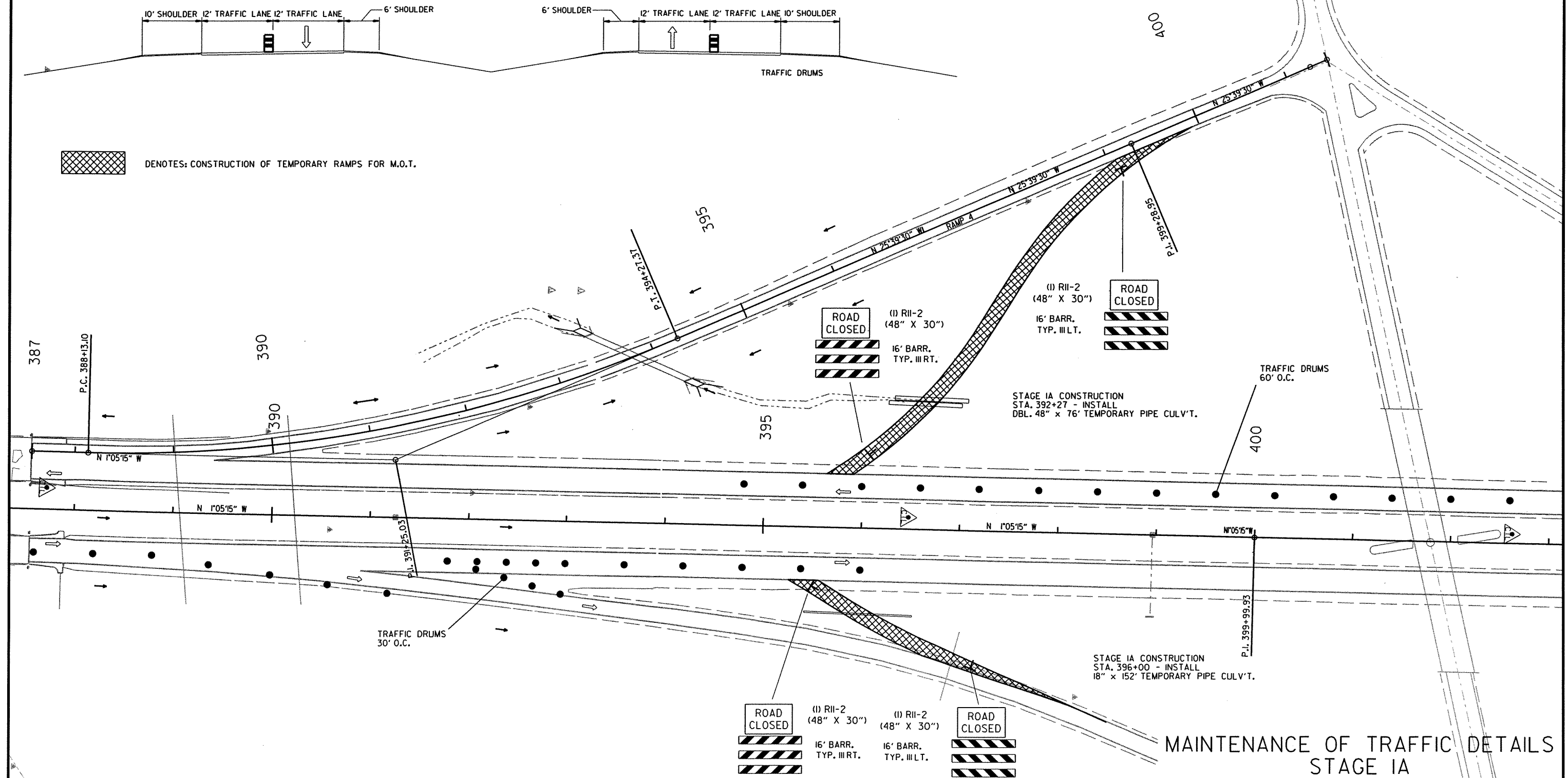
MAINTENANCE OF TRAFFIC
DETAIL OF RAMPS

SEQUENCE OF CONSTRUCTION
STAGE IA
SHIFT TRAFFIC TO INSIDE LANES AT BINGHAM RD. INTERCHANGE.
CONSTRUCT TEMPORARY RAMPS BETWEEN MAIN LANES AND RAMPS.

STAGE IA QUANTITIES
TRAFFIC DRUMS = 168 EACH

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB NO. 880614		52	219	

2 MAINTENANCE OF TRAFFIC DETAILS



MAINTENANCE OF TRAFFIC DETAILS
STAGE IA

SEQUENCE OF CONSTRUCTION

STAGE IB
SHIFT TRAFFIC TO THE OUTSIDE LANES ON MAIN LANES AT BINGHAM RD. INTERCHANGE.
CONSTRUCT CROSSOVERS, TEMPORARY RAMPS, AND ACCEL. LANES IN MEDIAN AT BINGHAM RD. INTERCHANGE.
RECONSTRUCT INSIDE SHOULDERS WHERE SHOWN IN PLANS.

STAGE IB QUANTITIES
TRAFFIC DRUMS - 26/EACH

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
11-16-12				6	ARK.		53	219
				JOB NO.		BB0614	53	219

2 MAINTENANCE OF TRAFFIC DETAILS



DENOTES: CONSTRUCTION OF TEMPORARY CROSSOVERS FOR M.O.T. AND SHOULDER RECONSTRUCTION

(1) RII-2
(48" X 30")
16' BARR.
TYP. III LT.



(1) RII-2
(48" X 30")
16' BARR.
TYP. III LT.



(1) RII-2
(48" X 30")
16' BARR.
TYP. III LT.



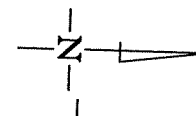
STAGE IB CONSTRUCTION
STA. 377+33 - INSTALL
18" X 536' TEMPORARY PIPE CULV'T.

(1) RII-2
(48" X 30")
16' BARR.
TYP. III LT.



BEGIN SHLDR. RECONSTRUCTION
STA. 380+50

TRAFFIC DRUMS
60' O.C.



387

P.C. 388+13.10

390

390

395

(5) RII-2
(48" X 30")
16' BARR.
TYP. III LT.



(1) RII-2
(48" X 30")
16' BARR.
TYP. III RT.

400

END SHOULDER RECONSTRUCTION
STA. 385+75

LORANCE CREEK

BEGIN SHLDR. RECONSTRUCTION
STA. 394+00

TRAFFIC DRUMS
60' O.C.

STAGE IB CONSTRUCTION
STA. 393+90 - INSTALL
18" X 326' TEMPORARY PIPE CULV'T.



(1) RII-2
(48" X 30")
16' BARR.
TYP. III RT.

RAMP 1

MATCH LINE

MAINTENANCE OF TRAFFIC DETAILS
STAGE IB

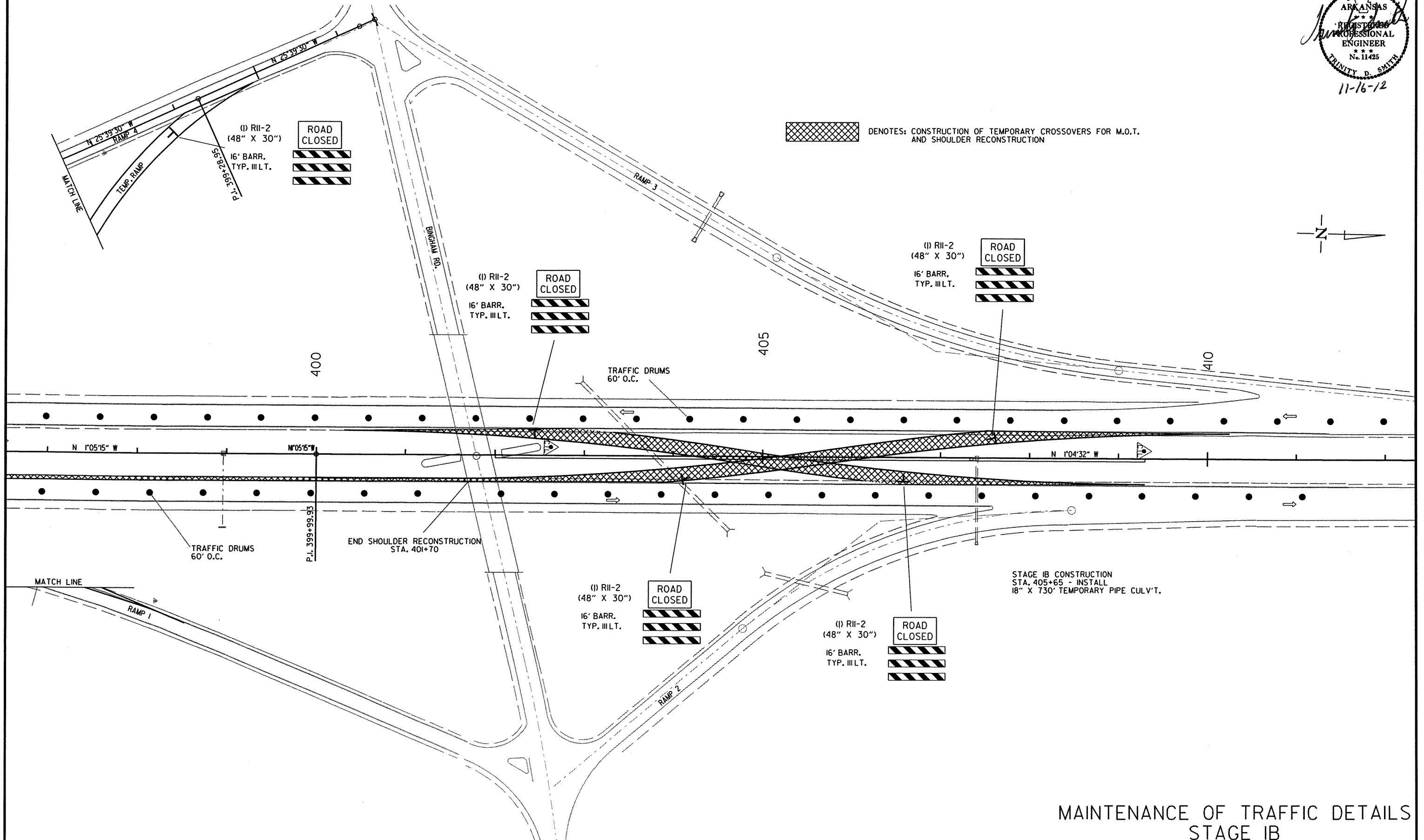
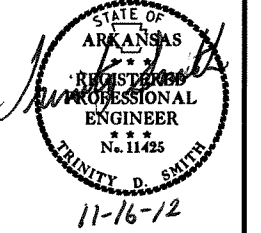


DENOTES: CONSTRUCTION OF TEMPORARY CROSSOVERS FOR M.O.T. AND SHOULDER RECONSTRUCTION

SEQUENCE OF CONSTRUCTION
 STAGE IB
 SHIFT TRAFFIC TO THE OUTSIDE LANES ON MAIN LANES AT BINGHAM RD. INTERCHANGE.
 CONSTRUCT CROSSOVERS, TEMPORARY RAMPS, AND ACCEL. LANES IN MEDIAN AT BINGHAM RD. INTERCHANGE.
 RECONSTRUCT INSIDE SHOULDERS WHERE SHOWN IN PLANS.

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
11-16-12				6	ARK.		54	219
				JOB NO. BB0614				

2 MAINTENANCE OF TRAFFIC DETAILS



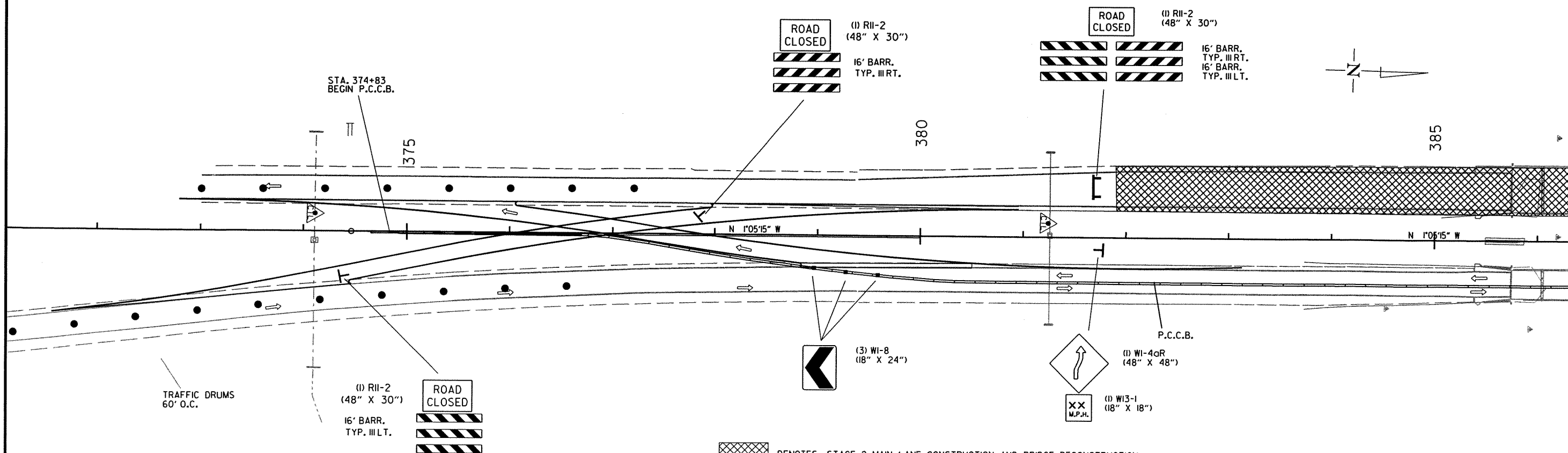
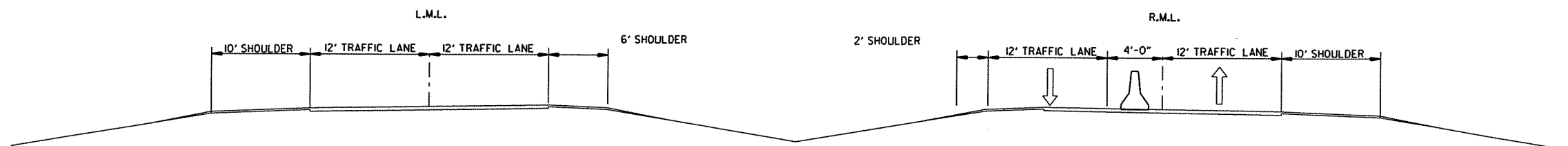
SEQUENCE OF CONSTRUCTION
 STAGE 2
 INSTALL PRECAST CONCRETE BARRIER ONTO EXISTING NB LANES IN AREAS SHOWN.
 ROUTE SB TRAFFIC TO INSIDE LANE OF NB LANES THROUGH CROSSOVERS.
 RECONSTRUCT SB BRIDGE, APPROACH CUTTERS, & SLABS OVER LORANCE CREEK.
 RECONSTRUCT SB LANES BETWEEN STA. 382+35 TO STA. 392+80.
 RECONSTRUCT I-530 SB ENTRANCE RAMP AT BINGHAM RD. INTERCHANGE.

*NOTE: THE END OF THE 500' STABILIZATION ZONE AS SHOWN IN THE LANE
 CLOSURE DETAIL SHALL BE AT STATION 345+00 ON THE NORTHBOUND
 MAIN LANES OF I-530.

STAGE 2 QUANTITIES
 TRAFFIC DRUMS = 149 EACH
 FURNISH AND INSTALL PRECAST CONCRETE BARRIER WALL = 3586 LIN. FT.
 REMOVAL OF PERMANENT PAVEMENT MARKINGS = 7762 LIN. FT.
 CONSTRUCTION PAVEMENT MARKINGS = 16909 LIN. FT.
 REMOVABLE CONSTRUCTION PAVEMENT MARKINGS = 150 LIN. FT.

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
11-16-12				6	ARK.			
				JOB NO.		BB0614	55	219

② MAINTENANCE OF TRAFFIC DETAILS



XXXXX DENOTES: STAGE 2 MAIN LANE CONSTRUCTION AND BRIDGE RECONSTRUCTION

MAINTENANCE OF TRAFFIC DETAILS
 STAGE 2

8/29/2012

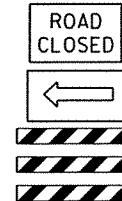
RB0614.DGN

SEQUENCE OF CONSTRUCTION

STAGE 2
 INSTALL PRECAST CONCRETE BARRIER ONTO EXISTING NB LANES IN AREAS SHOWN.
 ROUTE SB TRAFFIC TO INSIDE LANE OF NB LANES THROUGH CROSSOVERS.
 RECONSTRUCT SB BRIDGE, APPROACH GUTTERS, & SLABS OVER LORANCE CREEK.
 RECONSTRUCT SB LANES BETWEEN STA. 382+35 TO STA. 392+80.
 RECONSTRUCT I-530 SB ENTRANCE RAMP AT BINGHAM RD. INTERCHANGE.

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
11-16-12				6	ARK.			
				JOB NO.		BB0614	56	219

2 MAINTENANCE OF TRAFFIC DETAILS



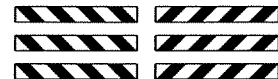
(1) RII-2
 (48" X 30")

(1) WI-6
 (48" X 24")

16' BARR.
 TYP. III RT.

ROAD
 CLOSED

(1) RII-2
 (48" X 30")



16' BARR.
 TYP. III RT.
 16' BARR.
 TYP. III LT.

ROAD
 CLOSED

(1) RII-2
 (48" X 30")



16' BARR.
 TYP. III RT.

ROAD
 CLOSED

(1) RII-2
 (48" X 30")



16' BARR.
 TYP. III RT.
 16' BARR.
 TYP. III LT.

ROAD
 CLOSED

(2) RII-2
 (48" X 30")



16' BARR.
 TYP. III RT.

ROAD
 CLOSED

(1) RII-2
 (48" X 30")



16' BARR.
 TYP. III LT.

ROAD
 CLOSED

(1) RII-2
 (48" X 30")



16' BARR.
 TYP. III LT.



DENOTES: STAGE 2 MAIN LANE CONSTRUCTION AND BRIDGE RECONSTRUCTION

STA. 387+55
 BEGIN P.C.C.B.

(4) OM-3R
 (12" X 36")

STA. 389+28
 END P.C.C.B.

MAINTENANCE OF TRAFFIC DETAILS
 STAGE 2

8/29/2012

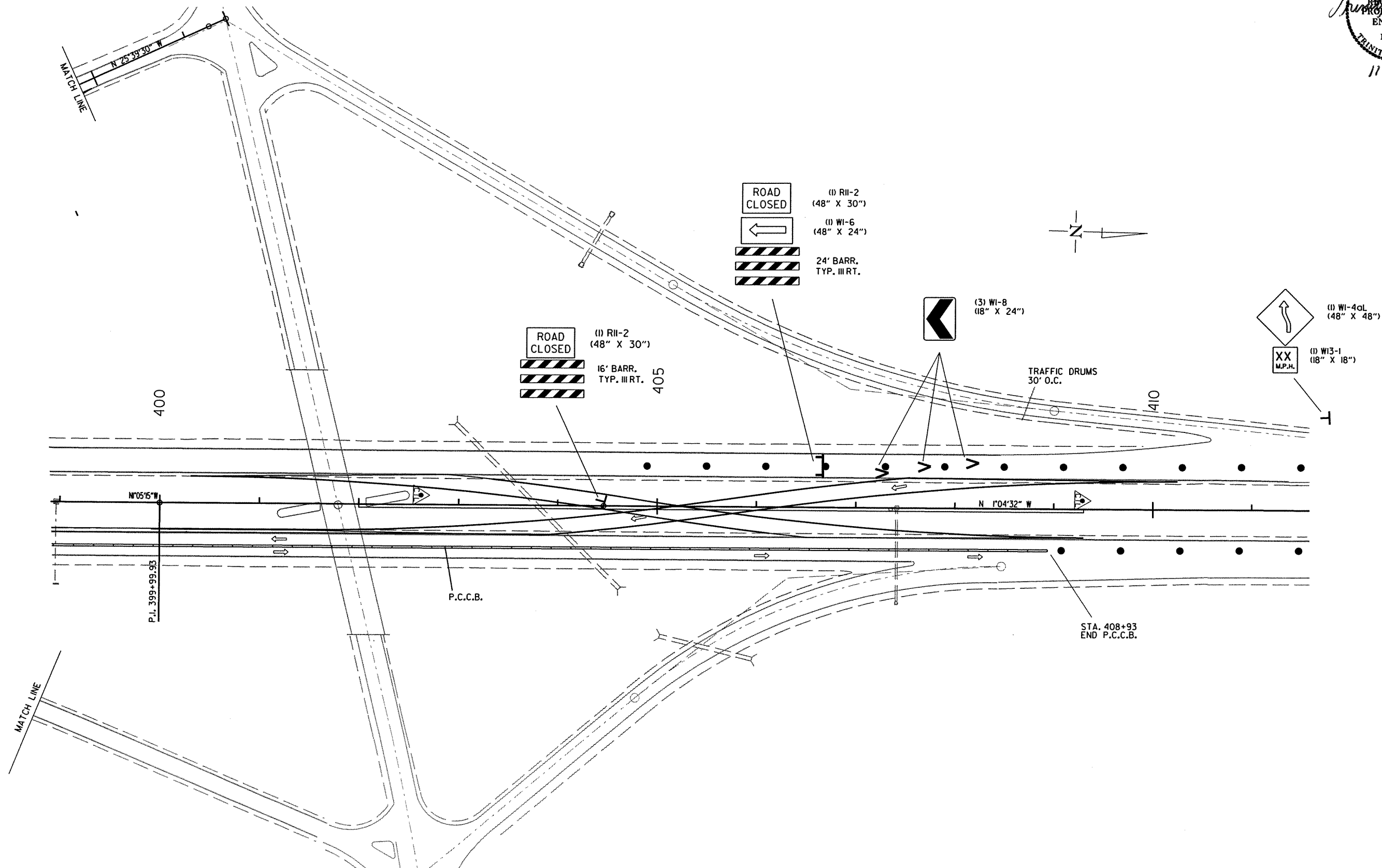
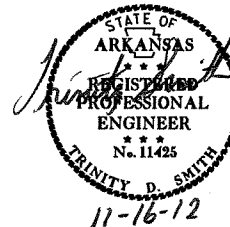
RB0614.DGN

SEQUENCE OF CONSTRUCTION

STAGE 2
 INSTALL PRECAST CONCRETE BARRIER ONTO EXISTING NB LANES IN AREAS SHOWN.
 ROUTE SB TRAFFIC TO INSIDE LANE OF NB LANES THROUGH CROSSOVERS.
 RECONSTRUCT SB BRIDGE, APPROACH CUTTERS, & SLABS OVER LORANCE CREEK.
 RECONSTRUCT SB LANES BETWEEN STA. 382+35 TO STA. 392+80.
 RECONSTRUCT I-530 SB ENTRANCE RAMP AT BINGHAM RD. INTERCHANGE.

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
11-16-12				6	ARK.			
						JOB NO. 880614	57	219

② MAINTENANCE OF TRAFFIC DETAILS



MAINTENANCE OF TRAFFIC DETAILS
 STAGE 2

8/29/2012

RB0614.DGN

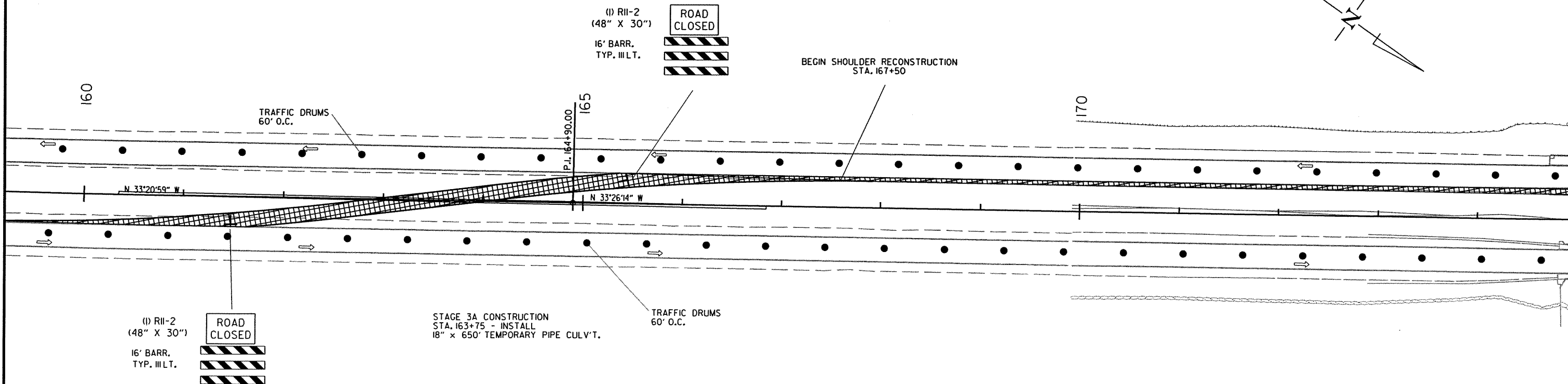
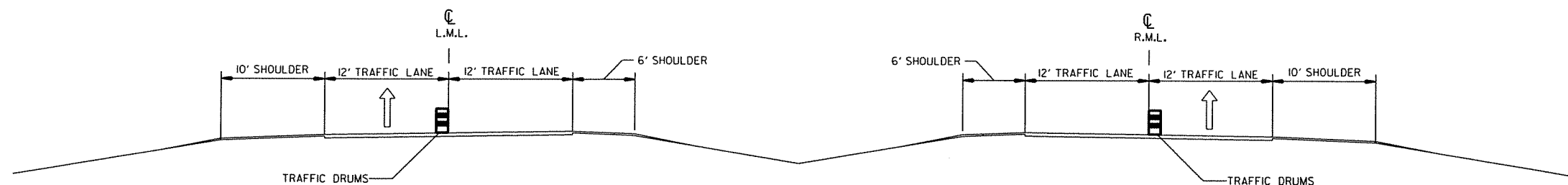
SEQUENCE OF CONSTRUCTION
 STAGE 3A
 ROUTE SB TRAFFIC ONTO OUTSIDE OF RECONSTRUCTED SB LANES.
 LEAVE NB TRAFFIC ON OUTSIDE NB LANES.
 CONSTRUCT TEMPORARY CROSSOVERS IN AREAS SHOWN.
 RECONSTRUCT SHOULDERS IN AREAS SHOWN

STAGE 3A QUANTITIES
 TRAFFIC DRUMS = 1462 EACH
 NOTE: P.C.C.B. IN PLACE FROM STAGE 2

RIGHT MAIN LANE LEFT SHOULDER RECONSTRUCTION
 STA. 56+20.00 TO STA. 58+20.00
 STA. 59+80.00 TO STA. 60+60.00
 STA. 283+18.00 TO STA. 285+18.00
 STA. 286+80.00 TO STA. 287+60.00

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
11-16-12				6	ARK.			
						JOB NO. 880614	58	219

② MAINTENANCE OF TRAFFIC DETAILS

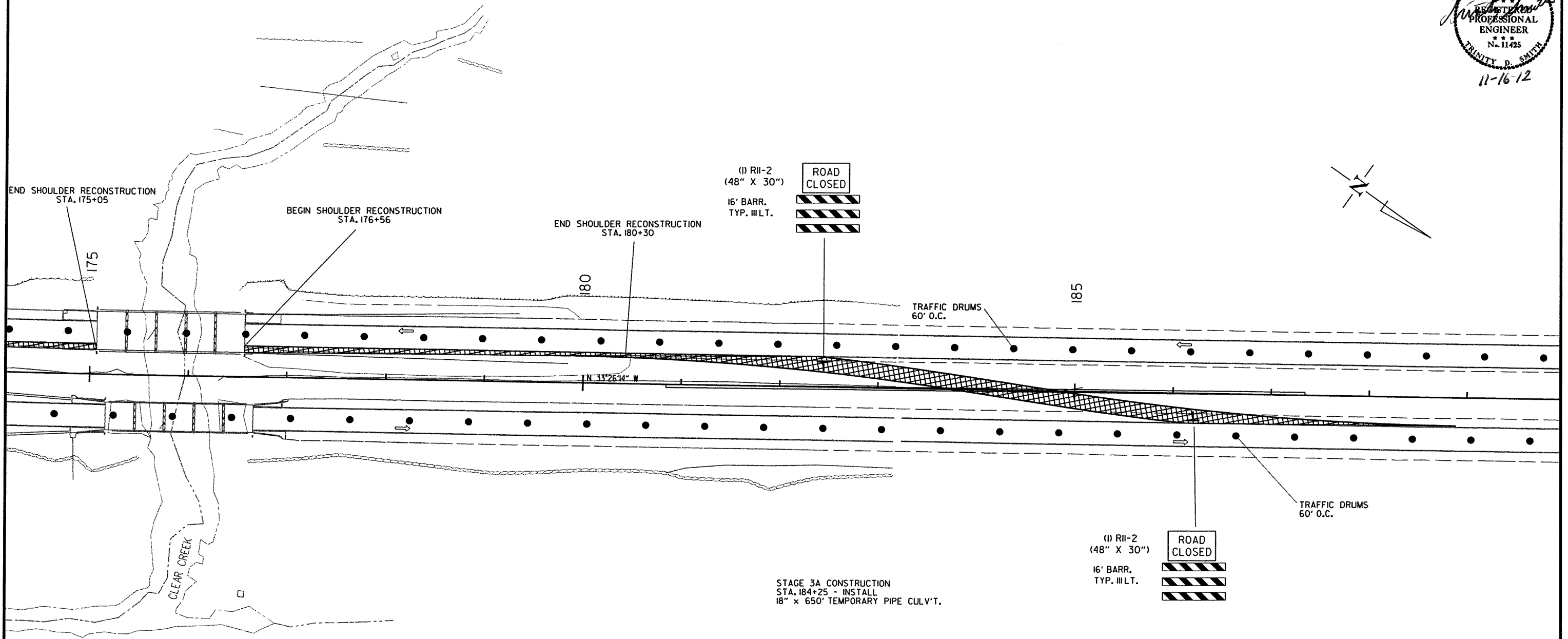
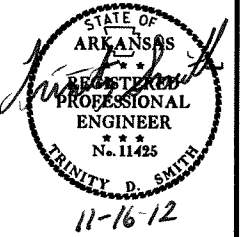


MAINTENANCE OF TRAFFIC DETAILS
 STAGE 3A

SEQUENCE OF CONSTRUCTION
STAGE 3A
ROUTE SB TRAFFIC ONTO OUTSIDE OF RECONSTRUCTED SB LANES.
LEAVE NB TRAFFIC ON OUTSIDE NB LANES.
CONSTRUCT TEMPORARY CROSSOVERS IN AREAS SHOWN.
RECONSTRUCT SHOULDERS IN AREAS SHOWN

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
11-16-12				6	ARK.			
				JOB NO.		880614	59	219

② MAINTENANCE OF TRAFFIC DETAILS



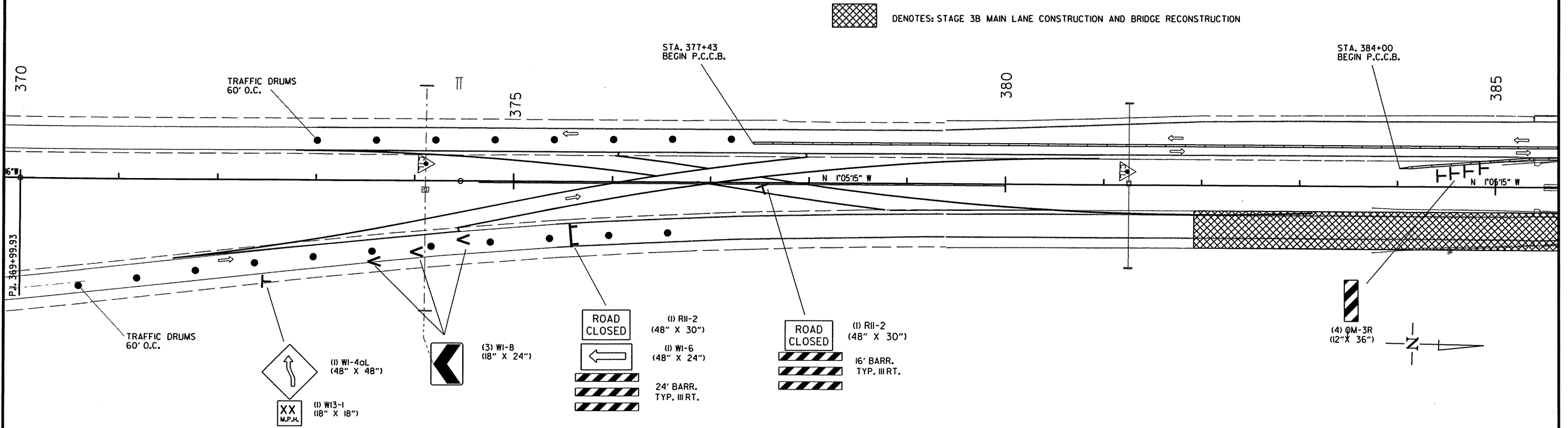
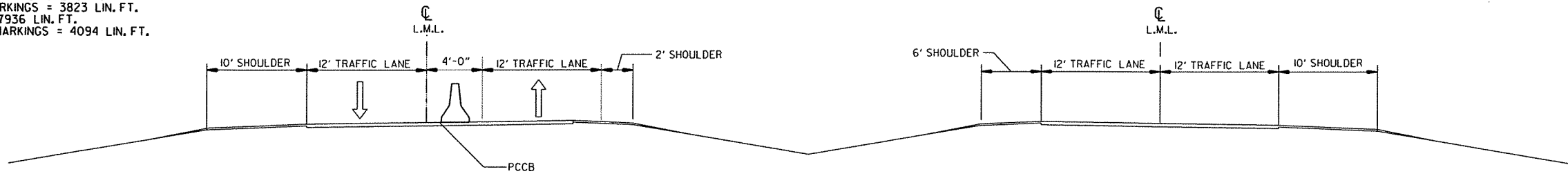
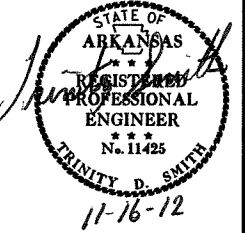
MAINTENANCE OF TRAFFIC DETAILS
STAGE 3A

SEQUENCE OF CONSTRUCTION
 STAGE 3B
 INSTALL PRECAST CONCRETE BARRIER ONTO EXISTING SB LANES IN AREAS SHOWN.
 ROUTE NB TRAFFIC ONTO INSIDE OF SB LANES THROUGH CROSSOVERS.
 RECONSTRUCT NB BRIDGES, APPROACH GUTTERS, & SLABS OVER LORANCE CREEK.
 RECONSTRUCT NB LANES BETWEEN STA. 382+35 TO STA. 392+80.

STAGE 3B QUANTITIES
 TRAFFIC DRUMS = 1326 EACH
 RELOCATING PRECAST CONCRETE BARRIER = 3146 LIN. FT.
 REMOVAL OF PERMANENT PAVEMENT MARKINGS = 3823 LIN. FT.
 CONSTRUCTION PAVEMENT MARKINGS = 17936 LIN. FT.
 REMOVABLE CONSTRUCTION PAVEMENT MARKINGS = 4094 LIN. FT.

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
11-16-12				6	ARK.		60	219
				JOB NO.		BB0614	60	219

② MAINTENANCE OF TRAFFIC DETAILS



8/28/2012

RB0614.DGN

MAINTENANCE OF TRAFFIC DETAILS
 STAGE 3B

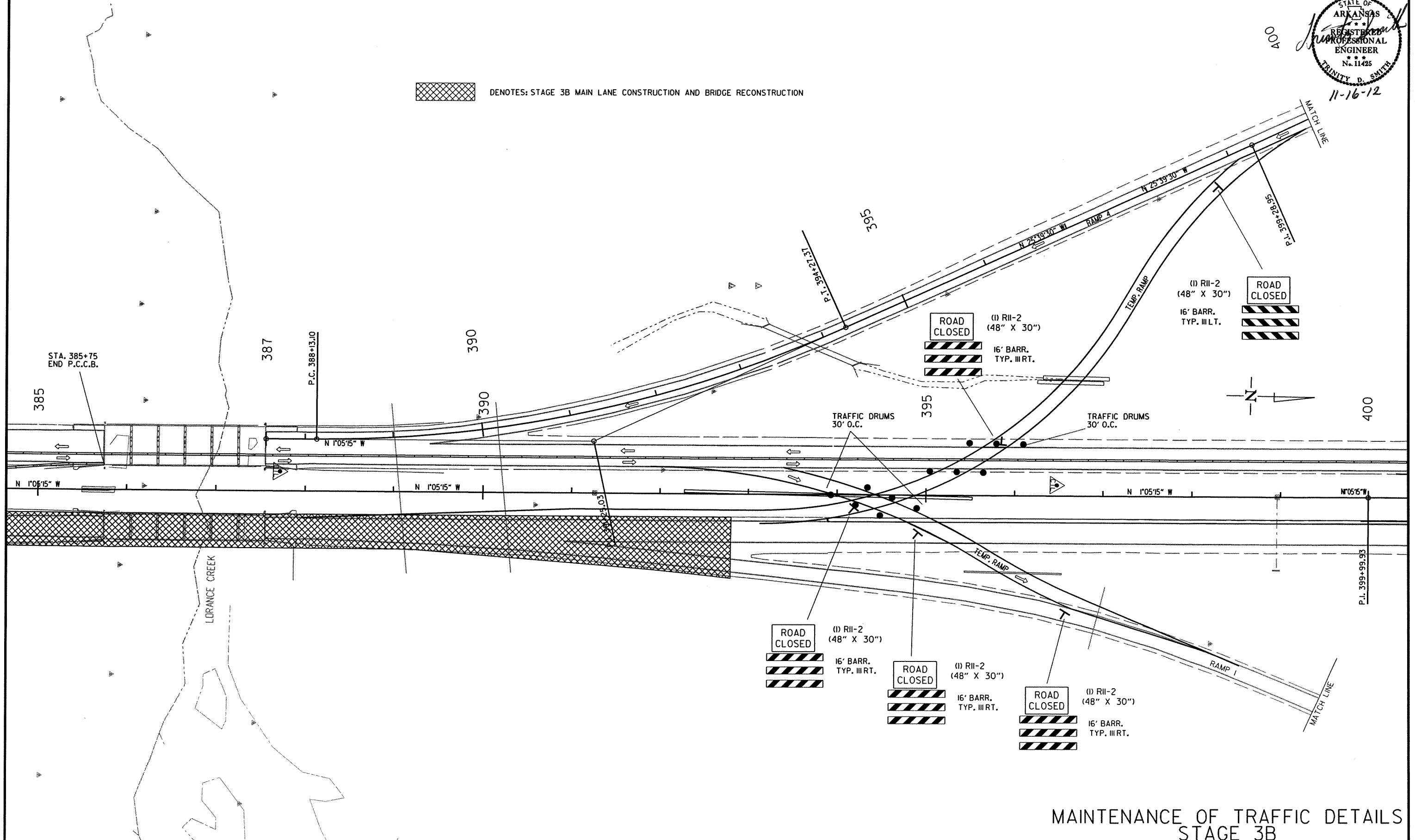
SEQUENCE OF CONSTRUCTION
STAGE 3B
INSTALL PRECAST CONCRETE BARRIER ONTO EXISTING SB LANES IN AREAS SHOWN.
ROUTE NB TRAFFIC ONTO INSIDE OF SB LANES THROUGH CROSSOVERS.
RECONSTRUCT NB BRIDGES, APPROACH CUTTERS, & SLABS OVER LORANCE CREEK.
RECONSTRUCT NB LANES BETWEEN STA. 382+35 TO STA. 392+80.

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
11-16-12				6	ARK.		61	219
				JOB NO.		880614	61	219

2 MAINTENANCE OF TRAFFIC DETAILS



 DENOTES: STAGE 3B MAIN LANE CONSTRUCTION AND BRIDGE RECONSTRUCTION



8/28/2012

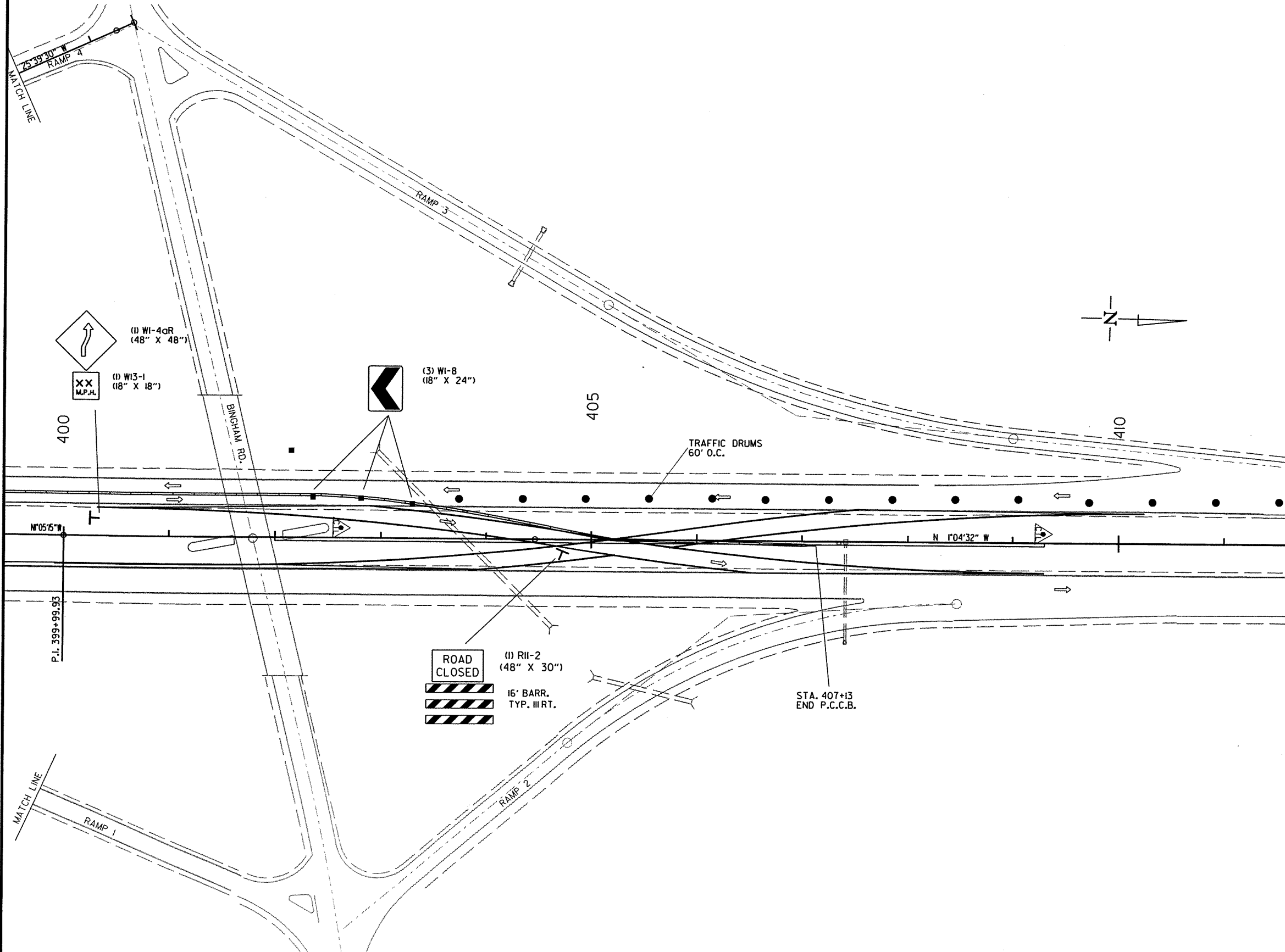
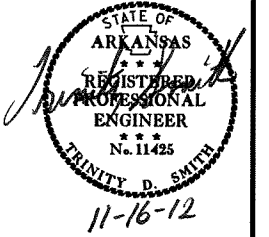
R880614.DGN

MAINTENANCE OF TRAFFIC DETAILS
STAGE 3B

SEQUENCE OF CONSTRUCTION
STAGE 3B
INSTALL PRECAST CONCRETE BARRIER ONTO EXISTING SB LANES IN AREAS SHOWN.
ROUTE NB TRAFFIC ONTO INSIDE OF SB LANES THROUGH CROSSOVERS.
RECONSTRUCT NB BRIDGES, APPROACH GUTTERS, & SLABS OVER LORANCE CREEK.
RECONSTRUCT NB LANES BETWEEN STA. 382+35 TO STA. 392+80.

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
11-16-12				6	ARK.			
				JOB NO.		BB0614	62	219

② MAINTENANCE OF TRAFFIC DETAILS

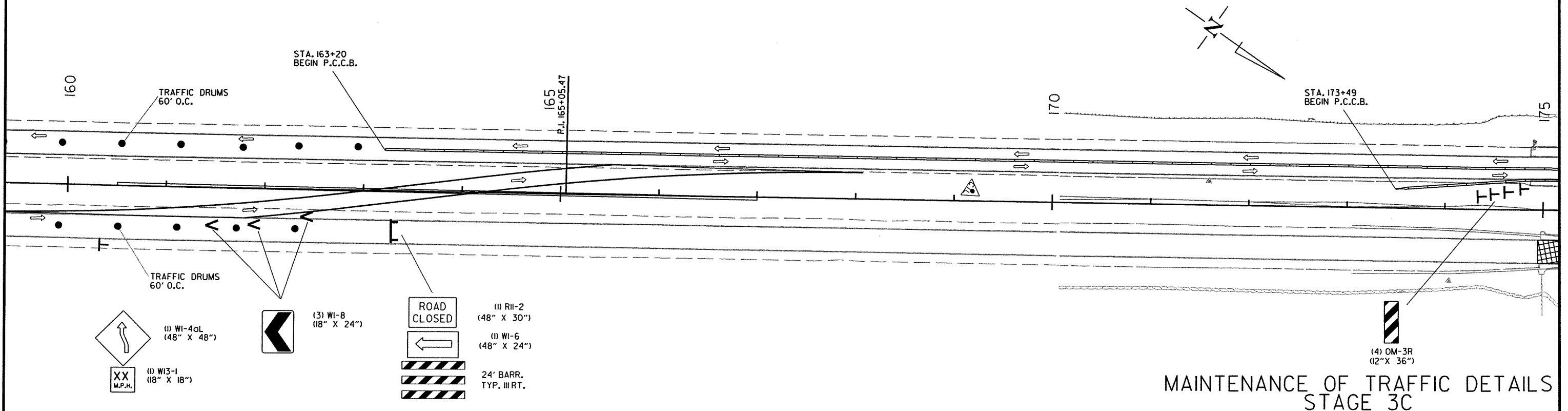
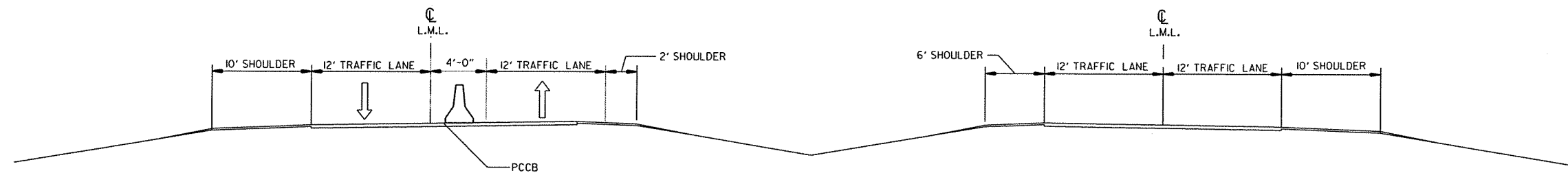
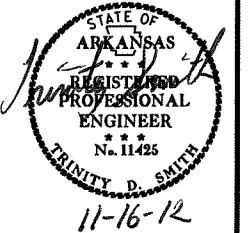


STAGE 3C
RECONSTRUCT NB BRIDGES, APPROACH GUTTERS, & SLAB OVER CLEAR CREEK.

STAGE 3C QUANTITIES
TRAFFIC DRUMS = 1314 EACH
FURNISH AND INSTALL PRECAST CONCRETE BARRIER WALL = 2126 LIN. FT.
RELOCATING PRECAST CONCRETE BARRIER WALL = 440 LIN. FT.

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. AID DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
11-16-12				6	ARK.			
						JOB NO. B80614	63	219

② MAINTENANCE OF TRAFFIC DETAILS



RBB0614_DHAL 7-27-12.DCN 8/17/2012

MAINTENANCE OF TRAFFIC DETAILS
STAGE 3C

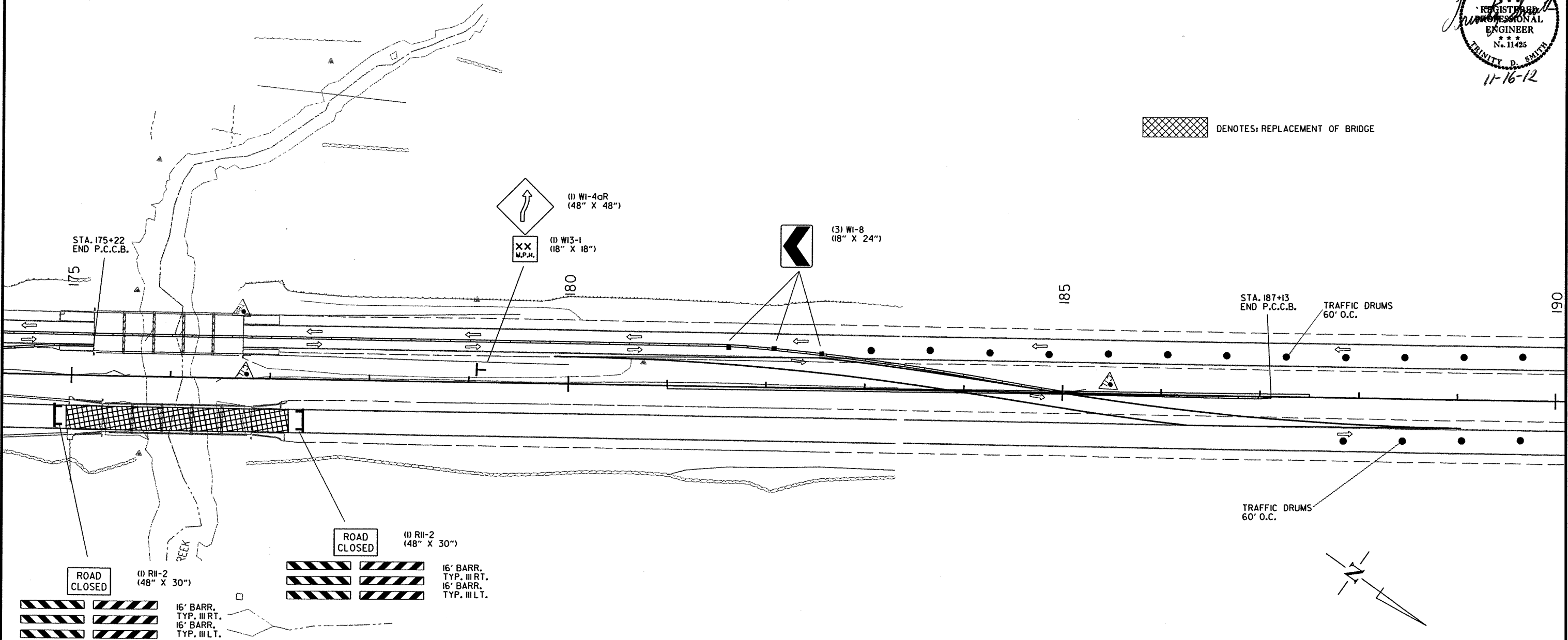
STAGE 3C
RECONSTRUCT NB BRIDGES, APPROACH GUTTERS, & SLAB OVER CLEAR CREEK.

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
11-16-12				6	ARK.			
				JOB NO.	BB0614		64	219

② MAINTENANCE OF TRAFFIC DETAILS



 DENOTES: REPLACEMENT OF BRIDGE

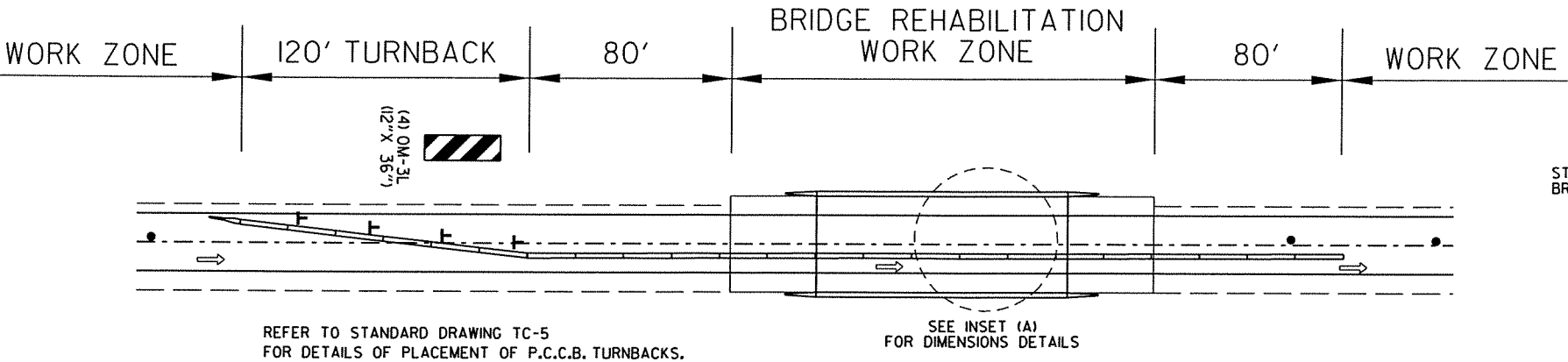
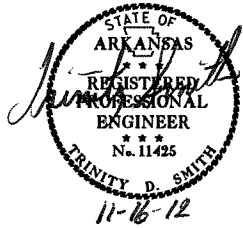


MAINTENANCE OF TRAFFIC DETAILS
STAGE 3C

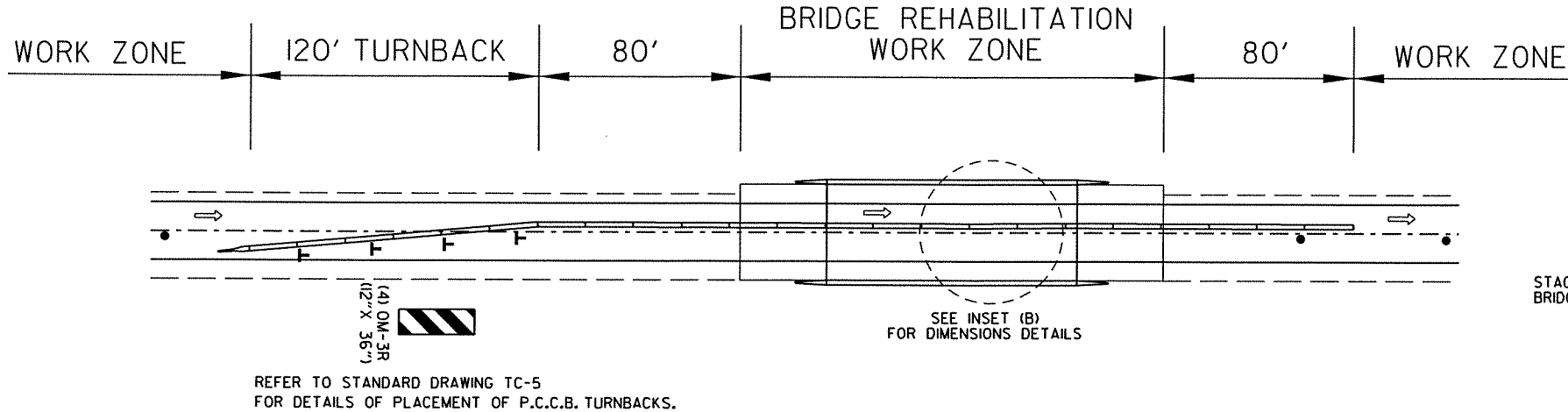
SEQUENCE OF CONSTRUCTION
STAGE 3D
CONDUCT HYDRODEMOLITION/REHABILITATION OPERATIONS AT CAMPBELL CREEK & MAPLE CREEK.

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
11-16-12				6	ARK.			
				JOB NO. BB0614		65	219	

2 MAINTENANCE OF TRAFFIC DETAILS

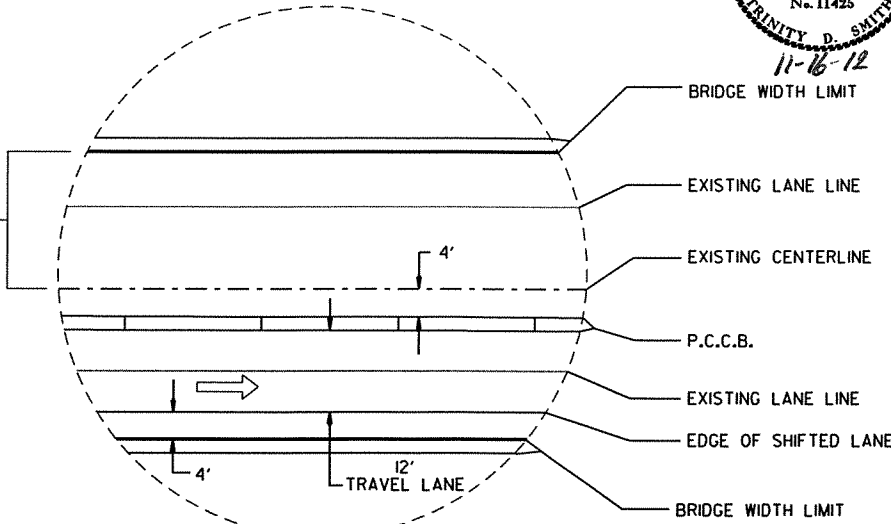


DIVERSION FOR LT. LANE BRIDGE DECK REHABILITATION AND
APPROACH GUTTER AND SLAB RECONSTRUCTION
2 SETS OF THIS NEEDED FOR JOB BB0614.



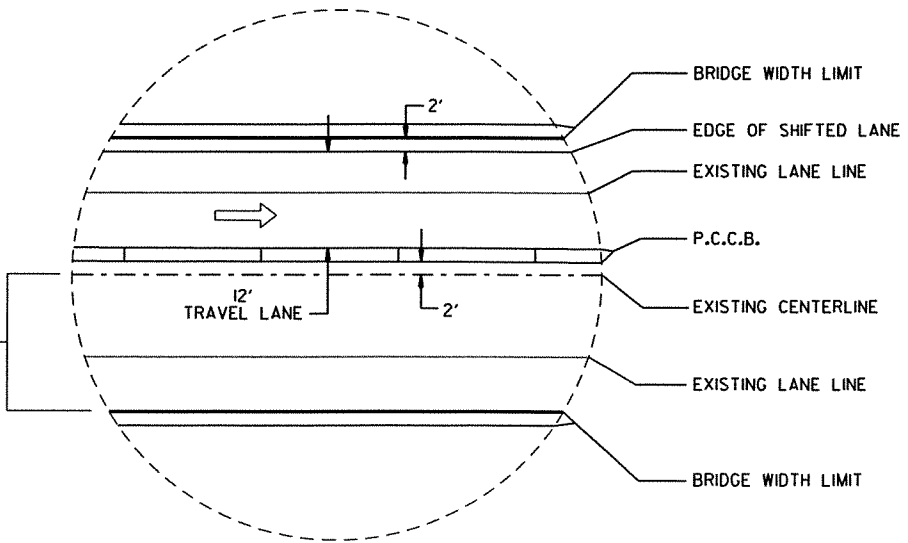
DIVERSION FOR RT. LANE BRIDGE DECK REHABILITATION AND
APPROACH GUTTER AND SLAB RECONSTRUCTION
2 SETS OF THIS NEEDED FOR JOB BB0614.

STAGE 1 CONSTRUCTION:
BRIDGE DECK REHABILITATION



INSET A

STAGE 2 CONSTRUCTION:
BRIDGE DECK REHABILITATION



INSET B

SHOULDER RECONSTRUCTION STATION LIMITS
NOTE: SEE SHEET 7 FOR SHOULDER RECONSTRUCTION TYPICAL SECTION
RIGHT MAIN LANE RIGHT SHOULDER RECONSTRUCTION
STA. 56+20.00 TO STA. 58+20.00
STA. 59+80.00 TO STA. 60+60.00
STA. 283+18.00 TO STA. 285+18.00
STA. 286+80.00 TO STA. 287+60.00

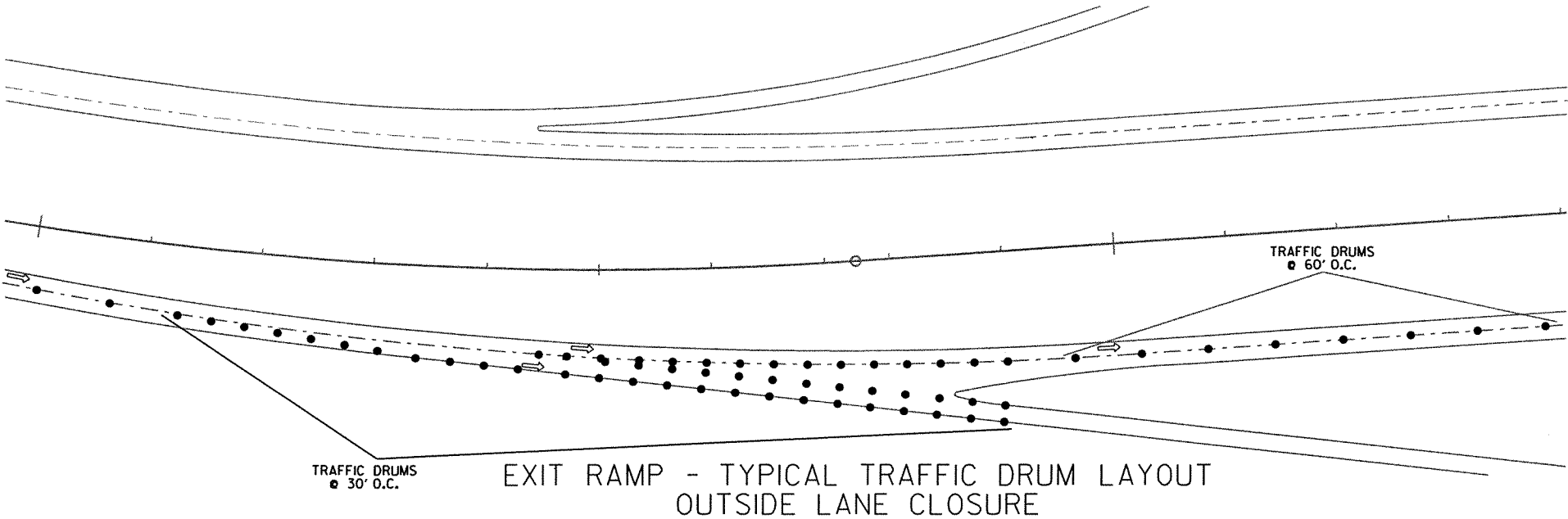
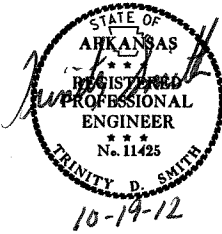
MAINTENANCE OF TRAFFIC DETAILS
STAGE 3D
WORK ZONE - BRIDGE DECK REHABILITATION

10/11/2012

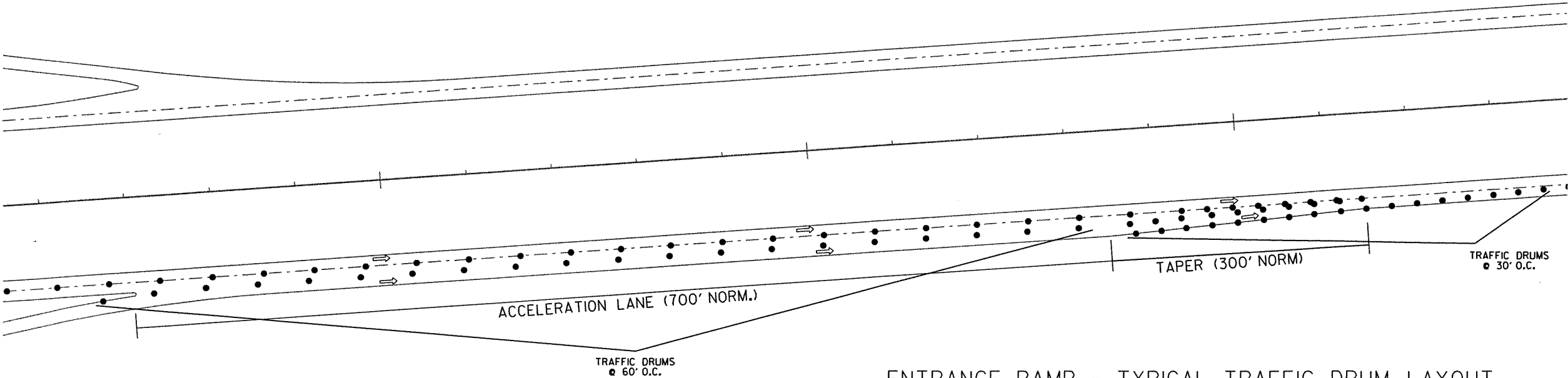
RB0614.DGN

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB NO. BB0614		66	219	

② MAINTENANCE OF TRAFFIC DETAILS



EXIT RAMP - TYPICAL TRAFFIC DRUM LAYOUT
OUTSIDE LANE CLOSURE



ENTRANCE RAMP - TYPICAL TRAFFIC DRUM LAYOUT
OUTSIDE LANE CLOSURE

HENSLEY RD.:
NORTHBOUND ENTRANCE = 48 TRAFFIC DRUMS
NORTHBOUND EXIT = 53 TRAFFIC DRUMS

SOUTHBOUND ENTRANCE = 48 TRAFFIC DRUMS
SOUTHBOUND EXIT = 53 TRAFFIC DRUMS

WOODSON LATERAL RD.:
NORTHBOUND ENTRANCE = 48 TRAFFIC DRUMS
NORTHBOUND EXIT = 53 TRAFFIC DRUMS

SOUTHBOUND ENTRANCE = 48 TRAFFIC DRUMS
SOUTHBOUND EXIT = 53 TRAFFIC DRUMS

HWY. 167:
NORTHBOUND ENTRANCE = 69 TRAFFIC DRUMS

SOUTHBOUND EXIT = 53 TRAFFIC DRUMS
SOUTHBOUND ENTRANCE = 48 TRAFFIC DRUMS

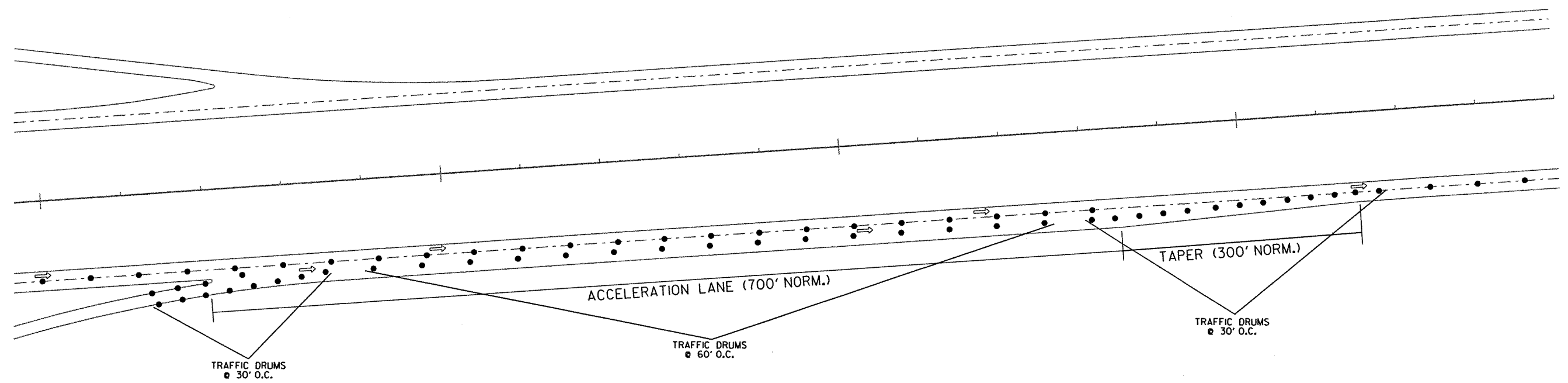
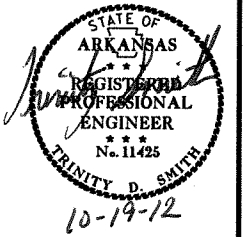
BINGHAM RD.:
NORTHBOUND ENTRANCE = 48 TRAFFIC DRUMS
NORTHBOUND EXIT = 53 TRAFFIC DRUMS

SOUTHBOUND ENTRANCE = 48 TRAFFIC DRUMS
SOUTHBOUND EXIT = 53 TRAFFIC DRUMS

MAINTENANCE OF TRAFFIC
MILL AND INLAY - STAGE 4
DETAIL OF RAMPS WITH LANE CLOSURE

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
						JOB NO. BB0614	67	219

2 MAINTENANCE OF TRAFFIC DETAILS



ENTRANCE RAMP - TYPICAL TRAFFIC DRUM LAYOUT
ACCELERATION LANE CLOSURE

HENSLEY RD.:
NORTHBOUND ENTRANCE = 36 TRAFFIC DRUMS
SOUTHBOUND ENTRANCE = 36 TRAFFIC DRUMS

WOODSON LATERAL RD.:
NORTHBOUND ENTRANCE = 36 TRAFFIC DRUMS
SOUTHBOUND ENTRANCE = 36 TRAFFIC DRUMS

HWY. 167:
NORTHBOUND ENTRANCE = 54 TRAFFIC DRUMS
SOUTHBOUND ENTRANCE = 36 TRAFFIC DRUMS

BINGHAM RD.:
NORTHBOUND ENTRANCE = 36 TRAFFIC DRUMS
SOUTHBOUND ENTRANCE = 36 TRAFFIC DRUMS

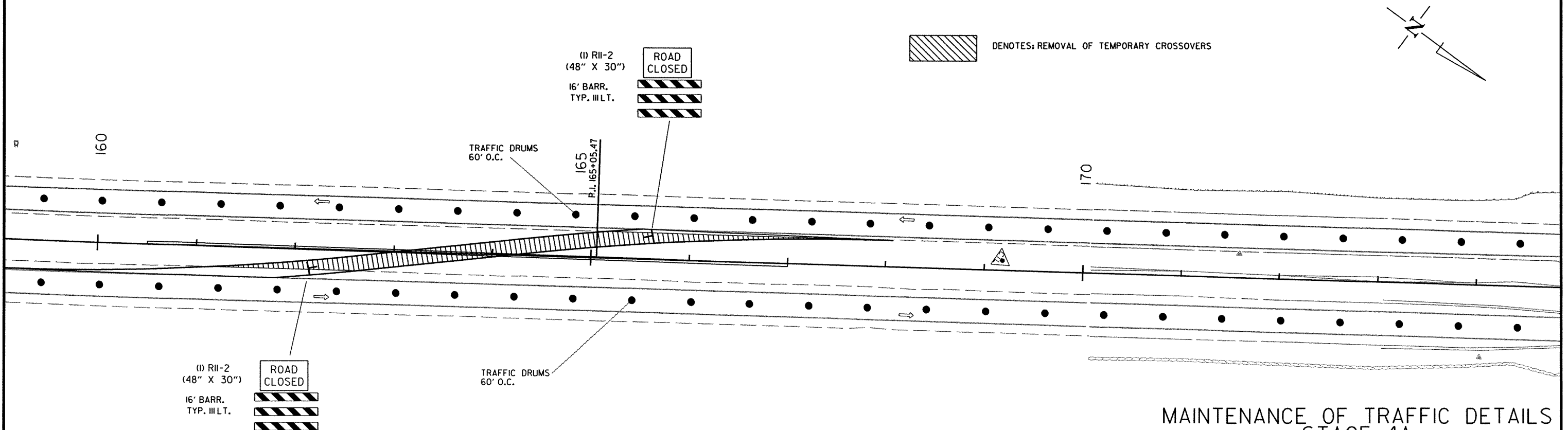
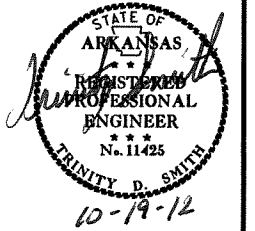
MAINTENANCE OF TRAFFIC
MILL AND INLAY - STAGE 4
DETAIL OF RAMPS WITH LANE CLOSURE

SEQUENCE OF CONSTRUCTION
STAGE 4A
ROUTE NB TRAFFIC TO OUTSIDE OF RECONSTRUCTED NB LANES.
RETAIN SB TRAFFIC IN OUTSIDE SB LANE.
REMOVE PCCB AND REPLACE WITH TRAFFIC DRUMS.
OBLITERATE TEMPORARY CROSSOVERS AND TEMPORARY RAMPS IN MEDIAN.
MILL AND INLAY INSIDE LANES.

STAGE 4A QUANTITIES
TRAFFIC DRUMS = 2238 EACH
CONSTRUCTION PAVEMENT MARKINGS = 100799 LIN. FT.

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB NO.		BB0614	68	219

② MAINTENANCE OF TRAFFIC DETAILS



MAINTENANCE OF TRAFFIC DETAILS
STAGE 4A

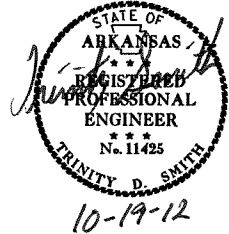
8/29/2012

RB0614.DGN

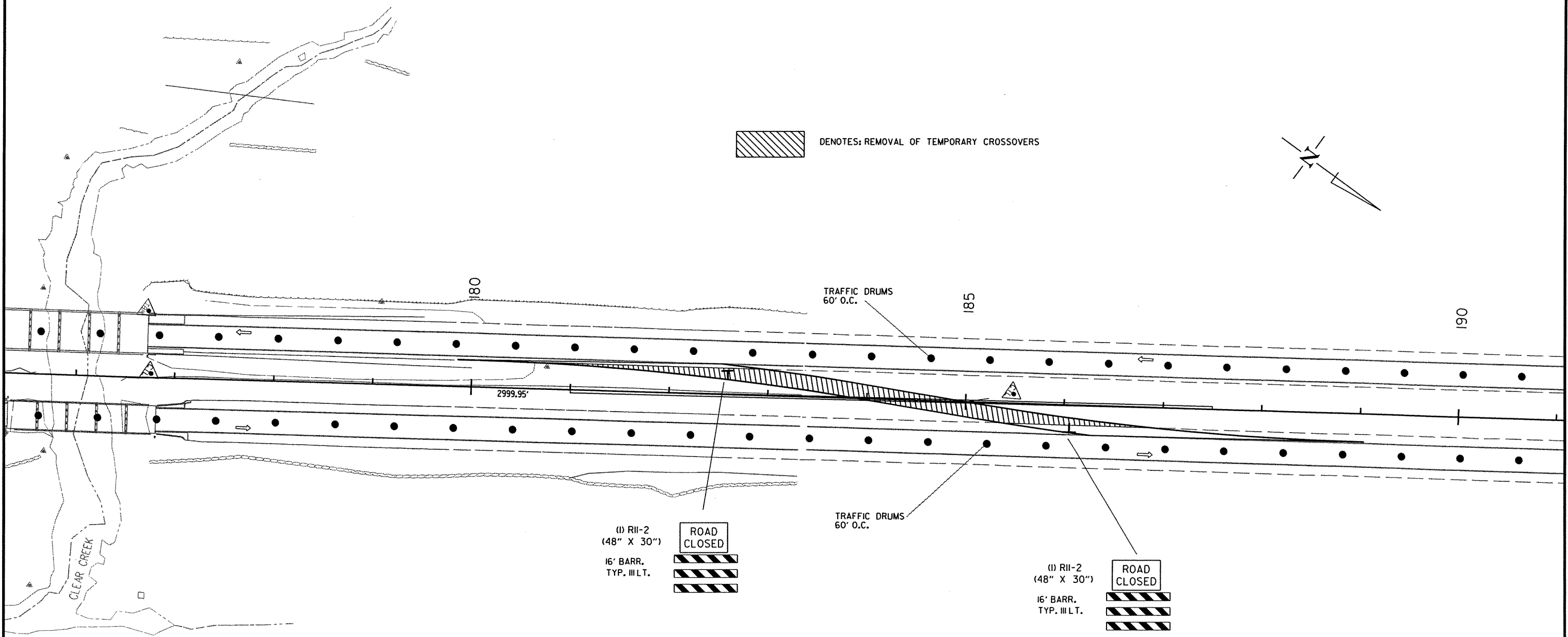
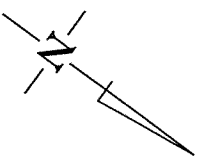
SEQUENCE OF CONSTRUCTION
STAGE 4A
ROUTE NB TRAFFIC TO OUTSIDE OF RECONSTRUCTED NB LANES.
RETAIN SB TRAFFIC IN OUTSIDE SB LANE.
REMOVE PCCB AND REPLACE WITH TRAFFIC DRUMS.
OBLITERATE TEMPORARY CROSSOVERS AND TEMPORARY RAMPS IN MEDIAN.
MILL AND INLAY INSIDE LANES.

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB NO.		BB0614	69	219

② MAINTENANCE OF TRAFFIC DETAILS



DENOTES: REMOVAL OF TEMPORARY CROSSOVERS



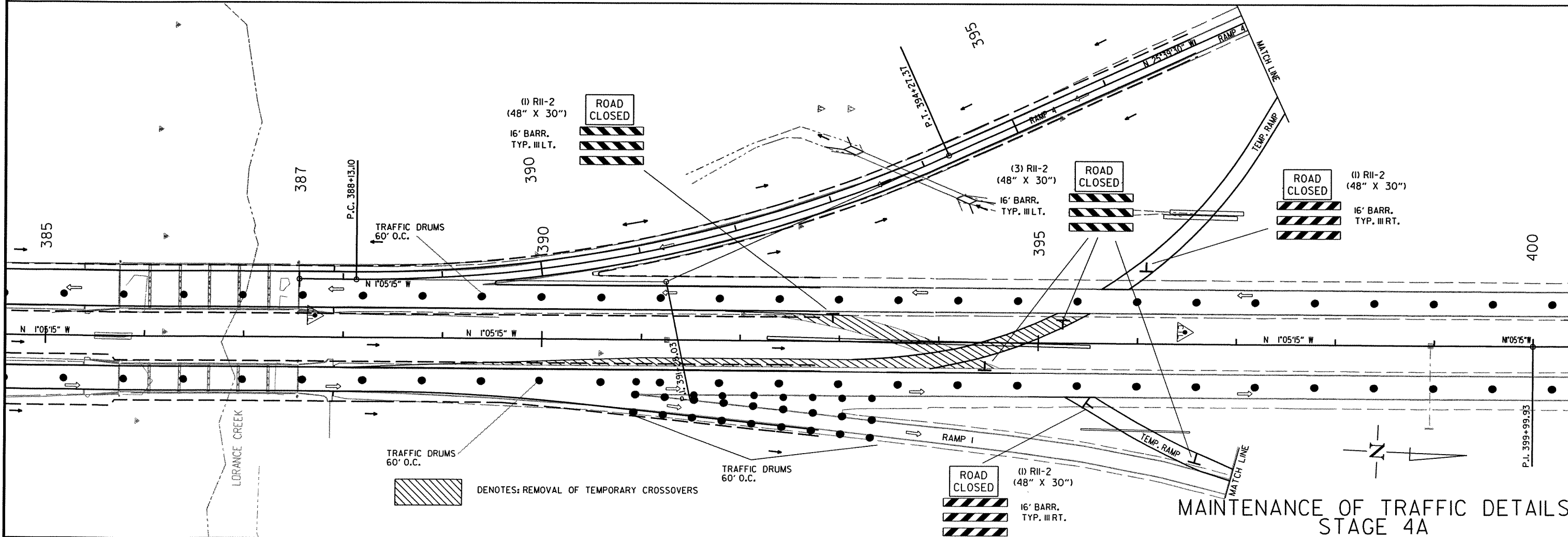
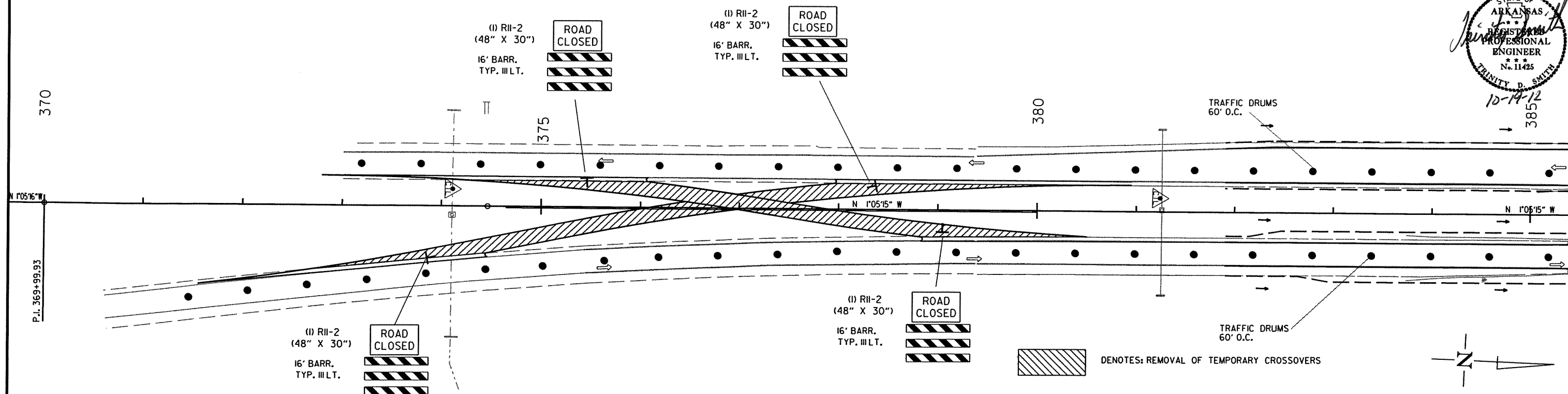
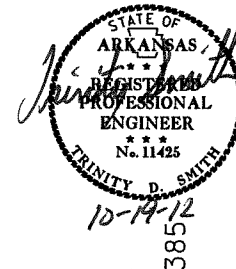
8/29/2012
RB0614.DGN

MAINTENANCE OF TRAFFIC DETAILS
STAGE 4A

SEQUENCE OF CONSTRUCTION
STAGE 4A
ROUTE NB TRAFFIC TO OUTSIDE OF RECONSTRUCTED NB LANES.
RETAIN SB TRAFFIC IN OUTSIDE SB LANE.
REMOVE PCCB AND REPLACE WITH TRAFFIC DRUMS.
OBLITERATE TEMPORARY CROSSOVERS AND TEMPORARY RAMPS IN MEDIAN.
MILL AND INLAY INSIDE LANES.

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.		70	219
				JOB NO.		BB0614		

② MAINTENANCE OF TRAFFIC DETAILS



8/29/2012

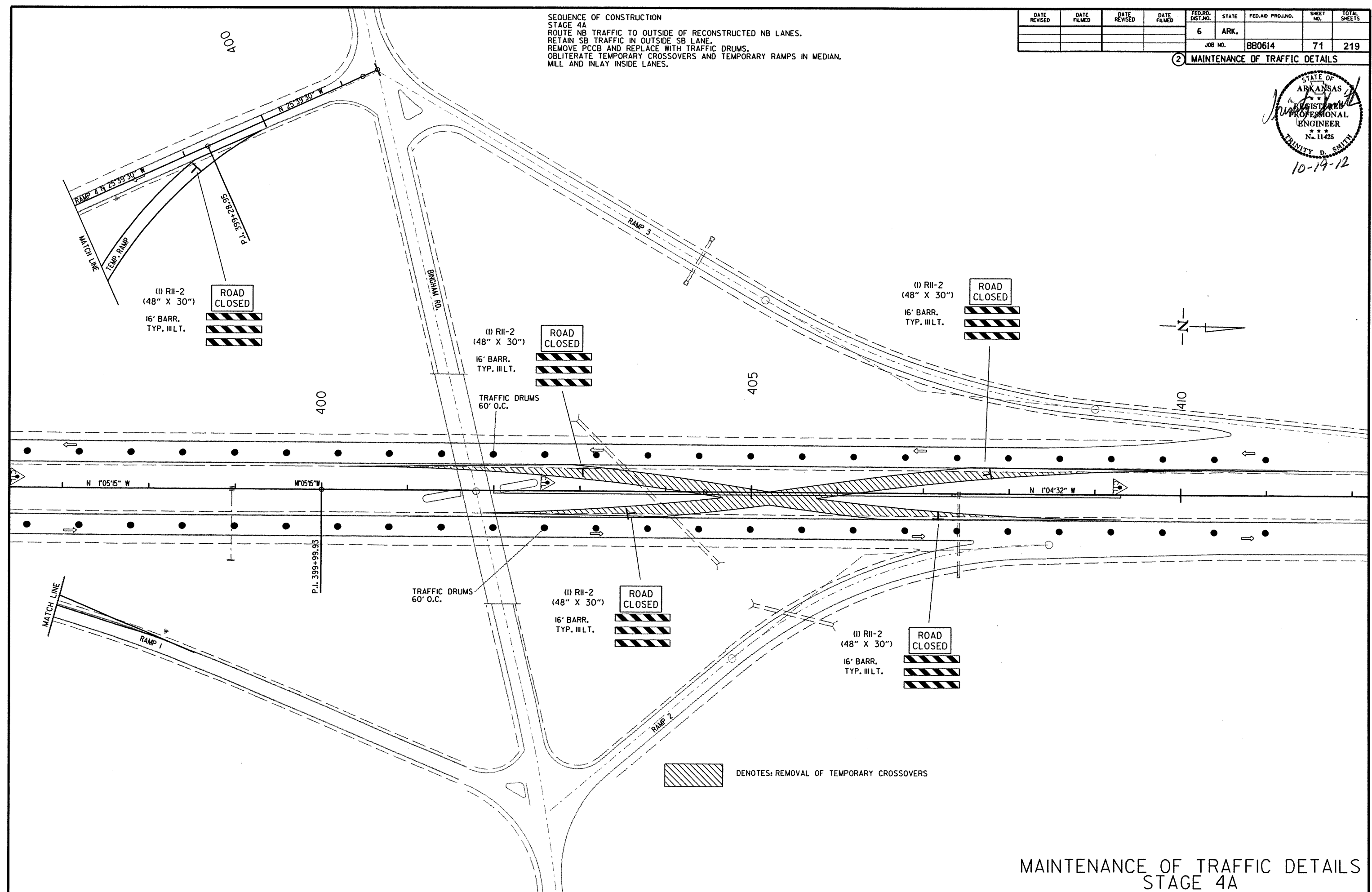
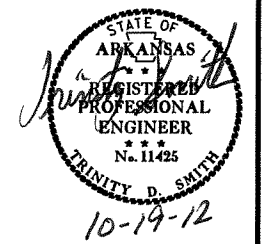
RB0614.DGN

MAINTENANCE OF TRAFFIC DETAILS
STAGE 4A

SEQUENCE OF CONSTRUCTION
 STAGE 4A
 ROUTE NB TRAFFIC TO OUTSIDE OF RECONSTRUCTED NB LANES.
 RETAIN SB TRAFFIC IN OUTSIDE SB LANE.
 REMOVE PCCB AND REPLACE WITH TRAFFIC DRUMS.
 OBLITERATE TEMPORARY CROSSOVERS AND TEMPORARY RAMPS IN MEDIAN.
 MILL AND INLAY INSIDE LANES.

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.		71	219
				JOB NO. 880614				

② MAINTENANCE OF TRAFFIC DETAILS



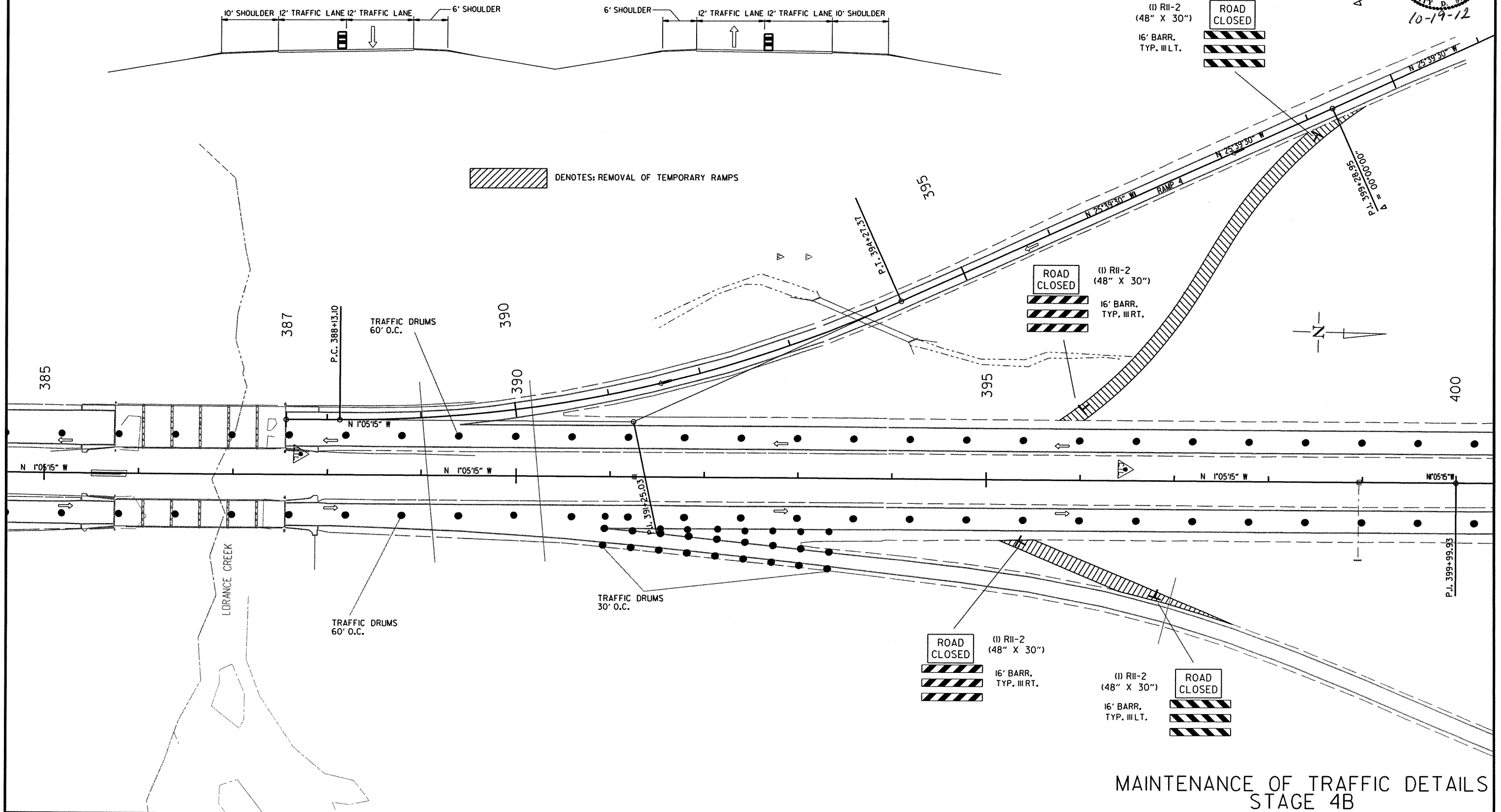
MAINTENANCE OF TRAFFIC DETAILS
 STAGE 4A

SEQUENCE OF CONSTRUCTION
STAGE 4B
SHIFT NB TRAFFIC TO INSIDE OF NB LANES.
SHIFT SB TRAFFIC TO INSIDE OF SB LANES.
OBLITERATE REMAINING TEMPORARY RAMPS.
OUTSIDE OF PROPOSED MAIN LANES.
MILL AND INLAY OUTSIDE LANES.

STAGE 4A QUANTITIES
TRAFFIC DRUMS = 1829 EACH
CONSTRUCTION PAVEMENT MARKINGS = 100799 LIN. FT.

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.		72	219
				JOB NO.	BB0614		72	219

② MAINTENANCE OF TRAFFIC DETAILS



10/11/2012

RBB0614.DGN

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB NO.	880614	73	219	

NOTE: MAINTAIN MINIMUM 12' LANE WIDTH ON LANE REMAINING OPEN

INSTALL WIRE ROPE SAFETY FENCE ON MEDIAN DITCH FORESLOPE

500' BUFFER
TRAFFIC DRUMS @ 60' O.C. = 8 EACH

MAX. 2 MILE WORK AREA WILL TAKE 352 TRAFFIC DRUMS EACH SIDE
TRAFFIC DRUMS @ 30' O.C. IN CLOSED LANE
TRAFFIC DRUMS @ 30' O.C. ON OPPOSITE MEDIAN SHOULDER

TRAFFIC DRUMS @ 20' O.C. = 33 EACH
1000' ACCEL. LANE FOR WORK TRUCKS

C.L. MEDIAN I-530

VARIABLE WIDTH: 60'-0" NORMAL

C.L. LANES

6'-0" SHLDR. RETAIN

0.02'/'

EXISTING SLOPE

WIRE ROPE SAFETY FENCE

6'-0" SHLDR. RETAIN

0.02'/'

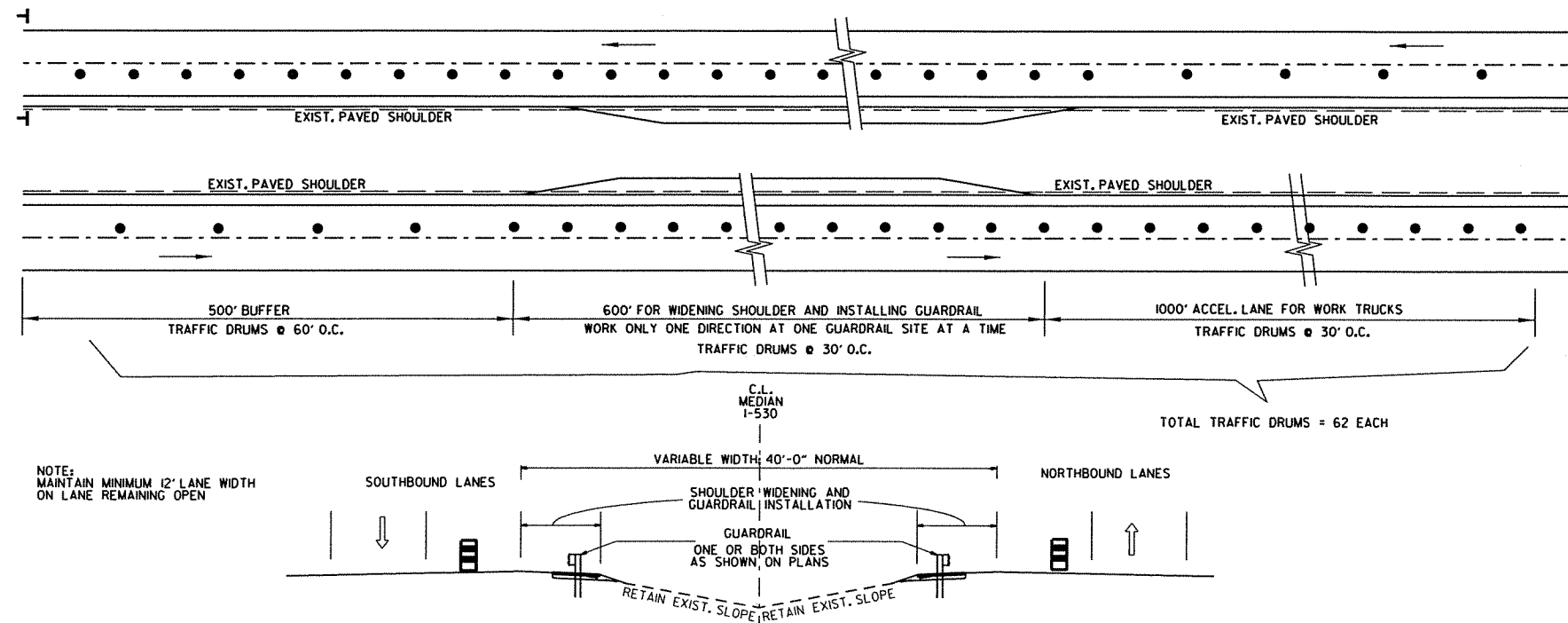
EXISTING SLOPE OR ADJUST TO 4:1 OR FLATTER

4'-0" CONCRETE DITCH PAVING (TYPE B)

SPEED LIMIT 65

(2) R2-1 (48" X 60")

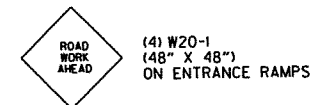
SPEED
LIMIT
65



NOTE: REFER TO SP-MAINTENANCE OF TRAFFIC FOR LANE CLOSURE LIMITATIONS AND RESTRICTIONS.

QUANTITY OF TRAFFIC DRUMS PROVIDED IN THE CONTRACT IS THE MAXIMUM NUMBER REQUIRED FOR CLOSING ONE LANE EACH DIRECTION IN WORK AREA.

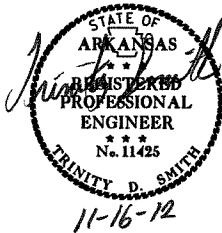
NOTE: REFER TO SHEET 49 FOR LANE CLOSURE DETAIL.



STAGE 5 - MAINTENANCE OF TRAFFIC
WORK ZONE FOR WRSF INSTALLATION

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
11-16-12				6	ARK.			
					JOB NO.	BB0614	74	219

2 QUANTITIES



ADVANCE WARNING SIGNS AND DEVICES

SIGN NUMBER	DESCRIPTION	SIGN SIZE	STAGE 1A	STAGE 1B	STAGE 2	STAGE 3A	STAGE 3B	STAGE 3C	STAGE 3D	STAGE 4A	STAGE 4B	STAGE 5	MAXIMUM NUMBER REQUIRED	TOTAL SIGNS REQUIRED		PORTABLE CHANGEABLE MESSAGE SIGN	ADVANCE WARNING ARROW PANEL	TRAFFIC DRUMS	BARRICADES (TYPE III)		FURNISHING & INSTALLING PRECAST CONC. BARRIER	RELOCATING PRECAST CONCRETE BARRIER	MOBILE SPEED NOTIFICATION SYSTEM							
														LIN. FT. - EACH					NO.	SQ. FT.				WEEK	DAY	EACH	RIGHT	LEFT	LIN. FT.	EACH
W20-1	ROAD WORK 1 MILE	48"x48"	4	4	4	4	4	4	4	4	4	4	4	4	4	64.0														
W20-1	ROAD WORK 1/2 MILE	48"x48"	4	4	4	4	4	4	4	4	4	4	4	4	4	64.0														
W20-1	ROAD WORK 1500 FT.	48"x48"	4	4	4	4	4	4	4	4	4	4	4	4	4	64.0														
W20-1	ROAD WORK AHEAD	48"x48"	18	18	18	18	18	18	18	18	18	18	18	18	18	288.0														
W20-5	RIGHT LANE CLOSED 1 MILE	48"x48"	4	4	4	4	4	4	4	4	4	4	4	4	4	64.0														
W20-5	RIGHT LANE CLOSED 1/2 MILE	48"x48"	4	4	4	4	4	4	4	4	4	4	4	4	4	64.0														
W20-5	RIGHT LANE CLOSED 1500 FT.	48"x48"	4	4	4	4	4	4	4	4	4	4	4	4	4	64.0														
W4-2 RT.	RIGHT LANE CLOSING GRAPHIC	48"x48"	4	4	4	4	4	4	4	4	4	4	4	4	4	64.0														
G20-2	END ROAD WORK	48"x24"	18	18	18	18	18	18	18	18	18	18	18	18	18	144.0														
G20-1	ROAD WORK NEXT xx MILES	60"x24"	2	2	2	2	2	2	2	2	2	2	2	2	2	20.0														
W1-4AR	REVERSE CURVE RT.	48"x48"			1		1	1	1				1	1	16.0															
W1-4AL	REVERSE CURVE LT.	48"x48"			1		2	1	1				2	2	32.0															
W13-1	SPEED LIMIT (ADVISORY)	24"x24"			1		2	2	2				2	2	8.0															
R11-2	ROAD CLOSED	48"x30"	4	16	13	4	11	11	11	19	4	4	19	19	190.0															
OM-3L	OBJECT MARKER	12"x36"							4				4	4	12.0															
OM-3R	OBJECT MARKER	12"x36"			4		4	4	4				4	4	12.0															
W1-6	LARGE ARROW	48"x24"	6	12	14	24	14	14	14	24	18	6	24	24	192.0															
W1-8	CHEVRONS	18"x24"			6		6	6	12				12	12	36.0															
R4-1	DO NOT PASS	24"x30"	4	4	4	4	4	4	4	4	4	4	4	4	20.0															
R2-5A	REDUCED SPEED AHEAD	48"x60"	4	4	4	4	4	4	4	4	4	4	4	4	80.0															
RSP-1	SHOULDER CLOSED	48"x30"	2	2	2	2	2	2	2	2	2	2	2	2	20.0															
R2-1	SPEED LIMIT 60	48"x60"	4	4	4	4	4	4	4	4	4	4	4	4	80.0															
R2-1	SPEED LIMIT 70	48"x60"	4	4	4	4	4	4	4	4	4	4	4	4	80.0															
R55-1	FINES DOUBLE IN WORK ZONES	36"x60"	4	4	4	4	4	4	4	4	4	4	4	4	60.0															
R2-2	TRUCK SPEED LIMIT 65	48"x60"	4	4	4	4	4	4	4	4	4	4	4	4	80.0															
SPECIAL	MERGE NOW WITH ARROW	48"x48"	4	4	4	4	4	4	4	4	4	4	4	4	64.0															
R1-2	YIELD	60"x60"x60"											1	1	10.9															
	PORTABLE CHANGEABLE MESSAGE SIGN		4	4	4	4	4	4	4	4	4	4	4			384														
	ADVANCE WARNING ARROW PANEL		2	2	2	2	2	2	2	2	2	2	2				1344													
	TRAFFIC DRUMS		168	261	149	1462	1326	1314	1314	2238	1829	1829	2238					2238												
	TYPE III BARRICADE-RT. (16')		2	2	9		8	8	8	2	2	2	9						144											
	TYPE III BARRICADE-LT. (16')		2	14	5	4	3	3	3	17	2	2	17							272										
	TYPE III BARRICADE-RT. (24')				1		2	2	2				2						48											
	FURNISHING AND INSTALLING PRECAST CONCRETE BARRIER				3586			2126	906				5712							5712										
	RELOCATING PRECAST CONCRETE BARRIER						3146	440					3586								3586									
	MOBILE SPEED NOTIFICATION SYSTEM												2									2								
TOTALS:															1892.9	384	1344	2238	192	272	5712	3586	2							
NOTE: THIS IS A LIGHT TRAFFIC VOLUME ROAD AS DEFINED BY SECTION 201.02, STANDARD SPECIFICATIONS FOR ROADWAY CONSTRUCTION, 2011 EDITION.															1892.9	384	1344	2238	192	272	5712	3586	2							

NOTE: THIS IS A HIGH TRAFFIC VOLUME ROAD AS DEFINED IN SECTION 604.03, STANDARD SPECIFICATIONS FOR HIGHWAY CONSTRUCTION, 2003 EDITION.

CONSTRUCTION PAVEMENT MARKINGS AND PERMANENT PAVEMENT MARKINGS

DESCRIPTION	STAGE 2	STAGE 3B	STAGE 4A	STAGE 4B	END OF JOB	REMOVAL OF PERMANENT PAVEMENT MARKINGS	CONSTRUCTION PAVEMENT MARKINGS	REMOVABLE CONSTRUCTION PAVEMENT MARKINGS	RAISED PAVEMENT MARKERS	HIGH PERFORMANCE PAVEMENT MARKINGS			HIGH PERFORMANCE CONTRAST PAVEMENT MARKING	
										TYPE II (WHITE/RED) EACH	4"		8" WHITE	4"
									WHITE		YELLOW	WHITE		
	LIN. FT. - EACH						LIN. FT.			LIN. FT.				
REMOVAL OF PERMANENT PAVEMENT MARKINGS	7762	3823				11585								
CONSTRUCTION PAVEMENT MARKINGS	16909	17936	100799	100799			236443							
REMOVABLE CONSTRUCTION PAVEMENT MARKINGS	150	4094						4244						
RAISED PAVEMENT MARKERS TYPE II (WHITE/RED)					1763				1763					
HIGH PERFORMANCE PAVEMENT MARKINGS WHITE (4")					105151					105151				
HIGH PERFORMANCE PAVEMENT MARKINGS YELLOW (4")					89382						89382			
HIGH PERFORMANCE PAVEMENT MARKINGS WHITE (8")					5558							5558		
HIGH PERFORMANCE CONTRAST PAVEMENT MARKING WHITE (4")					1250								1250	
HIGH PERFORMANCE CONTRAST PAVEMENT MARKING WHITE (8")					257									257
TOTALS:						11585	236443	4244	1763	105151	89382	5558	1250	257
NOTE: THIS IS A HIGH TRAFFIC HIGH VOLUME ROAD. ALL SURFACES ARE CONCRETE.														

NOTE: THIS IS A HIGH TRAFFIC VOLUME ROAD AS DEFINED IN SECTION 604.03, STANDARD SPECIFICATIONS FOR HIGHWAY CONSTRUCTION, 2003 EDITION.

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
11-16-12				6	ARK.			
				JOB NO.		BB0614	75	219

2 QUANTITIES



CLEARING AND GRUBBING

STATION	STATION	LOCATION	CLEARING	GRUBBING
			STATION	
353+32	362+84	I-530 N.B. ACCEL. LANE EXTENSION AT HWY. 167	10	10
381+90	388+00	I-530 MAIN LANES	7	7
391+00	392+80	I-530 MAIN LANES	2	2
390+00	396+97	BINGHAM RD. RAMP 4	7	7
395+00	399+50	BINGHAM RD. TEMPORARY RAMP 4	5	5
TOTALS:			31	31

BENCH MARKS

STATION	LOCATION	EACH
175+05	I-530 N.B. MAIN LANES BRIDGE END	1
385+67	I-530 S.B. MAIN LANES BRIDGE END	1
385+67	I-530 N.B. MAIN LANES BRIDGE END	1
393+38	BINGHAM RD. INTERCHANGE RAMP 4 - R.C. BOX CULVERT - PLACE ON RT. HEADWALL	1
TOTAL:		4

NOTE: SHOWN FOR INFORMATION ONLY. BENCH MARKS SHALL BE FURNISHED AND PLACED BY STATE FORCES.

REMOVAL AND DISPOSAL OF ITEMS

STATION	STATION	LOCATION	GUARDRAIL	TERMINAL ANCHOR POSTS	IMPACT ATTENUATION BARRIER	PIER PROTECTION	APPROACH GUTTERS
			LIN. FT.	EACH	EACH	LIN. FT.	EACH
56+55	58+55	I-530 N.B. MAIN LANE ON LEFT	200	1			
56+55	58+55	I-530 N.B. MAIN LANE ON RIGHT	200	1			
173+15	175+15	I-530 N.B. MAIN LANE ON LEFT	200	1			
173+15	175+15	I-530 N.B. MAIN LANE ON RIGHT	200	1			
283+70	285+70	I-530 N.B. MAIN LANE ON LEFT	200	1			
283+70	285+70	I-530 N.B. MAIN LANE ON RIGHT	200	1			
337+54	339+64	I-530 N.B. MAIN LANE ON RIGHT	200	1			
337+77	339+87	I-530 N.B. MAIN LANE ON LEFT	200	1			
384+00	385+75	I-530 N.B. MAIN LANE ON LEFT	175	1			
384+00	385+75	I-530 N.B. MAIN LANE ON RIGHT	175	1			
387+65	389+65	I-530 S.B. MAIN LANE ON LEFT	200	1			
387+65	389+40	I-530 S.B. MAIN LANE ON RIGHT	175	1			
74+39		HENSLEY RD. OVERPASS			2		
250+66		WOODSON LATERAL RD. OVERPASS			2		
401+77		BINGHAM RD. OVERPASS			2		
337+17	337+54	I-530 N.B. MAIN LANE ON RIGHT				37	
337+40	337+77	I-530 N.B. MAIN LANE ON LEFT				37	
58+55		I-530 N.B. MAIN LANE ON LEFT					1
58+55		I-530 N.B. MAIN LANE ON RIGHT					1
175+06		I-530 N.B. MAIN LANE ON LEFT					1
175+06		I-530 N.B. MAIN LANE ON RIGHT					1
176+57		I-530 N.B. MAIN LANE ON LEFT					1
176+57		I-530 N.B. MAIN LANE ON RIGHT					1
285+70		I-530 N.B. MAIN LANE ON LEFT					1
285+70		I-530 N.B. MAIN LANE ON RIGHT					1
385+67		I-530 N.B. MAIN LANE ON LEFT					1
385+67		I-530 N.B. MAIN LANE ON RIGHT					1
385+67		I-530 S.B. MAIN LANE ON LEFT					1
385+67		I-530 S.B. MAIN LANE ON RIGHT					1
387+49		I-530 N.B. MAIN LANE ON LEFT					1
387+49		I-530 N.B. MAIN LANE ON RIGHT					1
387+49		I-530 S.B. MAIN LANE ON LEFT					1
387+49		I-530 S.B. MAIN LANE ON RIGHT					1
TOTALS:			2325	12	6	74	16

GUARDRAIL

STATION	STATION	LOCATION	GUARDRAIL (TYPE A)	THRIE BEAM GUARDRAIL TERMINAL	GUARDRAIL TERMINAL (TYPE 2)	TERMINAL ANCHOR POST (TYPE 1)
			LIN. FT.	EACH		
964+07.60	969+07.60	I-530 N.B. MAIN LANES - INSIDE	450		1	1
967+15.60	972+15.60	I-530 S.B. MAIN LANES - INSIDE	450		1	1
55+26.85	58+45.60	I-530 N.B. MAIN LANES - INSIDE	250	1	1	
55+76.85	58+45.60	I-530 N.B. MAIN LANES - OUTSIDE	200	1	1	
70+30.00	75+30.00	I-530 N.B. MAIN LANES - INSIDE	450		1	1
73+20.00	78+20.00	I-530 S.B. MAIN LANES - INSIDE	450		1	1
171+77.82	174+96.57	I-530 N.B. MAIN LANES - INSIDE	250	1	1	
172+27.82	174+96.57	I-530 N.B. MAIN LANES - OUTSIDE	200	1	1	
246+56.00	251+56.00	I-530 N.B. MAIN LANES - INSIDE	450		1	1
249+42.00	254+42.00	I-530 S.B. MAIN LANES - INSIDE	450		1	1
282+41.80	285+60.55	I-530 N.B. MAIN LANES - INSIDE	250	1	1	
282+91.80	285+60.55	I-530 N.B. MAIN LANES - OUTSIDE	200	1	1	
337+54.00	340+97.75	I-530 S.B. MAIN LANES - OUTSIDE	325	1	1	
337+77.00	341+70.75	I-530 S.B. MAIN LANES - INSIDE	275	1	1	
382+38.97	385+57.72	I-530 N.B. MAIN LANES - INSIDE	250	1	1	
382+88.97	385+57.72	I-530 N.B. MAIN LANES - OUTSIDE	200	1	1	
387+58.02	390+76.77	I-530 S.B. MAIN LANES - INSIDE	250	1	1	
387+58.02	390+76.77	I-530 S.B. MAIN LANES - OUTSIDE	250	1	1	
398+00.00	403+00.00	I-530 N.B. MAIN LANES - INSIDE	450		1	1
400+51.00	405+51.00	I-530 S.B. MAIN LANES - INSIDE	450		1	1
TOTALS:			6500	12	20	8

SELECTED PIPE BEDDING

LOCATION	SELECTED PIPE BEDDING
	CU.YD.
ENTIRE PROJECT TO BE USED IF AND WHERE DIRECTED BY THE ENGINEER	10
TOTAL:	10

NOTE: QUANTITY ESTIMATED.
SEE SECTION 104.03 OF THE STD. SPECS.

QUANTITIES

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
11-16-12				6	ARK.			
				JOB NO.		BB0614	76	219

2 QUANTITIES



EARTHWORK

STATION	STATION	LOCATION / DESCRIPTION	UNCLASSIFIED EXCAVATION	COMPACTED EMBANKMENT	* SOIL STABILIZATION
			CU. YD.		TON
ENTIRE	PROJECT	MAIN LANES	4706	4855	
389+52.49	396+96.95	BINGHAM RD. INTERCHANGE RAMP 4	1255	2755	
353+32.03	362+84.43	N.B. ACCELERATION LANE EXTENSION AT HWY. 167	309	170	
ENTIRE	PROJECT	M.O.T. CROSSTOVERS		5300	
ENTIRE	PROJECT	M.O.T. CROSSTOVERS - REMOVAL	8350		
395+13.12	399+92.71	BINGHAM RD. INTERCHANGE TEMPORARY RAMP 4		1500	
395+13.12	399+92.71	BINGHAM RD. INTERCHANGE TEMPORARY RAMP 4 - REMOVAL	1820		
395+26.92	398+27.06	BINGHAM RD. INTERCHANGE TEMPORARY RAMP 1		750	
395+26.92	398+27.06	BINGHAM RD. INTERCHANGE TEMPORARY RAMP 1 - REMOVAL	785		
* ENTIRE	PROJECT	TO BE USED IF AND WHERE DIRECTED BY THE ENGINEER			500
TOTALS:			17225	15330	500

* QUANTITY ESTIMATED.
SEE SECTION 104.03 OF THE STD. SPECS.

NOTE: EARTHWORK QUANTITIES SHOWN ABOVE SHALL BE PAID AS PLAN QUANTITY.

EROSION CONTROL

STATION	STATION	LOCATION	PERMANENT EROSION CONTROL					TEMPORARY EROSION CONTROL									
			SEEDING	LIME	MULCH COVER	WATER	SECOND SEEDING APPLICATION	TEMPORARY SEEDING	MULCH COVER	WATER	SAND BAG DITCH CHECKS	ROCK DITCH CHECKS	DROP INLET SILT FENCE	SILT FENCE	SEDIMENT BASIN	OBLITERATION OF SEDIMENT BASIN	*SEDIMENT REMOVAL & DISPOSAL
			ACRE	TON	ACRE	M.GAL.	ACRE	ACRE	ACRE	M.GAL.	(E-5) BAG	(E-6) CU.YD.	(E-7) LIN. FT.	(E-11) LIN. FT.	(E-14) CU.YD.	CU.YD.	CU. YD.
ENTIRE	PROJECT	STAGE 1A						1.00	1.00	20.4	88	12		1313			49
ENTIRE	PROJECT	STAGE 1B						6.00	6.00	122.4	154	18	125	660			37
ENTIRE	PROJECT	STAGE 2									66	12		1309			61
ENTIRE	PROJECT	STAGE 3A									110	9	50	646			
ENTIRE	PROJECT	STAGE 3B									44	12		1149			
ENTIRE	PROJECT	STAGE 4A											750				
ENTIRE	PROJECT	STAGE 5									2046	264	1625				
ENTIRE	PROJECT	END OF JOB	6.49	12.98	6.49	662.0	6.49										7
*ENTIRE	PROJECT	TO BE USED IF AND WHERE DIRECTED BY THE ENGINEER	1.62	3.25	1.62	165.5	1.62	1.75	1.75	35.7	638	81	25	1269	400	400	448
TOTALS:			8.11	16.23	8.11	827.5	8.11	8.75	8.75	178.5	3146	408	2575	6346	400	400	610

BASIS OF ESTIMATE:
LIME 2 TONS / ACRE OF SEEDING
WATER 102.0 M.G. / ACRE OF SEEDING
WATER 20.4 M.G. / ACRE OF TEMPORARY SEEDING
SAND BAG DITCH CHECKS 22 BAGS / LOCATION
ROCK DITCH CHECKS 3 CU.YD./LOCATION

NOTE: THE TEMPORARY EROSION CONTROL DEVICES SHOWN ABOVE AND ON THE PLANS SHALL BE INSTALLED IN SUCH A SEQUENCE AS TO DETER EROSION AND SEDIMENTATION ON U.S. WATERWAYS AS EXPLAINED BY THE NATIONAL POLLUTANT DISCHARGE ELIMINATION SYSTEM PERMIT.

*QUANTITIES ARE ESTIMATED.
SEE SECTION 104.03 OF THE STD. SPECS.

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
11-16-12				6	ARK.			
				JOB NO.		880614	77	219

2 QUANTITIES



11-16-12

CONCRETE DITCH PAVING

STATION	STATION	LOCATION	F.A.P. HSIP-530-1(5)9				F.A.P. BIM-B530(204)
			LENGTH	WIDTH	CONC. DITCH PAVING (TYPE B)	SOLID SODDING	WATER
			LIN. FT.	FEET	SQ. YD.	SQ. YD.	M. GAL.
771+85.00	967+05.60	I-530 S.B. MAIN LANES - INSIDE	19520.60	4	8676.00	4337.91	54.7
968+55.00	1102+58.60	I-530 N.B. MAIN LANES - INSIDE	13403.60	4	5792.00	2978.58	37.5
1103+63.70	1114+51.24	I-530 S.B. MAIN LANES - INSIDE	1087.54	4	483.00	241.68	3.0
1113+85.90	1124+55.00	I-530 S.B. MAIN LANES - INSIDE	1069.10	4	475.00	237.58	3.0
1125+60.00	1137+75.00	I-530 S.B. MAIN LANES - INSIDE	1215.00	4	540.00	270.00	3.4
1138+25.00	1142+97.10	I-530 S.B. MAIN LANES - INSIDE	472.10	4	210.00	104.91	1.3
1142+94.90	1150+09.00	I-530 S.B. MAIN LANES - INSIDE	714.10	4	317.00	158.69	2.0
0+00.00	00+88.70	I-530 S.B. MAIN LANES - INSIDE	88.70	4	39.00	19.71	0.2
02+00.00	5+00.00	I-530 S.B. MAIN LANES - INSIDE	300.00	4	133.33	66.67	0.8
0+10.50	5+01.32	I-530 S.B. MAIN LANES - INSIDE	490.82	4	218.14	109.07	1.4
5+00.00	58+45.00	I-530 S.B. MAIN LANES - INSIDE	5345.00	4	2375.56	1187.78	15.0
59+55.00	70+95.00	I-530 N.B. MAIN LANES - INSIDE	1140.00	4	506.67	253.33	3.2
74+95.00	174+96.00	I-530 N.B. MAIN LANES - INSIDE	10001.00	4	4444.89	2222.44	28.0
176+67.00	247+17.00	I-530 N.B. MAIN LANES - INSIDE	7050.00	4	3133.33	1566.67	19.7
251+17.00	285+60.00	I-530 N.B. MAIN LANES - INSIDE	3443.00	4	1530.22	765.11	9.6
286+70.00	322+42.00	I-530 N.B. MAIN LANES - INSIDE	3572.00	4	1587.56	793.78	10.0
375+21.00	385+56.00	I-530 N.B. MAIN LANES - INSIDE	1035.00	4	460.00	230.00	2.9
387+60.00	398+52.00	I-530 N.B. MAIN LANES - INSIDE	1092.00	4	485.33	242.67	3.1
TOTALS:					31407.03	15786.58	198.8

BASIS OF ESTIMATE:

WATER.....12.6 GAL. / SQ. YD. OF SOLID SODDING.

WIRE ROPE SAFETY FENCE

STATION	STATION	LOCATION	F.A.P. HSIP-530-1(5)9		
			WIRE ROPE SAFETY FENCE	* WRSF ANCHOR	WRSF MAINTENANCE MATERIALS
			LIN. FT	EACH	LUMP SUM
771+85.00	967+05.60	I-530 S.B. MAIN LANES - INSIDE	19520.60	2	
968+55.00	1102+58.60	I-530 N.B. MAIN LANES - INSIDE	13403.60	2	
1103+63.70	1114+51.24	I-530 S.B. MAIN LANES - INSIDE	1087.54	1	
1113+85.90	1124+55.00	I-530 S.B. MAIN LANES - INSIDE	1069.10	1	
1125+60.00	1137+75.00	I-530 S.B. MAIN LANES - INSIDE	1215.00	2	
1138+25.00	1142+97.10	I-530 S.B. MAIN LANES - INSIDE	472.10	1	
1142+94.90	1150+09.00	I-530 S.B. MAIN LANES - INSIDE	714.10		
0+00.00	00+88.70	I-530 S.B. MAIN LANES - INSIDE	88.70	1	
02+00.00	5+00.00	I-530 S.B. MAIN LANES - INSIDE	300.00	1	
0+10.50	5+01.32	I-530 S.B. MAIN LANES - INSIDE	490.82		
5+00.00	58+45.00	I-530 S.B. MAIN LANES - INSIDE	5345.00	1	
59+55.00	70+95.00	I-530 N.B. MAIN LANES - INSIDE	1140.00	2	
74+95.00	174+96.00	I-530 N.B. MAIN LANES - INSIDE	10001.00	2	
176+67.00	247+17.00	I-530 N.B. MAIN LANES - INSIDE	7050.00	2	
251+17.00	285+60.00	I-530 N.B. MAIN LANES - INSIDE	3443.00	2	
286+70.00	322+42.00	I-530 N.B. MAIN LANES - INSIDE	3572.00	2	
375+21.00	385+56.00	I-530 N.B. MAIN LANES - INSIDE	1035.00	2	
387+60.00	398+52.00	I-530 N.B. MAIN LANES - INSIDE	1092.00	2	
ENTIRE	PROJECT				1.00
TOTALS:			71039.56	26	1.00

* SHOWN FOR INFORMATION ONLY.

4" PIPE UNDERDRAIN

STATION	STATION	LOCATIONS	4" PIPE UNDERDRAINS	UNDERDRAIN OUTLET PROTECTORS
			LIN. FT.	EACH
ENTIRE PROJECT TO BE USED IF AND WHERE DIRECTED BY THE ENGINEER			500	2
TOTALS:			500	2

* NOTE: QUANTITIES ARE ESTIMATED.

SEE SECTION 104.03 OF THE STD. SPECS.

CONCRETE BARRIER WALL

STATION	STATION	LOCATION	PIER PROTECTION (TYPE A)
			LIN. FT.
337+17	337+54	I-530 S.B. MAIN LANE - INSIDE	37
337+40	337+77	I-530 S.B. MAIN LANE - OUTSIDE	37
TOTAL:			74

RUMBLE STRIPS IN ASPHALT SHOULDERS

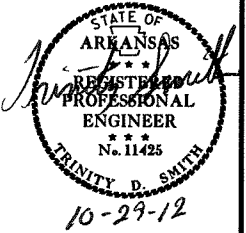
STATION	STATION	LOCATION	* RUMBLE STRIPS IN ASPHALT SHOULDERS
			LIN. FT.
56+20	58+20	I-530 N.B. MAIN LANE - INSIDE	200
59+80	60+60	I-530 N.B. MAIN LANE - INSIDE	80
283+18	287+60	I-530 N.B. MAIN LANE - INSIDE	442
286+80	287+60	I-530 N.B. MAIN LANE - INSIDE	80
56+20	58+20	I-530 N.B. MAIN LANE - OUTSIDE	200
59+80	60+60	I-530 N.B. MAIN LANE - OUTSIDE	80
283+18	287+60	I-530 N.B. MAIN LANE - OUTSIDE	442
286+80	287+60	I-530 N.B. MAIN LANE - OUTSIDE	80
158+99	162+61	I-530 N.B. MAIN LANE - INSIDE	362
165+51	174+83	I-530 S.B. MAIN LANE -INSIDE	932
176+81	182+52	I-530 S.B. MAIN LANE -INSIDE	571
186+11	189+03	I-530 N.B. MAIN LANE - INSIDE	292
371+56	374+46	I-530 N.B. MAIN LANE - INSIDE	290
372+79	376+08	I-530 S.B. MAIN LANE -INSIDE	329
377+95	385+30	I-530 S.B. MAIN LANE -INSIDE	735
378+93	385+30	I-530 N.B. MAIN LANE - INSIDE	637
387+86	389+53	I-530 S.B. MAIN LANE -INSIDE	167
387+86	403+68	I-530 S.B. MAIN LANE -INSIDE	1582
387+86	409+30	I-530 N.B. MAIN LANE - INSIDE	2144
387+86	393+34	I-530 N.B. MAIN LANE - INSIDE	548
389+53	392+90	I-530 S.B. MAIN LANE -INSIDE	337
394+90	395+28	I-530 N.B. MAIN LANE - INSIDE	38
395+64	395+91	I-530 S.B. MAIN LANE -INSIDE	27
408+48	411+24	I-530 S.B. MAIN LANE -INSIDE	276
TOTAL:			10871

* QUANTITY ESTIMATED.

SEE SECTION 104.03 OF THE STD. SPECS.

TO BE USED IF AND WHERE DIRECTED BY THE ENGINEER

QUANTITIES



STRUCTURES

STATION	LOCATION	DESCRIPTION	REINFORCED CONCRETE PIPE CULVERT (CLASS III)	FLARED END SECTIONS FOR R.C. PIPE CULVERTS	TEMPORARY PIPE CULVERTS		SPAN	HEIGHT	LENGTH	CLASS S CONCRETE- ROADWAY	REINF. STEEL- ROADWAY (GRADE 60)	UNCL.EXC. FOR STR.- ROADWAY	SOLID SODDING	WATER	STD. DWG. NOS.	
			24"	24"	18"	48"										
			LIN. FT.	EACH	LIN. FT.											LIN. FT.
163+75	I-530 TEMP. CROSSOVER	18" TEMPORARY PIPE			650											
184+25	I-530 TEMP. CROSSOVER	18" TEMPORARY PIPE			650											
357+37	HWY. 167 RAMP 1 TURNOUT	EXTEND R.C. PIPE CULVERT	22	1									8	0.1		FES-1, PCC-1
377+33	I-530 TEMP. CROSSOVER	18" TEMPORARY PIPE			536											
393+81	BINGHAM RD. RAMP 4	EXTEND R.C. BOX CULVERT					6	4	26	26.12	2508	18	21	0.3		RCB-1, RCB-2, RCB-3, R-100X-X1, W-X45, W-X453-1, R-145X-1
393+90	I-530 TEMP. CROSSOVER	18" TEMPORARY PIPE			326											
396+00	BINGHAM RD. TEMP. RAMP 1	18" TEMPORARY PIPE			110											
396+27	BINGHAM RD. TEMP. RAMP 4	DBL. 48" TEMPORARY PIPE				152										
405+65	I-530 TEMP. CROSSOVER	18" TEMPORARY PIPE			730											
TOTALS:			22	1	3002	152				26.12	2508	18	29	0.4		

BASIS OF ESTIMATE:
WATER..... 12.6 GAL. / SQ. YD. OF SOLID SODDING.

NOTE: FOR R.C. PIPE CULVERT INSTALLATIONS USE TYPE 3 BEDDING UNLESS OTHERWISE SPECIFIED.

APPROACH GUTTERS AND SLABS

STATION	STATION	LOCATION	APPROACH GUTTERS		APPROACH SLABS			REINFORCING STEEL RDWY. (GR 60)	AGGREGATE BASE CRS. (CLASS 7)
			TYPE SPECIAL 1	TYPE SPECIAL 2	TYPE SPECIAL 1	TYPE SPECIAL 2	TYPE SPECIAL 3		
			CU.YD.					POUND	TON
58+20.00	58+55.00	I-530 N.B. MAIN LANES - BRIDGE NO. B3323 - INSIDE	14.05					2362	
58+20.00	58+55.00	I-530 N.B. MAIN LANES - BRIDGE NO. B3323			55.44			6400	27.00
58+20.00	58+55.00	I-530 N.B. MAIN LANES - BRIDGE NO. B3323 - OUTSIDE	20.32					3427	
174+69.47	175+05.97	I-530 N.B. MAIN LANES - BRIDGE NO. B7266 - INSIDE		13.19				1238	
174+69.47	175+05.97	I-530 N.B. MAIN LANES - BRIDGE NO. B7266				49.40		5802	36.75
174+69.47	175+05.97	I-530 N.B. MAIN LANES - BRIDGE NO. B7266 - OUTSIDE		19.79				1758	
176+57.47	176+93.97	I-530 N.B. MAIN LANES - BRIDGE NO. B7266 - INSIDE		13.19				1238	
176+57.47	176+93.97	I-530 N.B. MAIN LANES - BRIDGE NO. B7266				49.40		5802	36.75
176+57.47	176+93.97	I-530 N.B. MAIN LANES - BRIDGE NO. B7266 - OUTSIDE		19.79				1758	
285+34.95	285+69.95	I-530 N.B. MAIN LANES - BRIDGE NO. B3326 - INSIDE	14.05					2362	
285+34.95	285+69.95	I-530 N.B. MAIN LANES - BRIDGE NO. B3326			55.44			6400	27.00
285+34.95	285+69.95	I-530 N.B. MAIN LANES - BRIDGE NO. B3326 - OUTSIDE	20.32					3427	
385+30.62	385+67.12	I-530 N.B. MAIN LANES - BRIDGE NO. B7267 - INSIDE		13.19				1238	
385+30.62	385+67.12	I-530 N.B. MAIN LANES - BRIDGE NO. B7267				49.40		5802	36.75
385+30.62	385+67.12	I-530 N.B. MAIN LANES - BRIDGE NO. B7267 - OUTSIDE		19.79				1758	
387+48.62	387+85.12	I-530 N.B. MAIN LANES - BRIDGE NO. B7267 - INSIDE		13.19				1238	
387+48.62	387+85.12	I-530 N.B. MAIN LANES - BRIDGE NO. B7267				49.40		5802	36.75
387+48.62	387+85.12	I-530 N.B. MAIN LANES - BRIDGE NO. B7267 - OUTSIDE		19.79				1758	
385+30.62	385+67.12	I-530 S.B. MAIN LANES - BRIDGE NO. A7267 - INSIDE		13.19				1238	
385+30.62	385+67.12	I-530 S.B. MAIN LANES - BRIDGE NO. A7267					74.10	8663	42.75
385+30.62	385+67.12	I-530 S.B. MAIN LANES - BRIDGE NO. A7267 - OUTSIDE		13.19				1238	
387+48.62	387+85.12	I-530 S.B. MAIN LANES - BRIDGE NO. A7267 - INSIDE		13.19				1238	
387+48.62	387+85.12	I-530 S.B. MAIN LANES - BRIDGE NO. A7267					74.10	8663	42.75
387+48.62	387+85.12	I-530 S.B. MAIN LANES - BRIDGE NO. A7267 - OUTSIDE		13.19				1238	
TOTALS:			68.74	184.68	110.88	197.60	148.20	81848	286.50

NOTE: USE T=15" FOR 6' AND 10' SHOULDERS.

COLD MILLING ASPHALT PAVEMENT

STATION	STATION	LOCATION	AVG. WIDTH	COLD MILLING ASPHALT PAVEMENT
			FEET	SQ. YD.
11+48.68	58+55.00	I-530 N.B. MAIN LANES	24.50	12811.65
59+45.00	175+05.97	I-530 N.B. MAIN LANES	24.50	31471.53
176+57.47	285+69.95	I-530 N.B. MAIN LANES	24.50	29706.20
286+59.95	316+11.41	I-530 N.B. MAIN LANES	24.50	8034.53
316+11.41	349+84.42	I-530 N.B. MAIN LANES	24.50	9182.08
350+00.00	379+21.87	I-530 N.B. MAIN LANES	24.50	7953.98
379+21.87	381+90.00	I-530 N.B. MAIN LANES	24.50	729.91
392+80.00	406+89.93	I-530 N.B. MAIN LANES	24.50	3838.14
11+48.68	58+55.00	I-530 S.B. MAIN LANES	24.50	12811.65
59+45.00	175+05.97	I-530 S.B. MAIN LANES	24.50	31471.53
176+57.47	285+66.25	I-530 S.B. MAIN LANES	24.50	29696.12
286+56.25	349+84.42	I-530 S.B. MAIN LANES	24.50	17226.69
350+00.00	381+90.00	I-530 S.B. MAIN LANES	24.50	8683.89
392+80.00	406+89.93	I-530 S.B. MAIN LANES	24.50	3838.14
60+30.17	66+51.49	HENSLEY RD. RAMP 1 TURNOUT	VAR.	840.30
59+86.42	69+87.48	HENSLEY RD. RAMP 4 TURNOUT/ ACCEL. LANE	VAR.	1154.00
84+33.69	94+41.20	HENSLEY RD. RAMP 2 TURNOUT/ ACCEL. LANE	VAR.	1212.80
84+99.21	90+48.87	HENSLEY RD. RAMP 3 TURNOUT/ ACCEL. LANE	VAR.	904.40
231+14.18	237+71.14	WOODSON RD. RAMP 1 TURNOUT	VAR.	880.00
235+26.88	245+26.81	WOODSON RD. RAMP 4 TURNOUT/ ACCEL. LANE	VAR.	1155.70
255+93.91	265+93.81	WOODSON RD. RAMP 2 TURNOUT/ ACCEL. LANE	VAR.	1155.80
263+77.84	270+06.51	WOODSON RD. RAMP 3 TURNOUT/ ACCEL. LANE	VAR.	867.60
342+35.71	346+96.22	HWY. 167 RAMP 1 TURNOUT	VAR.	619.50
379+46.64	381+90.00	BINGHAM RD. RAMP 4 TURNOUT/ ACCEL. LANE	VAR.	203.90
392+80.00	394+82.78	BINGHAM RD. RAMP 1 TURNOUT	VAR.	244.40
TOTAL:				216694.44

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
11-16-12				6	ARK.			
				JOB NO.		880614	79	219

2 QUANTITIES



11-16-12

BASE AND SURFACING

STATION	STATION	LOCATION	LENGTH	AGGREGATE BASE COURSE (CLASS 7)		TACK COAT				ACHM BASE COURSE (1 1/2")				ACHM BINDER COURSE (1")				ACHM SURFACE COURSE (1/2")									
				TON / STATION	TON	TOTAL WID. FEET	SQ. YD.	GALLONS / SQ. YD.	GALLON	AVG. WID. FEET	SQ. YD.	POUND / SQ. YD.	PG 76-22 TON	AVG. WID. FEET	SQ. YD.	POUND / SQ. YD.	PG 76-22 TON	AVG. WID. FEET	SQ. YD.	POUND / SQ. YD.	PG 76-22 TON	AVG. WID. FEET	SQ. YD.	POUND / SQ. YD.	PG 76-22 TON	TOTAL TONS	
MAIN LANES - MILL AND INLAY																											
11+48.68	58+55.00	I-530 N.B. MAIN LANES	4706.32			24.50	12811.6	0.10	1281.2													24.50	12811.6	220	1409.3	1409.3	
59+45.00	175+05.97	I-530 N.B. MAIN LANES	11560.97			24.50	31471.5	0.10	3147.2													24.50	31471.5	220	3461.9	3461.9	
176+57.47	285+69.95	I-530 N.B. MAIN LANES	10912.48			24.50	29706.2	0.10	2970.6													24.50	29706.2	220	3267.7	3267.7	
286+59.95	316+11.41	I-530 N.B. MAIN LANES	2951.46			24.50	8034.5	0.10	803.5													24.50	8034.5	220	883.8	883.8	
316+11.41	349+84.42	I-530 N.B. MAIN LANES	3373.01			24.50	9182.1	0.10	918.2													24.50	9182.1	220	1010.0	1010.0	
350+00.00	379+21.87	I-530 N.B. MAIN LANES	2921.87			24.50	7954.0	0.10	795.4													24.50	7954.0	220	874.9	874.9	
379+21.87	381+90.00	I-530 N.B. MAIN LANES	268.13			24.50	729.9	0.10	73.0													24.50	729.9	220	80.3	80.3	
392+80.00	406+89.93	I-530 N.B. MAIN LANES	1409.93			24.50	3838.1	0.10	383.8													24.50	3838.1	220	422.2	422.2	
11+48.68	58+55.00	I-530 S.B. MAIN LANES	4706.32			24.50	12811.6	0.10	1281.2													24.50	12811.6	220	1409.3	1409.3	
59+45.00	175+05.97	I-530 S.B. MAIN LANES	11560.97			24.50	31471.5	0.10	3147.2													24.50	31471.5	220	3461.9	3461.9	
176+57.47	285+66.25	I-530 S.B. MAIN LANES	10908.78			24.50	29696.1	0.10	2969.6													24.50	29696.1	220	3266.6	3266.6	
286+56.25	349+84.42	I-530 S.B. MAIN LANES	6328.17			24.50	17226.7	0.10	1722.7													24.50	17226.7	220	1894.9	1894.9	
350+00.00	381+90.00	I-530 S.B. MAIN LANES	3190.00			24.50	8683.9	0.10	868.4													24.50	8683.9	220	955.2	955.2	
392+80.00	406+89.93	I-530 S.B. MAIN LANES	1409.93			24.50	3838.1	0.10	383.8													24.50	3838.1	220	422.2	422.2	
MAIN LANES - MILL AND INLAY - ADDITIONAL FOR ENTRANCE AND EXIT RAMP																											
60+30.17	66+51.49	HENSLEY RD. RAMP 1 TURNOUT	621.32			VAR.	840.3	0.10	84.0													VAR.	840.3	220	92.4	92.4	
59+86.42	69+87.48	HENSLEY RD. RAMP 4 TURNOUT/ ACCEL. LANE	1001.06			VAR.	1154.0	0.10	115.4													VAR.	1154.0	220	126.9	126.9	
84+33.69	94+41.20	HENSLEY RD. RAMP 2 TURNOUT/ ACCEL. LANE	1007.51			VAR.	1212.8	0.10	121.3													VAR.	1212.8	220	133.4	133.4	
84+99.21	90+48.87	HENSLEY RD. RAMP 3 TURNOUT/ ACCEL. LANE	549.66			VAR.	904.4	0.10	90.4													VAR.	904.4	220	99.5	99.5	
231+14.18	237+71.14	WOODSON LATERAL RD. RAMP 1 TURNOUT	656.96			VAR.	880.0	0.10	88.0													VAR.	880.0	220	96.8	96.8	
235+26.88	245+26.81	WOODSON LATERAL RD. RAMP 4 TURNOUT/ ACCEL. LANE	999.93			VAR.	1155.7	0.10	115.6													VAR.	1155.7	220	127.1	127.1	
255+93.91	265+93.81	WOODSON LATERAL RD. RAMP 2 TURNOUT/ ACCEL. LANE	999.90			VAR.	1155.8	0.10	115.6													VAR.	1155.8	220	127.1	127.1	
263+77.84	270+06.51	WOODSON LATERAL RD. RAMP 3 TURNOUT/ ACCEL. LANE	628.67			VAR.	867.6	0.10	86.8													VAR.	867.6	220	95.4	95.4	
342+35.71	346+96.22	HWY. 167 RAMP 1 TURNOUT	460.51			VAR.	619.5	0.10	62.0													VAR.	619.5	220	68.1	68.1	
379+46.64	381+90.00	BINGHAM RD. RAMP 4 TURNOUT/ ACCEL. LANE	243.36			VAR.	203.9	0.10	20.4													VAR.	203.9	220	22.4	22.4	
392+80.00	394+82.78	BINGHAM RD. RAMP 1 TURNOUT	202.78			VAR.	244.4	0.10	24.4													VAR.	244.4	220	26.9	26.9	
MAIN LANES - FULL DEPTH CONSTRUCTION																											
353+32.03	362+84.43	I-530 N.B. ACCELERATION LANE EXTENSION AT HWY. 167	952.40	VAR.	1763.02	VAR.	4124.5	0.03	123.7	VAR.	1061.3	550	291.9	VAR.	1028.9	660	339.5	VAR.	1005.4	220	110.6	VAR.	1480.0	220	162.8	273.4	
381+90.00	382+38.84	I-530 MAIN LANES	48.84	956.75	467.28	274.10	1487.4	0.03	44.6	70.20	381.0	550	104.8	68.40	371.2	660	122.5	67.10	364.1	220	40.1	82.10	445.5	220	49.0	89.1	
382+38.84	385+30.62	I-530 MAIN LANES	291.78	961.75	2806.19	281.85	9137.6	0.03	274.1	72.17	2339.8	550	643.4	70.34	2280.4	660	752.5	69.00	2237.0	220	246.1	84.00	2723.3	220	299.6	545.7	
387+85.12	389+52.49	I-530 MAIN LANES	167.37	961.75	1609.68	281.85	5241.5	0.03	157.2	72.17	1342.1	550	369.1	70.34	1308.1	660	431.7	69.00	1283.2	220	141.2	84.00	1562.1	220	171.8	313.0	
389+52.49	392+80.00	I-530 MAIN LANES	327.51	943.00	3088.42	233.85	8509.8	0.03	255.3	60.17	2189.6	550	602.1	58.34	2123.0	660	700.6	57.00	2074.2	220	228.2	76.00	2765.6	220	304.2	532.4	
387+85.12	389+52.49	I-530 MAIN LANES (ADD'L FOR BINGHAM RD. RAMP 4 TURNOUT)	167.37	VAR.	16.63	VAR.	142.4	0.03	4.3	VAR.	35.6	550	9.8	VAR.	35.6	660	11.7	VAR.	35.6	220	3.9	VAR.	35.6	220	3.9	7.8	
387+85.12	392+80.00	I-530 MAIN LANES (ADD'L FOR BINGHAM RD. RAMP 1 TURNOUT)	494.88	VAR.	324.26	VAR.	2779.2	0.03	83.4	VAR.	694.8	550	191.1	VAR.	694.8	660	229.3	VAR.	694.8	220	76.4	VAR.	694.8	220	76.4	152.8	
389+52.49	397+55.95	I-530 ENTRANCE RAMP 4	803.46	384.75	3091.31	64.92	5795.6	0.03	173.9	17.08	1524.8	550	419.3	16.17	1443.5	660	476.4	15.50	1383.7	220	152.2	25.00	2231.8	220	245.5	397.7	
WIDENING FOR GUARDRAIL																											
54+16.25	58+45.00	I-530 N.B. MAIN LANES - INSIDE	428.75	VAR.	31.84																	VAR.	263.6	220	29.0	29.0	
54+48.25	58+45.00	I-530 N.B. MAIN LANES - OUTSIDE	396.75	VAR.	37.26																	VAR.	599.0	220	65.9	65.9	
70+42.86	75+44.82	I-530 N.B. MAIN LANES - INSIDE	501.96	VAR.	376.25																	VAR.	498.6	220	54.8	54.8	
73+34.17	78+34.68	I-530 S.B. MAIN LANES - INSIDE	500.51	VAR.	141.25																	VAR.	545.6	220	60.0	60.0	
170+77.82	174+69.47	I-530 N.B. MAIN LANES - INSIDE	391.65	VAR.	31.84																	VAR.	263.6	220	29.0	29.0	
171+09.82	174+69.47	I-530 N.B. MAIN LANES - OUTSIDE	359.65	VAR.	37.26																	VAR.	599.0	220	65.9	65.9	
246+64.60	251+66.84	I-530 N.B. MAIN LANES - INSIDE	502.24	VAR.	376.25																	VAR.	498.6	220	54.8	54.8	
249+56.23	254+56.79	I-530 S.B. MAIN LANES - INSIDE	500.56	VAR.	141.25																	VAR.	545.6	220	60.0	60.0	
281+41.20	285+69.95	I-530 N.B. MAIN LANES - INSIDE	428.75	VAR.	31.84																	VAR.	263.6	220	29.0	29.0	
281+73.20	285+69.95	I-530 N.B. MAIN LANES - OUTSIDE	396.75	VAR.	37.26																	VAR.	599.0	220	65.9	65.9	
381+38.97	385+57.72	I-530 N.B. MAIN LANES - INSIDE	418.75	VAR.	247.81																	VAR.	263.6	220	29.0		

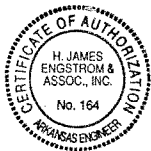
SCHEDULE OF BRIDGE QUANTITIES - JOB BB0614

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. ROAD NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
					ARK.			
					JOB NO.	BB0614		
B7266,A7267&B7267 QUANTITIES 53223								

BRIDGE NO. CODE NO.	NAME	PLATE	TITLE	UNIT OF STRUCTURE	ITEM NO.	205	801	802	802	803	SS & 804	SS & 804	805	805	805	805	805	812	816	816
					ITEM UNIT	REMOVAL OF EXISTING BRIDGE STRUCTURE (SITE _)	UNCLASSIFIED EXCAVATION FOR STRUCTURES-- BRIDGE	CLASS S CONCRETE-- BRIDGE	CLASS S(AE) CONCRETE-- BRIDGE	CLASS 1 PROTECTIVE SURFACE TREATMENT	REINFORCING STEEL-- BRIDGE (GRADE 60)	EPOXY COATED REINFORCING STEEL (GRADE 60)	CONCRETE PILING (18" IN. SQ.)	CONCRETE PILING (24" IN. SQ.)	TEST PILE (18 IN. SQ.)	TEST PILE (24 IN. SQ.)	PREBORING	BRIDGE NAME PLATE (TYPE D)	DUMPED RIPRAP	FILTER BLANKET
						LUMP SUM	CU. YD.	CU. YD.	CU. YD.	GAL.	LB.	LB.	LIN. FT.	LIN. FT.	LIN. FT.	LIN. FT.	LIN. FT.	EACH	CU. YD.	SQ. YD.
B7266	X020	CLEAR CREEK		END BENT 1			489	19.86			2895	473	312		57		60		334	669
				INTERMEDIATE BENT 2				16.92			2190	473		396						
				INTERMEDIATE BENT 3				16.92			2190	473		444		79				
				INTERMEDIATE BENT 4				16.92			2190	473		360						
				INTERMEDIATE BENT 5				16.92			2190	473		330						
				END BENT 6			400	19.86			2895	473	276				60		300	600
				151'-6" CONT. R.C. SLAB UNIT					433.00	15.8		110152								
				EXISTING BRIDGE NO. B3324 (SITE 2)		1														
				TOTALS FOR BRIDGE NO. B7266			889	107.40	433.00	15.8	14550	112990	588	1530	57	79	120	1	634	1269
A7267	X020	LORANCE CREEK		END BENT 1			310	23.00			3330	552	364		57		70		233	466
				INTERMEDIATE BENT 2				23.04			2588	552		420						
				INTERMEDIATE BENT 3				23.04			2588	552		441		68				
				INTERMEDIATE BENT 4				23.04			2588	552		441						
				INTERMEDIATE BENT 5				23.04			2588	552		378						
				INTERMEDIATE BENT 6				23.04			2588	552		357						
				END BENT 7			295	23.00			3330	552	336				70		404*	441
				181'-6" CONT. R.C. SLAB UNIT					608.30	22.1		152816								
				EXISTING BRIDGE NO. A3327 (SITE 4)		1														
TOTALS FOR BRIDGE NO. A7267			605	161.20	608.30	22.1	19600	156680	700	2037	57	68	140	1	637	907				
B7267	X020	LORANCE CREEK		END BENT 1			276	19.90			2895	473	312				60		207	414
				INTERMEDIATE BENT 2				16.92			2190	473		360						
				INTERMEDIATE BENT 3				16.92			2190	473		378						
				INTERMEDIATE BENT 4				16.92			2190	473		378						
				INTERMEDIATE BENT 5				16.92			2190	473		324						
				INTERMEDIATE BENT 6				16.92			2190	473		306						
				END BENT 7			333	19.90			2895	473	288				60		250	499
				181'-6" CONT. R.C. SLAB UNIT					518.70	18.9		131529								
				EXISTING BRIDGE NO. B3327 (SITE 5)		1														
TOTALS FOR BRIDGE NO. B7267			609	124.40	518.70	18.9	16740	134840	600	1746			120	1	457	913				
TOTALS FOR JOB NO. BB0614						2103	393.00	1560.00	①	50890	404510	1888	5313	114	147	380	3	1728	3089	

(1) See Dwg. No. 53224
for Job Total for this
Pay Item

* Includes 183 cu. yd.
scour hole repair



Handwritten signature of Bruce Edward Peterson
9/27/2012

BRIDGE ENGINEER

SHEET 1 OF 2
SCHEDULE OF BRIDGE QUANTITIES
CLEAR CREEK BRIDGE
AND LORANCE CREEK BRIDGES
BINGHAM RD. - GRANT CO. LINE (S)
PULASKI COUNTY

ROUTE 530 SEC. 1 & 3
ARKANSAS STATE HIGHWAY COMMISSION
LITTLE ROCK, ARK.

Engstrom/Modjeski and Masters

DRAWN BY: DSK DATE: 9-25-12 FILENAME: BBB0614_Q1.DGN
CHECKED BY: SPC DATE: 9-25-12 SCALE: As Noted
DESIGNED BY: SPC/TPL DATE: 9-25-12

BRIDGE NO. B7266, A7267 & B7267 DRAWING NO. 53223

*ALTERNATE BID ITEMS

[illegible]

② SUMMARY OF QUANTITIES AND REVISIONS



11-16-12

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB NO.		BB0614	83	219
(2) SURVEY CONTROL DETAILS								



SURVEY CONTROL COORDINATES

Coordinate System: ARKANSAS STATE PLANE - SOUTH ZONE BASED ON GPS CONTROL,
PROJECTED TO GROUND.
Units: U.S. SURVEY FOOT

Point Name	Northing	Easting	Elev	Feature	Description
620026a	1998150.7840	1223038.1870	274.759	GPS	
1	2010246.3840	1231663.2110	245.984	CTL	
2	2009370.3570	1231670.0800	247.676	CTL	
3	2008723.0360	1231690.5400	251.967	CTL	
4	1991559.3910	1240044.6440	245.330	CTL	
5	1990834.1470	1240529.5210	240.517	CTL	
6	1990198.7820	1240942.2530	241.311	CTL	
7	2011527.2610	1231642.3290	252.954	CTL	
8	2010860.9580	1231654.2090	248.506	CTL	
9	2008010.1960	1231700.5790	263.427	CTL	
10	2007309.4070	1231709.3760	270.410	CTL	
11	2006298.6510	1231732.3660	268.695	CTL	
12	2005458.7750	1231749.5010	267.335	CTL	
13	2004777.4870	1231711.5260	267.056	CTL	
14	2004172.5240	1231832.1980	266.567	CTL	
15	1997578.6420	1236064.6940	263.953	CTL	
16	1997128.1530	1236367.6200	264.349	CTL	
17	1996626.9520	1236689.5790	264.144	CTL	
18	1982931.1100	1245724.9860	253.960	CTL	
19	1982418.6470	1246064.1610	248.436	CTL	
20	1981891.9480	1246396.1830	242.950	CTL	
21	2005248.2730	1232131.2980	267.964	CTL	
22	2005874.6360	1232037.3710	268.251	CTL	
23	2006292.9680	1231990.4790	268.755	CTL	
620016	2002642.9960	1232762.5150	260.751	GPS	

*Note - Rebar and Cap - Standard - * Rebar with 2" Aluminum Cap stamped
*(standard markings common to all caps), or as indicated
(other markings indicated in the point description of the individual point).
USE CAF = 1.0 FOR STAKEOUT FOR THIS PROJECT
A PROJECT CAF WAS NOT COMPUTED ON THIS JOB DUE TO THE LENGTH OF THE JOB FOR THE GROUND COORDINATES LISTED ABOVE.
THIS CAF IS INTENDED FOR USE WITHIN THE PROJECT LIMITS.
GRID DISTANCE = GROUND DISTANCE X CAF.
GRID COORDINATES ARE STORED UNDER FILE NAME SBB00614G1.CTL
HORIZONTAL DATUM: NAD 83 (1997)
VERTICAL DATUM: NAVD 88 POSITIONAL ACCURACY THIRD ORDER, UNLESS SPECIFIED OTHERWISE
AT A SPECIFIC POINT.

REFERENCE POINTS (1500 SERIES) ARE TO BE USED TO ESTABLISH CONTROL
IF THE PRIMARY CONTROL POINTS LISTED ABOVE HAVE BEEN DESTROYED.
REFERENCE POINTS ARE NOT TO BE USED FOR VERTICAL CONTROL

BASIS OF BEARING:
ARKANSAS STATE PLANE GRID BEARINGS - 0302-SOUTH ZONE
DETERMINED FROM GPS CONTROL POINTS: 620016 - 620016A, 620026 - 620026A
CONVERGENCE ANGLE: 00 00 08 28.30 LEFT AT LT: 34-54-82.76 LON: 092-25-22.73
GRID AZIMUTH = ASTRONOMICAL AZIMUTH - CONVERGENCE ANGLE.

BINGHAM ROAD INTERCHANGE RAMP 4

POINT NO.	TYPE	STATION	NORTHING	EASTING
8033	POB	387+56.35	2009353.6621	1231633.7455
8034	PC	388+13.10	2009410.4021	1231632.6685
8040	PI	391+25.03	2009722.2755	1231626.7483
8036	PT	394+27.37	2010003.4465	1231491.6820
8037	PI	399+28.95	2010455.5710	1231274.4947
8038	PI	401+26.67	2010633.7960	1231188.8807
8039	POE	401+45.38	2010650.6571	1231180.7811

I-530 C.L.

POINT NO.	TYPE	STATION	NORTHING	EASTING
8058	PI	369+99.93	2007599.1759	1231725.5577
8059	PI	399+99.93	2010598.6356	1231668.6256

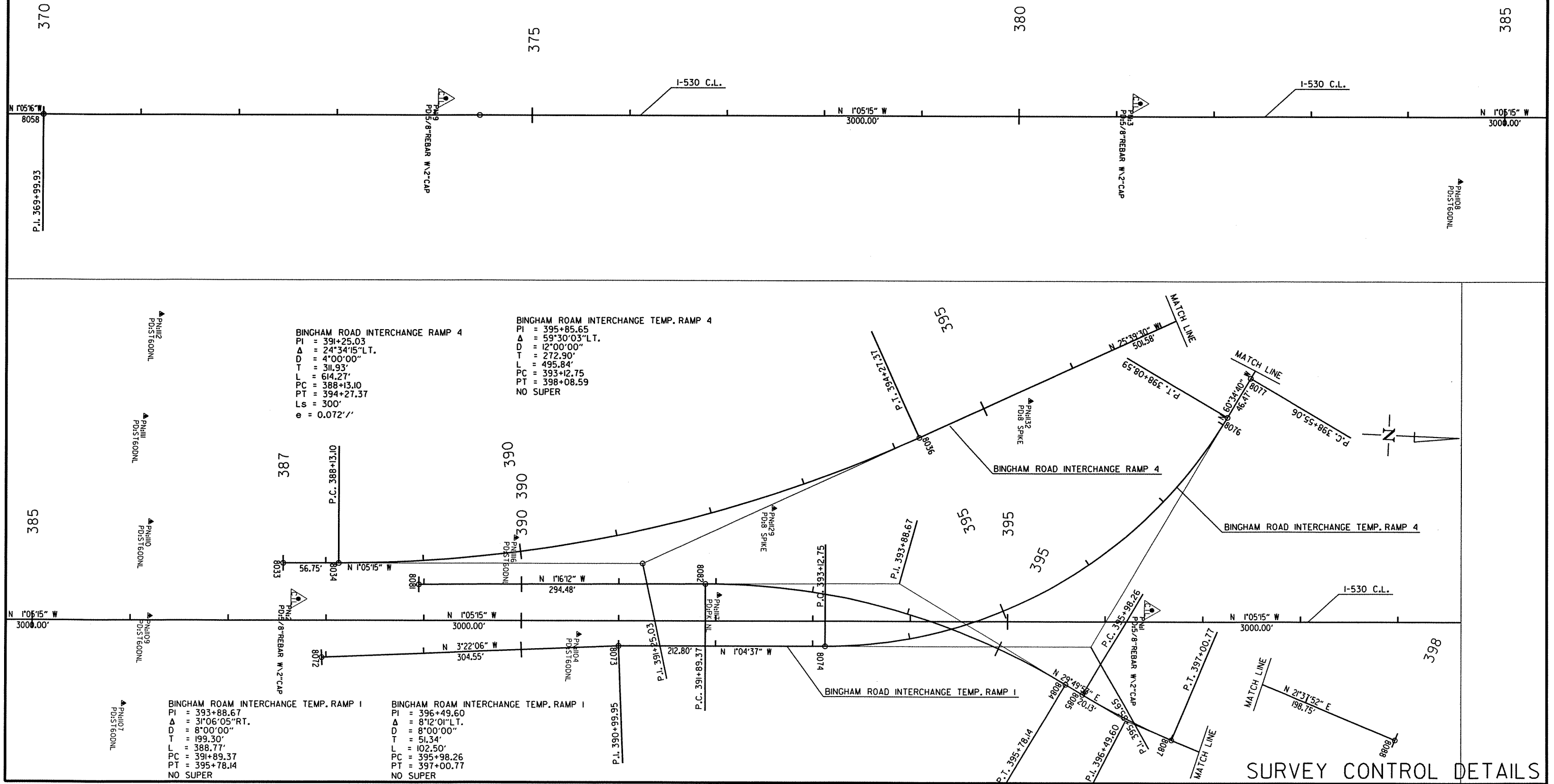
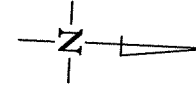
BINGHAM ROAD INTERCHANGE TEMP. RAMP 4

POINT NO.	TYPE	STATION	NORTHING	EASTING
8072	POB	387+95.40	2009395.2729	1231729.0354
8073	PI	390+99.95	2009699.2958	1231711.1411
8074	PC	393+12.75	2009912.0576	1231707.1415
8076	PT	398+08.59	2010318.9666	1231464.3116
8077	PC	398+55.06	2010341.7970	1231423.8309
8079	PT	401+46.48	2010551.3164	1231227.8159
8080	POE	401+65.82	2010568.7557	1231219.4575

BINGHAM ROAD INTERCHANGE TEMP. RAMP 1

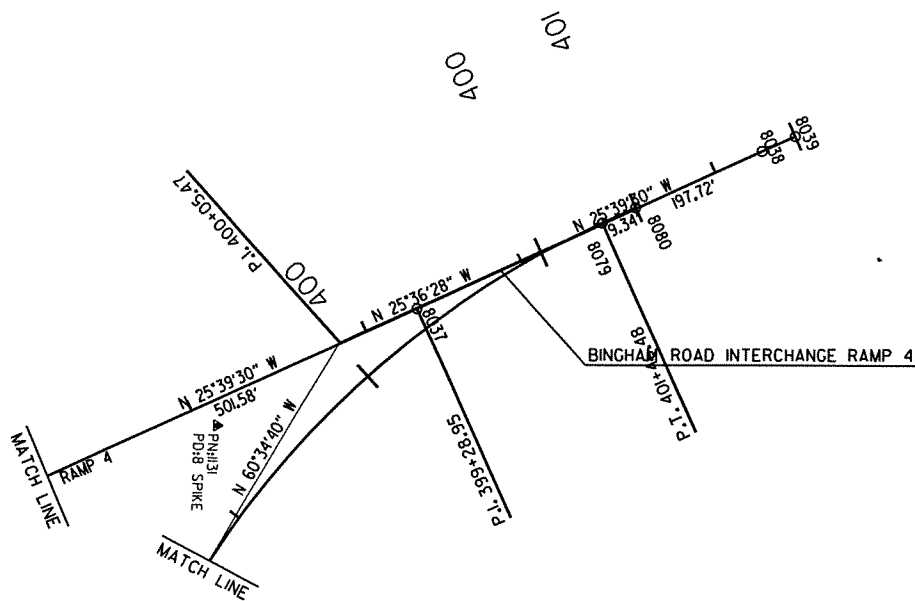
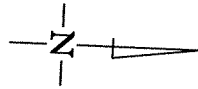
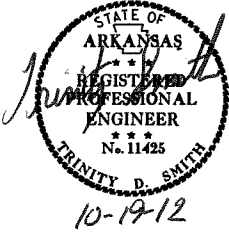
POINT NO.	TYPE	STATION	NORTHING	EASTING
8081	POB	388+94.89	2009493.0913	1231652.6608
8082	PC	391+89.37	2009787.4974	1231646.1338
8084	PT	395+78.14	2010159.6443	1231740.8596
8085	PC	395+98.26	2010177.1047	1231750.8720
8087	PT	397+00.77	2010269.3649	1231795.3360
8088	POE	398+99.52	2010454.1193	1231668.6020

2 SURVEY CONTROL DETAILS

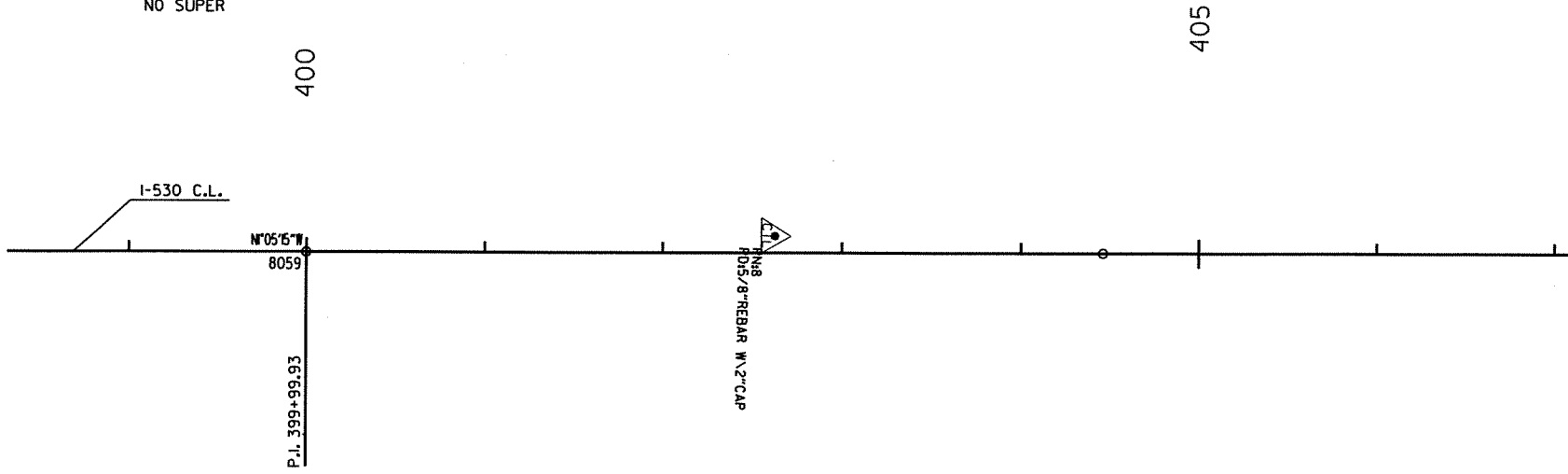


DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
						JOB NO. 880614	85	219

2 SURVEY CONTROL DETAILS



BINGHAM ROAD INTERCHANGE TEMP. RAMP 4
PI = 400+05.47
Δ = 34°58'12" RT.
D = 12°00'00"
T = 150.41'
L = 291.42'
PC = 398+55.06
PT = 401+46.48
NO SUPER



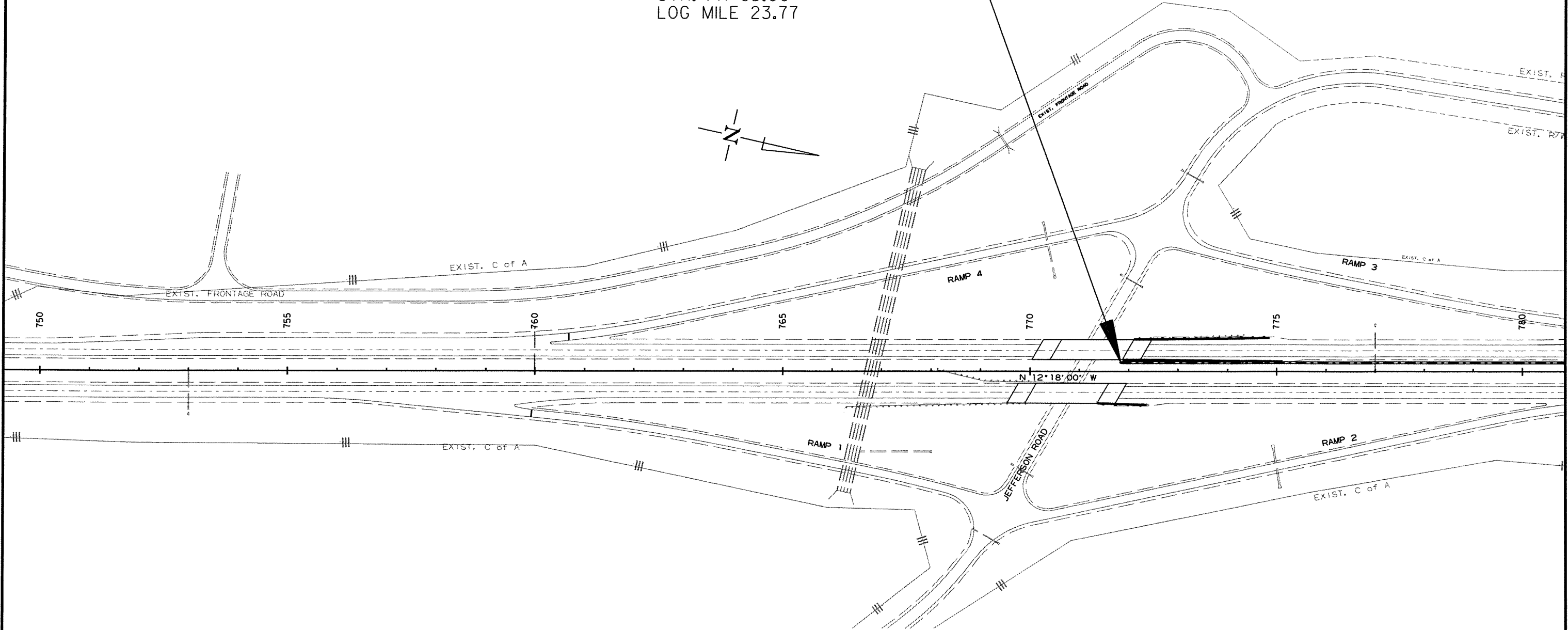
405

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB NO.		BB0614	86	219

2 PLAN SHEETS



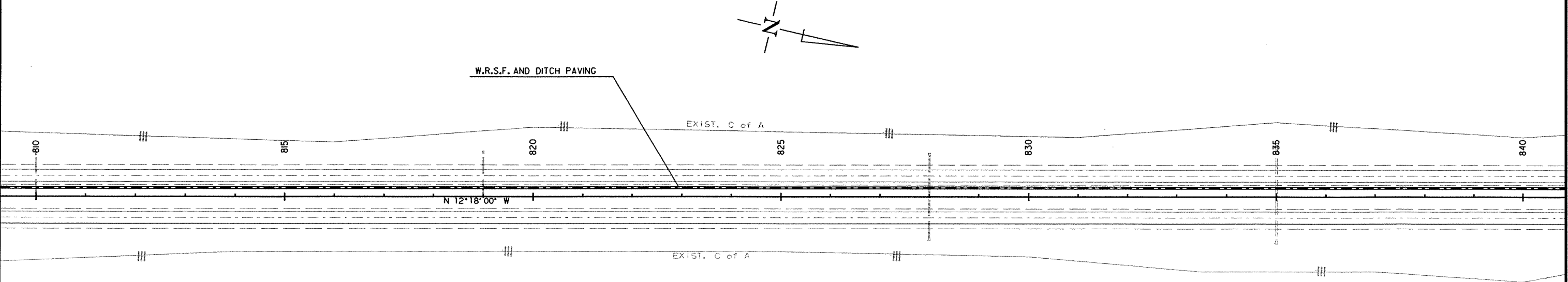
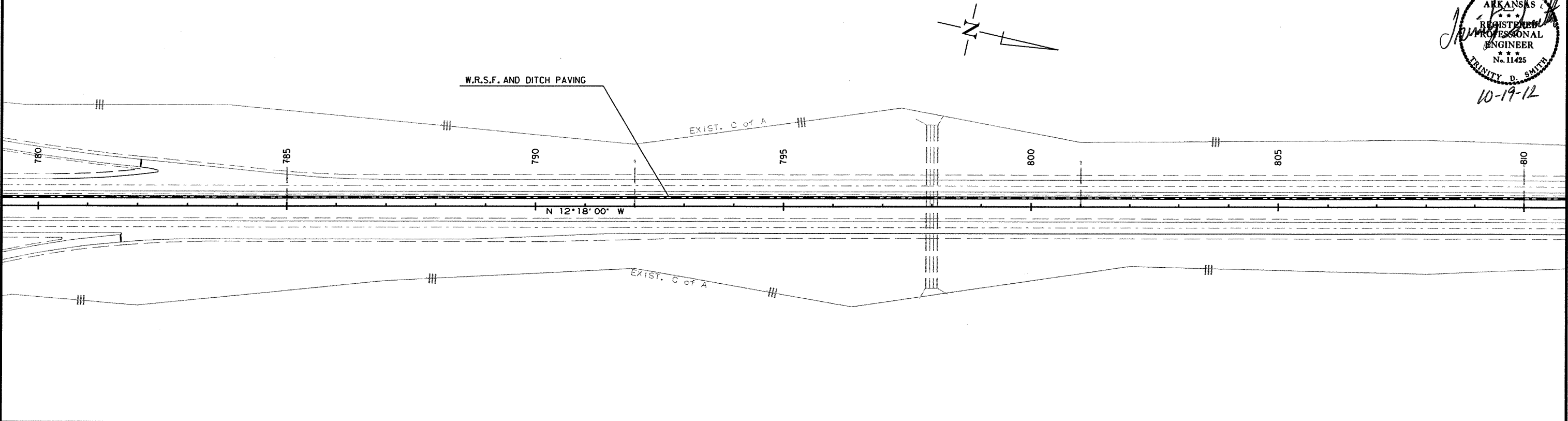
BEGIN W.R.S.F. AND DITCH PAVING
STA. 771+85.00
LOG MILE 23.77



EXISTING GUARDRAIL - RETAIN				
STA.	STA.	LOCATION	SIDE	LIN. FT.
771+36.33	772+36.33	RT. MAIN LANES	RT.	75
771+87.06	775+12.06	LT. MAIN LANES	RT.	250
772+10.16	774+85.16	LT. MAIN LANES	LT.	200

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB NO. BB0614			87	219

2 PLAN SHEETS

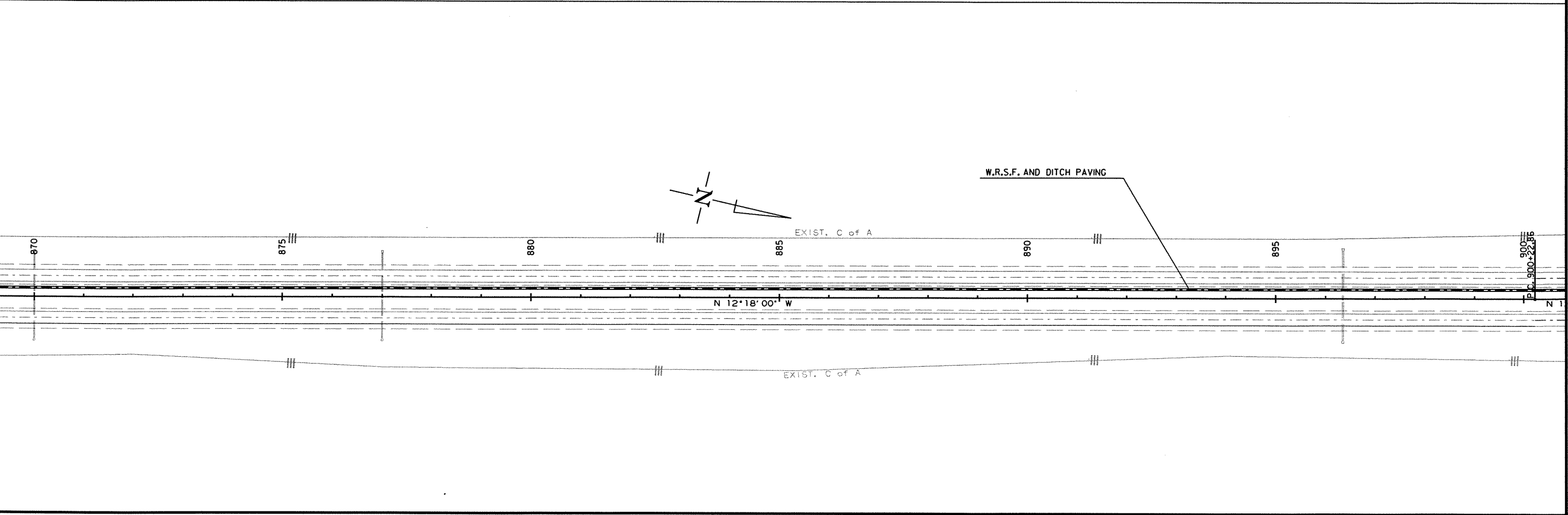
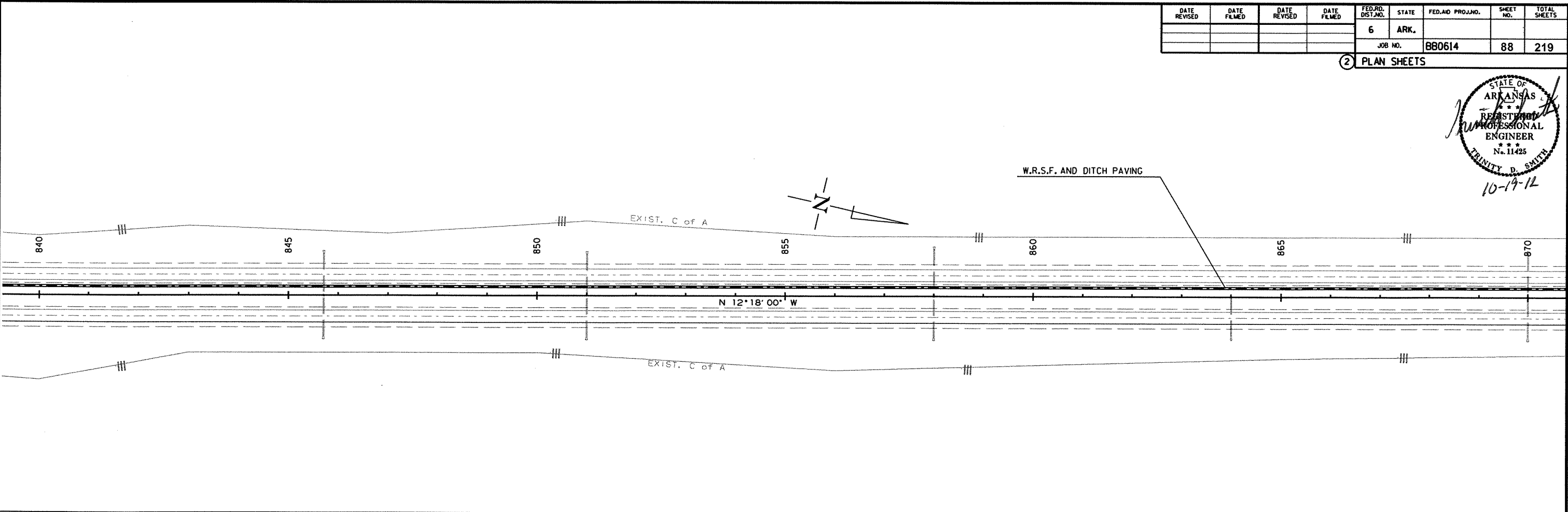
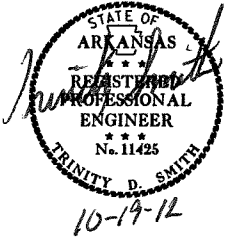


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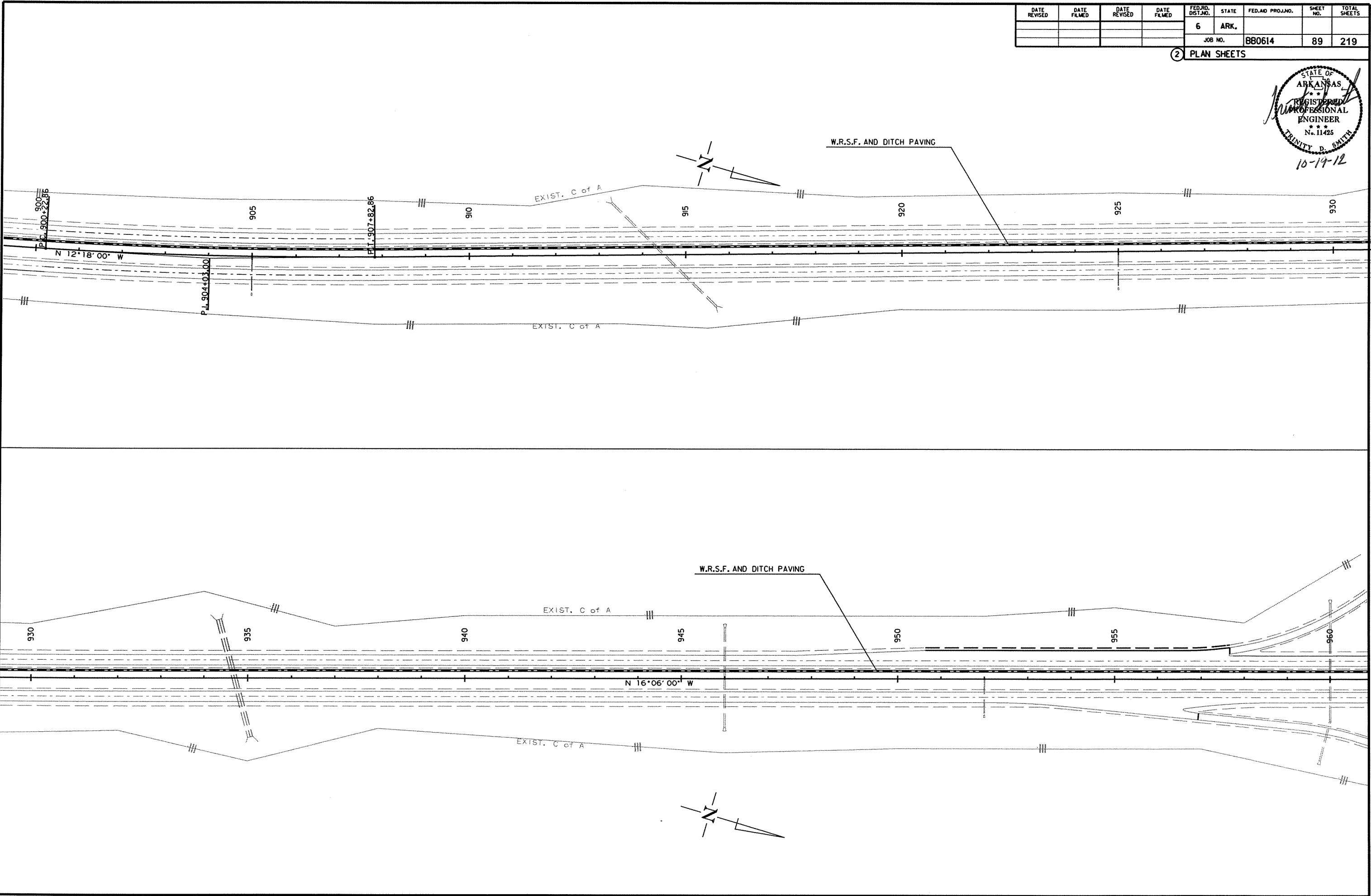
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				6	ARK.			
				JOB NO. 880614		88	219	

2 PLAN SHEETS



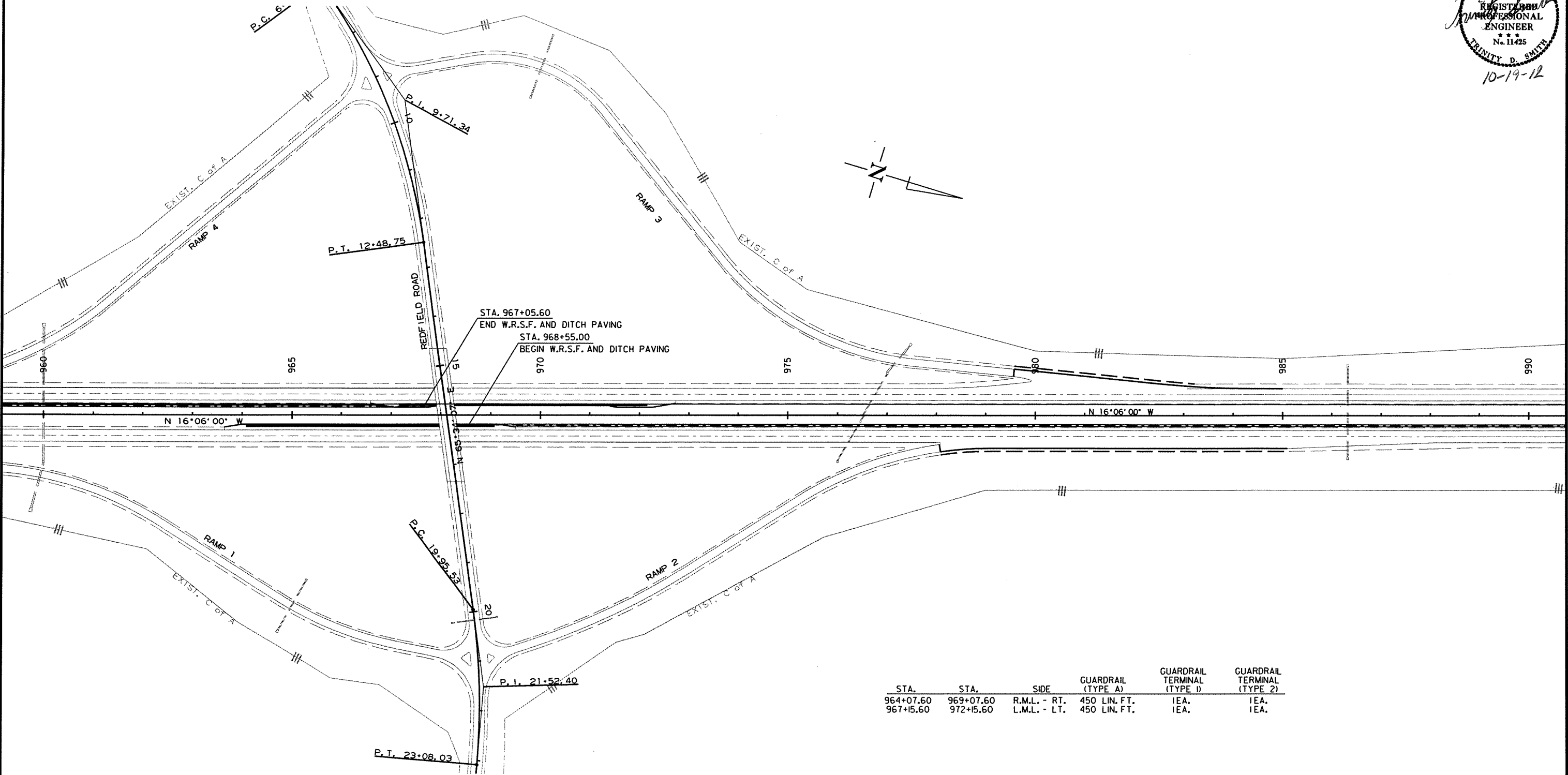
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				6	ARK.			
				JOB NO. 880614			89	219

2 PLAN SHEETS



DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB NO.		BB0614	90	219

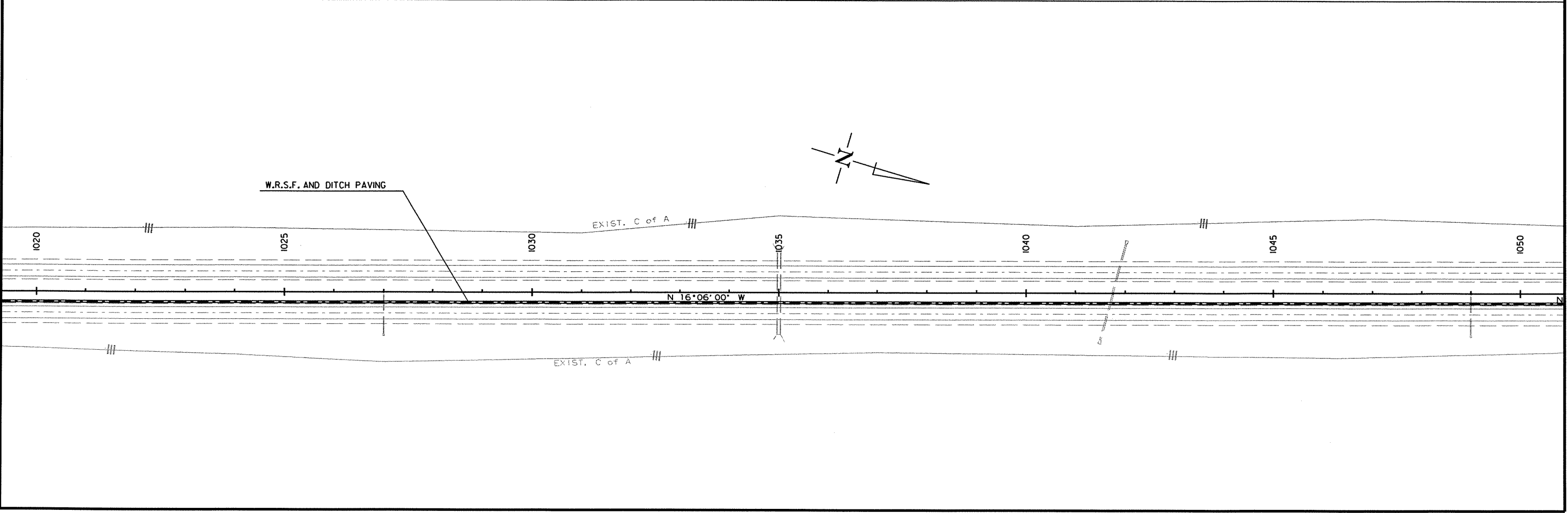
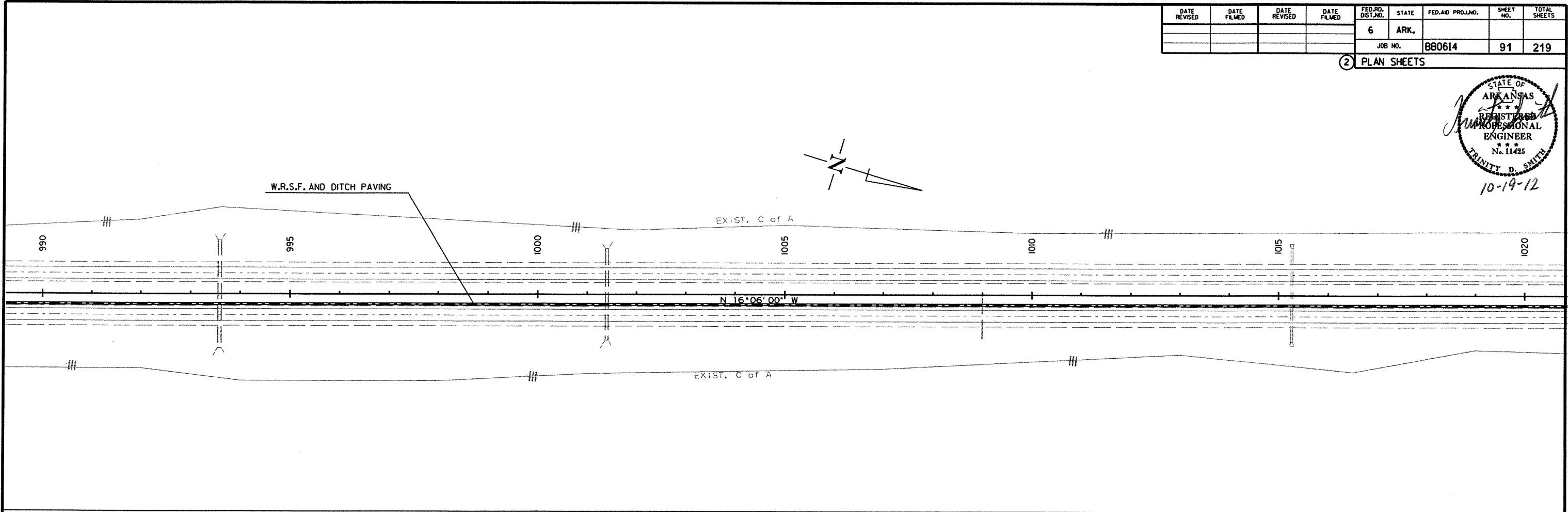
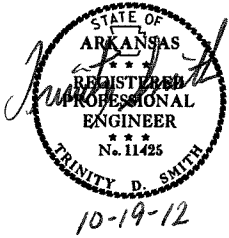
2 PLAN SHEETS



STA.	STA.	SIDE	GUARDRAIL (TYPE A)	GUARDRAIL TERMINAL (TYPE 1)	GUARDRAIL TERMINAL (TYPE 2)
964+07.60	969+07.60	R.M.L. - RT.	450 LIN. FT.	1EA.	1EA.
967+15.60	972+15.60	L.M.L. - LT.	450 LIN. FT.	1EA.	1EA.

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB NO.		BB0614	91	219

2 PLAN SHEETS

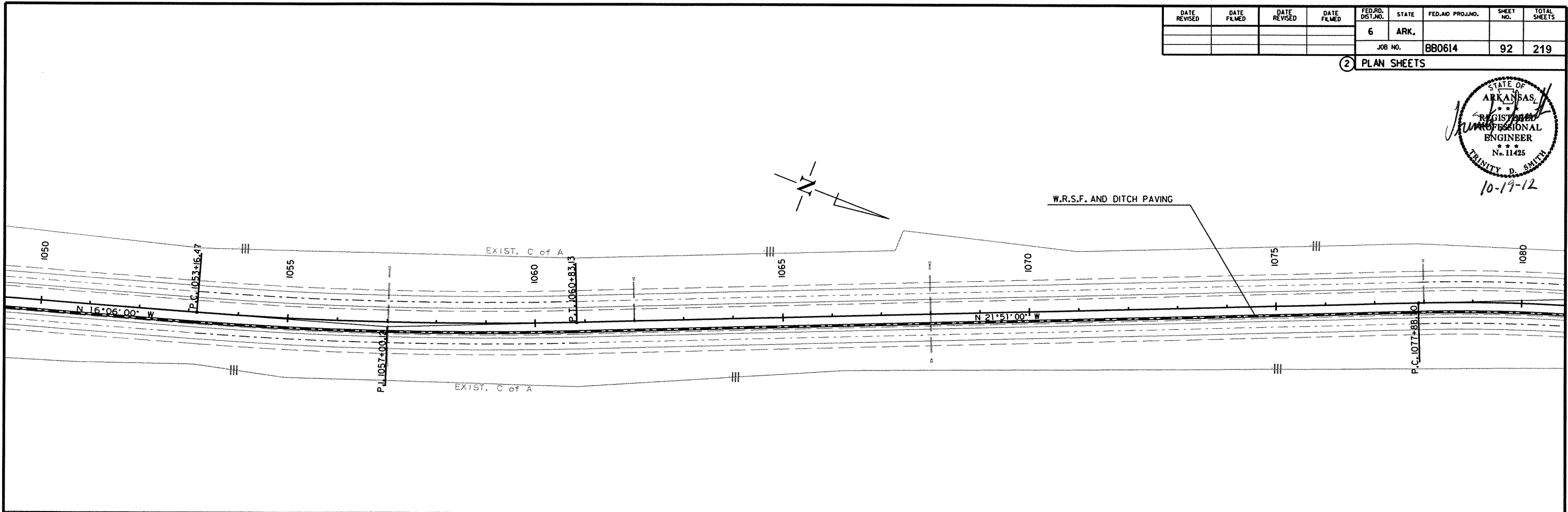


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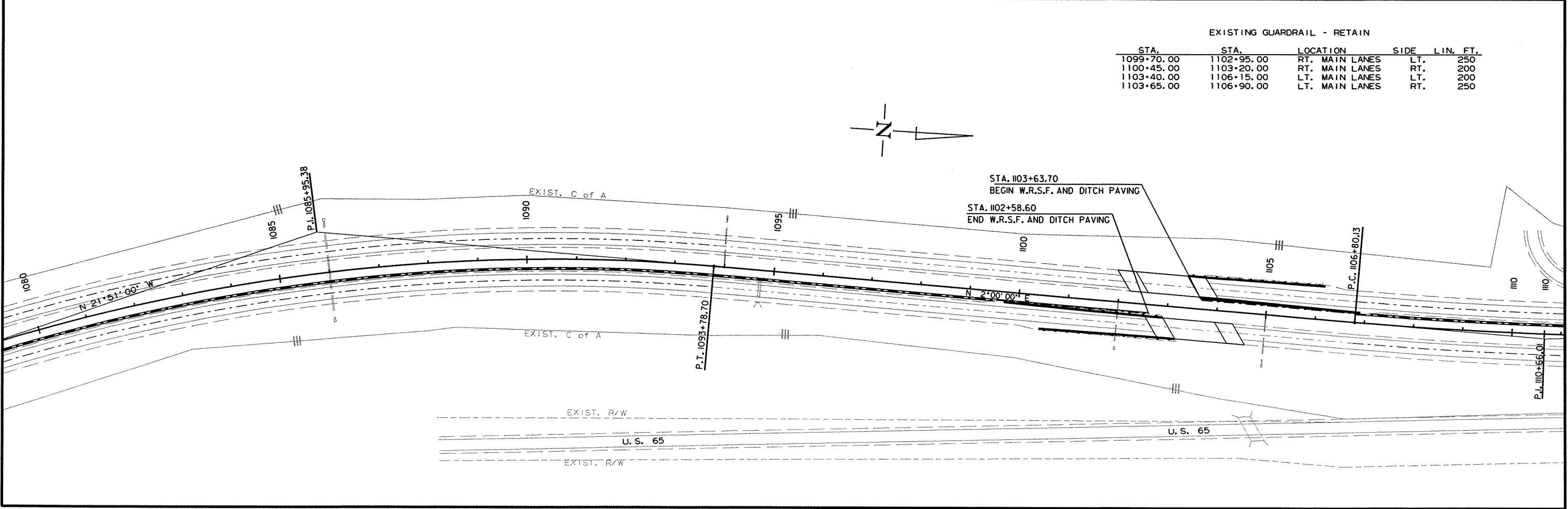
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DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB NO.		BB0614	92	219

2 PLAN SHEETS



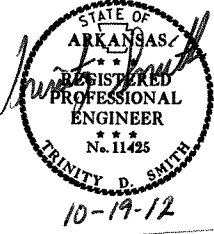
EXISTING GUARDRAIL - RETAIN				
STA.	STA.	LOCATION	SIDE	LIN. FT.
1099+70.00	1102+95.00	RT. MAIN LANES	LT.	250
1100+45.00	1103+20.00	RT. MAIN LANES	RT.	200
1103+40.00	1106+15.00	LT. MAIN LANES	LT.	200
1103+65.00	1106+90.00	LT. MAIN LANES	RT.	250



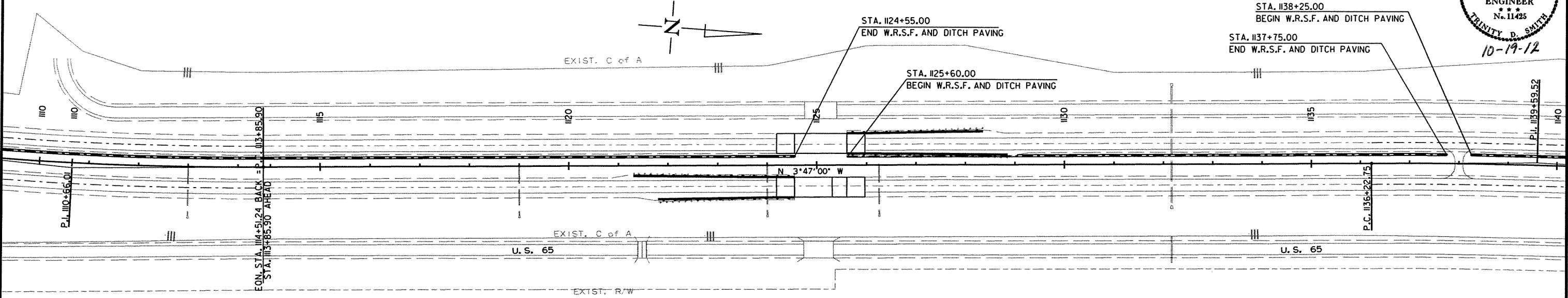
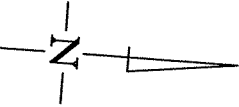
10/19/2012
RB0614.DGN

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB NO.		880614	93	219

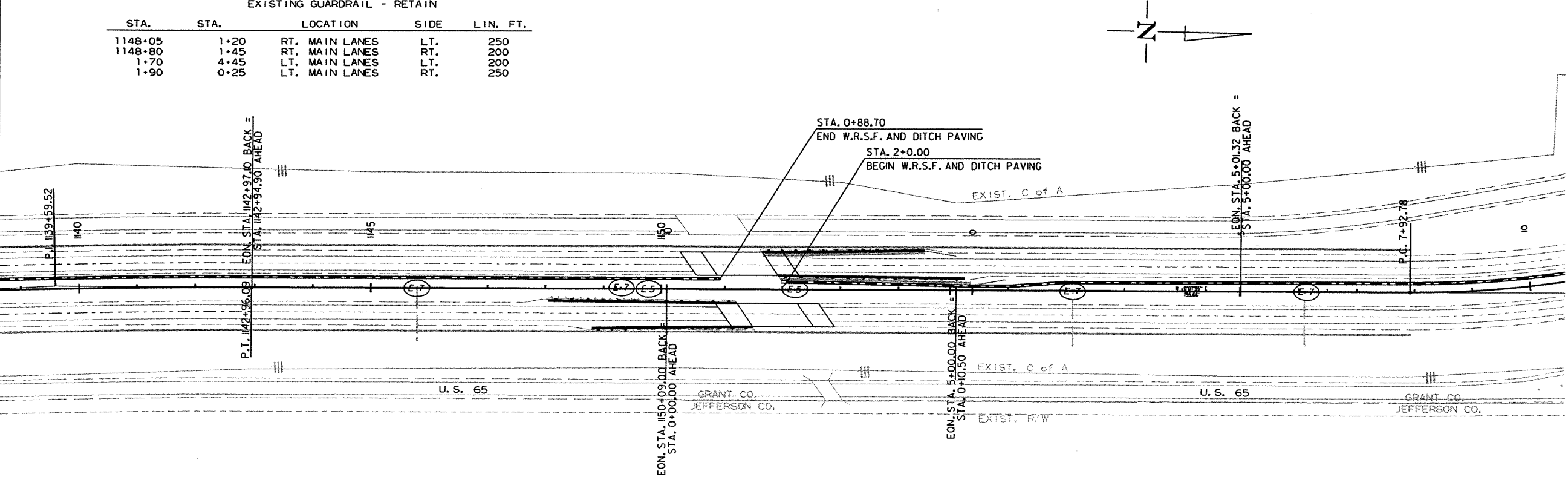
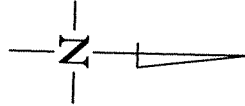
2 PLAN SHEETS



EXISTING GUARDRAIL - RETAIN				
STA.	STA.	LOCATION	SIDE	LIN. FT.
1121+80	1124+55	RT. MAIN LANES	RT.	200
1121+30	1124+55	RT. MAIN LANES	LT.	250
1125+60	1128+35	LT. MAIN LANES	LT.	250
1125+60	1128+85	LT. MAIN LANES	RT.	200



EXISTING GUARDRAIL - RETAIN				
STA.	STA.	LOCATION	SIDE	LIN. FT.
1148+05	1+20	RT. MAIN LANES	LT.	250
1148+80	1+45	RT. MAIN LANES	RT.	200
1+70	4+45	LT. MAIN LANES	LT.	200
1+90	0+25	LT. MAIN LANES	RT.	250

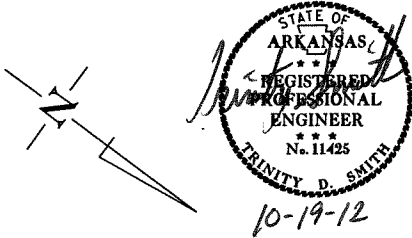


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DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB NO.		BB0614	94	219

2 PLAN SHEETS

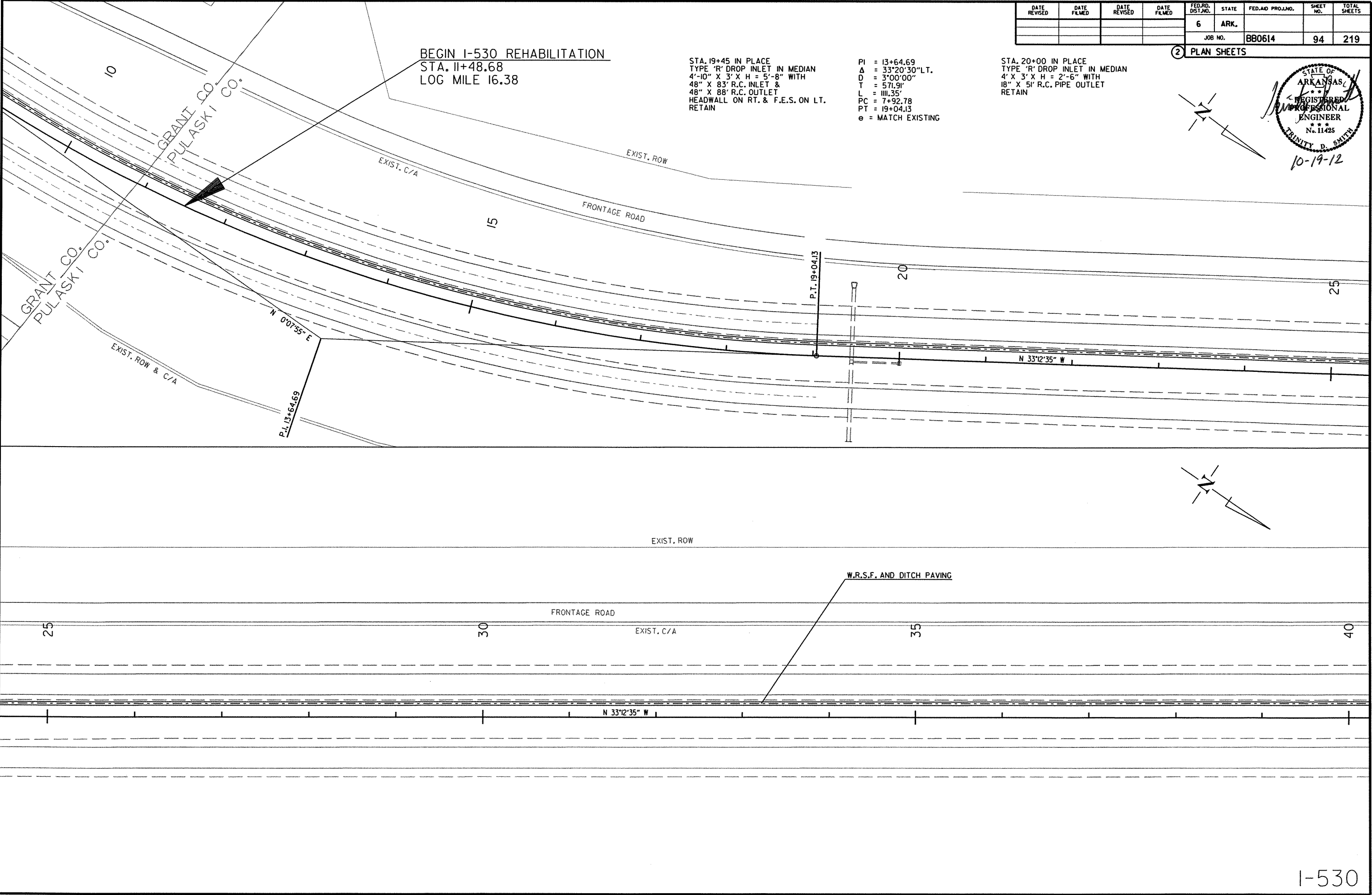


BEGIN I-530 REHABILITATION
STA. 11+48.68
LOG MILE 16.38

STA. 19+45 IN PLACE
TYPE 'R' DROP INLET IN MEDIAN
4'-10" X 3' X H = 5'-8" WITH
48" X 83' R.C. INLET &
48" X 88' R.C. OUTLET
HEADWALL ON RT. & F.E.S. ON LT.
RETAIN

PI = 13+64.69
Δ = 33°20'30" LT.
D = 3°00'00"
T = 571.91'
L = 1111.35'
PC = 7+92.78
PT = 19+04.13
e = MATCH EXISTING

STA. 20+00 IN PLACE
TYPE 'R' DROP INLET IN MEDIAN
4' X 3' X H = 2'-6" WITH
18" X 51' R.C. PIPE OUTLET
RETAIN



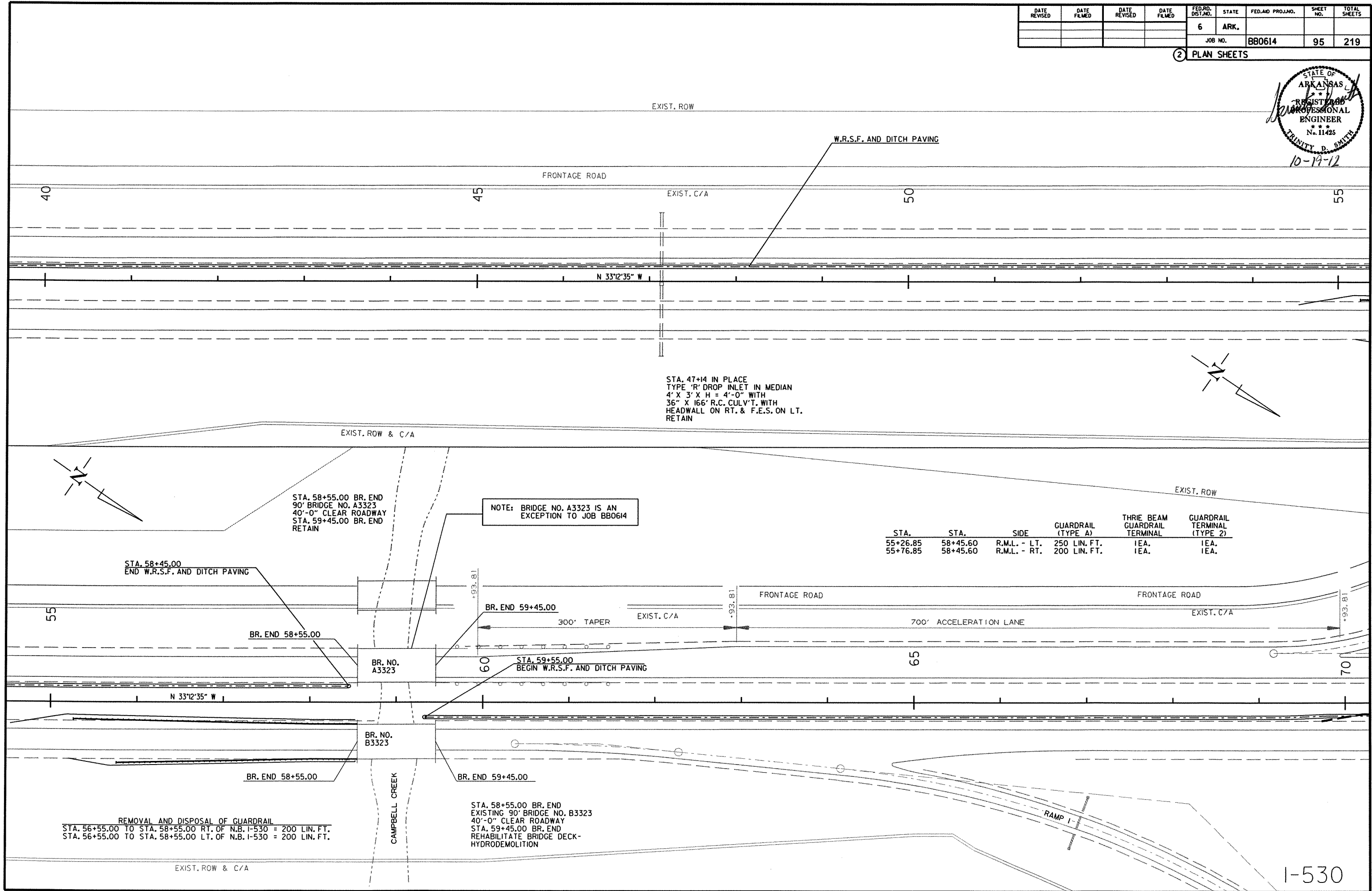
10/19/2012

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I-530

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB NO.		BB0614	95	219

2 PLAN SHEETS

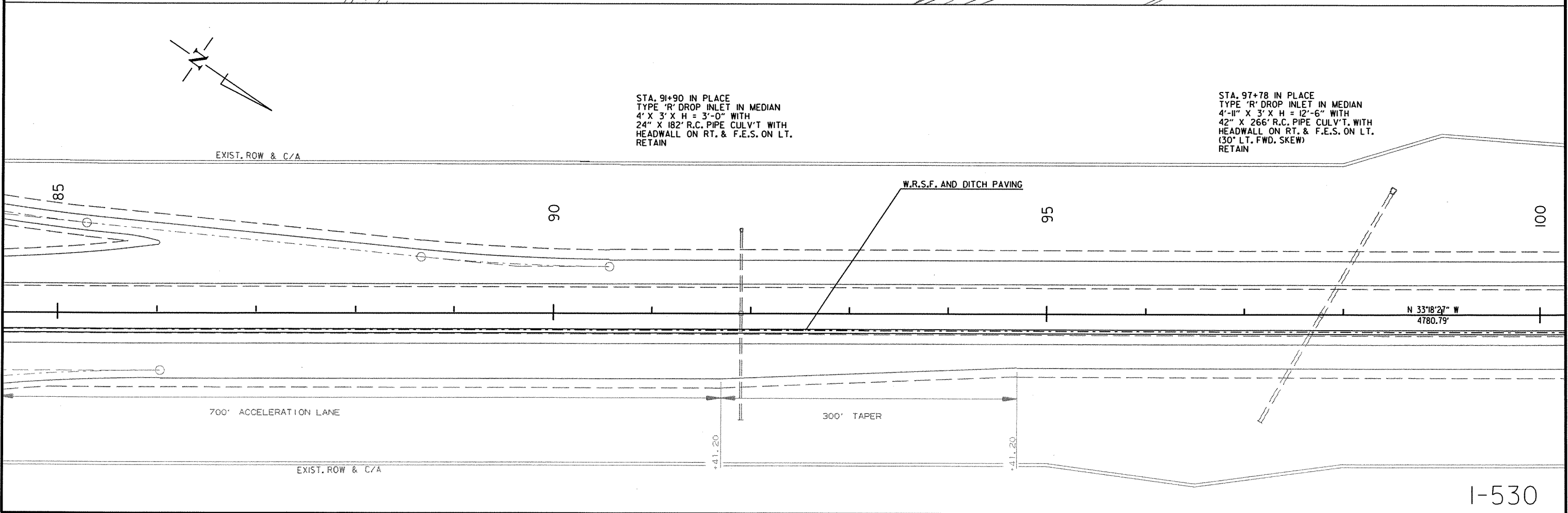
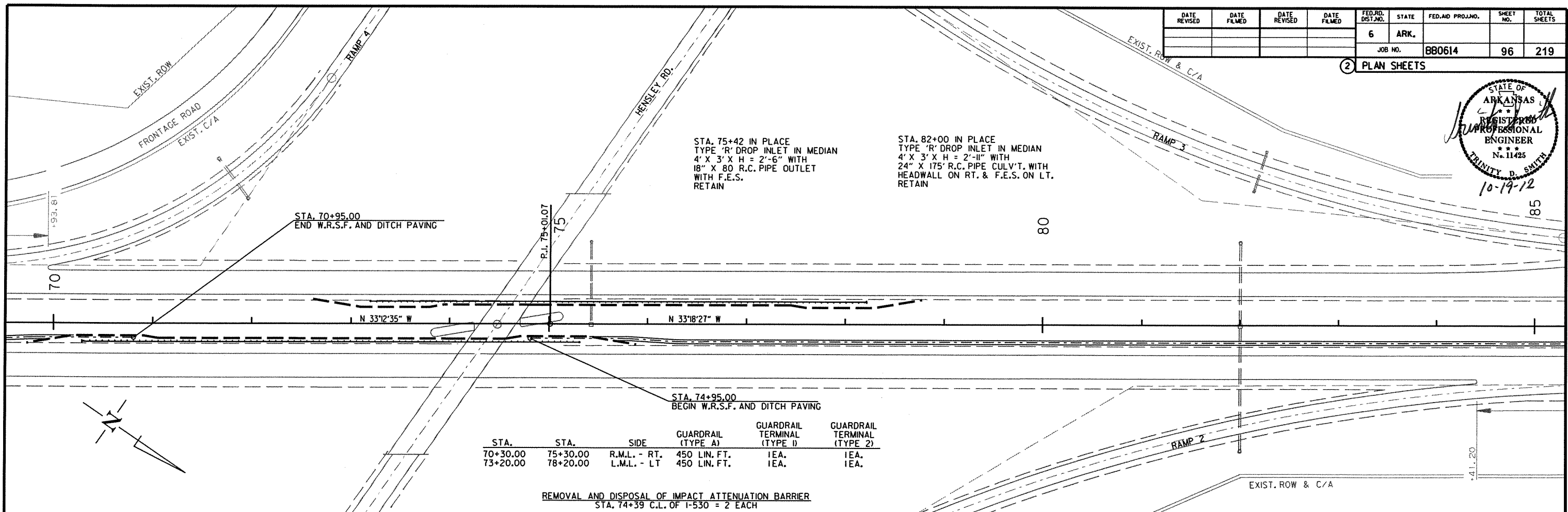
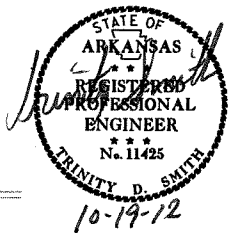


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DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB NO.		BB0614	96	219

2 PLAN SHEETS



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RBB0614.DGN

I-530

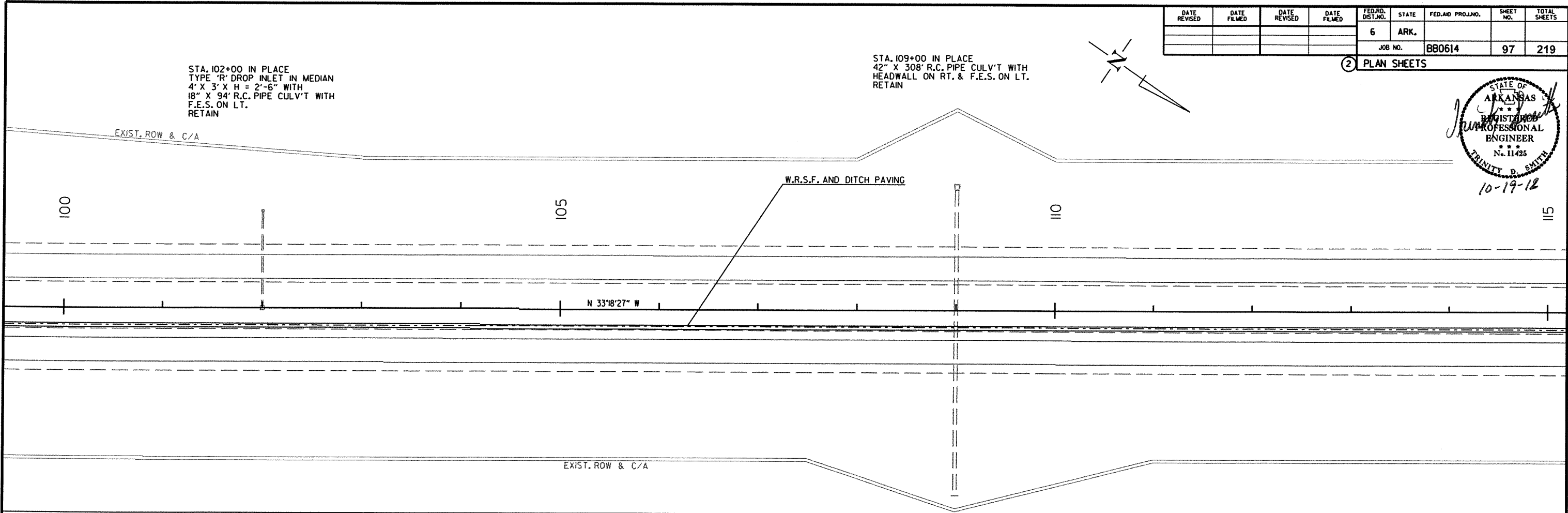
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				6	ARK.			
				JOB NO. 880614		97	219	

2 PLAN SHEETS

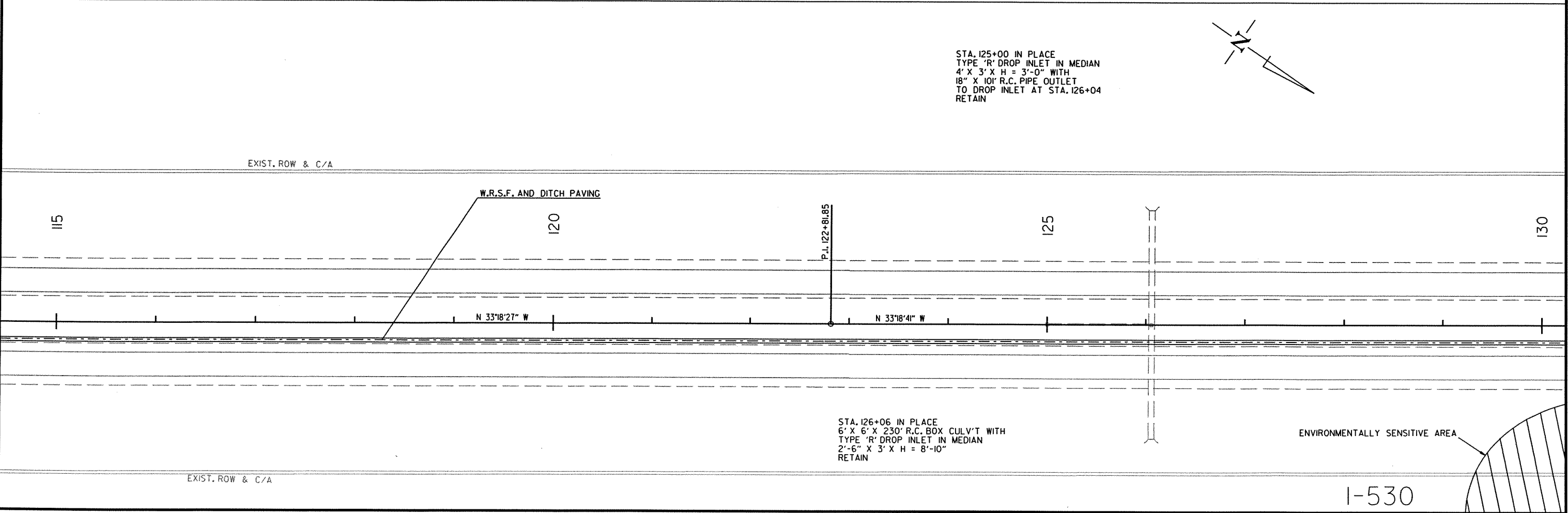


STA. 102+00 IN PLACE
TYPE 'R' DROP INLET IN MEDIAN
4' X 3' X H = 2'-6" WITH
18" X 94' R.C. PIPE CULV'T WITH
F.E.S. ON LT.
RETAIN

STA. 109+00 IN PLACE
42" X 308' R.C. PIPE CULV'T WITH
HEADWALL ON RT. & F.E.S. ON LT.
RETAIN



STA. 125+00 IN PLACE
TYPE 'R' DROP INLET IN MEDIAN
4' X 3' X H = 3'-0" WITH
18" X 101' R.C. PIPE OUTLET
TO DROP INLET AT STA. 126+04
RETAIN



ENVIRONMENTALLY SENSITIVE AREA

I-530

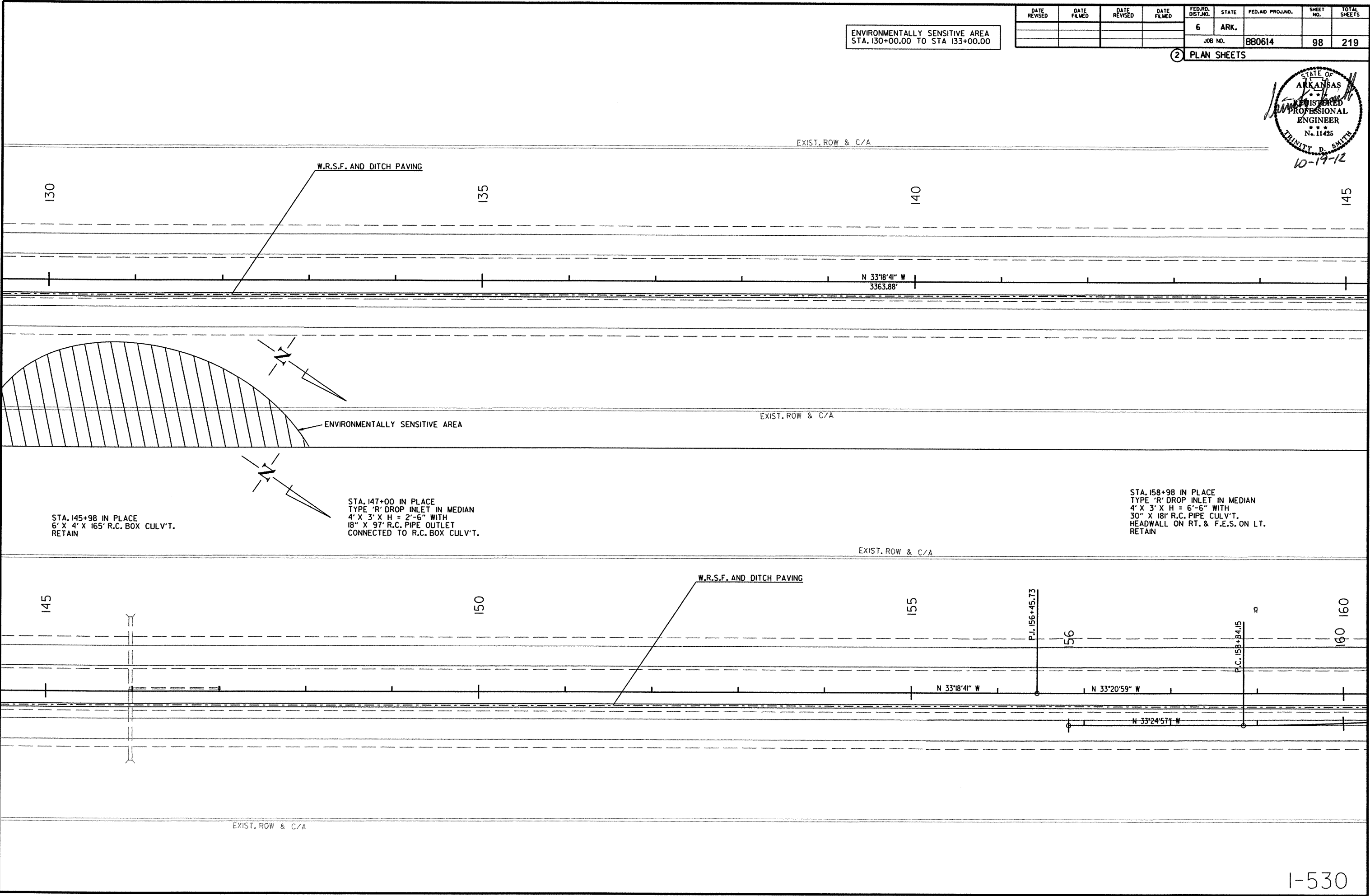
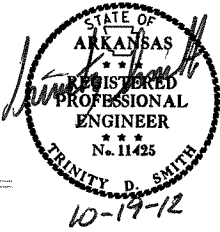
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ENVIRONMENTALLY SENSITIVE AREA
STA. 130+00.00 TO STA 133+00.00

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB NO.		880614	98	219

2 PLAN SHEETS



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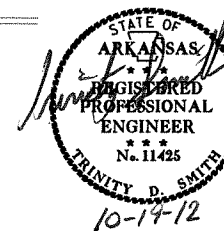
I-530 CROSSOVER
PI = 160+74.74
Δ = 7°36'44" T.
D = 2°00'00"
T = 190.59'
L = 380.62'
PC = 158+84.15
PT = 162+64.77
NO SUPER

I-530 CROSSOVER
PI = 166+04.97
Δ = 7°36'40" RT.
D = 2°00'00"
T = 190.56'
L = 380.56'
PC = 164+14.41
PT = 167+94.97
NO SUPER

SPECIAL FLOOD HAZARD LIMITS
STA. 168+80 TO STA. 177+00

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB NO.	BB0614		99	219

2 PLAN SHEETS



EXIST. ROW & C/A

STA. 169+00 IN PLACE
TYPE 'R' DROP INLET IN MEDIAN
4' X 3' X H = 9'-0" WITH
18" X 110' R.C. PIPE CULV'T.
WITH F.E.S. ON LT.
RETAIN

REMOVAL AND DISPOSAL OF GUARDRAIL
STA. 173+15.00 TO STA. 175+15.00 RT. OF N.B. I-530 = 200 LIN. FT.
STA. 173+15.00 TO STA. 175+15.00 LT. OF N.B. I-530 = 200 LIN. FT.

STA.	STA.	SIDE	GUARDRAIL (TYPE A)	THREE BEAM GUARDRAIL TERMINAL	GUARDRAIL TERMINAL (TYPE 2)
171+77.82	174+96.57	R.M.L. - LT.	250 LIN. FT.	IEA.	IEA.
172+27.82	174+96.57	R.M.L. - RT.	200 LIN. FT.	IEA.	IEA.

NOTE: BRIDGE NO. A3324 IS AN
EXCEPTION TO JOB BB0614

EXIST. ROW & C/A

I-530 CROSSOVER
PI = 181+72.31
Δ = 8°05'39" RT.
D = 2°00'00"
T = 202.69'
L = 404.71'
PC = 179+69.62
PT = 183+74.33
NO SUPER

I-530 CROSSOVER
PI = 186+95.03
Δ = 7°54'04" LT.
D = 2°00'00"
T = 197.84'
L = 395.05'
PC = 184+97.19
PT = 188+92.25
NO SUPER

STA. 175+16.67 BR. END
150.00' BRIDGE NO. A3324
40'-0" CLEAR ROADWAY
STA. 176+66.67 BR. END
RETAIN

BM: 5 5/8" REBAR W/ 2" CAP
4.21' LT. OF STA. 176+60.70
ELEV. 240.52

STA. 176+67.00
BEGIN W.R.S.F. AND DITCH PAVING

I-530 N.B. CONSTRUCT
STA. 175+05.97 BR. END
150.00' CONTINUOUS R.C.
SLAB UNIT (30.75' -
3 @ 30' - 30.75')
40'-0" CLEAR ROADWAY
BRIDGE NO. B7266
STA. 176+57.47 BR. END

EXIST. ROW & C/A

STA. 175+16.67 TO STA. 176+66.67 - IN PLACE
33'-6" X 150' BRIDGE CONSISTING OF REINFORCED CONCRETE
SLAB SPAN SUPERSTRUCTURES AND PRECAST CONCRETE PILE SUBSTRUCTURES.
REMOVE AS EXISTING BRIDGE STRUCTURE (SITE NO. 2) = 1.00 LUMP SUM

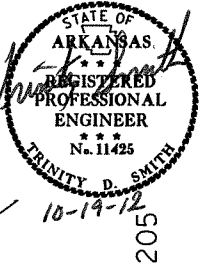
NOTE:
FOR CONSTRUCTION OF TEMPORARY WORK RAMPS
OR HAUL ROADS, THIS STREAM IS CLASSIFIED AS AN
INTERMITTENT STREAM. THE STREAM BANK ELEVATION IS
230 FT. MSL. BETWEEN STATIONS 175+40 AND 176+50. REFER TO
SECTION 110.06(C) OF THE 2003 STANDARD SPECIFICATIONS.

I-530

ENVIRONMENTALLY SENSITIVE AREA
STA. 203+00.00 TO STA 205+00.00

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB NO.		BB0614	100	219

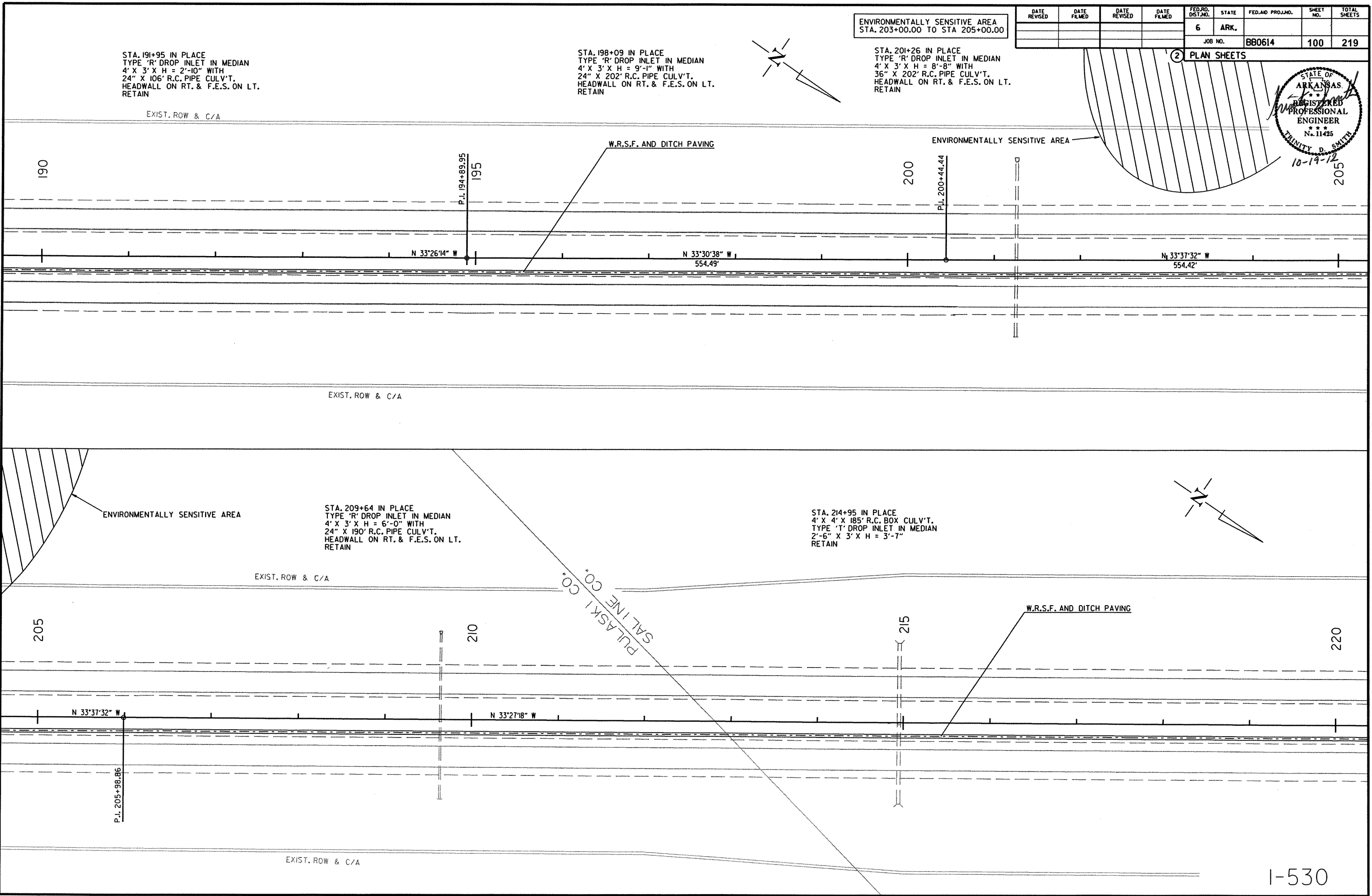
2 PLAN SHEETS



STA. 191+95 IN PLACE
TYPE 'R' DROP INLET IN MEDIAN
4' X 3' X H = 2'-10" WITH
24" X 106' R.C. PIPE CULV'T.
HEADWALL ON RT. & F.E.S. ON LT.
RETAIN

STA. 198+09 IN PLACE
TYPE 'R' DROP INLET IN MEDIAN
4' X 3' X H = 9'-1" WITH
24" X 202' R.C. PIPE CULV'T.
HEADWALL ON RT. & F.E.S. ON LT.
RETAIN

STA. 201+26 IN PLACE
TYPE 'R' DROP INLET IN MEDIAN
4' X 3' X H = 8'-8" WITH
36" X 202' R.C. PIPE CULV'T.
HEADWALL ON RT. & F.E.S. ON LT.
RETAIN



10/19/2012

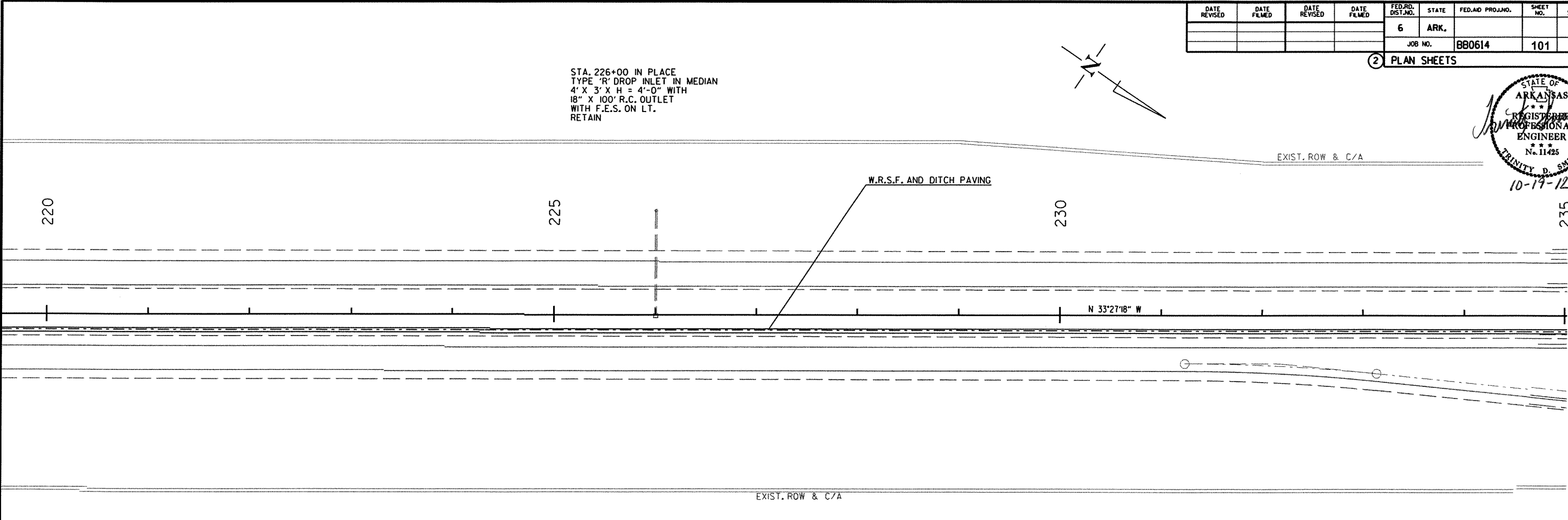
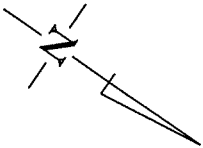
RB0614.DGN

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB NO.		BB0614	101	219

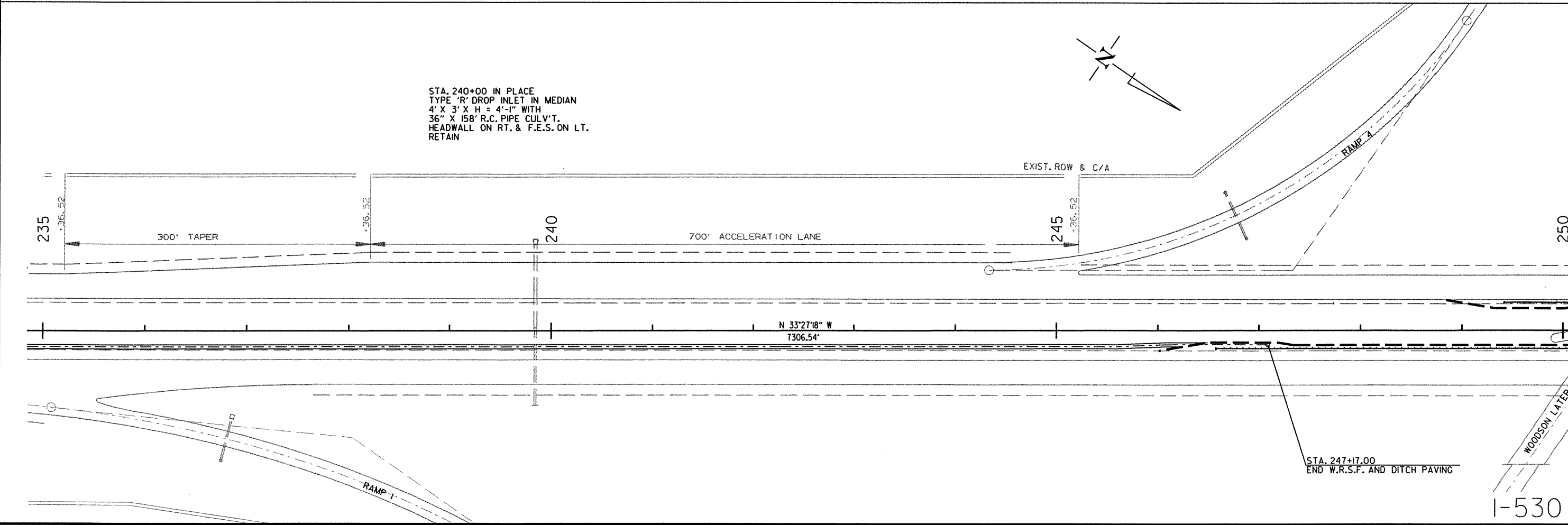
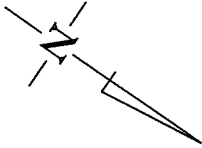
2 PLAN SHEETS



STA. 226+00 IN PLACE
TYPE 'R' DROP INLET IN MEDIAN
4' X 3' X H = 4'-0" WITH
18" X 100' R.C. OUTLET
WITH F.E.S. ON LT.
RETAIN



STA. 240+00 IN PLACE
TYPE 'R' DROP INLET IN MEDIAN
4' X 3' X H = 4'-1" WITH
36" X 158' R.C. PIPE CULV'T.
HEADWALL ON RT. & F.E.S. ON LT.
RETAIN



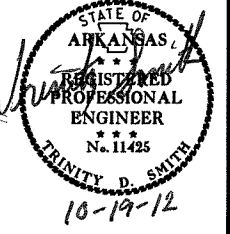
10/19/2012

RB0614.DGN

I-530

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB NO.		BB0614	102	219

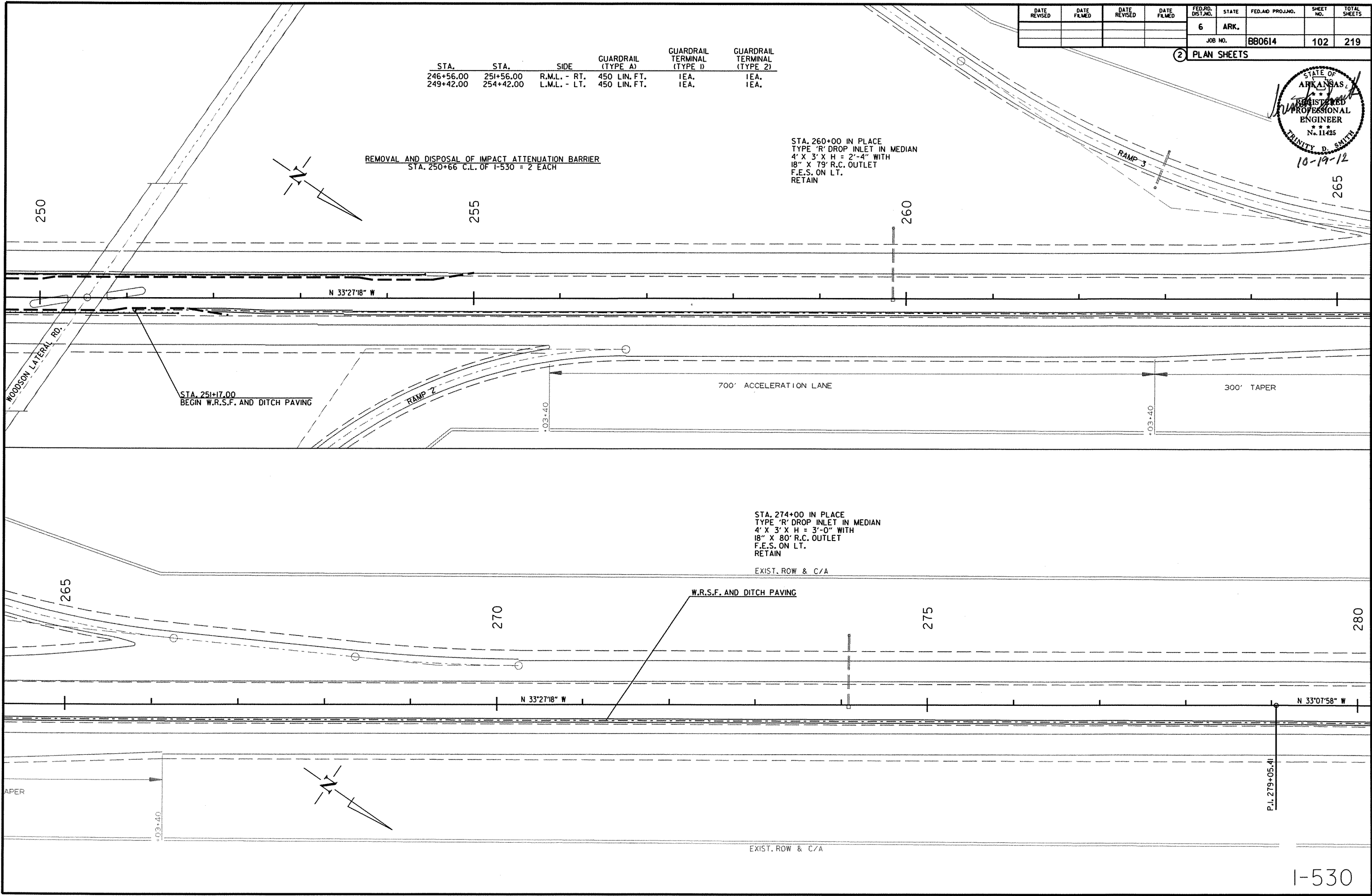
2 PLAN SHEETS



STA.	STA.	SIDE	GUARDRAIL (TYPE A)	GUARDRAIL TERMINAL (TYPE 1)	GUARDRAIL TERMINAL (TYPE 2)
246+56.00	251+56.00	R.M.L. - RT.	450 LIN. FT.	1EA.	1EA.
249+42.00	254+42.00	L.M.L. - LT.	450 LIN. FT.	1EA.	1EA.

REMOVAL AND DISPOSAL OF IMPACT ATTENUATION BARRIER
STA. 250+66 C.L. OF I-530 = 2 EACH

STA. 260+00 IN PLACE
TYPE 'R' DROP INLET IN MEDIAN
4' X 3' X H = 2'-4" WITH
18" X 79' R.C. OUTLET
F.E.S. ON LT.
RETAIN



10/19/2012

RB0614.DGN

I-530

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB NO.	BB0614		103	219

2 PLAN SHEETS



10-19-12

NOTE: BRIDGE NO. A3326A IS AN
EXCEPTION TO JOB BB0614

STA.	STA.	SIDE	GUARDRAIL (TYPE A)	THREE BEAM GUARDRAIL TERMINAL	GUARDRAIL TERMINAL (TYPE 2)
282+41.80	285+60.55	R.M.L. - LT.	250 LIN. FT.	IEA.	IEA.
282+91.80	285+60.55	R.M.L. - RT.	200 LIN. FT.	IEA.	IEA.

EXIST. ROW & C/A

REMOVAL AND DISPOSAL OF GUARDRAIL
STA. 283+70.00 TO STA. 285+70.00 RT. OF N.B. I-530 = 200 LIN. FT.
STA. 283+70.00 TO STA. 285+70.00 LT. OF N.B. I-530 = 200 LIN. FT.

BR. END 285+66.25

BR. END 286+56.25

BR. NO.
A3326

BR. NO.
B3326

STA. 285+69.95 BR. END
90' BRIDGE NO. B3326
40'-0" CLEAR ROADWAY
STA. 286+59.95 BR. END
REHABILITATE BRIDGE DECK-
HYDRODEMOLITION

BR. END 285+69.95

STA. 285+60.00
END W.R.S.F. AND DITCH PAVING

STA. 286+70.00
BEGIN W.R.S.F. AND DITCH PAVING

EXIST. ROW & C/A

MAPLE CREEK

STA. 303+95 IN PLACE
TYPE 'R' DROP INLET IN MEDIAN
4' X 3' X H = 4'-6" WITH
24" X 176' R.C. PIPE CULV'T.
HEADWALL ON RT. & F.E.S. ON LT.
RETAIN

EXIST. ROW & C/A

STA. 294+94 IN PLACE
TYPE 'R' DROP INLET IN MEDIAN
4' X 3' X H = 3'-2" WITH
24" X 165' R.C. PIPE CULV'T.
HEADWALL ON RT. & F.E.S. ON LT.
RETAIN

W.R.S.F. AND DITCH PAVING

N 33°07'58" W
3485.83'

EXIST. ROW & C/A

I-530

10/19/2012

RBB0614.DGN

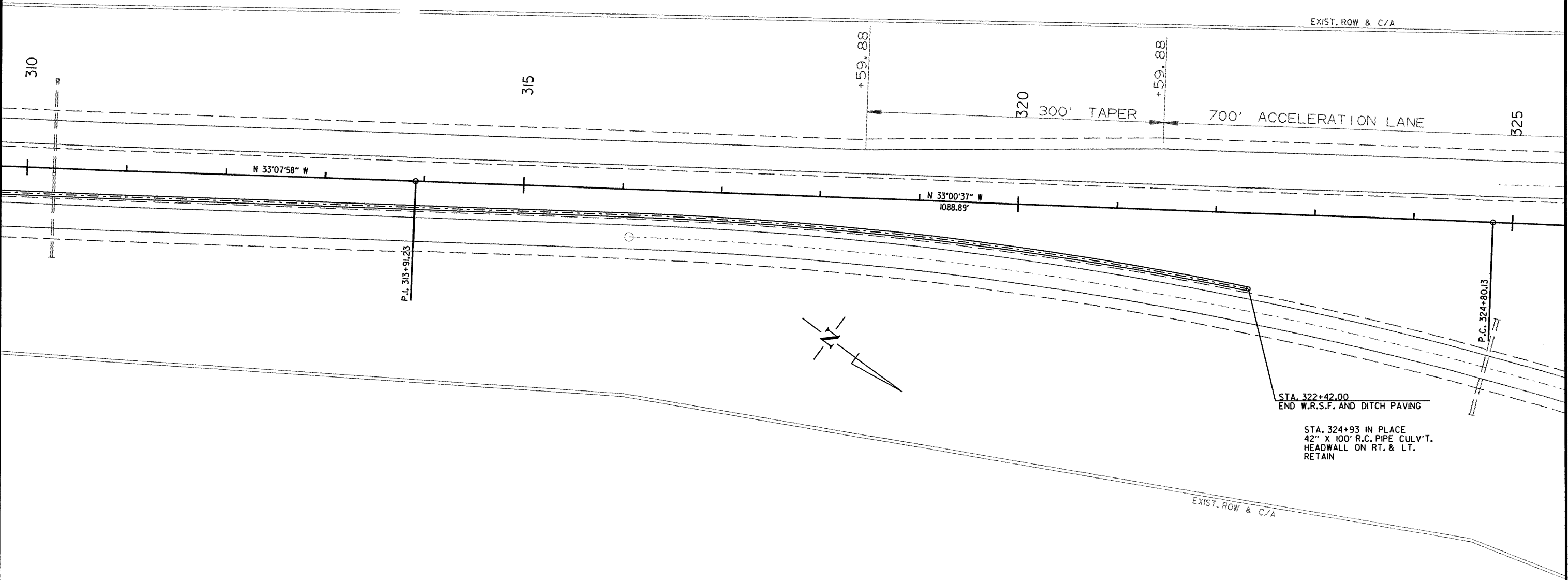
DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
						JOB NO. 880614	104	219

2 PLAN SHEETS



PI = 335+75.37
Δ = 31°59'56" RT.
D = 1°30'00"
T = 1095.24'
L = 2133.25'
PC = 324+80.13
PT = 346+13.38
e = MATCH EXISTING

STA. 310+43 IN PLACE
TYPE 'R' DROP INLET IN MEDIAN
4' X 3' X H = 6'-2" WITH
24" X 80' R.C. PIPE INLET &
24" X 88' R.C. PIPE OUTLET
HEADWALL ON RT. & F.E.S. ON LT.
RETAIN



STA. 322+42.00
END W.R.S.F. AND DITCH PAVING

STA. 324+93 IN PLACE
42" X 100' R.C. PIPE CULV'T.
HEADWALL ON RT. & LT.
RETAIN

EXIST. ROW & C/A

10/19/2012

RB80614.DGN

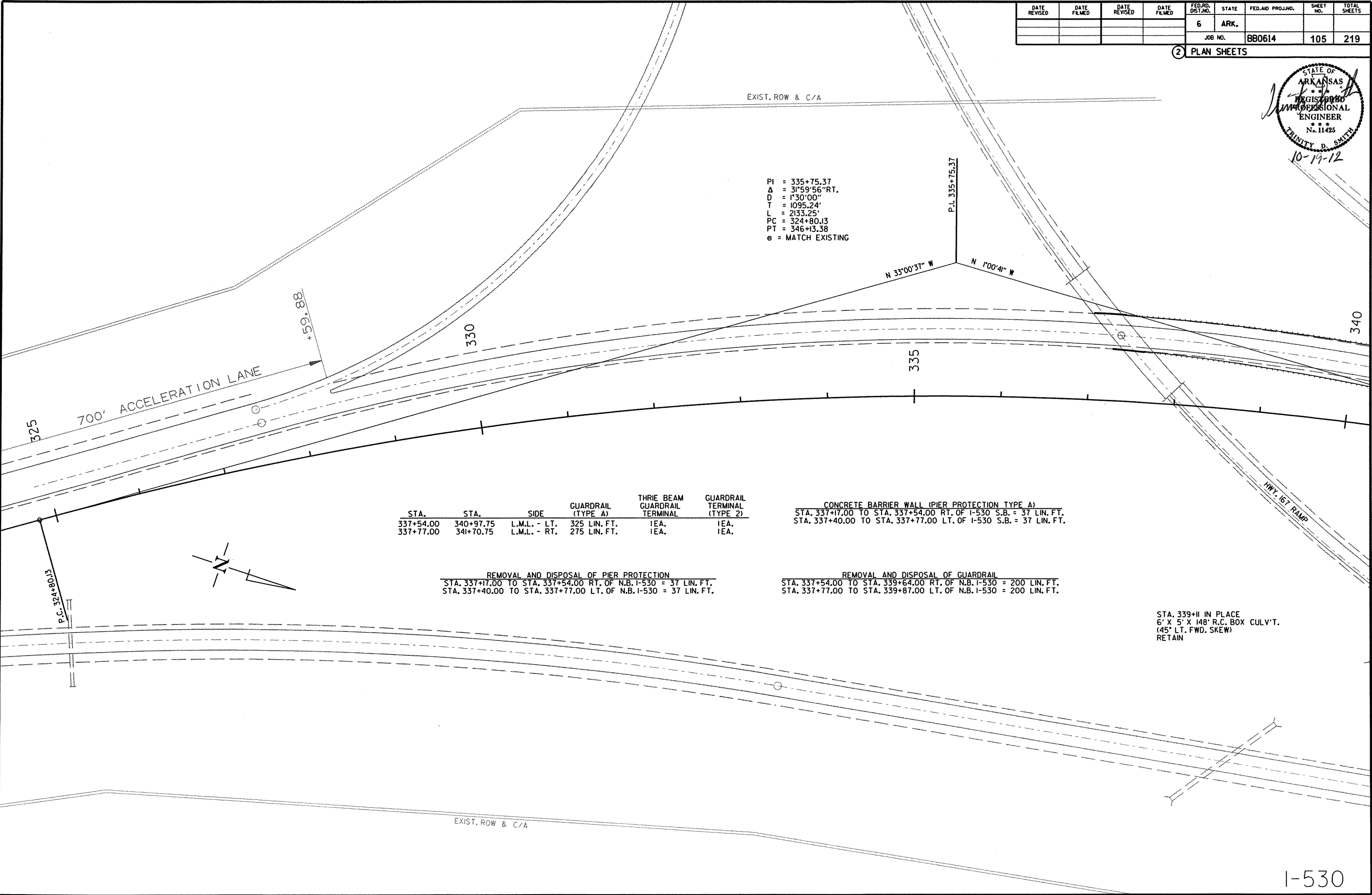
DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB NO.		BB0614	105	219

2 PLAN SHEETS



PI = 335+75.37
Δ = 31°59'56" RT.
D = 1°30'00"
T = 1095.24'
L = 2133.25'
PC = 324+80.13
PT = 346+13.38
e = MATCH EXISTING

P.I. 335+75.37
N 33°00'37" W
N 1°00'41" W



STA.	STA.	SIDE	GUARDRAIL (TYPE A)	THREE BEAM GUARDRAIL TERMINAL	GUARDRAIL TERMINAL (TYPE 2)
337+54.00	340+97.75	L.M.L. - LT.	325 LIN. FT.	IEA.	IEA.
337+77.00	341+70.75	L.M.L. - RT.	275 LIN. FT.	IEA.	IEA.

CONCRETE BARRIER WALL (PIER PROTECTION TYPE A)
STA. 337+17.00 TO STA. 337+54.00 RT. OF I-530 S.B. = 37 LIN. FT.
STA. 337+40.00 TO STA. 337+77.00 LT. OF I-530 S.B. = 37 LIN. FT.

REMOVAL AND DISPOSAL OF PIER PROTECTION
STA. 337+17.00 TO STA. 337+54.00 RT. OF N.B. I-530 = 37 LIN. FT.
STA. 337+40.00 TO STA. 337+77.00 LT. OF N.B. I-530 = 37 LIN. FT.

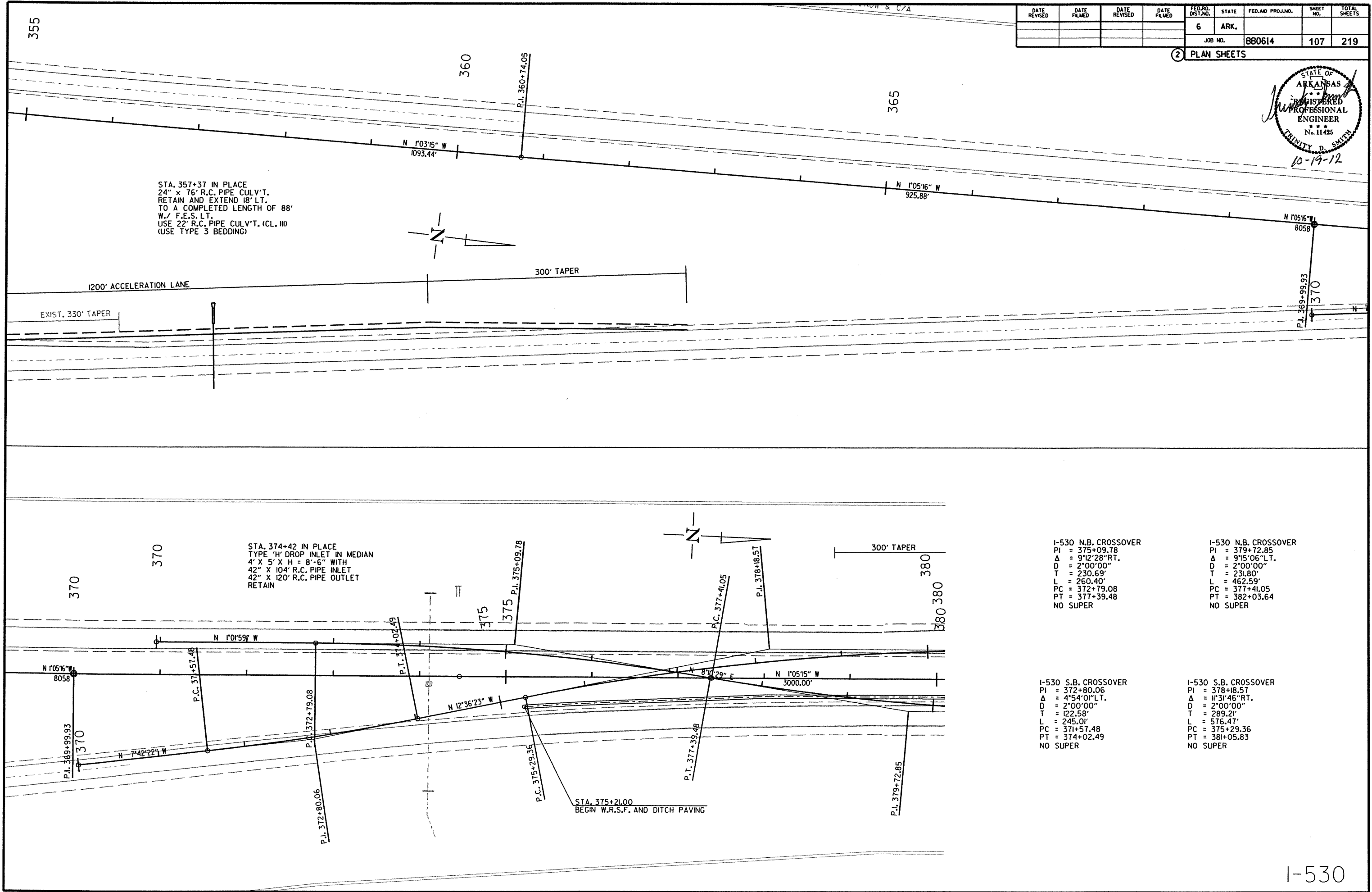
REMOVAL AND DISPOSAL OF GUARDRAIL
STA. 337+54.00 TO STA. 339+64.00 RT. OF N.B. I-530 = 200 LIN. FT.
STA. 337+77.00 TO STA. 339+87.00 LT. OF N.B. I-530 = 200 LIN. FT.

STA. 339+11 IN PLACE
6' X 5' X 148' R.C. BOX CULV'T.
(45° LT. FWD. SKEW)
RETAIN

I-530

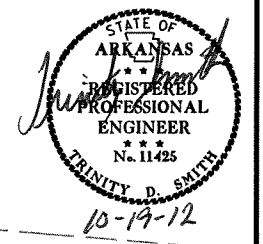
10/19/2012

RBB0614.DGN



DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB NO.	880614		107	219

2 PLAN SHEETS



I-530 N.B. CROSSOVER PI = 375+09.78 Δ = 912'28" RT. D = 2'00'00" T = 230.69' L = 260.40' PC = 372+79.08 PT = 377+39.48 NO SUPER	I-530 N.B. CROSSOVER PI = 379+72.85 Δ = 915'06" LT. D = 2'00'00" T = 231.80' L = 462.59' PC = 377+41.05 PT = 382+03.64 NO SUPER
I-530 S.B. CROSSOVER PI = 372+80.06 Δ = 4'54'01" LT. D = 2'00'00" T = 122.58' L = 245.01' PC = 371+57.48 PT = 374+02.49 NO SUPER	I-530 S.B. CROSSOVER PI = 378+18.57 Δ = 11'31'46" RT. D = 2'00'00" T = 289.21' L = 576.47' PC = 375+29.36 PT = 381+05.83 NO SUPER

STA. 385+74.88 TO STA. 387+49.62 - IN PLACE
46'-8" X 180' BRIDGE CONSISTING OF
REINFORCED CONCRETE SLAB SPAN
SUPERSTRUCTURES AND PRECAST
CONCRETE PILE SUBSTRUCTURES.
REMOVE AS EXISTING BRIDGE STRUCTURE
BRIDGE NO. A3327
(SITE NO. 4) = 1.00 LUMP SUM

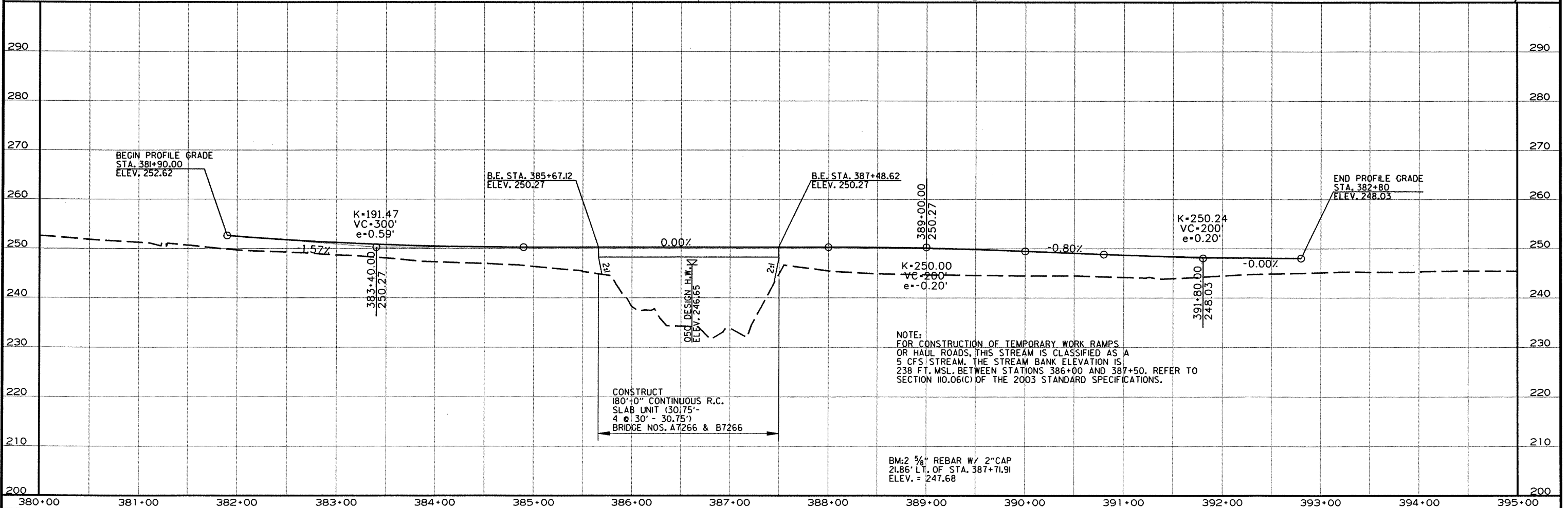
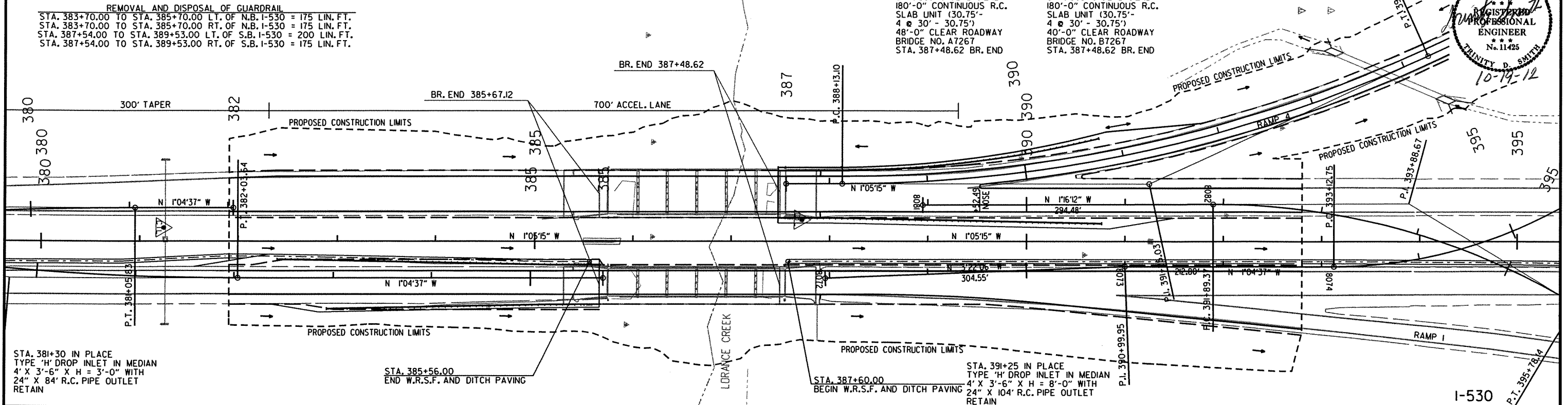
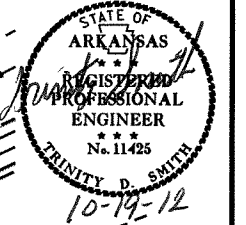
STA. 385+74.88 TO STA. 387+49.62 - IN PLACE
33'-6" X 180' BRIDGE CONSISTING OF
REINFORCED CONCRETE SLAB SPAN
SUPERSTRUCTURES AND PRECAST
CONCRETE PILE SUBSTRUCTURES.
REMOVE AS EXISTING BRIDGE STRUCTURE
BRIDGE NO. B3327
(SITE NO. 5) = 1.00 LUMP SUM

STA.	STA.	SIDE	GUARDRAIL (TYPE A)	THREE BEAM GUARDRAIL TERMINAL	GUARDRAIL TERMINAL (TYPE 2)
382+37.97	385+56.72	R.M.L. - LT.	250 LIN. FT.	IEA.	IEA.
382+87.97	385+56.72	R.M.L. - RT.	200 LIN. FT.	IEA.	IEA.
387+59.02	390+77.77	L.M.L. - LT.	250 LIN. FT.	IEA.	IEA.
387+59.02	390+77.77	R.M.L. - RT.	250 LIN. FT.	IEA.	IEA.

SPECIAL FLOOD HAZARD LIMITS
STA. 383+30 TO STA. 403+50

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				6	ARK.			
				JOB NO.		BB0614	108	219

2 PLAN AND PROFILE SHEETS



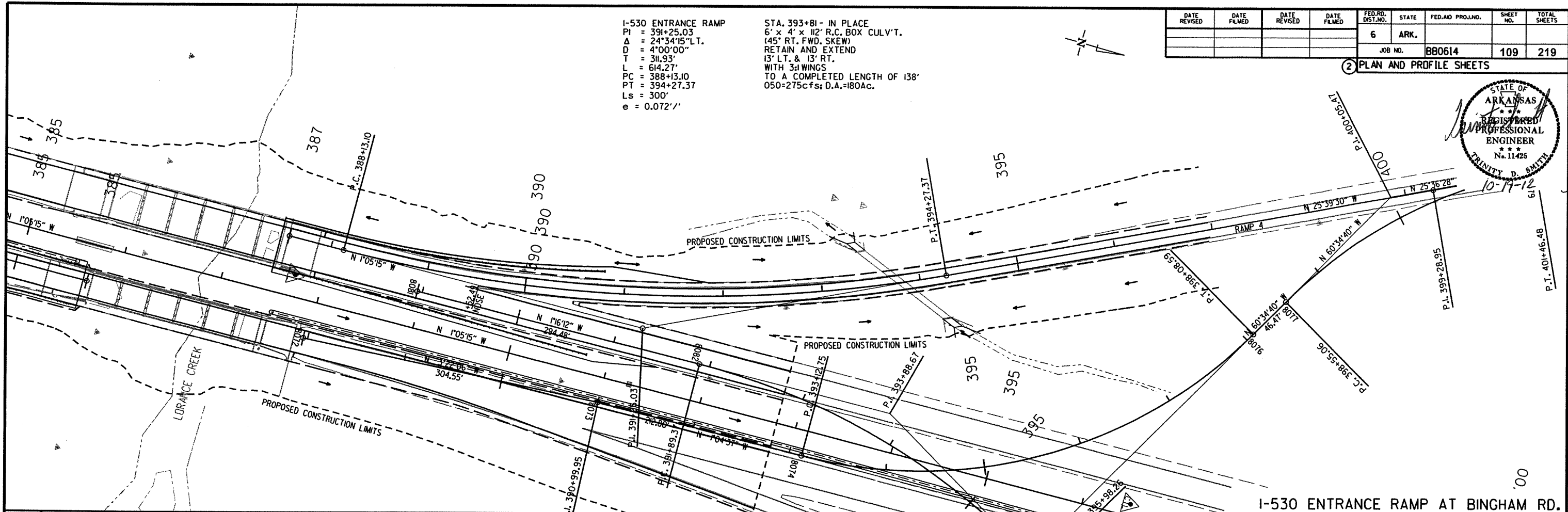
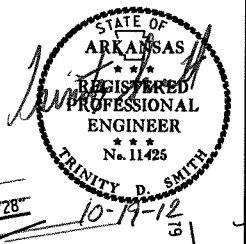
10/19/2012
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I-530 ENTRANCE RAMP
PI = 391+25.03
Δ = 24°34'15" LT.
D = 4°00'00"
T = 311.93'
L = 614.27'
PC = 388+13.10
PT = 394+27.37
LS = 300'
e = 0.072'/'

STA. 393+81- IN PLACE
6' x 4' x 112' R.C. BOX CULV'T.
(45° RT. FWD. SKEW)
RETAIN AND EXTEND
13' LT. & 13' RT.
WITH 3:1 WINGS
TO A COMPLETED LENGTH OF 138'
050=275cfs; D.A.=180Ac.

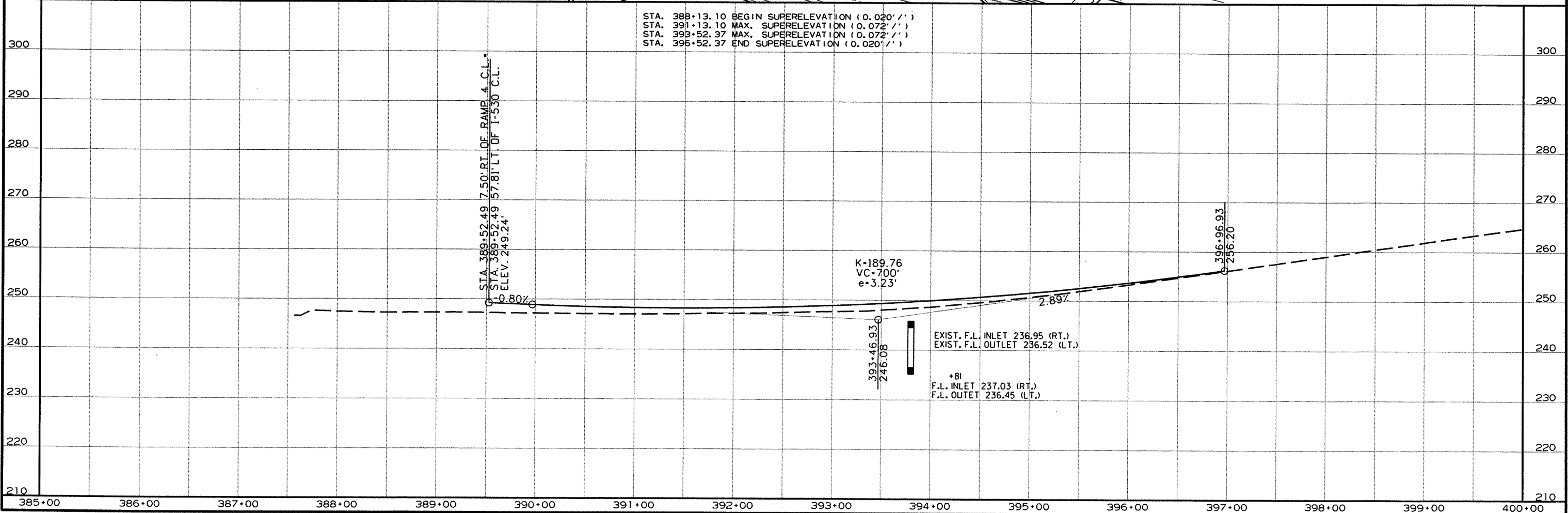
DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB NO.	BB0614		109	219

② PLAN AND PROFILE SHEETS



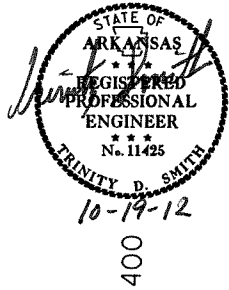
I-530 ENTRANCE RAMP AT BINGHAM RD.

STA. 388+13.10 BEGIN SUPERELEVATION (0.020'/'')
STA. 391+13.10 MAX. SUPERELEVATION (0.072'/'')
STA. 393+52.37 MAX. SUPERELEVATION (0.072'/'')
STA. 396+52.37 END SUPERELEVATION (0.020'/'')



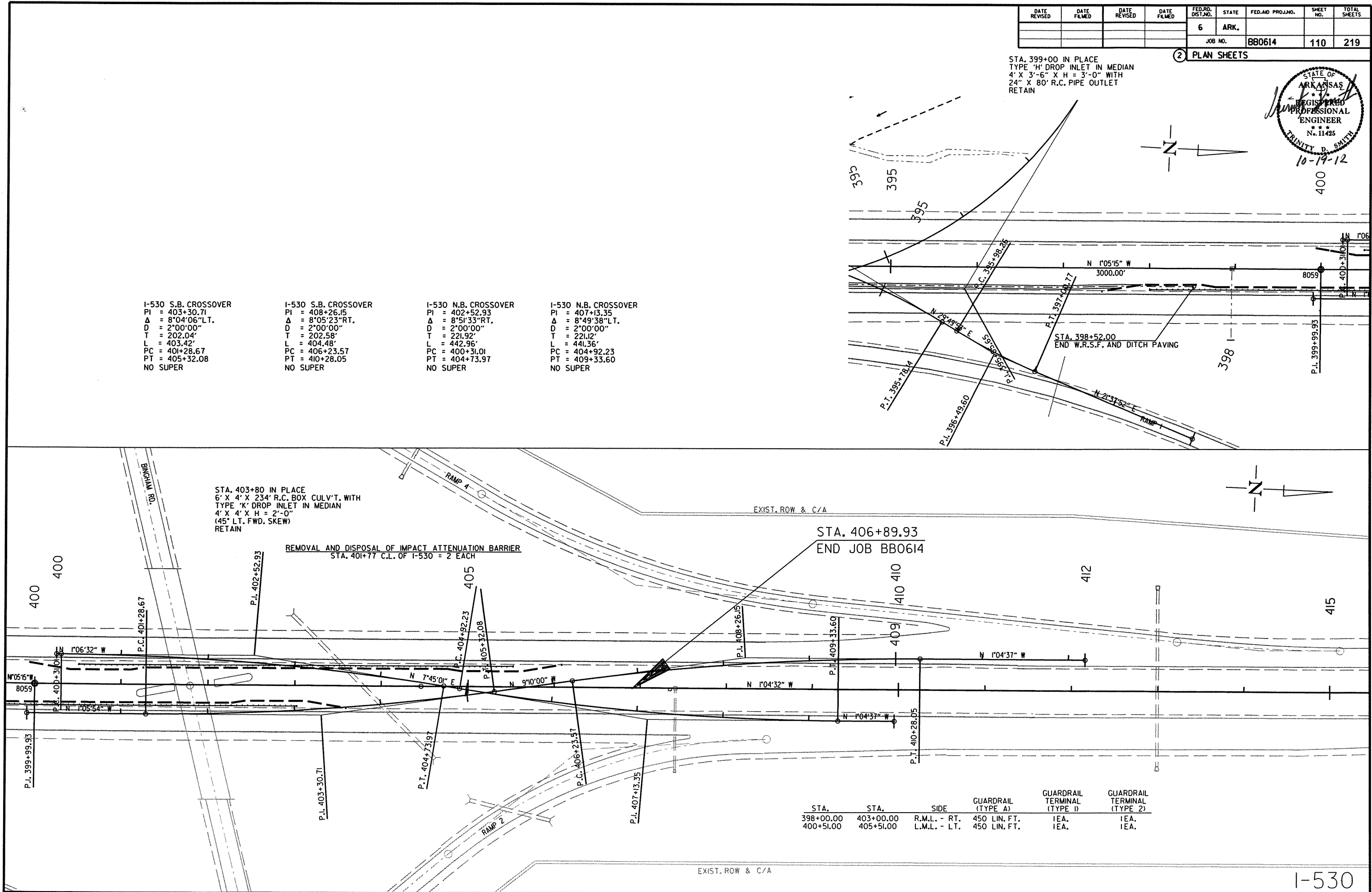
DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB NO.	BB0614		110	219

2 PLAN SHEETS



STA. 399+00 IN PLACE
TYPE 'H' DROP INLET IN MEDIAN
4' X 3'-6" X H = 3'-0" WITH
24" X 80' R.C. PIPE OUTLET
RETAIN

I-530 S.B. CROSSOVER	I-530 S.B. CROSSOVER	I-530 N.B. CROSSOVER	I-530 N.B. CROSSOVER
PI = 403+30.71	PI = 408+26.15	PI = 402+52.93	PI = 407+13.35
Δ = 8°04'06" L.T.	Δ = 8°05'23" RT.	Δ = 8°51'33" RT.	Δ = 8°49'38" L.T.
D = 2'00'00"	D = 2'00'00"	D = 2'00'00"	D = 2'00'00"
T = 202.04'	T = 202.58'	T = 221.92'	T = 221.12'
L = 403.42'	L = 404.48'	L = 442.96'	L = 441.36'
PC = 401+28.67	PC = 406+23.57	PC = 400+31.01	PC = 404+92.23
PT = 405+32.08	PT = 410+28.05	PT = 404+73.97	PT = 409+33.60
NO SUPER	NO SUPER	NO SUPER	NO SUPER



STA.	STA.	SIDE	GUARDRAIL (TYPE A)	GUARDRAIL TERMINAL (TYPE D)	GUARDRAIL TERMINAL (TYPE 2)
398+00.00	403+00.00	R.M.L. - RT.	450 LIN. FT.	1EA.	1EA.
400+51.00	405+51.00	L.M.L. - LT.	450 LIN. FT.	1EA.	1EA.

SIGNING SUMMARY OF QUANTITIES

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB NO.		BB0614	111	219

(2) SIGNING QUANTITIES SHEET

ITEM NUMBER	ITEM	TOTAL	UNIT
SP & 202	REMOVAL AND DISPOSAL OF GUARDRAIL	350	LIN. FT.
SP	REMOVAL AND DISPOSAL OF IMPACT ATTENUATION BARRIERS	1	EACH
SP	REMOVAL AND DISPOSAL OF OVERHEAD SIGN STRUCTURE	2	EACH
SS & 303	AGGREGATE BASE COURSE (CLASS 7)	140	TON
SP, SS & 407	MINERAL AGGREGATE IN ACHM SURFACE COURSE (1/2")	32	TON
SP, SS & 407	ASPHALT BINDER (PG 76-22) IN ACHM SURFACE COURSE (1/2")	2	TON
SS & 617	GUARDRAIL (TYPE A)	350	LIN FT
SS & 617	TERMINAL ANCHOR POSTS (TYPE 1)	2	EACH
SS & 617	GUARDRAIL TERMINAL (TYPE 2)	2	EACH
SP	STEEL OVERHEAD SIGN STRUCTURE (OH-530-60-03)	1	EACH
SS & 725	GUIDE SIGN - ROADSIDE MOUNTED (DEMOUNTABLE LEGEND)	1,392	SQ. FT.
SS & 725	GUIDE SIGN - OVERHEAD MOUNTED (DEMOUNTABLE LEGEND)	290	SQ. FT.
SP, SS & 726	STANDARD SIGNS	945	SQ. FT.
SS & 727	EXIT NUMBER PANEL (TYPE A)	250	SQ. FT.
SP & 729	CHANNEL POST SIGN SUPPORT (TYPE U1-1)	32	EACH
730	BREAKAWAY SIGN SUPPORT (TYPE G-1)	881	POUND
730	BREAKAWAY SIGN SUPPORT (TYPE G-2)	20,862	POUND

REMOVAL AND DISPOSAL

LOCATION	GUARDRAIL	IMPACT ATTENUATION	OVERHEAD SIGN
	LIN. FT.	BARRIERS EACH	STRUCTURE EACH
STA396+00	350		1
STA353+50		1	1
TOTAL:	350	1	2

MAIN LANES SIGNING QUANTITIES

SIGN NO./ LOCATION	STRUCTURE TYPE					BREAKAWAY SIGN SUPPORT														EXIT NUMBER PANEL				GUARDRAIL					
						GUIDE SIGN			STEEL SECT.		SIGN			STUB POST			FOOTINGS			SIGN POST	LEGEND	TYPE			TYPE A	TERM. ANCHOR	GUARDRAIL	AGG.	ACHM
	POST LENGTH		AND STUB								TYPE						TERM.	BASE CR	SURF. CR.										
	ST	CL	OH	BM	G-2	Length	Height	SQ. FT.	A-572	H - 1	H - 2	H - 3	H - 1	H - 2	H - 3	DIA.				DEPTH	EMBED.	AND STUB	A	B	C	LIN. FT.	POSTS TYPE 1	TYPE 2	(CL. 7)
						LIN. FT.			BEAM	LBS.	LIN. FT.				LIN. FT.				POUND		SQ. FT.								
EXIT 9																													
ML-530-332+20NB					1	11.50	8.00	92.00	V8	18.00	15.00	16.00		3.99	3.99		2.00	5.50	3.66	701.64	9	17.50							
ML-530-364+00NB					1	11.50	8.00	92.00	V8	21.00	15.00	16.00		4.99	4.99		2.50	7.00	4.66	860.58	9	17.50							
ML-530-385+00NB					1	14.50	6.00	87.00	V8	18.00	13.00	14.00		3.99	3.99		2.50	5.50	3.66	629.64	9	17.50							
ML-530-388+00NB					1	7.00	2.00	14.00	V6	9.00	9.00	10.00		2.33	2.33		1.50	3.00	2.00	212.94									
ML-530-388+00SB					1	7.00	2.00	14.00	V6	9.00	9.00	10.00		2.33	2.33		1.50	3.00	2.00	212.94									
ML-530-393+20NB					1	6.00	5.00	30.00	V6	9.00	12.00	13.00		2.33	2.33		1.50	3.00	2.00	266.94									
ML-530-410+00SB					1	6.00	5.00	30.00	V6	9.00	12.00	13.00		2.33	2.33		1.50	3.00	2.00	266.94									
LG-530-355+00NB					1	18.00	6.50	117.00	V8	18.00	13.00	14.00		3.99	3.99		2.50	5.50	3.66	629.64									
OH-530-60-03			1																				350	2	2	140	34		
OH-530-62-415+00SB-A						14.50	6.00	87.00												9	17.50								
OH-530-62-415+00SB-B						14.00	14.50	203.00												10	20.00								
EXIT 10																													
ML-530-343+00SB					1	6.00	5.00	30.00	V6	9.00	12.00	13.00		2.33	2.33		1.50	3.00	2.00	266.94									
EXIT 12																													
ML-530-317+00SB					1	12.00	6.00	72.00	V8	18.00	13.00	14.00		3.99	3.99		2.50	5.50	3.66	629.64	12	20.00							
ML-530-179+00NB					1	12.00	6.00	72.00	V8	18.00	13.00	14.00		3.99	3.99		2.50	5.50	3.66	629.64	12	20.00							
ML-530-179+00NB					1	15.00	4.00	60.00	V8	18.00	11.00	12.00		3.99	3.99		2.50	5.50	3.66	557.64	12	20.00							
ML-530-179+00NB					1	15.00	4.00	60.00	V8	18.00	11.00	12.00		3.99	3.99		2.50	5.50	3.66	557.64	12	20.00							
ML-530-168+00SB					1	14.00	6.50	91.00	V8	18.00	13.50	14.50		3.99	3.99		2.50	5.50	3.66	647.64									
ML-530-265+00SB					1	6.00	5.00	30.00	V6	9.00	12.00	13.00		2.33	2.33		1.50	3.00	2.00	266.94									
ML-530-236+00NB					1	6.00	5.00	30.00	V6	9.00	12.00	13.00		2.33	2.33		1.50	3.00	2.00	266.94									
ML-530-212+90NB					1	7.00	2.00	14.00	V6	9.00	9.00	10.00		2.33	2.33		1.50	3.00	2.00	212.94									
ML-530-212+90SB					1	7.00	2.00	14.00	V6	9.00	9.00	10.00		2.33	2.33		1.50	3.00	2.00	212.94									
EXIT 15																													
ML-530-144+00SB					1	12.00	6.00	72.00	V8	18.00	13.00	14.00		3.99	3.99		2.50	5.50	3.66	629.64	12	20.00							
ML-530-5+00NB					1	12.00	6.00	72.00	V8	18.00	13.00	14.00		3.99	3.99		2.50	5.50	3.66	629.64	12	20.00							
ML-530-92+00SB					1	15.00	4.00	60.00	V8	18.00	11.00	12.00		3.99	3.99		2.50	5.50	3.66	557.64	12	20.00							
ML-530-58+00NB					1	15.00	4.00	60.00	V8	18.00	11.00	12.00		3.99	3.99		2.50	5.50	3.66	557.64	12	20.00							
ML-530-35+00SB					1	14.00	6.50	91.00	V8	18.00	13.50	14.50		3.99	3.99		2.50	5.50	3.66	647.64									
ML-530-85+50SB					1	6.00	5.00	30.00	V6	9.00	12.00	13.00		2.33	2.33		1.50	3.00	2.00	266.94									
ML-530-65+10NB					1	6.00	5.00	30.00	V6	9.00	12.00	13.00		2.33	2.33		1.50	3.00	2.00	266.94									
ML-530-10+90NB					1	7.00	2.00	14.00	V6	9.00	9.00	10.00		2.33	2.33		1.50	3.00	2.00	212.94									
ML-530-10+90SB					1	7.00	2.00	14.00	V6	9.00	9.00	10.00		2.33	2.33		1.50	3.00	2.00	212.94									
GUIDE SIGNS ROADSIDE MOUNTED TOTALS:						1392.00																							
GUIDE SIGNS OVERHEAD MOUNTED TOTALS:						290.00																							
TOTALS:				1	27			1682.00												12012.12		250.00		350	2	2	140	34	

NOTE:
BREAKAWAY SIGN SUPPORT TOTAL IS CALCULATED BY TAKING THE LENGTH OF H-1, H-2, H-3 AND EACH STUB POST AND MULTIPLYING BY THE BEAM WEIGHT (LBS).

BASIS OF ESTIMATE:
NMAX = 205
MINERAL AGGREGATE 94.8%
ASPHALT BINDER (PG 76-22) 5.2%



STANDARD SIGNS FLAT SHEET

SIGN NO./	SIGN SUPPORTS				STANDARD		I-BEAM SIGN SUPPORT										SIGN POST
	TYPE			STEEL SECT.			SIGN POST LENGTH			STUB POST		FOOTINGS					
	U-1	G-1	G-2		EA.	EA.	SQ. FT.	BEAM	A-572	H - 1	H - 2	H - 1	H - 2	DIA.	DEPTH	EMBED.	
LOCATION	EA.	EA.	EA.	EA.				LBS.	LIN. FT.		LIN. FT.		LIN. FT.		LIN. FT.		POUND
SS-530-48+00SB				1		20.00	W6	9	12.00	13.00	1.99	1.99	2.00	3.00	2.00	260.82	
SS-530-105+00NB				1		20.00	W6	9	12.00	13.00	1.99	1.99	2.00	3.00	2.00	260.82	
SS-530-225+00SB				1		20.00	W6	9	12.00	13.00	1.99	1.99	2.00	3.00	2.00	260.82	
SS-530-275+00NB				1		20.00	W6	9	12.00	13.00	1.99	1.99	2.00	3.00	2.00	260.82	
SS-530-305+00SB				1		20.00	W6	9	12.00	13.00	1.99	1.99	2.00	3.00	2.00	260.82	
SS-530-371+00SB				1		20.00	W6	9	12.00	13.00	1.99	1.99	2.00	3.00	2.00	260.82	
SS-530-373+00NB				1		20.00	W6	9	12.00	13.00	1.99	1.99	2.00	3.00	2.00	260.82	
SS-530-62+00NB				1		20.00	W6	9	12.00	13.00	1.99	1.99	2.00	3.00	2.00	260.82	
SS-530-89+00SB				1		20.00	W6	9	12.00	13.00	1.99	1.99	2.00	3.00	2.00	260.82	
SS-530-232+90NB				1		20.00	W6	9	12.00	13.00	1.99	1.99	2.00	3.00	2.00	260.82	
SS-530-267+10SB				1		20.00	W6	9	12.00	13.00	1.99	1.99	2.00	3.00	2.00	260.82	
SS-530-389+50NB				1		20.00	W6	9	12.00	13.00	1.99	1.99	2.00	3.00	2.00	260.82	
SS-530-415+00SB				1		20.00	W6	9	12.00	13.00	1.99	1.99	2.00	3.00	2.00	260.82	
SS-530-100+00NB		1				14.38	W6	9	12.00		1.99	2.00	2.00	3.00	2.00	125.91	
SS-530-270+00NB		1				14.38	W6	9	12.00		1.99	2.00	2.00	3.00	2.00	125.91	
SS-530-370+00NB		1				14.38	W6	9	12.00		1.99	2.00	2.00	3.00	2.00	125.91	
SS-530-52+00SB		1				14.38	W6	9	12.00		1.99	2.00	2.00	3.00	2.00	125.91	
SS-530-231+00SB		1				14.38	W6	9	12.00		1.99	2.00	2.00	3.00	2.00	125.91	
SS-530-310+00SB		1				14.38	W6	9	12.00		1.99	2.00	2.00	3.00	2.00	125.91	
SS-530-376+00SB		1				14.38	W6	9	12.00		1.99	2.00	2.00	3.00	2.00	125.91	
SS-530-73+00NB			1			16.00	W6	9	11.00	12.00	1.99	1.99	2.00	3.00	2.00	242.82	
SS-530-80+00NB			1			16.00	W6	9	11.00	12.00	1.99	1.99	2.00	3.00	2.00	242.82	
SS-530-249+00SB			1			16.00	W6	9	11.00	12.00	1.99	1.99	2.00	3.00	2.00	242.82	
SS-530-253+00NB			1			16.00	W6	9	11.00	12.00	1.99	1.99	2.00	3.00	2.00	242.82	
SS-530-333+00SB			1			16.00	W6	9	11.00	12.00	1.99	1.99	2.00	3.00	2.00	242.82	
SS-530-396+00SB			1			16.00	W6	9	11.00	12.00	1.99	1.99	2.00	3.00	2.00	242.82	
SS-530-405+00NB			1			16.00	W6	9	11.00	12.00	1.99	1.99	2.00	3.00	2.00	242.82	
SS-530-343+00NB			1			16.00	W6	9	11.00	12.00	1.99	1.99	2.00	3.00	2.00	242.82	
SS-530-52+00NB			1			16.00	W6	9	11.00	12.00	1.99	1.99	2.00	3.00	2.00	242.82	
SS-530-66+00SB			1			16.00	W6	9	11.00	12.00	1.99	1.99	2.00	3.00	2.00	242.82	
SS-530-168+00NB			1			16.00	W6	9	11.00	12.00	1.99	1.99	2.00	3.00	2.00	242.82	
SS-530-183+00SB			1			16.00	W6	9	11.00	12.00	1.99	1.99	2.00	3.00	2.00	242.82	
SS-530-280+00NB			1			16.00	W6	9	11.00	12.00	1.99	1.99	2.00	3.00	2.00	242.82	
SS-530-293+00SB			1			16.00	W6	9	11.00	12.00	1.99	1.99	2.00	3.00	2.00	242.82	
SS-530-381+00NB			1			16.00	W6	9	11.00	12.00	1.99	1.99	2.00	3.00	2.00	242.82	
SS-530-394+00SB			1			16.00	W6	9	11.00	12.00	1.99	1.99	2.00	3.00	2.00	242.82	
SS-530-43+00SB			1			36.00	W6	9	10.00	11.00	1.99	1.99	2.00	3.00	2.00	224.82	
SS-530-110+00NB			1			36.00	W6	9	10.00	11.00	1.99	1.99	2.00	3.00	2.00	224.82	
SS-530-219+00SB			1			36.00	W6	9	10.00	11.00	1.99	1.99	2.00	3.00	2.00	224.82	
SS-530-285+00NB			1			36.00	W6	9	10.00	11.00	1.99	1.99	2.00	3.00	2.00	224.82	
SS-530-300+00SB			1			36.00	W6	9	10.00	11.00	1.99	1.99	2.00	3.00	2.00	224.82	
SS-530-366+00SB			1			36.00	W6	9	10.00	11.00	1.99	1.99	2.00	3.00	2.00	224.82	
SS-530-378+00NB			1			36.00	W6	9	10.00	11.00	1.99	1.99	2.00	3.00	2.00	224.82	
SS-530-59+45SB-A	1					3.00											
SS-530-59+45SB-B	1					3.00											
SS-530-58+55NB-A	1					3.00											
SS-530-58+55NB-B	1					3.00											
SS-530-176+66SB-A	1					3.00											
SS-530-176+66SB-B	1					3.00											
SS-530-175+16NB-A	1					3.00											
SS-530-175+16NB-B	1					3.00											
SS-530-285+69NB-A	1					3.00											
SS-530-285+69NB-B	1					3.00											
SS-530-286+56SB-A	1					3.00											
SS-530-286+56SB-B	1					3.00											
SS-530-385+74NB-A	1					3.00											
SS-530-385+74NB-B	1					3.00											
SS-530-387+64SB-A	1					3.00											
SS-530-387+64SB-B	1					3.00											
SS-530-400+00NB	1					1.25											
SS-530-400+00SB	1					1.25											
SS-530-345+00NB	1					1.87											
SS-530-345+00SB	1					1.87											
SS-530-288+50SB	1					1.87											
SS-530-288+50NB	1					1.87											
SS-530-239+60SB	1					1.87											
SS-530-239+60NB	1					1.87											
SS-530-186+80SB	1					1.87											
SS-530-186+80NB	1					1.87											
SS-530-134+00SB	1					1.87											
SS-530-134+00NB	1					1.87											
SS-530-81+20SB	1					1.87											
SS-530-81+20NB	1					1.87											
SS-530-28+40SB	1					1.87											
SS-530-28+40NB	1					1.87											
TOTALS:	32					945.34										9730.89	

2

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED.RD. DIST.NO.	STATE	FED.AID PROJ.NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB NO.		BB0614	112	219
SIGNING QUANTITIES SHEET								



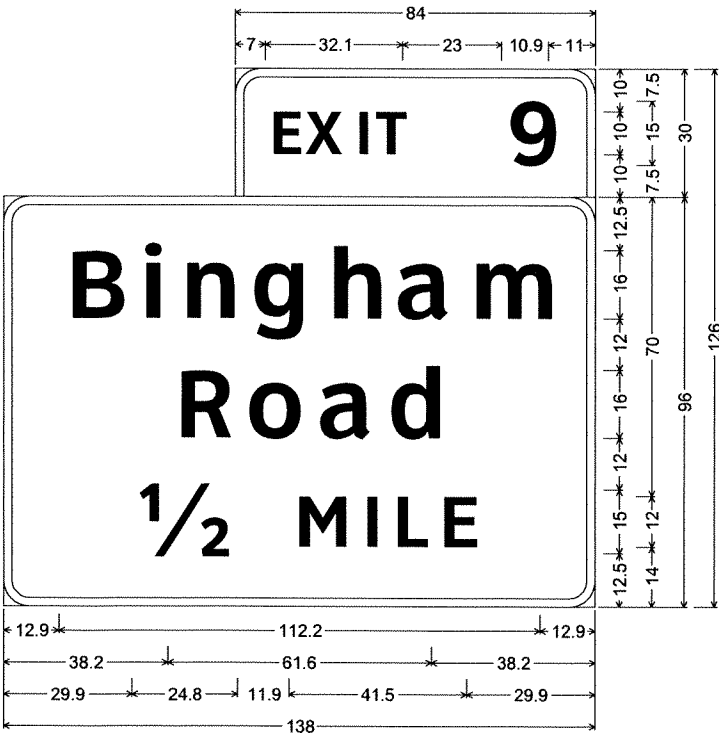
NOTES:

ALL EXISTING GUIDE SIGNS SHALL BE MAINTAINED IN SUCH A MANNER THAT THE SIGNS ARE FULLY VISIBLE, INTACT, AND ERECT FOR THE DURATION OF THE PROJECT, AND SHALL BE REMOVED WHEN THEIR USE IS NO LONGER REQUIRED. REMOVAL AND DISPOSAL OF ROADSIDE MOUNTED SIGNS, SUPPORTS AND FOUNDATIONS SHALL NOT BE PAID FOR SEPARATELY BUT SHALL BE CONSIDERED SUBSIDIARY TO OTHER ITEMS IN THE CONTRACT.

THE EXISTING SIGNS AND POSTS SHALL BECOME THE PROPERTY OF THE CONTRACTOR. THE EXISTING FOOTINGS SHALL BE REMOVED AND THE HOLES FILLED WITH A SUITABLE MATERIAL AND COMPACTED.

EXISTING LOGOS WILL BE RELOCATED TO THE NEW LOGO SIGN BY THE CONTRACTOR. THE LOGO INSTALLATION SHALL NOT BE PAID FOR SEPARATELY BUT SHALL BE CONSIDERED SUBSIDIARY TO OTHER ITEMS IN THE CONTRACT.

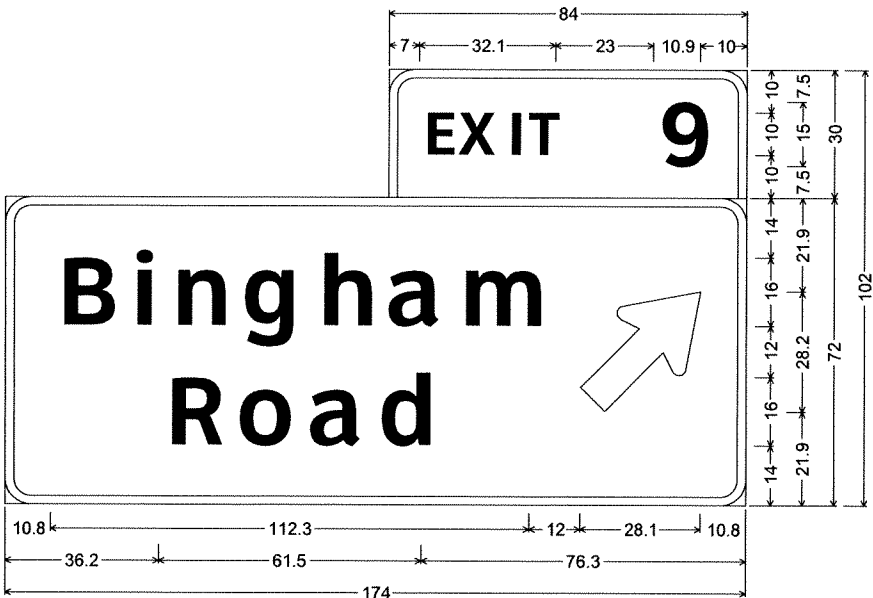
THE CLEARVIEW FONT SHALL FOLLOW THE SPACE TABLES FOR CLEARVIEW AND NOT SHS E-MODIFIED. THIS INCLUDES THE USE OF CLEARVIEW 5-W-R. FOR GENERAL GUIDANCE ON LETTER AND WORD SPACING REFER TO THE FHWA CLEARVIEW TYPEFACE SUPPLEMENT. ([HTTP://MUTCD.FHWA.DOT.GOV](http://MUTCD.FHWA.DOT.GOV))



ML-530-364+00NB;
6.0" Radius, 2.0" Border, White on Green;
[EXIT] ClearviewHwy-5-W; [9] ClearviewHwy-5-W;
6.0" Radius, 2.0" Border, White on Green;
[Bingham] ClearviewHwy-5-W; [Road] ClearviewHwy-5-W;
[1/2 MILE] ClearviewHwy-5-W;

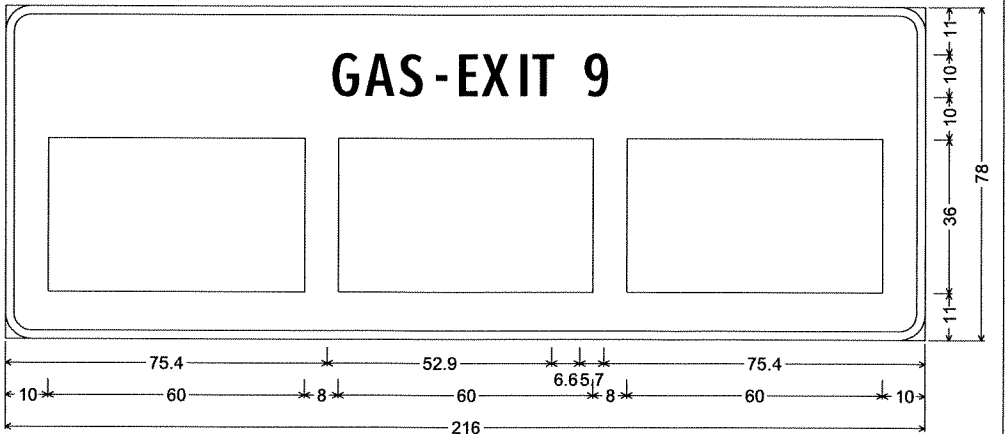


ML-530-332+20NB;
6.0" Radius, 2.0" Border, White on Green;
[EXIT] ClearviewHwy-5-W; [9] ClearviewHwy-5-W;
6.0" Radius, 2.0" Border, White on Green;
[Bingham] ClearviewHwy-5-W; [Road] ClearviewHwy-5-W;
[1 MILE] ClearviewHwy-5-W;

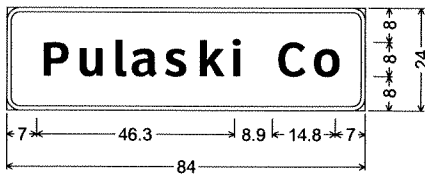


ML-530-385+00NB;
6.0" Radius, 2.0" Border, White on Green;
[EXIT] ClearviewHwy-5-W; [9] ClearviewHwy-5-W;
6.0" Radius, 2.0" Border, White on Green;
[Bingham] ClearviewHwy-5-W; [Road] ClearviewHwy-5-W;
Standard Arrow Custom 35.8" X 21.6" 45°;

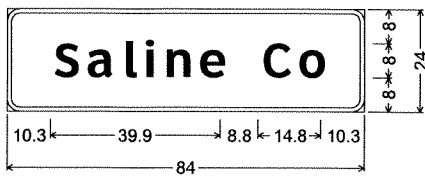
DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB NO.		BB0614	113	216
SIGN LAYOUT SHEET								
GUIDE SIGN - ROADSIDE MOUNTED (DEMOUNTABLE LEGEND)								



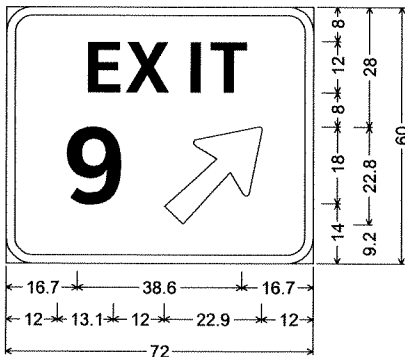
LG530-355+00NB; 6.0" Radius, 2.0" Border, White on Blue;
[GAS-EXIT 9] ClearviewHwy-2-W;



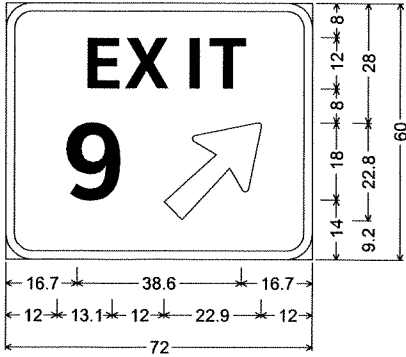
ML-530-388+00NB;
3.0" Radius, 1.0" Border, White on Green;
[Pulaski Co] ClearviewHwy-5-W;



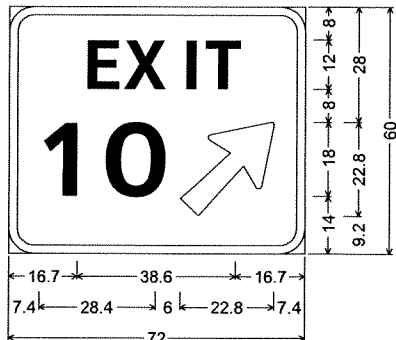
ML-530-388+00SB;
3.0" Radius, 1.0" Border, White on Green;
[Saline Co] ClearviewHwy-5-W;



ML-530-393+20NB;
6.0" Radius, 2.0" Border, White on Green;
[EXIT] ClearviewHwy-5-W;
[9] ClearviewHwy-5-W-R;
Arrow Custom - 29.0" 45°;



ML-530-410+00SB;
6.0" Radius, 2.0" Border, White on Green;
[EXIT] ClearviewHwy-5-W;
[9] ClearviewHwy-5-W-R;
Arrow Custom - 29.0" 45°;



ML-530-343+00SB;
6.0" Radius, 2.0" Border, White on Green;
[EXIT] ClearviewHwy-5-W;
[10] ClearviewHwy-5-W-R;
Arrow Custom - 29.0" 45°;



DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB NO.		BB0614	114	217
(2)				SIGN LAYOUT SHEET				

OH-530-60-03

SEE OVERHEAD SIGN STRUCTURE DETAILS
SHEETS FOR DESIGN SPECIFICATIONS

INSTALL
OH-530-62-415+00SB-A/B

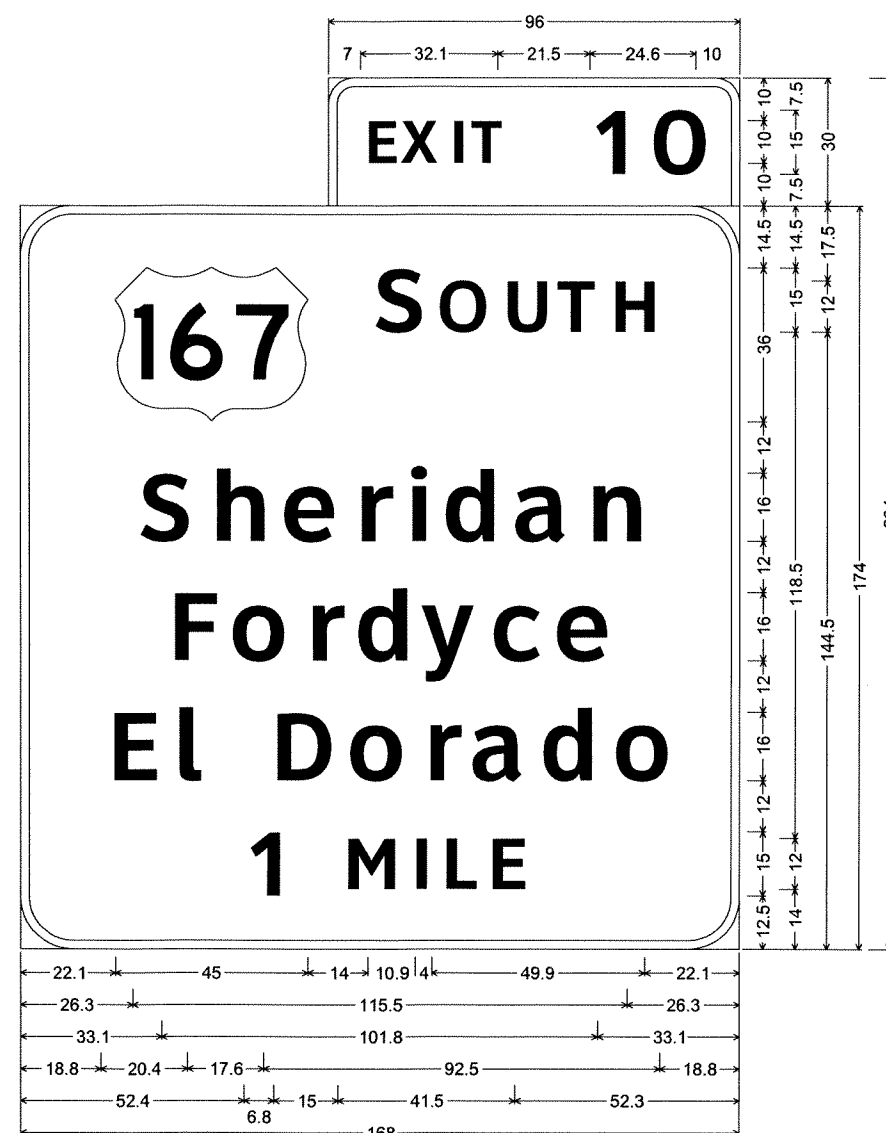
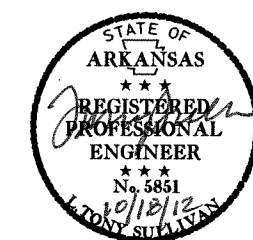
17.5 FT. MIN. VERTICAL
CLEARANCE OVER
THE HIGHEST POINT
OF ROADWAY

INSTALL

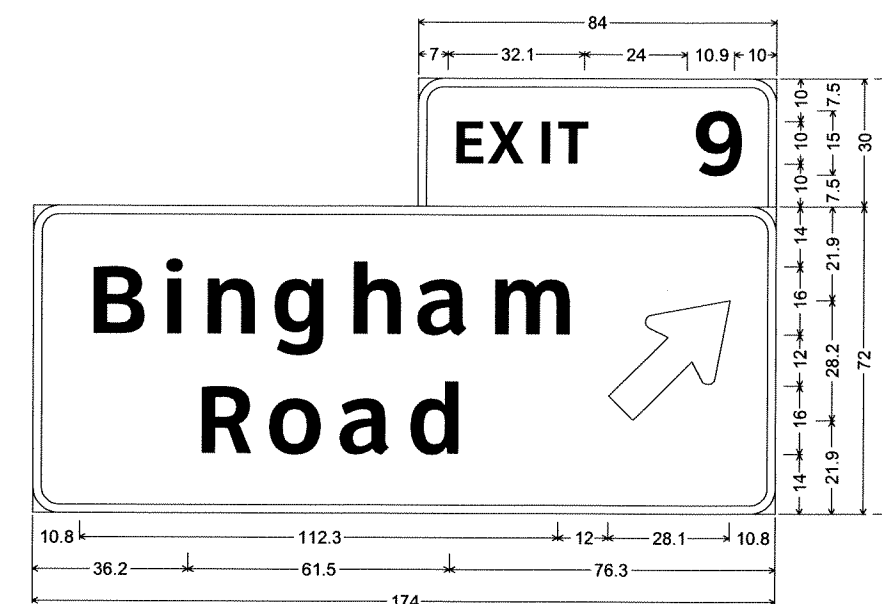
SEE ROADWAY PLANS FOR
GUARDRAIL DETAILS

7'-3" MIN.	SHOULDER	TRAFFIC LANE	TRAFFIC LANE	SHOULDER	7'-3" MIN.
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OH-530-62-415+00SB-A;
6.0" Radius, 2.0" Border, White on Green;
[EXIT] ClearviewHwy-5-W; [9] ClearviewHwy-5-W;
6.0" Radius, 2.0" Border, White on Green;
[Bingham] ClearviewHwy-5-W; [Road] ClearviewHwy-5-W;
Standard Arrow Custom 35.8" X 21.6" 45°.

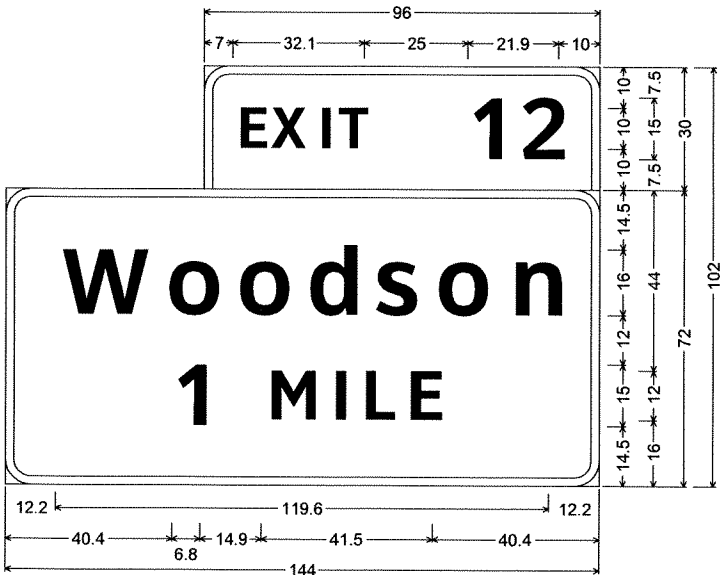


OH-530-62-415+00SB-B;
6.0" Radius, 2.0" Border, White on Green;
[EXIT] ClearviewHwy-5-W; [10] ClearviewHwy-5-W;
12.0" Radius, 2.0" Border, White on Green;
[S] ClearviewHwy-5-W; [OUTH] ClearviewHwy-5-W; [Sheridan] ClearviewHwy-5-W;
[Fordyce] ClearviewHwy-5-W; [El Dorado] ClearviewHwy-5-W;
[1] ClearviewHwy-5-W; [MILE] ClearviewHwy-5-W;

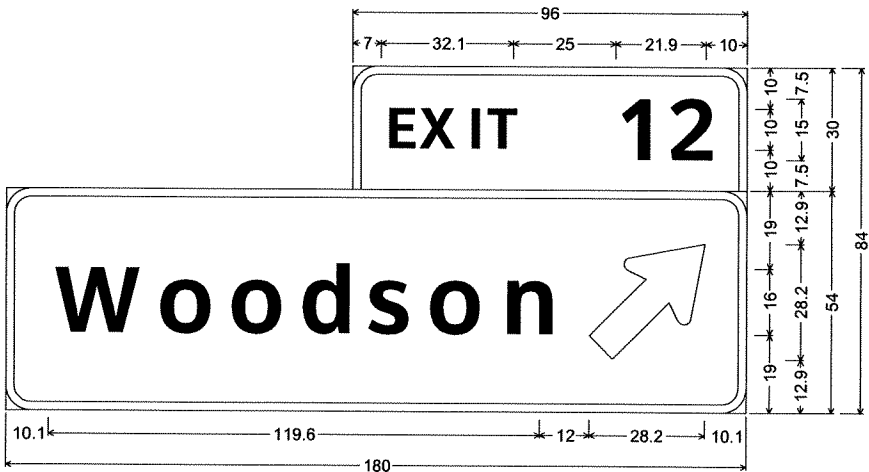


DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED.RD. DIST.NO.	STATE	FED.AID PROJ.NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB NO.		BB0614	115	219

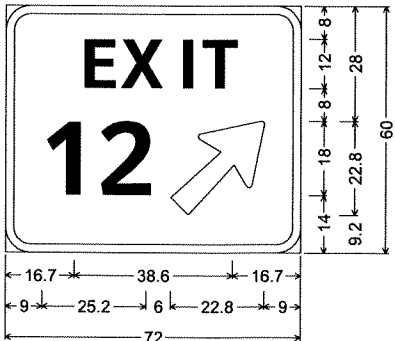
2 SIGN LAYOUT SHEET
GUIDE SIGN - ROADSIDE MOUNTED
(DEMOUNTABLE LEGEND)



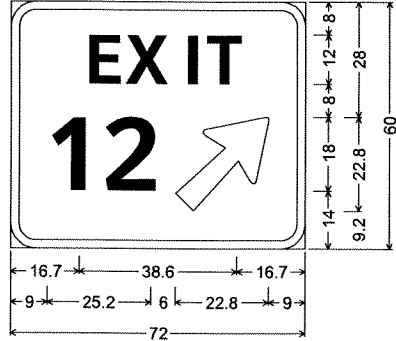
OH-530-62-415+00SB-A;
6.0" Radius, 2.0" Border, White on Green;
[EXIT] ClearviewHwy-5-W; [12] ClearviewHwy-5-W;
6.0" Radius, 2.0" Border, White on Green;
[Woodson] ClearviewHwy-5-W; [1 MILE] ClearviewHwy-5-W;



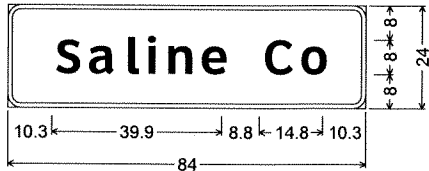
ML-530-270+00SB;
6.0" Radius, 2.0" Border, White on Green;
[EXIT] ClearviewHwy-5-W; [12] ClearviewHwy-5-W;
6.0" Radius, 2.0" Border, White on Green;
[Woodson] ClearviewHwy-5-W; Standard Arrow Custom 35.8" X 21.6" 45°;



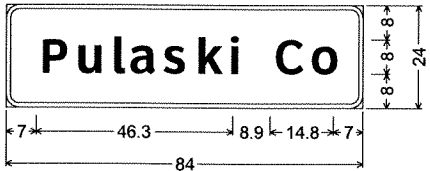
ML-530-236+00NB;
6.0" Radius, 2.0" Border, White on Green;
[EXIT] ClearviewHwy-5-W;
[12] ClearviewHwy-5-W-R;
Arrow Custom - 29.0" 45°;



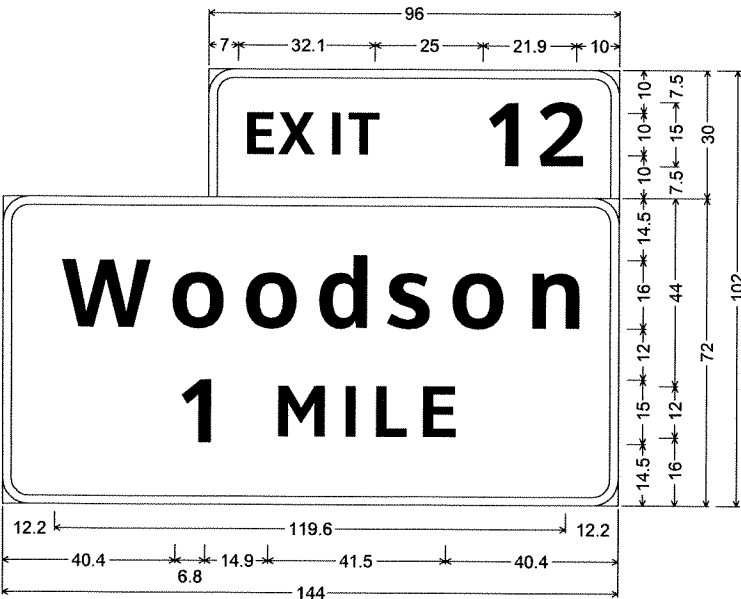
ML-530-265+00SB;
6.0" Radius, 2.0" Border, White on Green;
[EXIT] ClearviewHwy-5-W;
[12] ClearviewHwy-5-W-R;
Arrow Custom - 29.0" 45°;



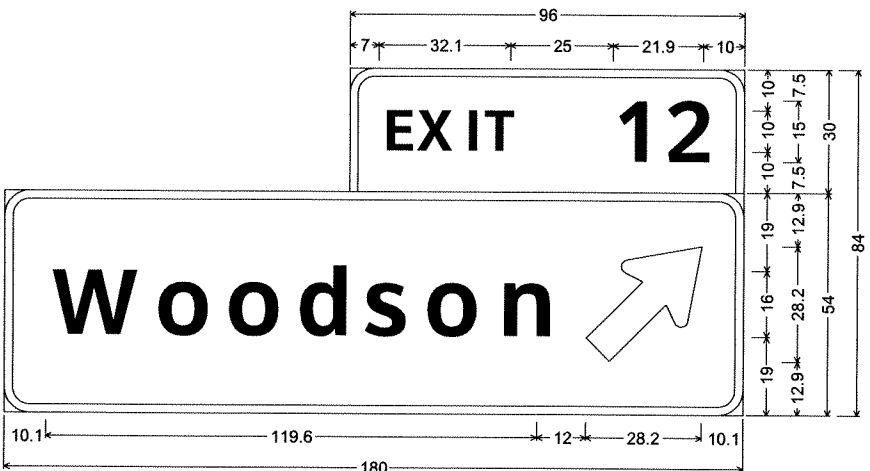
ML-530-212+90NB;
3.0" Radius, 1.0" Border, White on Green;
[Saline Co] ClearviewHwy-5-W;



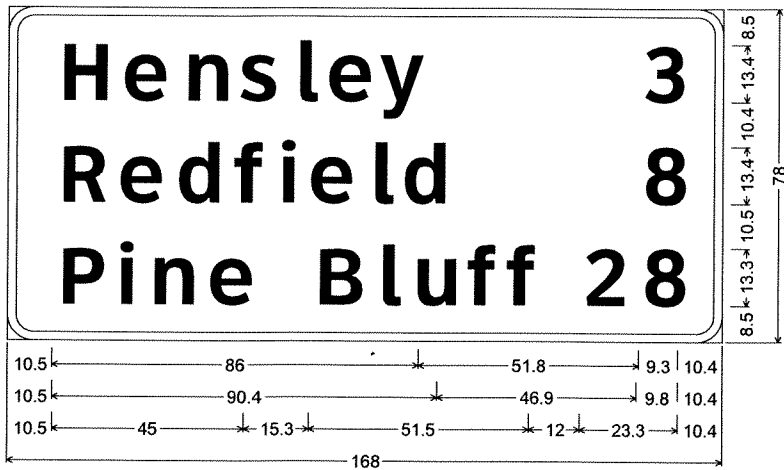
ML-530-212+90SB;
3.0" Radius, 1.0" Border, White on Green;
[Pulaski Co] ClearviewHwy-5-W;



ML-530-179+00NB;
6.0" Radius, 2.0" Border, White on Green;
[EXIT] ClearviewHwy-5-W; [12] ClearviewHwy-5-W;
6.0" Radius, 2.0" Border, White on Green;
[Woodson] ClearviewHwy-5-W; [1 MILE] ClearviewHwy-5-W;

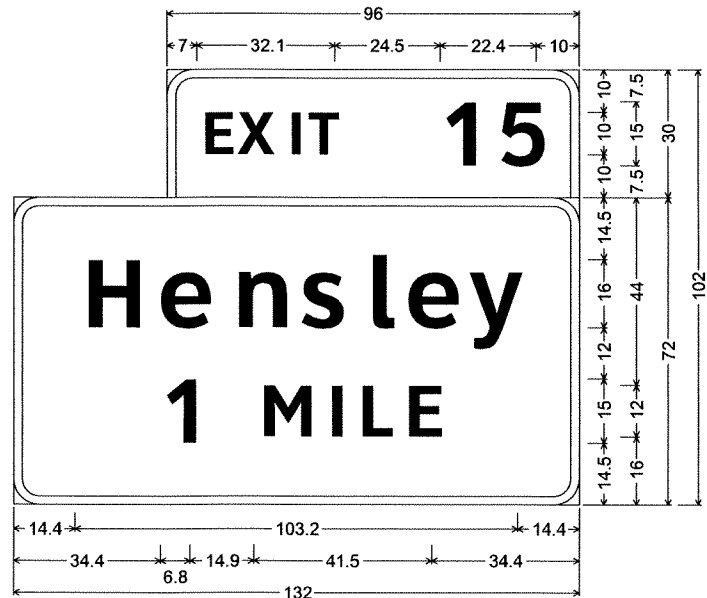


ML-530-230+00NB;
6.0" Radius, 2.0" Border, White on Green;
[EXIT] ClearviewHwy-5-W; [12] ClearviewHwy-5-W;
6.0" Radius, 2.0" Border, White on Green;
[Woodson] ClearviewHwy-5-W; Standard Arrow Custom 35.8" X 21.6" 45°;

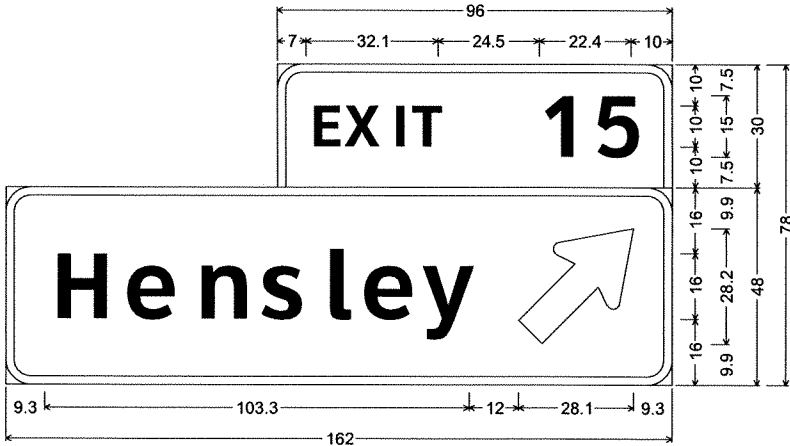


ML-530-209+00SB; 6.0" Radius, 2.0" Border, White on Green;
[Hensley] ClearviewHwy-5-W; [3] ClearviewHwy-5-W;
[Redfield] ClearviewHwy-5-W; [8] ClearviewHwy-5-W;
[Pine Bluff] ClearviewHwy-5-W; [28] ClearviewHwy-5-W;

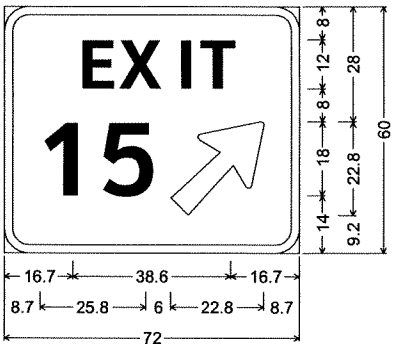




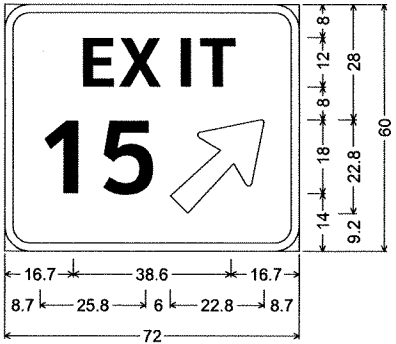
ML-530-144+00NB;
6.0" Radius, 2.0" Border, White on Green;
[EXIT] ClearviewHwy-5-W; [15] ClearviewHwy-5-W;
6.0" Radius, 2.0" Border, White on Green;
[Hensley] ClearviewHwy-5-W; [1 MILE] ClearviewHwy-5-W;



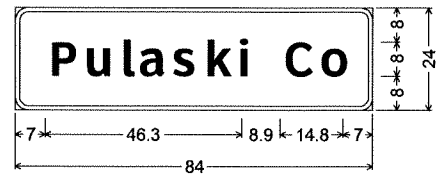
ML-530-92+00SB;
6.0" Radius, 2.0" Border, White on Green;
[EXIT] ClearviewHwy-5-W; [15] ClearviewHwy-5-W;
6.0" Radius, 2.0" Border, White on Green;
[Hensley] ClearviewHwy-5-W; Standard Arrow Custom 35.8" X 21.6" 45°;



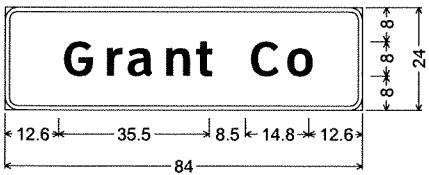
ML-530-65+10NB;
6.0" Radius, 2.0" Border, White on Green;
[EXIT] ClearviewHwy-5-W;
[15] ClearviewHwy-5-W-R;
Arrow Custom - 29.0" 45°;



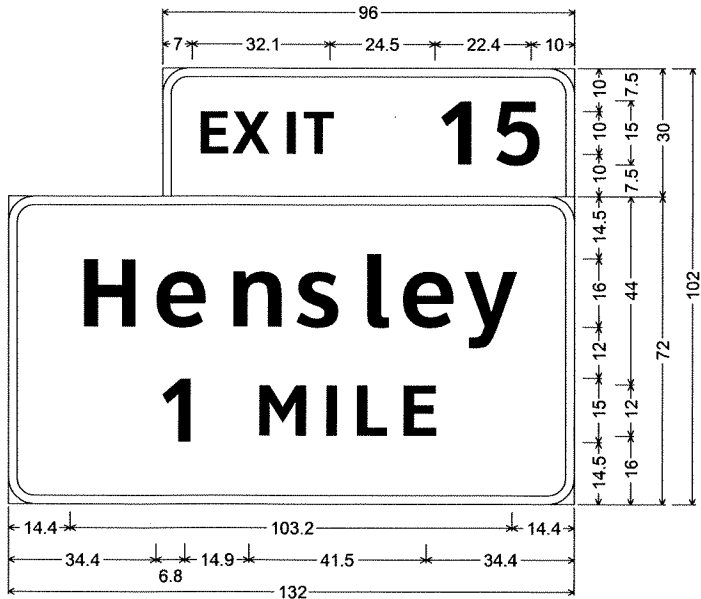
ML-530-85+50SB;
6.0" Radius, 2.0" Border, White on Green;
[EXIT] ClearviewHwy-5-W;
[15] ClearviewHwy-5-W-R;
Arrow Custom - 29.0" 45°;



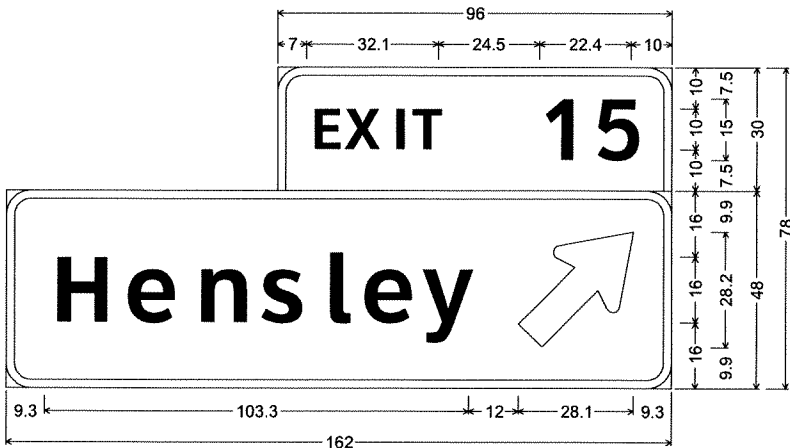
ML-530-10+90NB;
3.0" Radius, 1.0" Border, White on Green;
[Pulaski Co] ClearviewHwy-5-W;



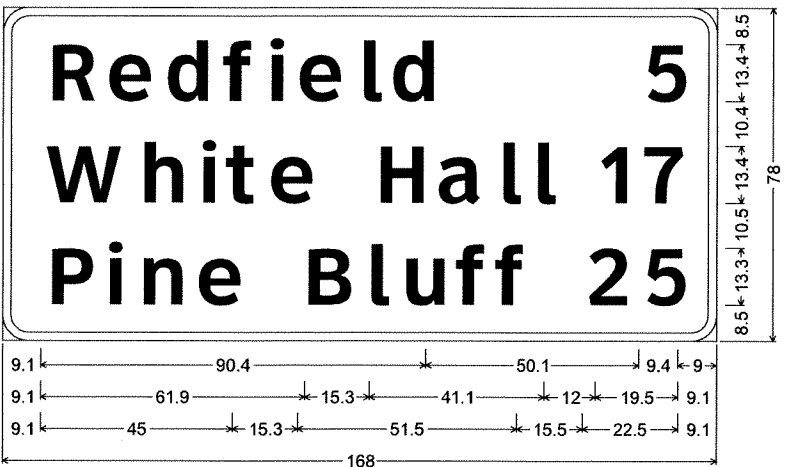
ML-530-10+90SB;
3.0" Radius, 1.0" Border, White on Green;
[Grant Co] ClearviewHwy-5-W;



ML-530-5+00NB;
6.0" Radius, 2.0" Border, White on Green;
[EXIT] ClearviewHwy-5-W; [15] ClearviewHwy-5-W;
6.0" Radius, 2.0" Border, White on Green;
[Hensley] ClearviewHwy-5-W; [1 MILE] ClearviewHwy-5-W;



ML-530-58+00NB;
6.0" Radius, 2.0" Border, White on Green;
[EXIT] ClearviewHwy-5-W; [15] ClearviewHwy-5-W;
6.0" Radius, 2.0" Border, White on Green;
[Hensley] ClearviewHwy-5-W; Standard Arrow Custom 35.8" X 21.6" 45°;

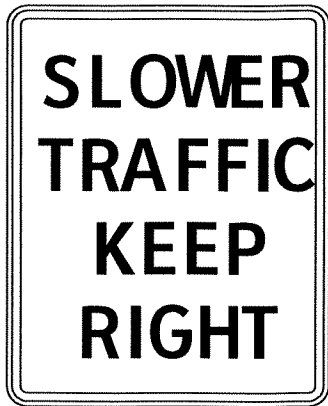


ML-530-35+00SB; 6.0" Radius, 2.0" Border, White on Green;
[Redfield] ClearviewHwy-5-W; [5] ClearviewHwy-5-W;
[White Hall] ClearviewHwy-5-W; [17] ClearviewHwy-5-W;
[Pine Bluff] ClearviewHwy-5-W; [25] ClearviewHwy-5-W;

2 SIGN LAYOUT SHEET
GUIDE SIGN - ROADSIDE MOUNTED
(DEMOUNTABLE LEGEND)

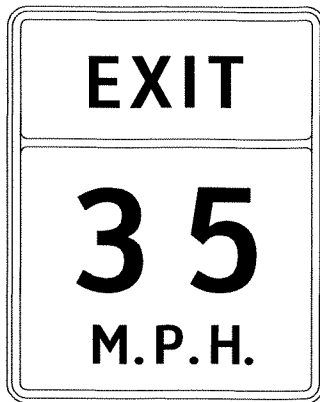


SS-530-48+00SB
SS-530-105+00NB
SS-530-225+00SB
SS-530-275+00NB
SS-530-305+00SB
SS-530-371+00SB
SS-530-373+00NB
G-2



R4-3
48"X60"

SS-530-62+00NB
SS-530-89+00SB
SS-530-232+90NB
SS-530-267+10SB
SS-530-389+50NB
SS-530-415+00SB
G-2



W13-3
48"X60"

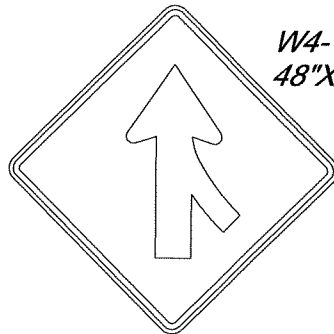
SS-530-100+00NB
SS-530-270+00NB
SS-530-370+00NB
G-1



M3-1
30"X15"

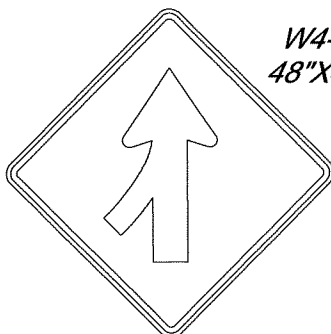
M1-1
45"X36"

SS-530-73+00NB
SS-530-80+00NB
SS-530-249+00SB
SS-530-253+00NB
SS-530-333+00SB
SS-530-396+00SB
SS-530-405+00NB
G-2



W4-1R
48"X48"

SS-530-343+00NB
G-2



W4-1L
48"X48"

SS-530-52+00SB
SS-530-231+00SB
SS-530-310+00SB
SS-530-376+00SB
G-1



M3-3
30"X15"

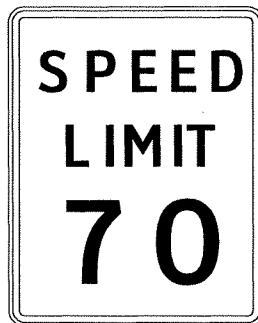
M1-1
45"X36"

SS-530-52+00NB
SS-530-66+00SB
SS-530-168+00NB
SS-530-183+00SB
SS-530-280+00NB
SS-530-293+00SB
SS-530-381+00NB
SS-530-394+00SB
G-2

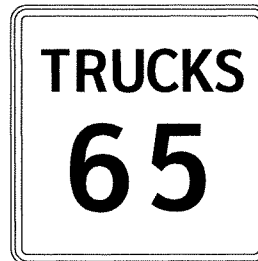


W8-13
48"X48"

SS-530-43+00SB
SS-530-110+00NB
SS-530-219+00SB
SS-530-285+00NB
SS-530-300+00SB
SS-530-366+00SB
SS-530-378+00NB
G-2



R2-1
48"X60"



R2-2
48"X48"

SS-530-59+45SB-A
SS-530-59+45SB-B
SS-530-58+55NB-A
SS-530-58+55NB-B
SS-530-176+66SB-A
SS-530-176+66SB-B
SS-530-175+16NB-A
SS-530-175+16NB-B
SS-530-285+69NB-A
SS-530-285+69NB-B
SS-530-286+56SB-A
SS-530-286+56SB-B
SS-530-385+74NB-A
SS-530-385+74NB-B
SS-530-387+64SB-A
SS-530-387+64SB-B
U1-1



OM-3L
12"X36"



OM-3R
12"X36"

SS-530-400+00NB
SS-530-400+00SB
U1-1



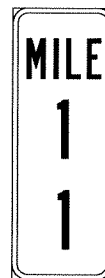
D10-2
10"X18"

SS-530-345+00NB
SS-530-345+00SB
U1-1



D10-2
10"X27"

SS-530-288+50SB
SS-530-288+50NB
U1-1



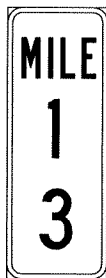
D10-2
10"X27"

SS-530-239+60SB
SS-530-239+60NB
U1-1



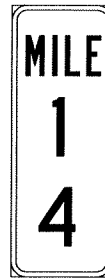
D10-2
10"X27"

SS-530-186+80SB
SS-530-186+80NB
U1-1



D10-2
10"X27"

SS-530-134+00SB
SS-530-134+00NB
U1-1



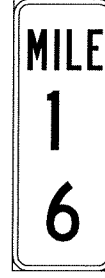
D10-2
10"X27"

SS-530-81+20SB
SS-530-81+20NB
U1-1



D10-2
10"X27"

SS-530-28+40SB
SS-530-28+40NB
U1-1

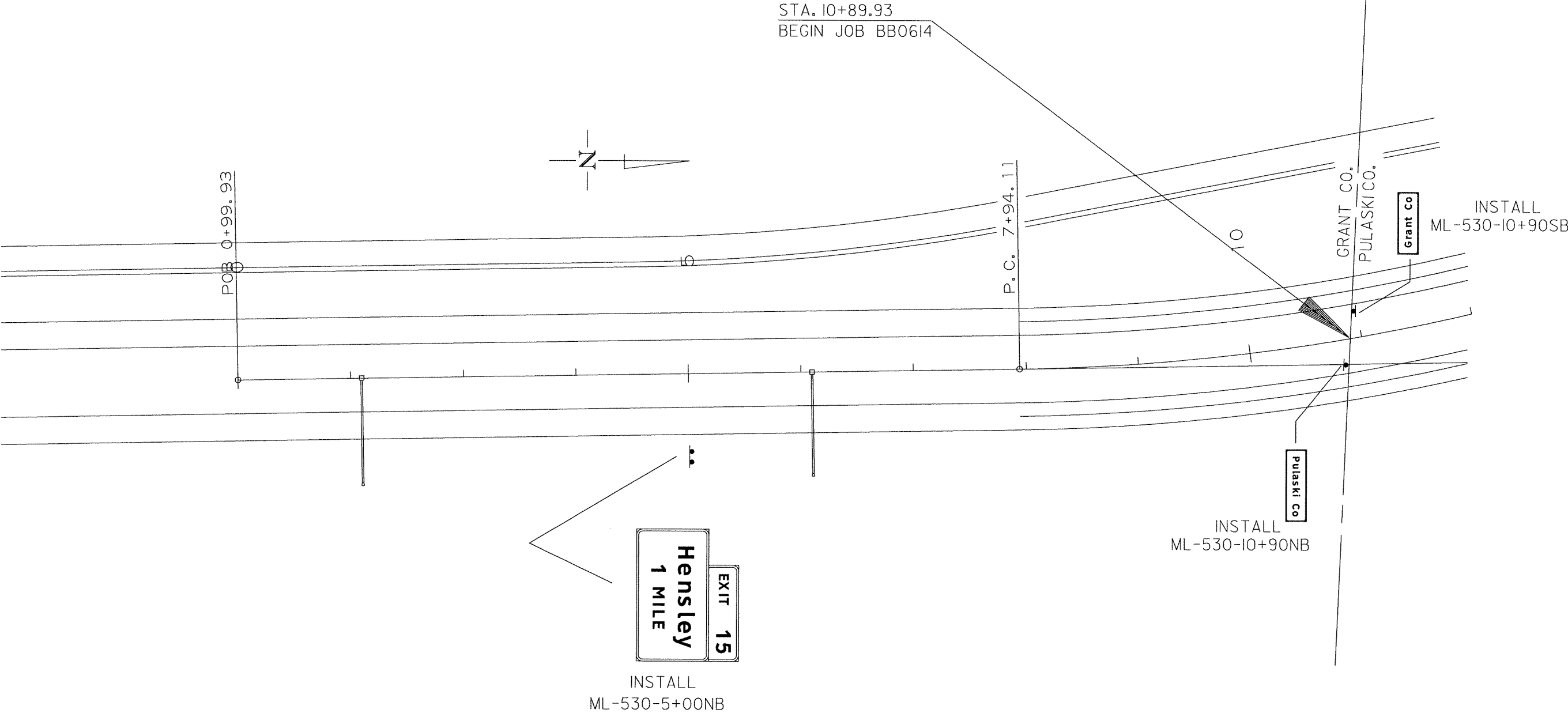


D10-2
10"X27"



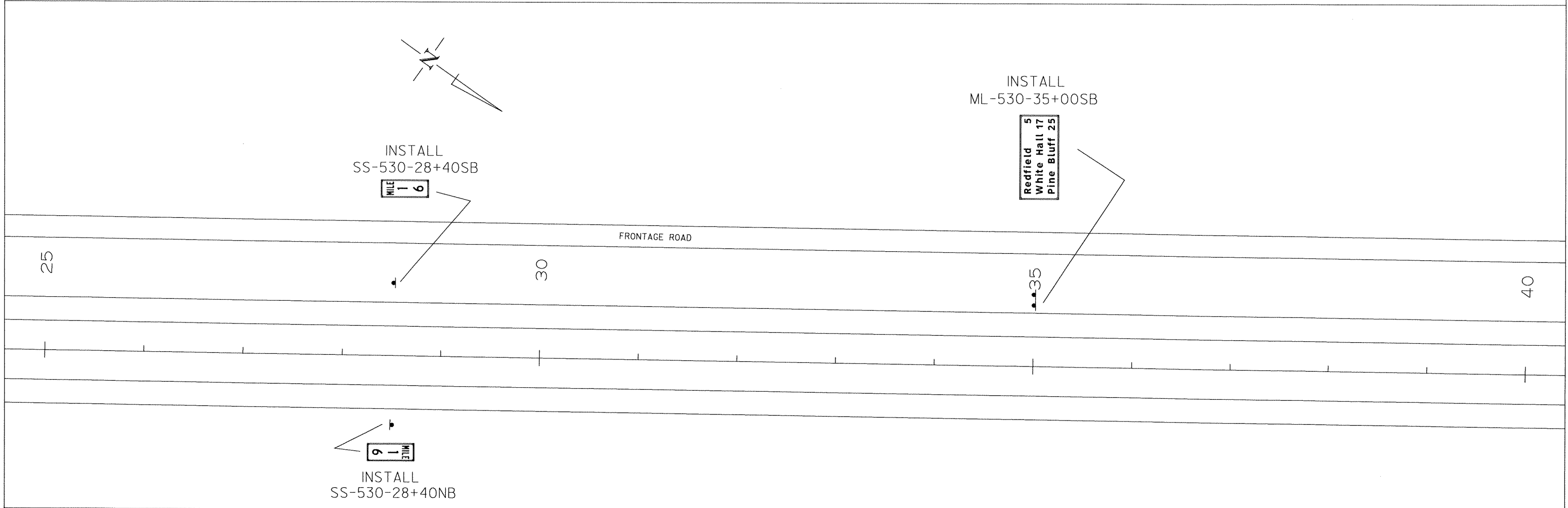
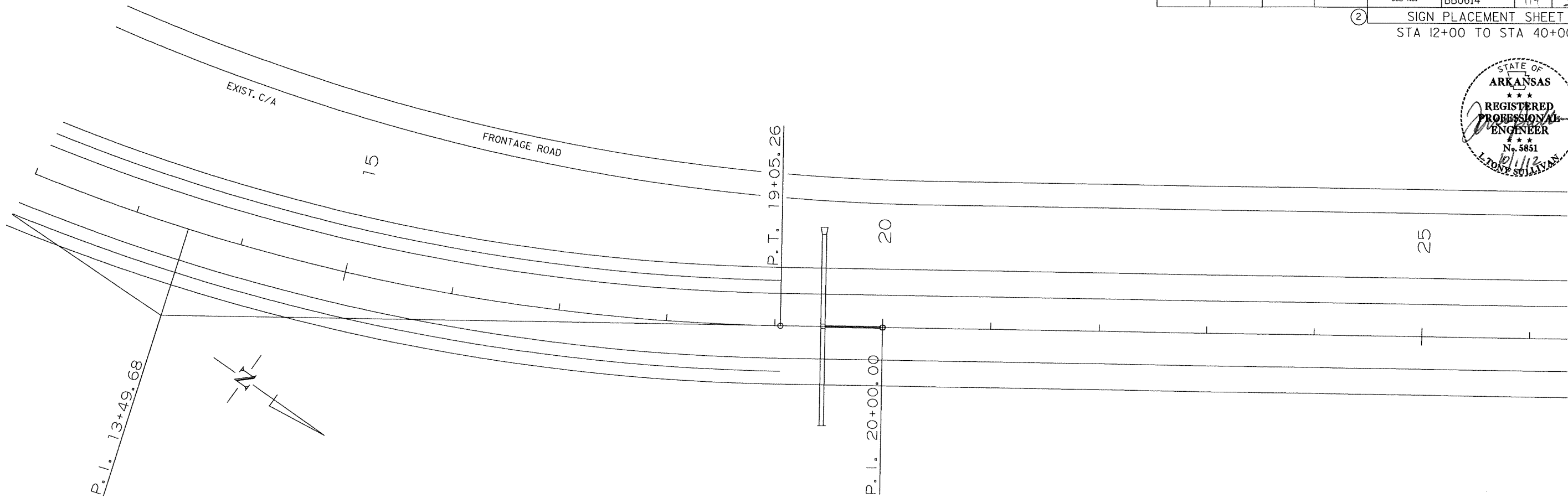
DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB NO.		BB0614	118	219

② SIGN PLACEMENT SHEET
STA 0+00 TO STA 12+00



DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			

② SIGN PLACEMENT SHEET
STA 12+00 TO STA 40+00



DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB NO. BB0614		120	219	

② SIGN PLACEMENT SHEET
STA 40+00 TO STA 70+00



INSTALL
SS-530-43+00SB



INSTALL
SS-530-48+00SB



INSTALL
SS-530-52+00SB



INSTALL
SS-530-52+00NB



INSTALL
SS-530-66+00SB



NOTE: BRIDGE NO. 3323A AND 3323B ARE
EXCEPTIONS TO JOB BB0614

BR. END 58+55.00

SS-530-59+45SB-A
INSTALL

BR. END 59+45.00
EXIST. C/A

300' TAPER

FRONTAGE ROAD

700' ACCELERATION LANE

FRONTAGE ROAD

INSTALL
SS-530-58+55NB-B

SS-530-59+45SB-B
INSTALL

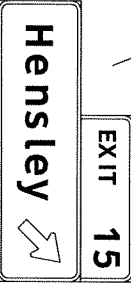
BR. END 58+55.00

BR. NO.
3323B

BR. END 59+45.00

INSTALL
SS-530-58+55NB-A

INSTALL
ML-530-58+00NB



CAMPBELL

INSTALL
SS-530-62+00NB

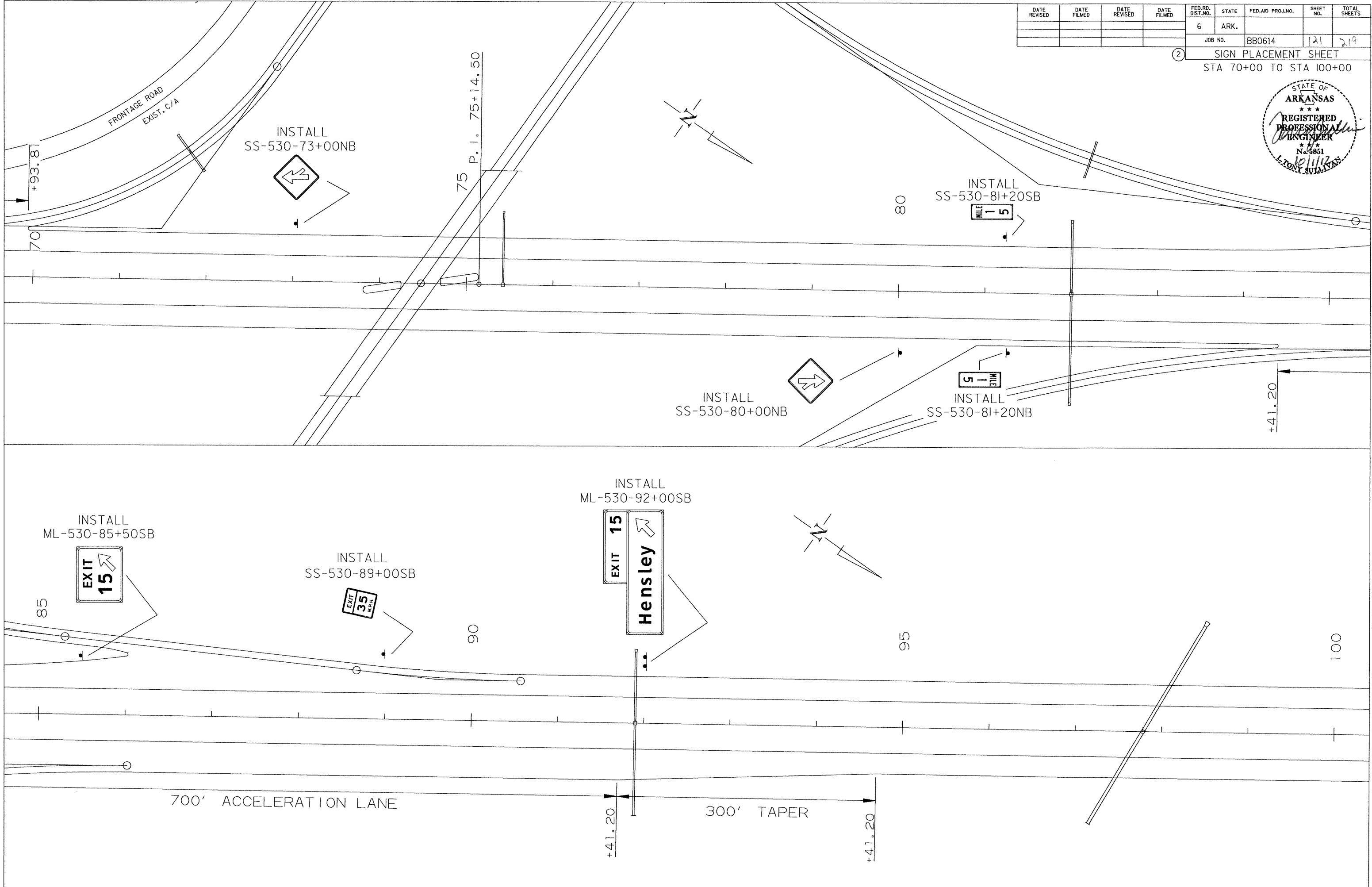


INSTALL
ML-530-65+00NB



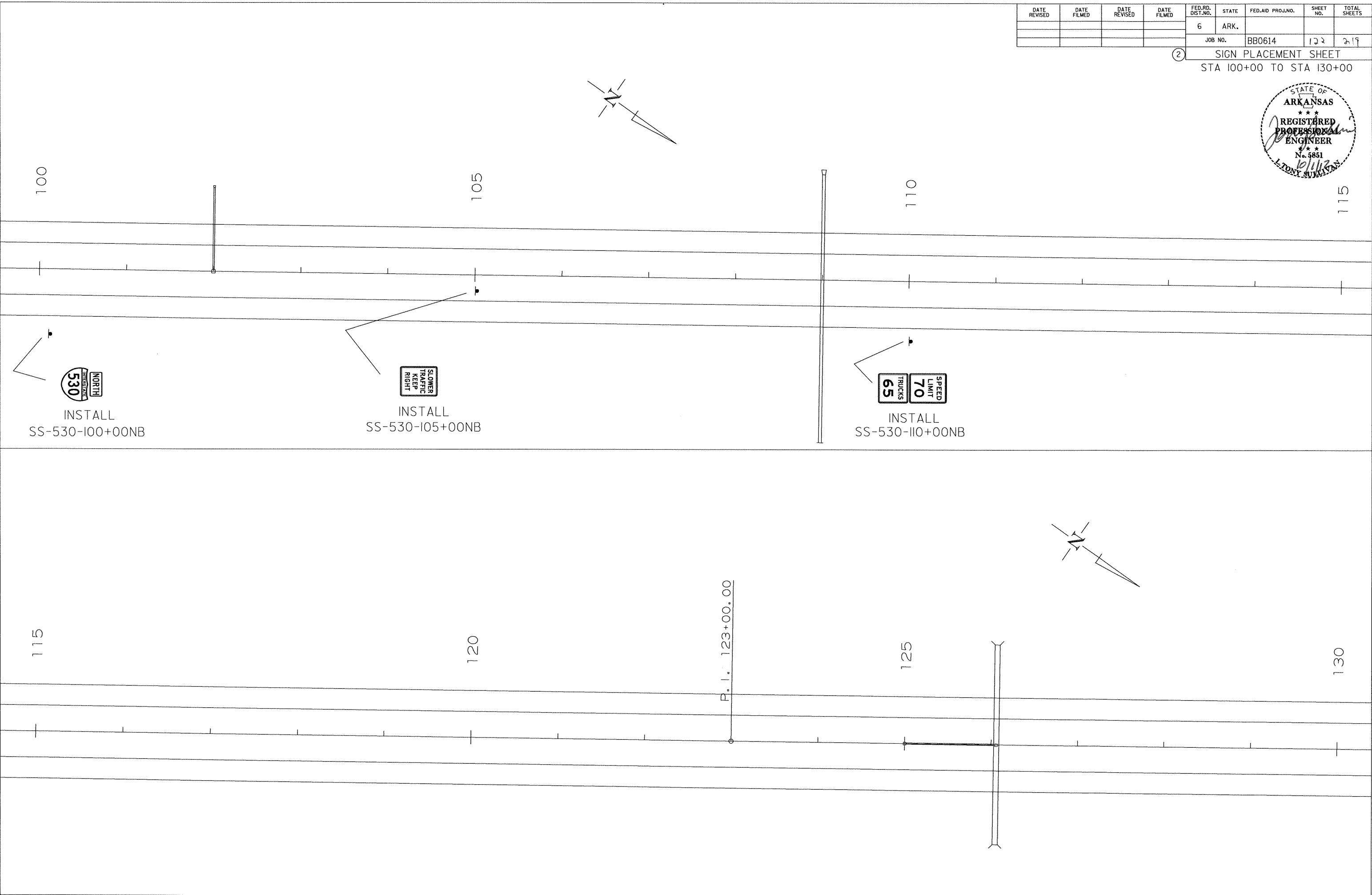
DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
						JOB NO. BB0614	121	219

② SIGN PLACEMENT SHEET
STA 70+00 TO STA 100+00



DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB NO. BB0614		122	219	

② SIGN PLACEMENT SHEET
STA 100+00 TO STA 130+00

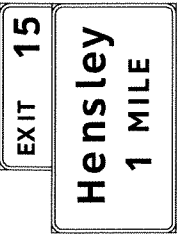


DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
						JOB NO. BB0614	123	219

② SIGN PLACEMENT SHEET
STA 130+00 TO STA 160+00



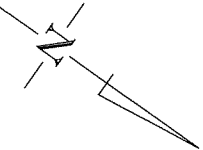
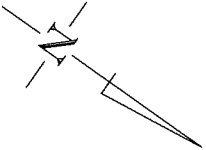
INSTALL
ML-530-144+00NB



INSTALL
SS-530-134+00SB



INSTALL
SS-530-134+00NB



130

135

140

145

145

150

155

160

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
						JOB NO. BB0614	124	219

② SIGN PLACEMENT SHEET
STA 160+00 TO STA 190+00



BR. END 175+16.67

BR. END 175+14.97

INSTALL
SS-530-168+00NB

INSTALL
SS-530-183+00SB

INSTALL
SS-530-186+80SB

MILE 1 3

MILE 1 3

INSTALL
SS-530-186+80NB

INSTALL
ML-530-179+00NB

EXIT 12
Woodson
1 MILE

NOTE: BRIDGE NO. 3324A IS AN
EXCEPTION TO JOB BB0614

INSTALL
SS-530-176+66SB-A
SS-530-176+66SB-B

BR. END 176+66.67

BR. END 176+74.97

INSTALL
SS-530-175+16NB-A
SS-530-175+16NB-B

CLEAR

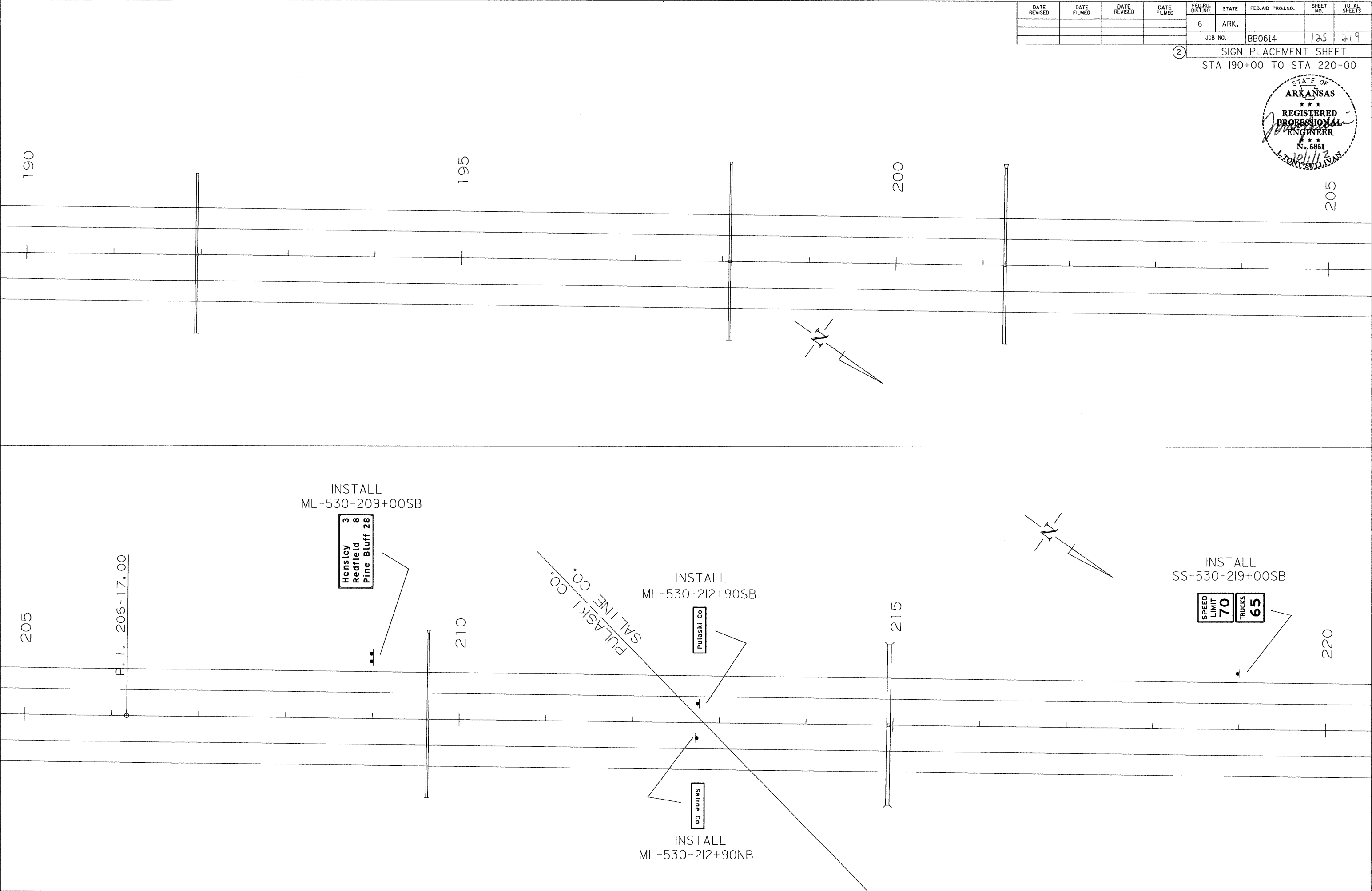
CREEK

BR. NO. 3324A

BR. NO. 3324B

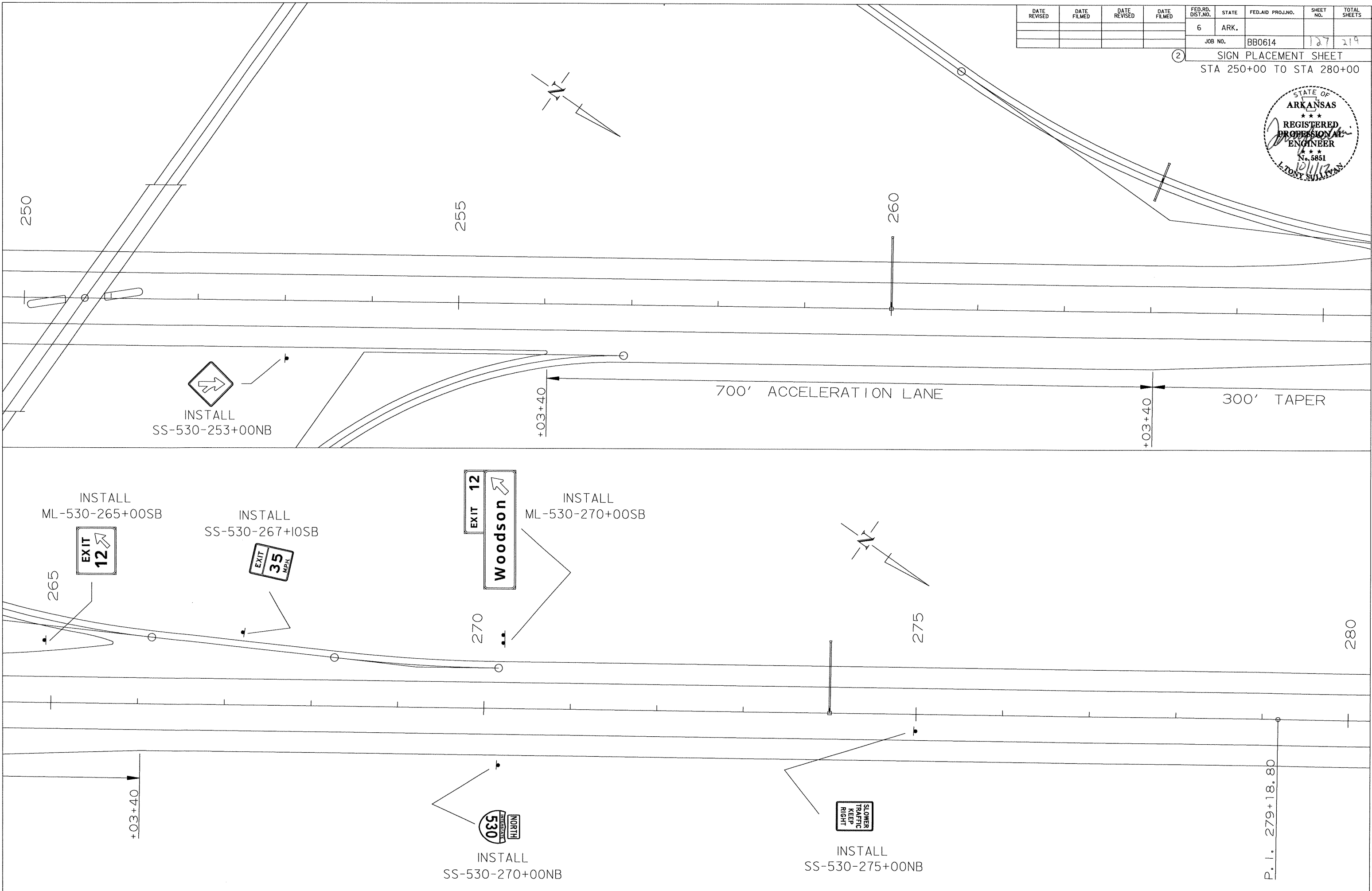
DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB NO.		BB0614	125	219

2 SIGN PLACEMENT SHEET
STA 190+00 TO STA 220+00



DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
						JOB NO.	BB0614	127 219

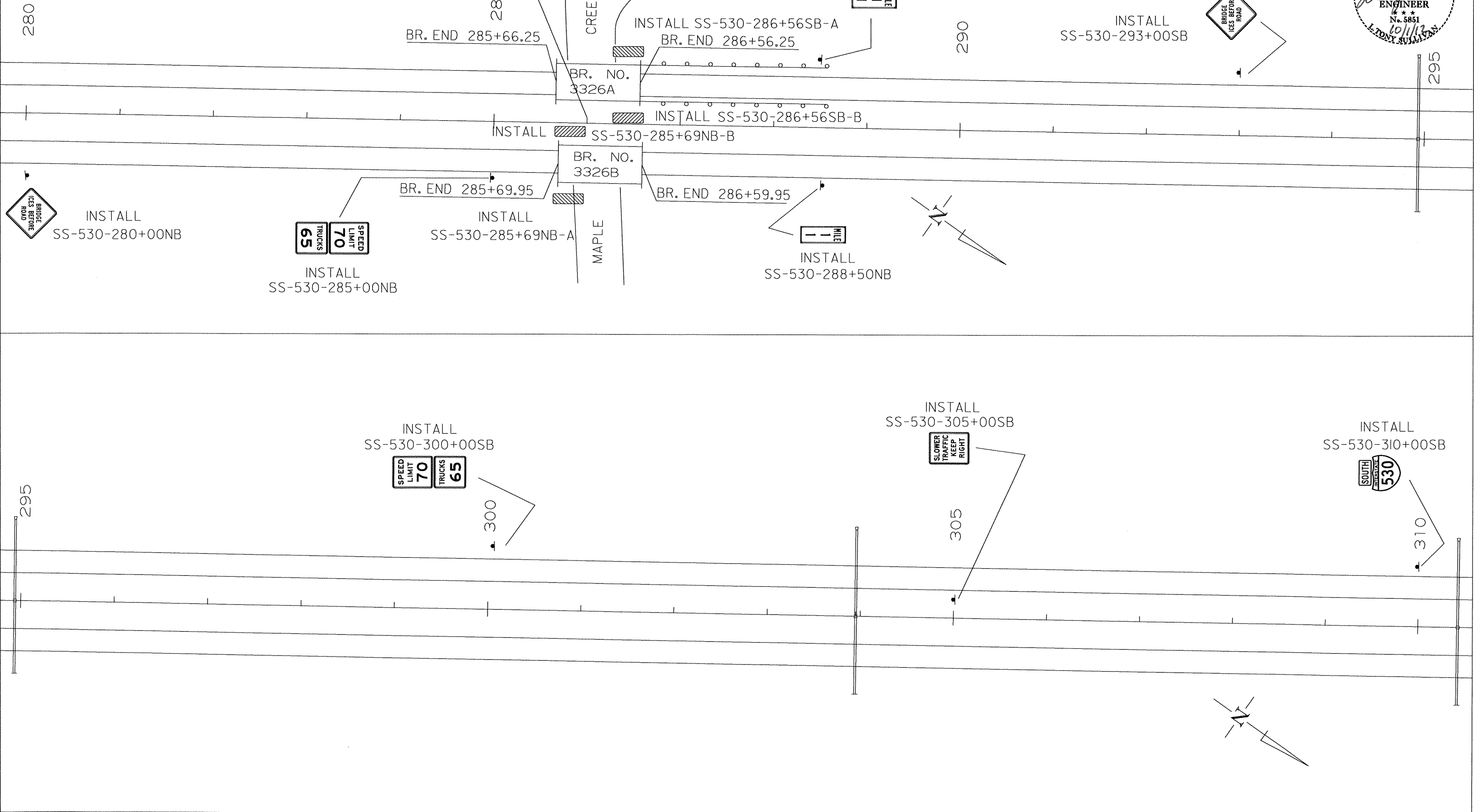
② SIGN PLACEMENT SHEET
STA 250+00 TO STA 280+00



NOTE: BRIDGE NO. 3326A & 3326B ARE EXCEPTIONS TO JOB BB0614

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
						JOB NO. BB0614	128	219

② SIGN PLACEMENT SHEET
STA 280+00 TO STA 310+00



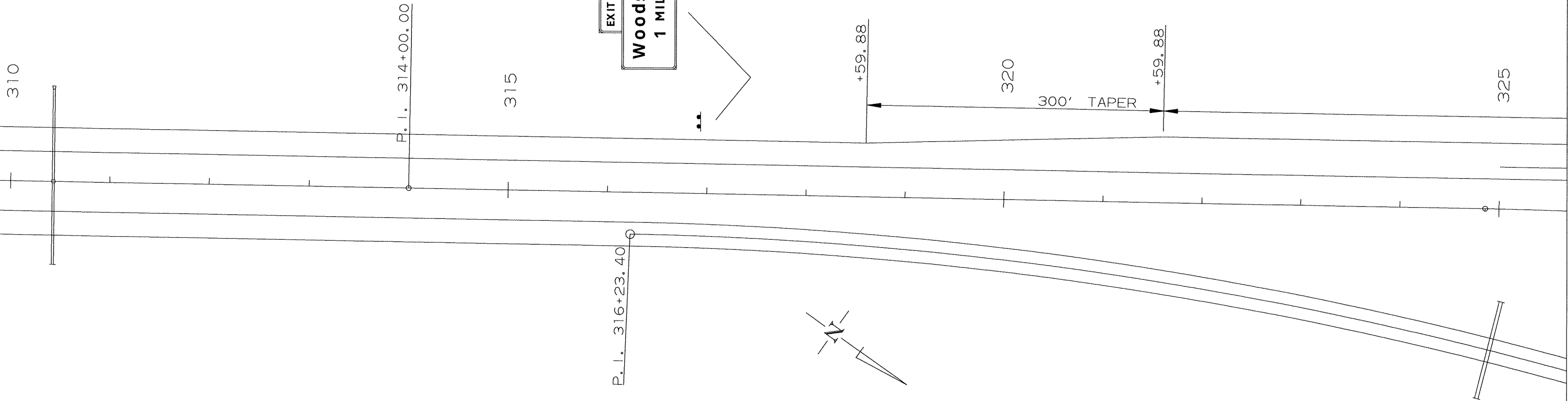
DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB NO.		BB0614	229	219

② SIGN PLACEMENT SHEET
STA 310+00 TO STA 325+00



INSTALL
ML-530-317+00SB

EXIT 12
Woodson
1 MILE



DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
						JOB NO. BB0614	130	219

2 SIGN PLACEMENT SHEET
STA 325+00 TO STA 340+00



INSTALL
SS-530-333+00SB



335

340

325

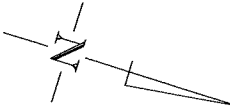
700' ACCELERATION LANE

+59.88

330

PC 327+67.95

PC 324+86.07



EXIT 9
Bingham
Road
1 MILE

INSTALL
ML-530-332+20NB

PT 333+41.08

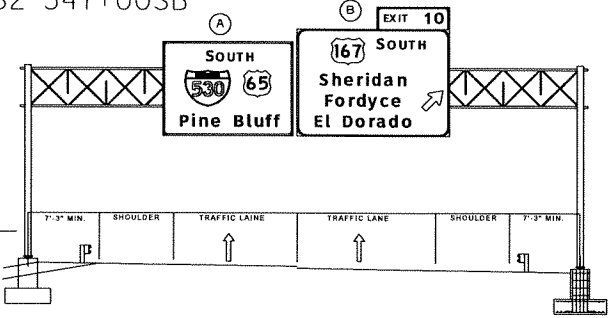
P.I. 335+51.68

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB NO.	BB0614		131	219

② SIGN PLACEMENT SHEET
STA 340+00 TO STA 355+00



MAINTAIN OVERHEAD SIGNS AND SIGN STRUCTURE
OH-530-62-347+00SB



INSTALL
ML-530-343+00SB



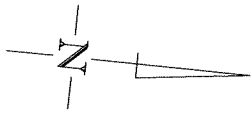
INSTALL
SS-530-345+00SB



345

PT 343+35.01

P.T. 346+17.23



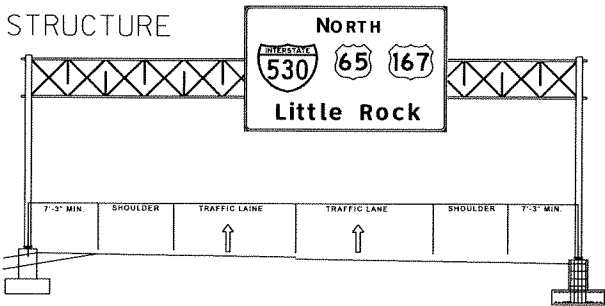
INSTALL
SS-530-343+00NB



INSTALL
SS-530-345+00NB



REMOVE OVERHEAD SIGNS AND SIGN STRUCTURE
OH-530-62-353+50NB



INSTALL
LG-530-355+00NB

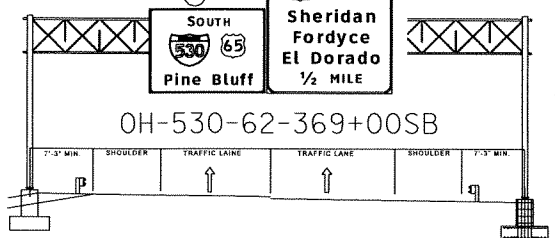


DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB NO.	BB0614		132	216

SIGN PLACEMENT SHEET
STA 355+00 TO STA 385+00

REMOVE OVERHEAD SIGNS,
SIGN STRUCTURE & GUARDRAIL

INSTALL
SS-530-366+00SB



OH-530-62-369+00SB

INSTALL
ML-530-364+00NB



INSTALL
SS-530-370+00NB



INSTALL
SS-530-371+00SB



INSTALL
SS-530-376+00SB



380

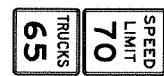
300' TAPER

BR. END 385+74.94

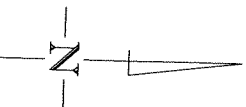
INSTALL
SS-530-373+00NB



INSTALL
SS-530-378+00NB



INSTALL
SS-530-381+00NB



BR. END 385+74.94

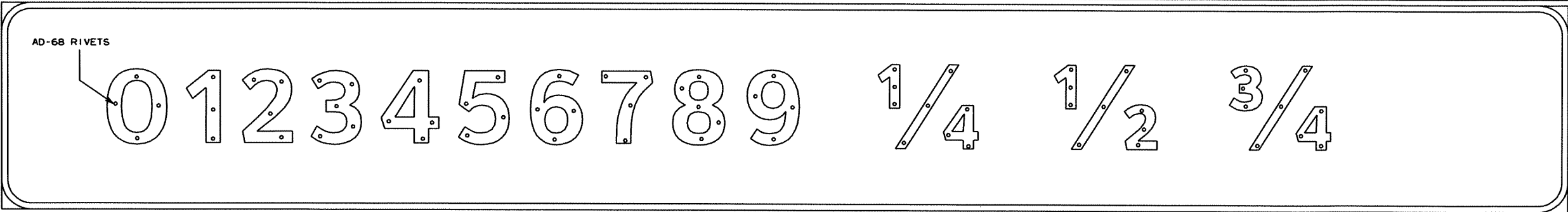
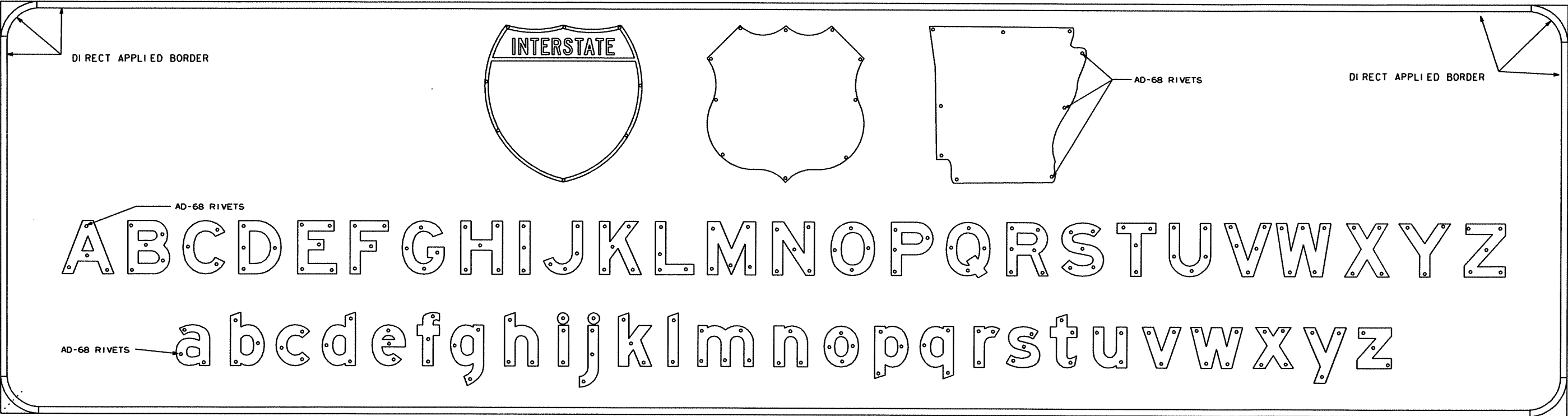


SIGN PLACEMENT SHEET

STA 385+00 TO STA 415+00



THE CONTRACTOR SHALL DRILL AND POP-RIVET LEGEND, SHIELDS, ARROWS, OR OTHER COPY AS SHOWN.

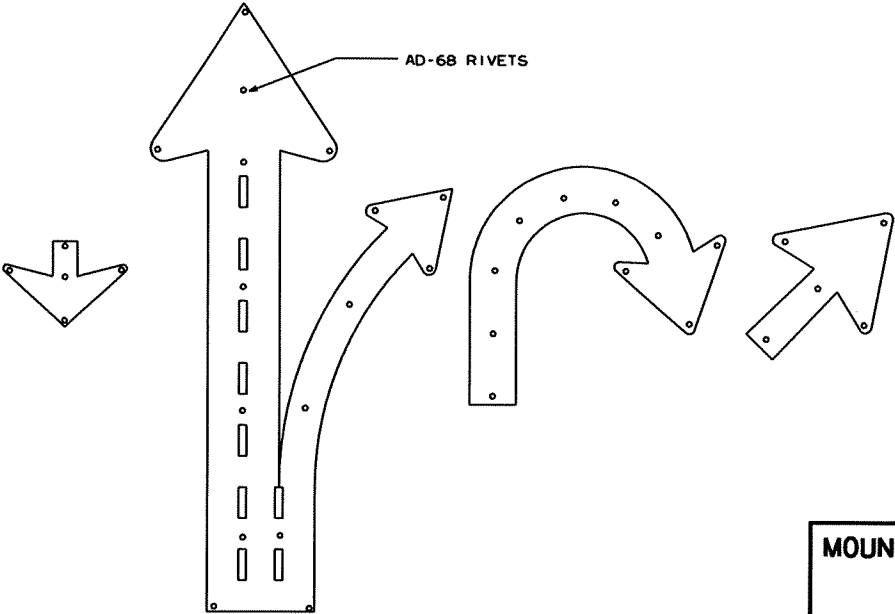


NOTE:

LEGEND ON GUIDE SIGNS ON THE MAIN LANES SHALL BE DEMOUNTABLE LEGEND.
LEGEND ON GUIDE SIGNS ON CROSS ROADS AND RAMPS SHALL BE DIRECT APPLIED.
THE DEMOUNTABLE AND DIRECT APPLIED LEGENDS SHALL BE TYPE IX SHEETING.

THE BACKGROUND ON ALL GUIDE SIGNS AND STANDARD SIGNS SHALL BE CONSTRUCTED USING TYPE III SHEETING.
TYPE IX SHEETING FOR BORDER, LEGEND, SHIELDS, ARROWS, OR OTHER COPY SHALL BE ORIENTED VERTICALLY AS PER MANUFACTURERS' DATUM MARKS, ORIENTATION MARKS, OR OTHER RECOMMENDATIONS.

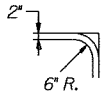
SIGN LEGEND, SHIELDS, ARROWS OR OTHER COPY SHALL BE APPLIED WITH RIVETS ONLY.
NO OTHER METHOD OF APPLYING CHARACTERS IS ALLOWED.



MOUNTING DETAILS FOR DEMOUNTABLE
LEGEND ON GUIDE SIGNS

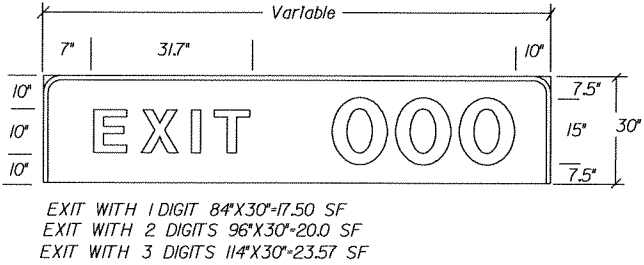
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				6	ARK.			
						JOB NO.	BB0614	135 219

2 EXIT PANEL DETAILS

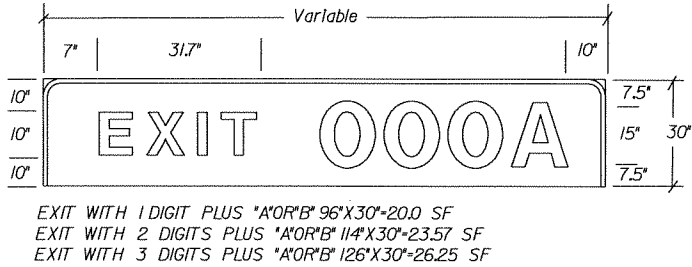


TYPICAL DETAIL

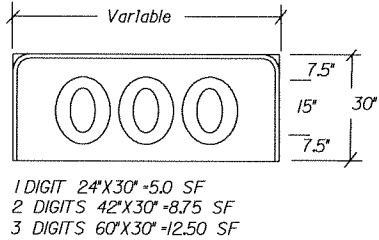
TYPE A



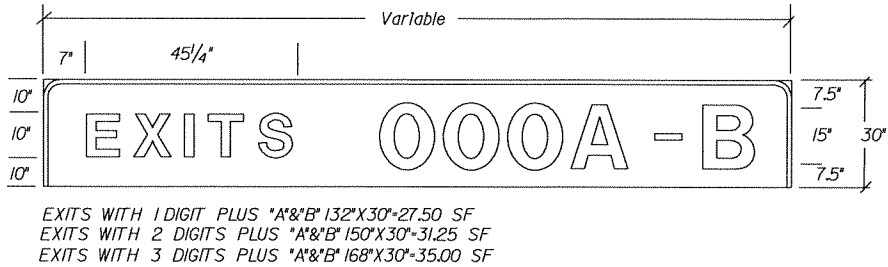
TYPE B



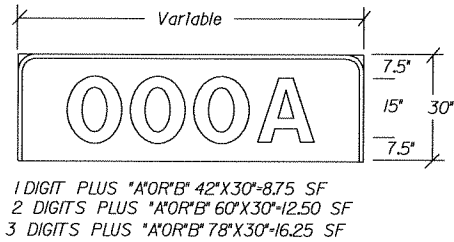
TYPE D

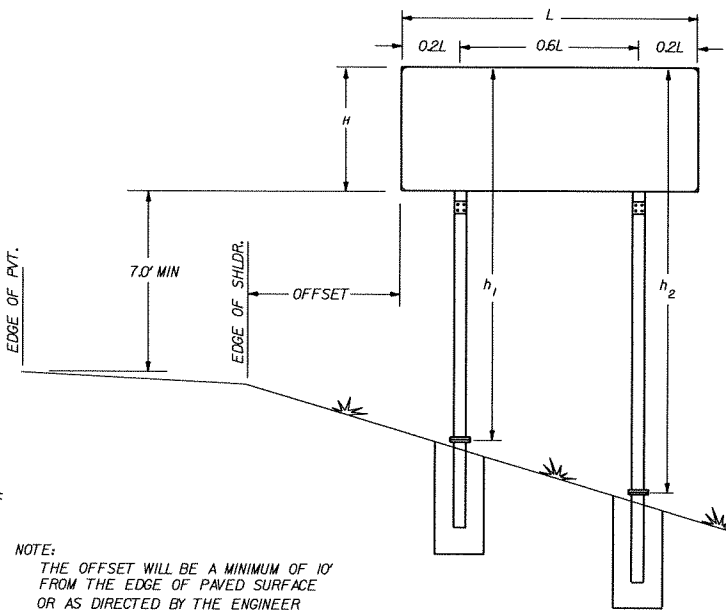
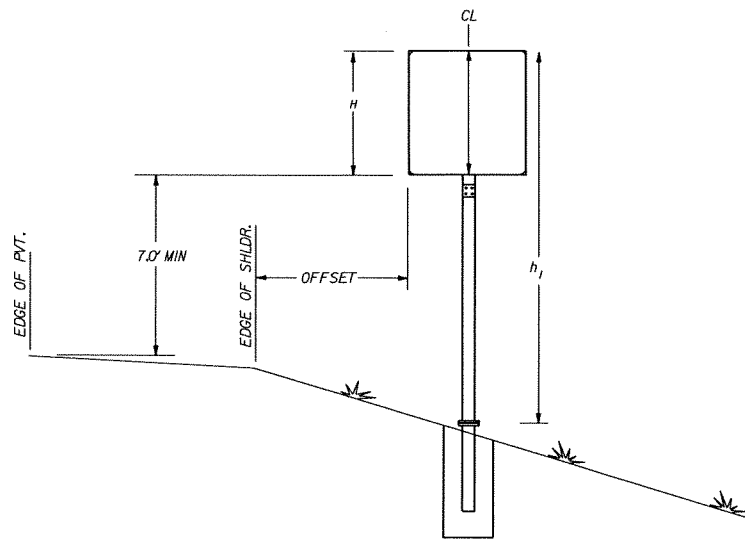
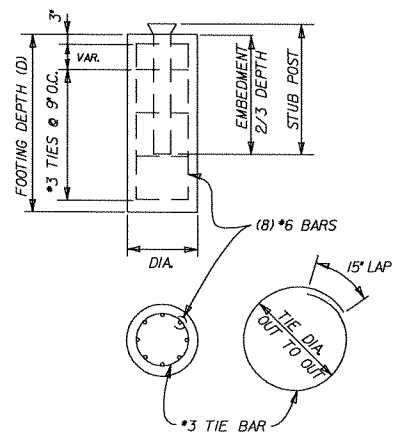


TYPE C

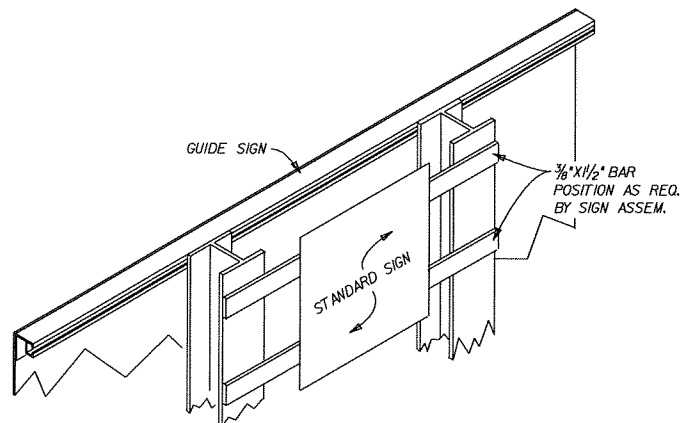


TYPE E

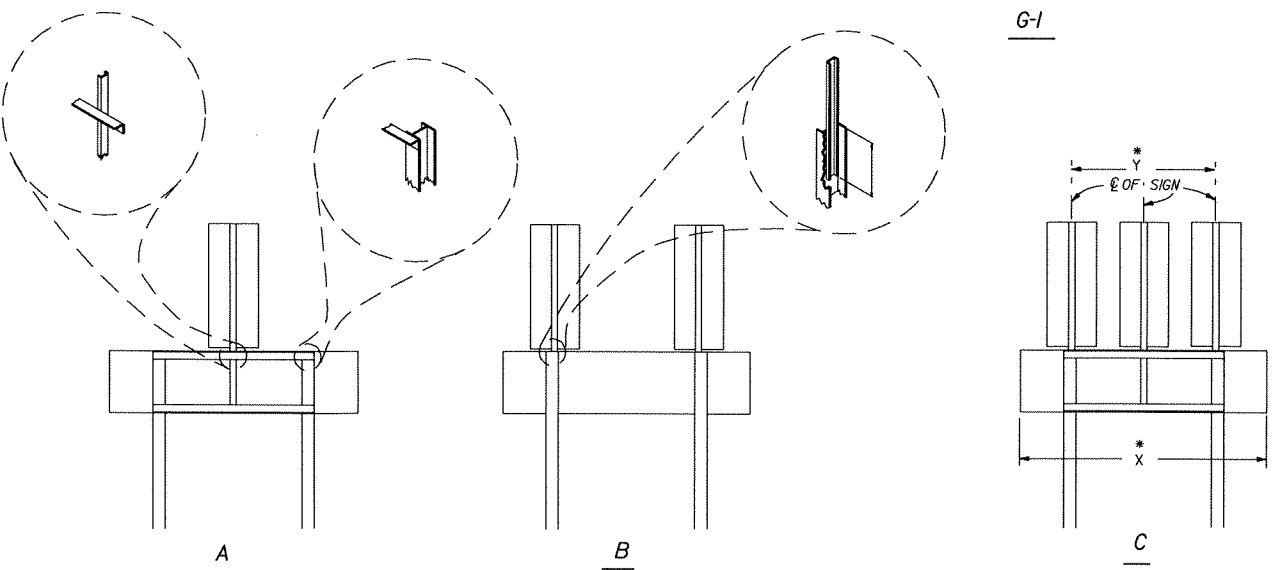




NOTE:
THE OFFSET WILL BE A MINIMUM OF 10'
FROM THE EDGE OF PAVED SURFACE
OR AS DIRECTED BY THE ENGINEER



SECONDARY SIGN INSTALLATION
ON BACKSIDE OF GUIDE SIGN



TRIPLE ASSEM.(C or D)

* Y	* X	TYPE ASSEM.
4'-4"	≥ 7'-6"	G-1C
4'-4"	< 7'-6"	G-1D
4'-10"	≥ 8'-0"	G-1C
4'-10"	< 8'-0"	G-1D
5'-4"	≥ 8'-6"	G-1C
5'-4"	< 8'-6"	G-1D
5'-10"	≥ 9'-0"	G-1C
5'-10"	< 9'-0"	G-1D

LEGEND
≥ GREATER THAN OR EQUAL TO
< LESS THAN

FOOTING QUANTITIES

FTG. DEPTH	NO. OF TIES	STUB PROJ.	18" DIA.		24" DIA.		30" DIA.		36" DIA.		"A"
			CU.YD.	* STEEL	CU.YD.	* STEEL	CU.YD.	* STEEL	CU.YD.	* STEEL	EMBED.
2'-6"	4	4"	0.06	30.5							7'-8"
3'-0"	4	4"	0.20	36.6							2'-0"
3'-6"	5	4"	0.23	44.2							2'-4"
4'-0"	6	4"	0.26	51.8	0.46	55.4					2'-8"
4'-6"	7	4"	0.29	59.5	0.52	63.6					3'-0"
5'-0"	7	4"	0.33	65.5	0.58	69.6	0.91	73.8			3'-4"
5'-6"	8	4"		0.64	77.9	1.00	82.6				3'-8"
6'-0"	9	4"		0.70	86.7	1.09	91.5	1.57	96.5		4'-0"
6'-6"	9	4"				1.18	97.5	1.70	102.6		4'-4"
7'-0"	10	4"				1.27	106.3	1.83	111.9		4'-8"
7'-6"	11	4"						1.96	121.3		5'-0"
8'-0"	11	4"						2.09	127.3		5'-4"

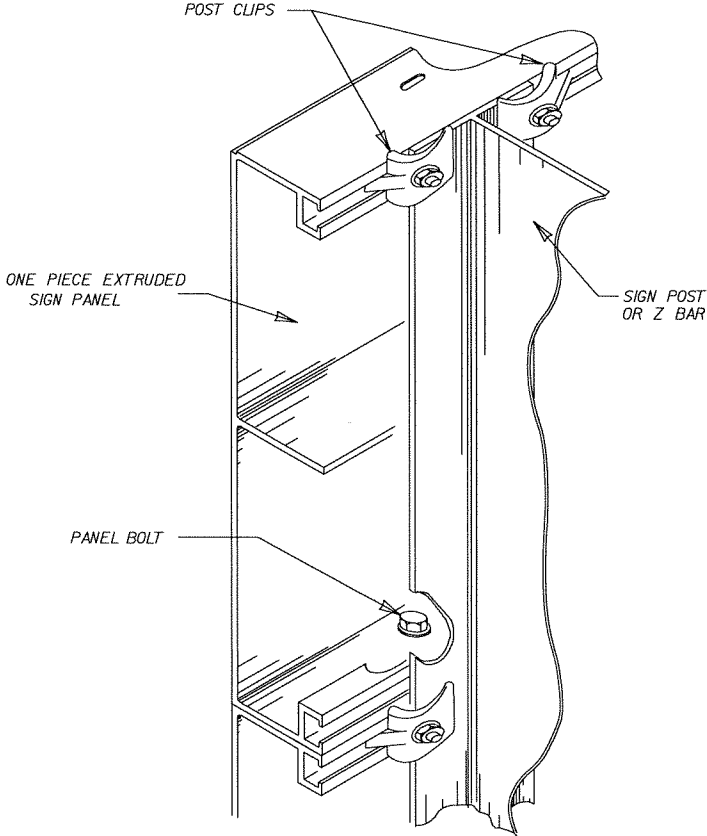
REINFORCING STEEL SCHEDULE

*3 TIE BARS				(8) #6 BARS				NO. TIES E.A.
FTG. DIA. IN.	DIA. IN.	LGTH.-E.A. FT./IN.	WEIGHT LBS.	FTG. DEPTH FT./IN.	LGTH.-E.A. FT./IN.	WEIGHT LBS.		
18"	12"	4'-4"	1629	2'-6"	2'-0"	24.03		4
				3'-0"	2'-6"	30.04		4
24"	18"	5'-11"	2225	3'-6"	3'-0"	36.05		5
				4'-0"	3'-6"	42.06		6
30"	24"	7'-6"	2820	4'-6"	4'-0"	48.06		7
				5'-0"	4'-6"	54.07		7
36"	30"	9'-0"	3384	5'-6"	5'-0"	60.08		8
				6'-0"	5'-6"	66.09		9
				6'-6"	6'-0"	72.10		9
				7'-0"	6'-6"	78.10		10
				7'-6"	7'-0"	84.11		11
				8'-0"	7'-6"	90.12		11



DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
						JOB NO. B80614	137	219

2 DETAILS OF GUIDE SIGN PANELS



SLOTTED HOLES (7/16" X 7/8")
DRILLED OR PUNCHED @ 12" O.C.
BEGINNING 6" FROM ONE END

SIGN POST
OR Z BAR

ALUMINUM PANEL BOLT
AND HEX NUT (3/8"-16X3/4")
AND (2) FLAT WASHERS
2" MAX SPACING

ALL MOUNTING HARDWARE
SHALL COMPLY WITH THE
MATERIALS SECTION OF 724
OF THE STANDARD SPECIFICATIONS
UNLESS OTHERWISE SPECIFIED.
ALUMINUM POST CLIP
(ASTM B108 ALLOY 356-T6)
OR ASTM B26 ALLOY 356-T6)
ALUMINUM POST CLIP BOLT
AND FLAT WASHER (3/8"-16X1 3/4")
ALUMINUM STOP NUT

MOUNTING HARDWARE

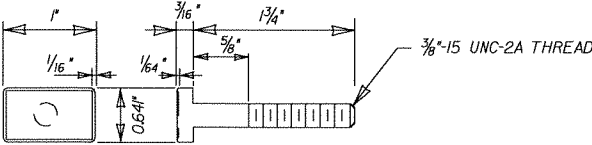
USE DOUBLE POST CLIPS
AT TOP AND BOTTOM OF SIGN

ALTERNATE POST CLIPS

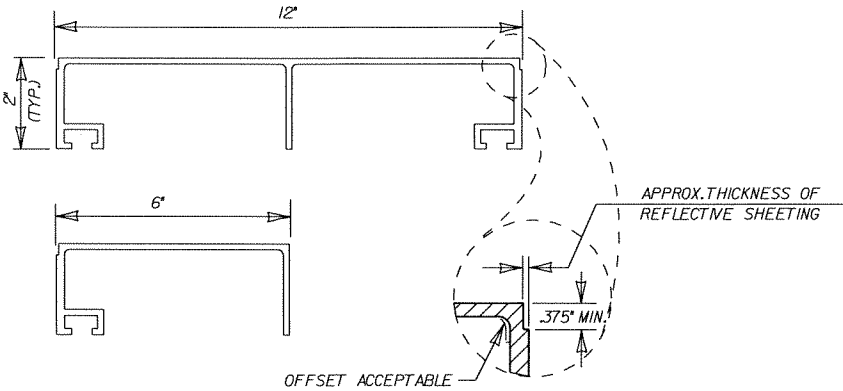
SIGN PANEL

SIGN POST

POST CLIP PLACEMENT

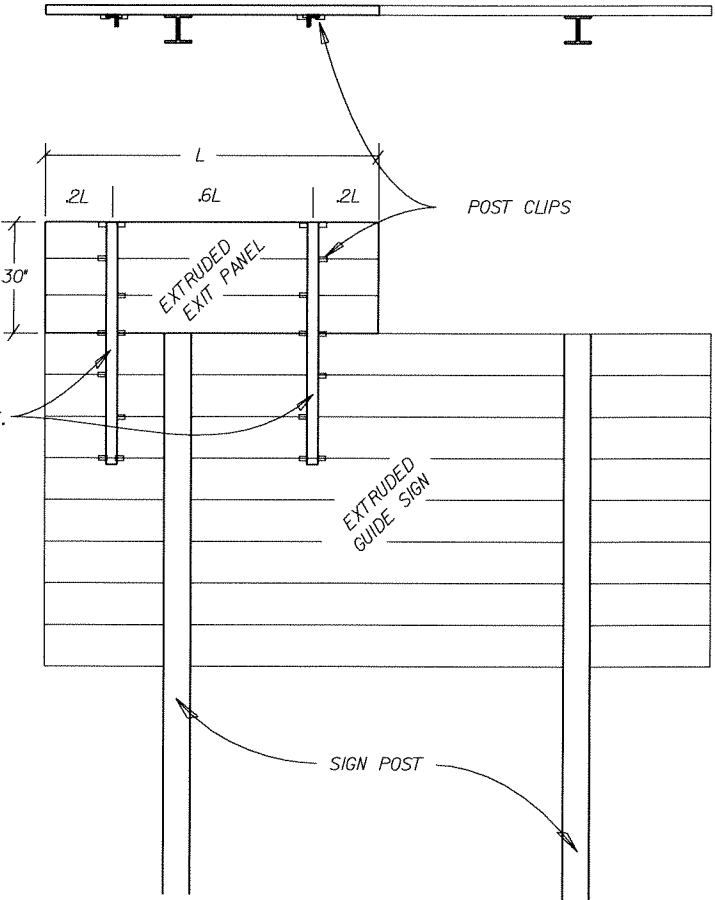


POST CLIP BOLT

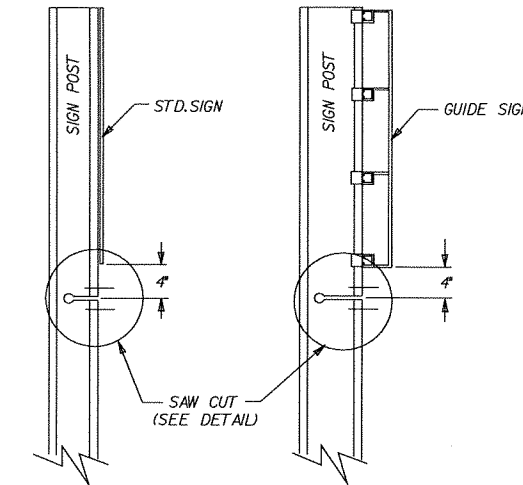


ONE PIECE EXTRUDED
SIGN PANELS

NOTE: EXIT NUMBER PANELS SHALL HAVE WHITE LEGENDS AND BORDERS. THE BACKGROUND COLOR WILL BE AS USE SPECIFIES. SHEETING TYPE WILL BE THE SAME AS THE GUIDE SIGN WHICH THE EXIT PANEL IS ATTACHED OR AS SPECIFIED IN THE PLANS. PAYMENT FOR ALL POST CLIPS, BOLTS, AND ANGLES SHALL BE SUBSIDIARY TO THE ITEM "EXIT NUMBER PANEL".



NOTE: BOLT HOLES, USED IN THE MOUNTING OF STANDARD SIGNS SHALL BE LOCATED IN THE FLANGE ADJACENT TO THE NEAR EDGE OF PAVEMENT FOR SINGLE POST ASSEMBLIES AND IN THE OUTSIDE FLANGES FOR MULTIPLE POST ASSEMBLIES.

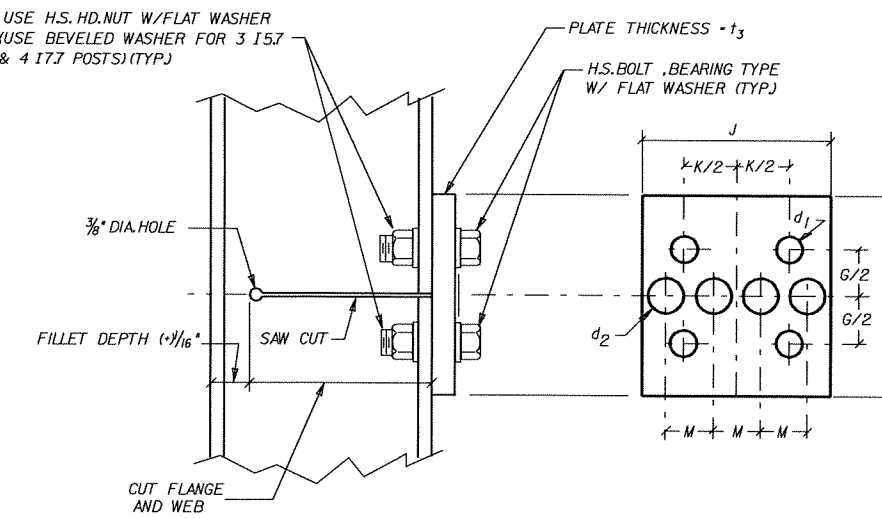


NOTE: POST SHALL BE SAW CUT AFTER GALVANIZING AND THE CUT SURFACE TREATED, AFTER PLATE IS INSTALLED AND ALL BOLTS FULLY TIGHTENED, WITH AN APPROVED ZINC SOLDER MEETING THE FEDERAL SPEC. 0-6-93 (STICK ONLY).

Standard Signs

Guide Signs

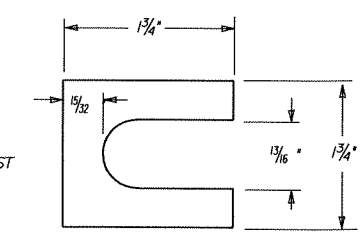
USE H.S. HD. NUT W/ FLAT WASHER (USE BEVELED WASHER FOR 3 15.7 & 4 17.7 POSTS) (TYP.)



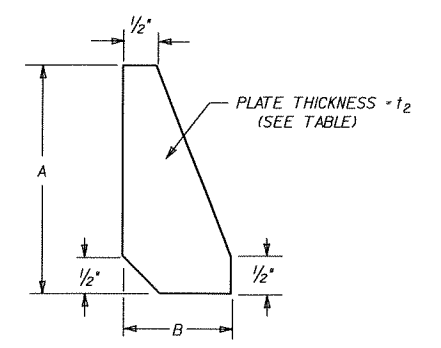
NOTE: USE H.S. HEX HEAD BOLTS, HEX HEAD NUTS AND BEVEL OR FLAT WASHERS (WHERE REQ.) UNDER NUTS. ALL HOLES SHALL BE DRILLED. ALL PLATE CUTS SHALL PREFERABLY BE SAW CUTS, HOWEVER FLAME CUTTING WILL BE PERMITTED PROVIDED ALL EDGES ARE GROUND. METAL PROJECTING BEYOND THE PLANE OF THE PLATE FACE WILL NOT BE PERMITTED. STEEL FUSE PLATES SHALL CONFORM TO THE REQUIREMENTS OF ASTM-A36, ASTM-A441, ASTM-572 GRADE 50, OR ASTM-A588 MAY BE SUBSTITUTED FOR A36 AT THE OPTION OF THE FABRICATOR. STEEL USED SHALL HAVE AN ULTIMATE TENSILE STRENGTH NOT TO EXCEED 80 KSI.

FUSE PLATE DETAILS

FURNISH 2 ~ .012" +/- THICK AND 2 ~ .032" +/- THICK SHIMS PER POST



SHIM DETAIL



STIFFENER PLATE DETAIL

GENERAL NOTES

TIGHTEN THE HIGH STRENGTH BOLTS IN THE BASE CONNECTION ONLY TO THE TORQUE SHOWN. DO NOT OVERTIGHTEN.

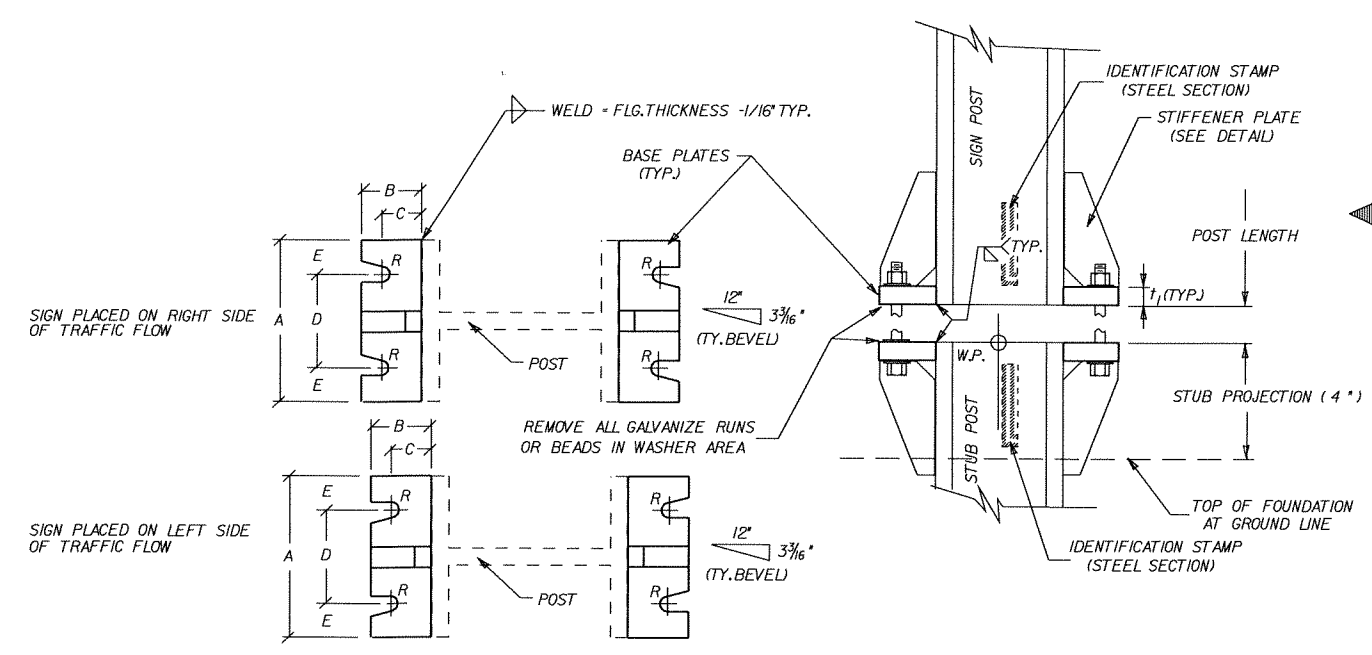
BASE PLATES AND STIFFENER PLATES SHALL BE OF THE SAME MATERIAL AS THE PRIMARY SUPPORT POSTS WHICH THEY ARE WELDED.

FOR FOOTING DETAILS REFER TO PLANS.

EACH STUB POST AND SIGN POST SHALL HAVE A PERMANENT IDENTIFYING STAMP WHICH SPECIFIES THE STEEL SECTION USED. IF THE CONTRACTOR ELECTS TO SHIP THE STUB POST SEPARATE FROM THE SIGN POST A MATCH MARK SYSTEM WILL BE REQUIRED.

NOTE: SECTIONS SHOWN ARE FOR INSTALLATION ON THE RIGHT SHOULDER AND IN THE GORE. BOLT HOLES IN BASE PLATE ARE SLOTTED AND BEVELED AS SHOWN. USE H.S. BOLTS WITH HEX HD., HEX NUT AND THREE FLAT WASHERS FOR EACH BOLT. SEE TABLE FOR BOLT DIA. AND TORQUE.

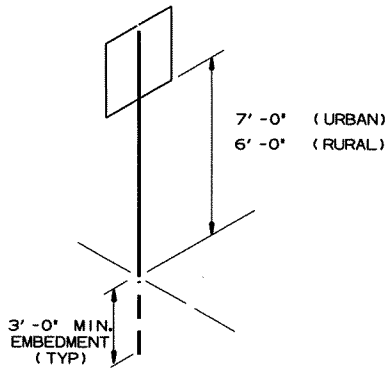
NOTE: ASSEMBLE SIGN POST TO STUB POST USING THE BOLTS SPEC. IN THE TABLE AND AS SHOWN IN THE ELEVATION DETAILS. THERE SHALL BE THREE FLAT WASHERS ON EACH BOLT LOCATED AS SHOWN IN THE ELEVATIONS. USE A SHIM TO PLUMB THE SIGN POST, THEN TIGHTEN THE BOLTS USING A 12" TO 15" WRENCH UNTIL THE WASHERS AND SHIMS ARE SEATED AND THE BOLT THREADS ARE CLEAR. THEN LOOSEN EACH BOLT IN TURN AND RETIGHTEN IN A SYSTEMATIC ORDER TO THE PRESCRIBED TORQUE (SEE TABLE). THE BURR THREADS ADJACENT TO THE BACK SIDE OF THE NUT TO PREVENT LOOSENING.



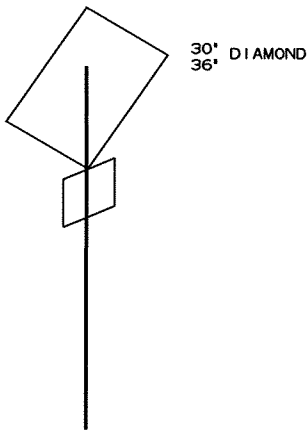
SIGN POST AND STUB POST



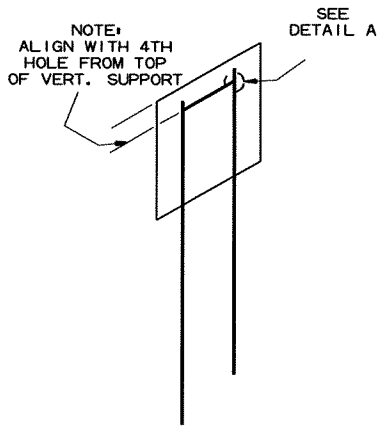
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				6	ARK.			
					JOB NO.	BB0614	139	219
② U-CHANNEL POST ASSEMBLIES								



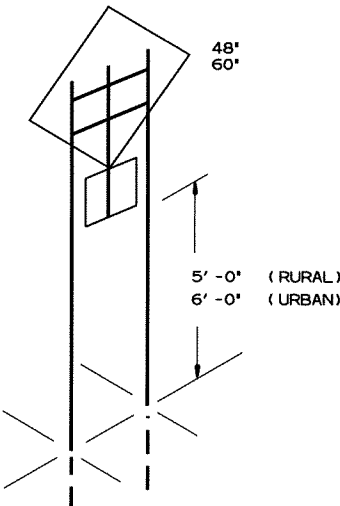
U1-1



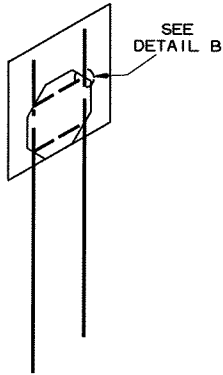
U1-2



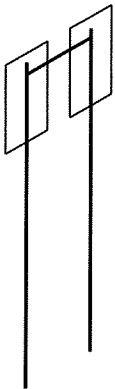
U2-1



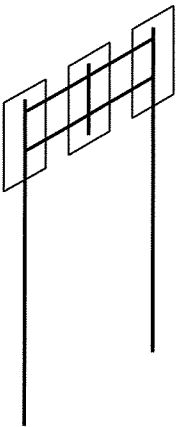
U2-2



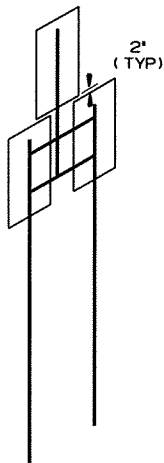
U2-3



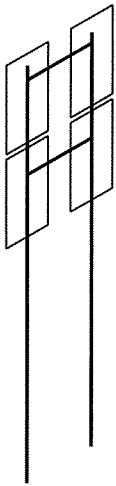
U2-4



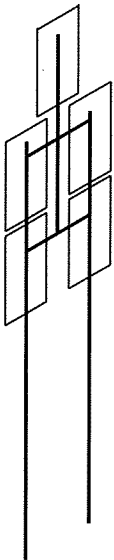
U2-5



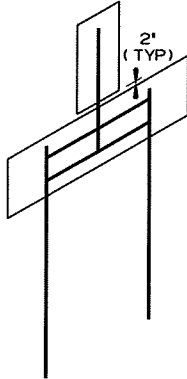
U2-6



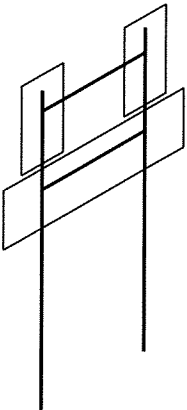
U2-7



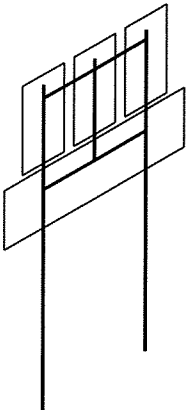
U2-8



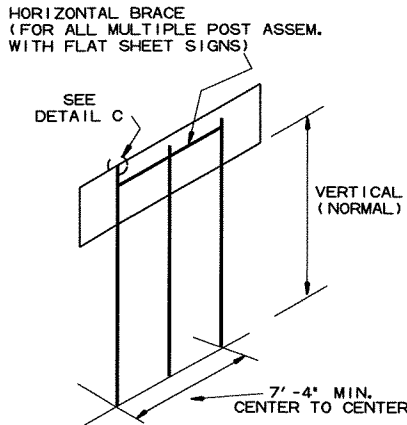
U2-9



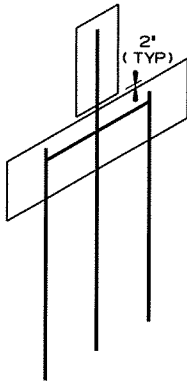
U2-10



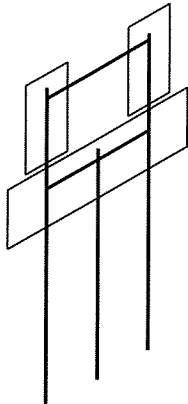
U2-11



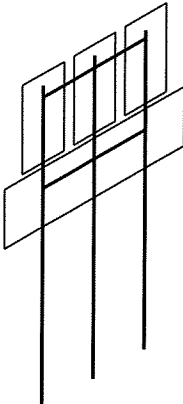
U3-1



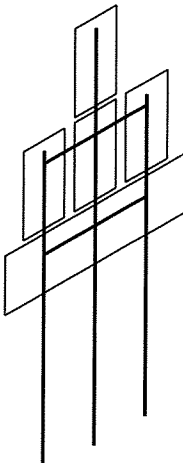
U3-2



U3-3



U3-4



U3-5

NOTES:

SIGNS AT LEAST 8' IN LENGTH MAY BE INSTALLED ON THREE 3 LB. POST. IN NO CASE SHALL THERE BE MORE THAN TWO 3 LB. POSTS WITHIN A 7' PATH.

SPLICES NECESSARY TO ATTAIN PROPER MOUNTING HEIGHT SHALL BE AS SHOWN IN THE U-CHANNEL POST ASSEMBLY DETAILS.

NORMAL INSTALLATIONS WILL REQUIRE 1/4" DIA. CARRIAGE BOLTS TO MOUNT SIGNS TO POST AND 5/16" DIA. CARRIAGE BOLTS TO ASSEMBLE THE VARIOUS POST SUPPORTS.

ALL SIGN POSTS SHALL BE PLUMB.

THE POST FOR "TYPE U" SUPPORTS SHALL BE HOT DIP GALVANIZED.

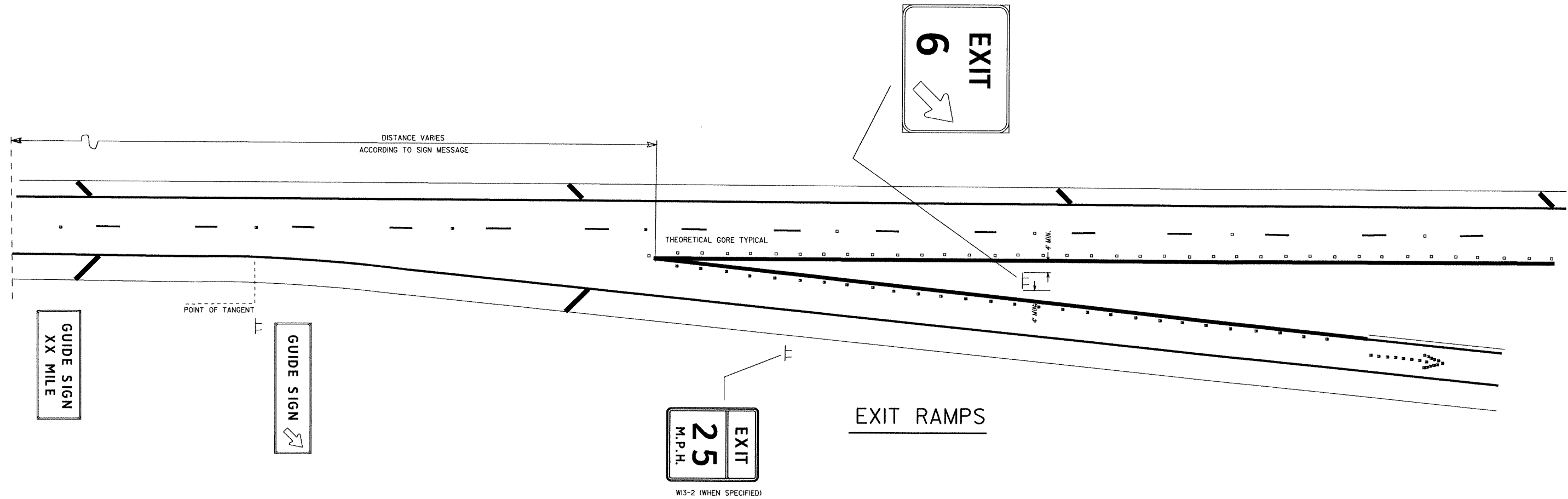
THE POSTS SHALL BE A FLANGED CHANNEL SECTION WEIGHING NOT LESS THAN 3.00 POUNDS PER FOOT (4.46 KG/M).



U-CHANNEL POST ASSEMBLIES

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB NO.		BB0614	141	219

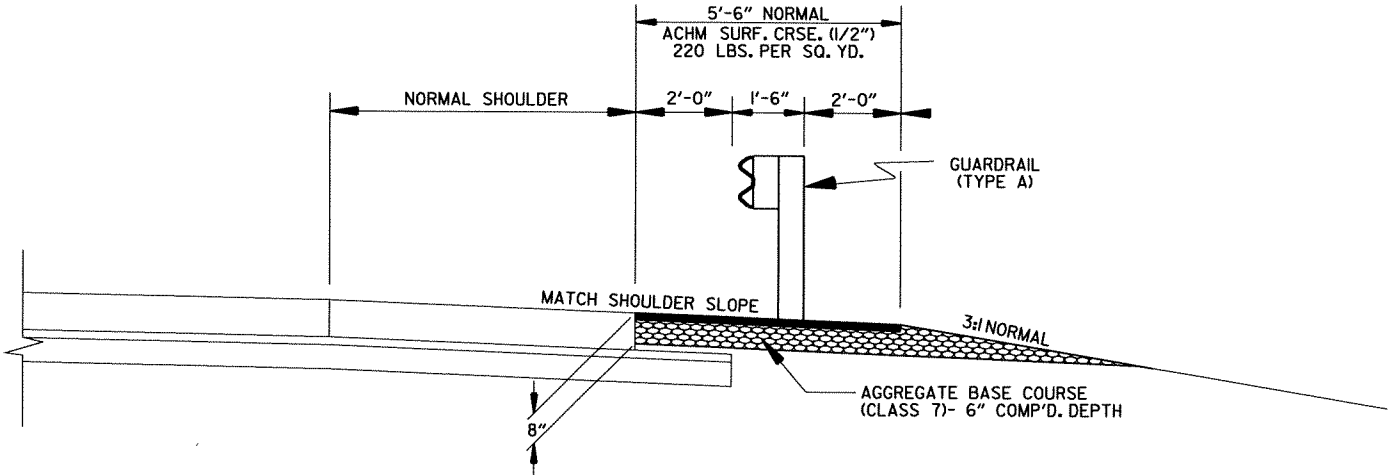
2 DETAIL OF SIGN PLACEMENT



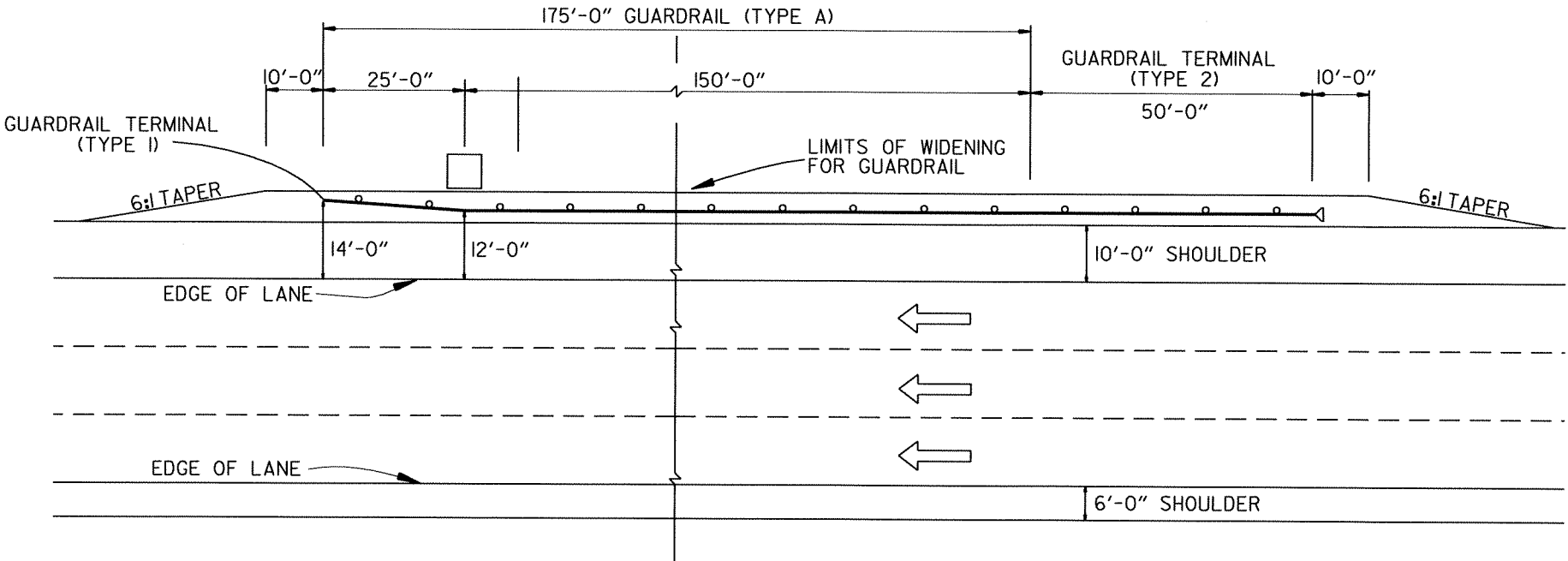
DETAIL OF SIGN PLACEMENT

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
						JOB NO.	BB0614	142 219

2 TYPICAL GUARDRAIL WIDENING (SIGN)



TYPICAL SECTION OF WIDENING FOR GUARDRAIL



TYPICAL LAYOUT OF GUARDRAIL ALONG ROADWAY



Use Type Special 2 approach gutters (W = 6'-0" and 10'-0") and Type Special 2 approach slab at each end of bridge. See Dwg. Nos. 53236 and 53238.

For right of way data, see roadway plans.

Dumped riprap 1'-6" thick placed on filter blanket. Top of riprap at Elev. 235.00. See Std. Dwg. No. 1891F.

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. ROAD NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
					ARK.			
				JOB NO.		BB0614	142	219
				B7266		LAYOUT		53225

GENERAL NOTES:

BENCH MARK: AHTD Control Point 5-2" aluminum cap on 5/8" rebar 4.21' Lt., Sta. 176+60.7, Elevation 240.52.

DESIGN SPECIFICATIONS: AASHTO LRFD Bridge Design Specifications 6th Edition (2012).

LIVE LOADING: HL 93

SEISMIC ZONE: 2

MATERIALS AND STRENGTHS:

Class S Concrete (substructure) f'c = 3,500 psi
Class S(AE) Concrete (superstructure) f'c = 4,000 psi
Reinforcing Steel (AASHTO M31 or M53, Gr. 60) fy = 60,000 psi

BORING LOGS: Boring Logs may be obtained from the Programs and Contracts Division.

CONCRETE PILING: Piles for Bents 2-5 shall be 24" square precast prestressed concrete and shall be driven to a minimum ultimate bearing capacity of 117 tons per pile. Piles for Bents 1 and 6 shall be 18" square precast prestressed concrete and shall be driven to a minimum ultimate bearing capacity of 70 tons per pile. All piles shall be driven to a tip elevation of 206.0 or lower. All piling shall be driven with an approved air, steam, or diesel hammer. Piling in end bents shall be driven after embankment to bottom of cap is in place. Lengths of piling shown are assumed for estimating quantities only. Actual lengths to be determined in the field. Drive one 79' test pile in Bent No. 3, and one 57' test pile in Bent No. 1.

The new bridge is positioned to avoid interference with the existing piling. The Contractor shall verify measurements before driving any piling. Any adjustments necessary to fit the new bridge to the existing conditions shall be submitted to the Engineer for approval.

PREBORING: Preboring is required in Bents 1 and 6 to a depth of 10' below bottom of cap. Prebored holes shall be 6" greater than the pile and shall be backfilled with sand or pea gravel after piles have been driven. The Contractor shall be responsible for keeping prebored holes free of debris prior to backfilling which may require the use of temporary casings or other methods. The required preboring will be paid for at the unit price bid for the pay item "Preboring". The method used to keep the prebored hole free from debris will not be paid for directly but will be considered subsidiary to the pay item "Preboring".

Preboring or other methods approved by the Engineer may be required in Bents 1-6 to achieve minimum penetration requirements. The Contractor shall be responsible for keeping prebored holes free of debris which may require the use of temporary casings or other methods. Preboring in Bents 2-5, preboring below the required preboring in Bents 1 and 6, and the method used to keep the prebored hole free from debris will not be paid for directly but will be considered subsidiary to the item "Concrete Piling (24" Sq.) and "Concrete Piling (18" Sq.)".

DRIVING SYSTEM: The driving system approval and ultimate bearing capacity determination for piling shall be based on the requirements of subsection 805.09(b) "Method B - Wave Equation Analysis (WEAP)". It is estimated that a minimum rated hammer energy of 27,000 foot pounds per blow for the end bents and 40,000 foot pounds per blow for the intermediate bents will be required to obtain the ultimate bearing capacity.

BRIDGE DECK: The concrete bridge deck shall be given a fine finish as specified for final finishing in Subsection 802.19 for Class 5 Tined Bridge Roadway Surface Finish.

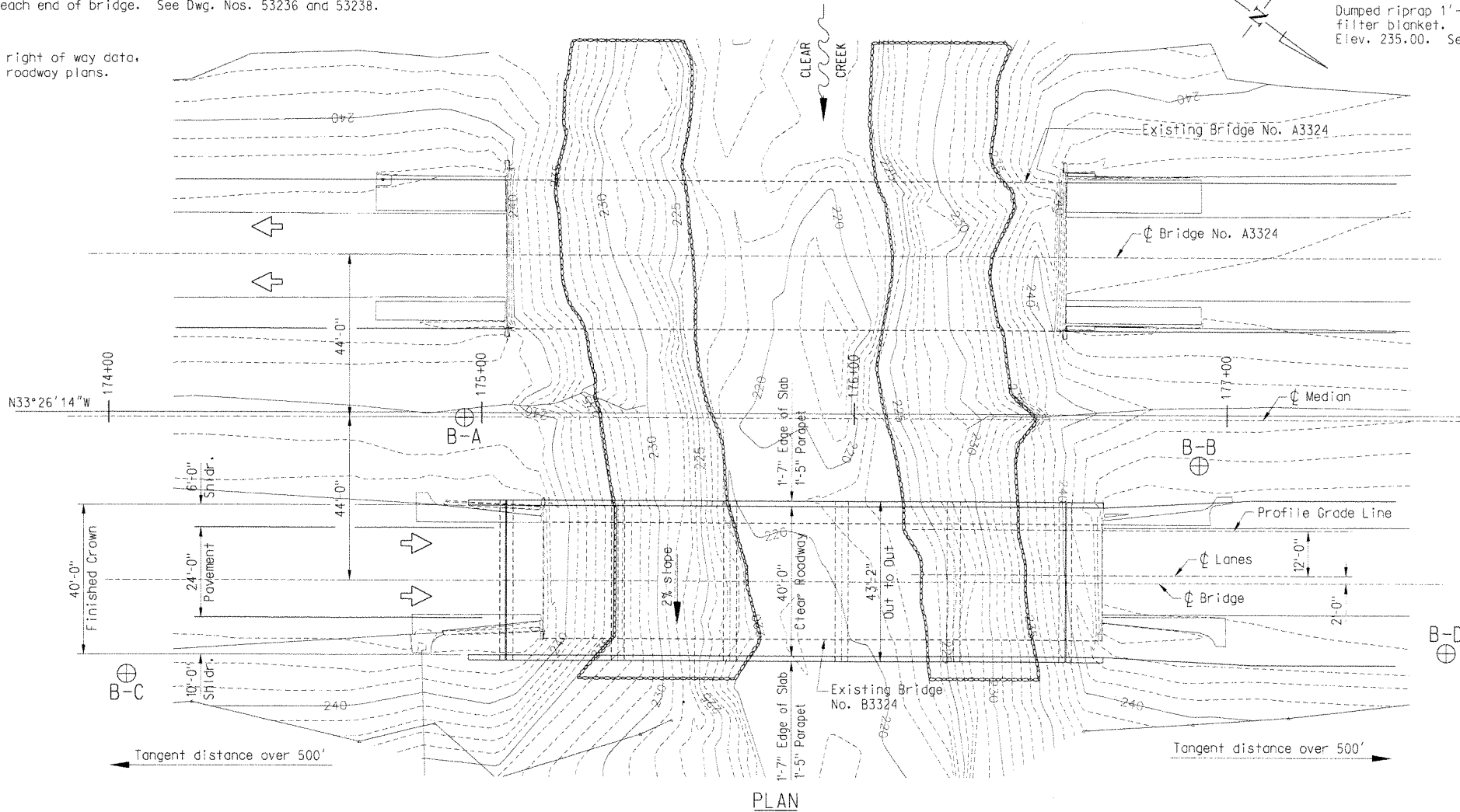
CLASS 1 PROTECTIVE SURFACE TREATMENT: Class 1 Protective Surface Treatment shall be applied to the roadway surface and the face and top of parapet rail.

EXISTING BRIDGE: Existing Bridge B3324 is 33'-6" wide and 150' long and consists of reinforced concrete slab span superstructures and precast concrete pile substructures. Existing Bridge B3324 is located at the same location as the proposed bridge.

REMOVE AND SALVAGE: Existing Bridge B3324 shall be removed in accordance with Section 205. All materials from the removed bridge shall become the property of the Contractor. Existing Bridge A3324 is to remain.

For description of boring material, see sheets 53227 - 53228.

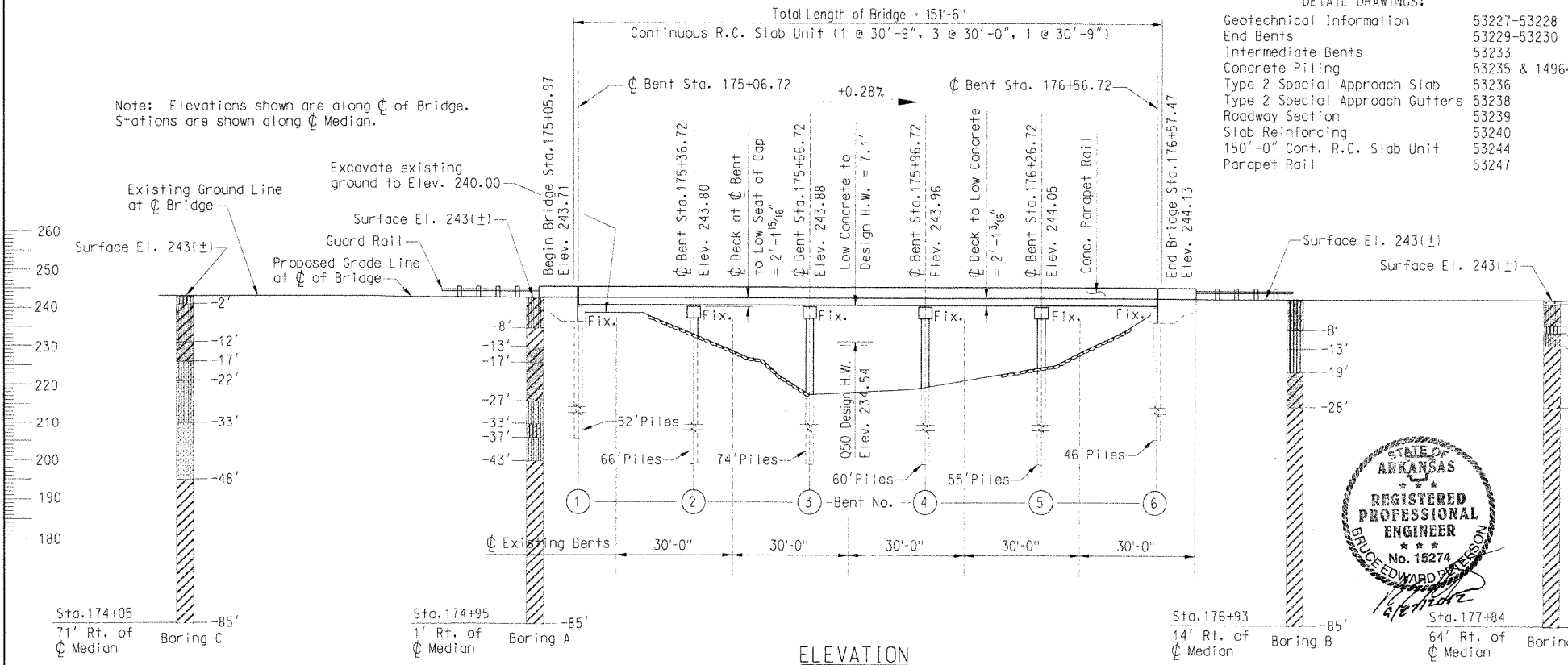
MAINTENANCE OF TRAFFIC: See Roadway Plans.



PLAN

DETAIL DRAWINGS:

Geotechnical Information	53227-53228
End Bents	53229-53230
Intermediate Bents	53233
Concrete Piling	53235 & 14964
Type 2 Special Approach Slab	53236
Type 2 Special Approach Gutters	53238
Roadway Section	53239
Slab Reinforcing	53240
150'-0" Cont. R.C. Slab Unit	53244
Parapet Rail	53247



ELEVATION

HYDRAULIC DATA

FLOOD DESCRIPTION	FREQUENCY	DISCHARGE	*NATURAL WATER SURFACE ELEV.	WATER SURFACE ELEV. WITH BACKWATER
	YEARS	CFS	FEET	FEET
DESIGN	50	5,968	232.94	234.54
BASE	100	7,006	233.60	235.27
EXTREME	500	9,623	235.16	237.02
OVERTOPPING	>500	-	-	-

* Unconstricted Water Surface without Structure or Roadway Approaches.
Drainage Area = 28.6 Square Miles.
Historical H.W. Elev. = 233.0 Ft.
Proposed low bridge chord elevation = 241.61 Ft.
Exist. 100 Yr. Elev. = 235.26 Ft.

LAYOUT OF BRIDGE OVER CLEAR CREEK

BINGHAM RD. - GRANT CO. LINE (S)
PULASKI COUNTY

ROUTE 530 SEC. 3

ARKANSAS STATE HIGHWAY COMMISSION
LITTLE ROCK, ARK.

Engstrom / Modjeski and Masters

DRAWN BY: PMR DATE: 9-25-12 FILENAME: BBB0614B2_1.DGN
CHECKED BY: SPC DATE: 9-25-12 SCALE: 1" = 20'-0"
DESIGNED BY: SPC DATE: 9-25-12
BRIDGE NO. B7266 DRAWING NO. 53225



For Bridge A, use Type 2 Special approach gutter (W = 6'-0") and Type 3 Special approach slab at each end of bridge. For Bridge B, use Type 2 Special Approach Gutter (W = 6'-0" and 10'-0") and Type Special 2 approach slab at each end of bridge. See Dwg. Nos. 53236 - 53238.

For right of way data, see roadway plans.

Dumped riprap 1'-6" thick placed on filter blanket. Top of riprap at Elev. 247.00. See Std. Dwg. No. 1891F.

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. ROAD NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
					ARK.			
				JOB NO.		BB0614	149	219
				A7267 & B7267	LAYOUT			53226

GENERAL NOTES:

BENCH MARK: AHTD Control Point 3-2" aluminum cap on 5/8" rebar 13.7' Lt., Sta. 381+24.3, Elevation 251.97.

DESIGN SPECIFICATIONS: AASHTO LRFD Bridge Design Specifications 6th Edition (2012).

LIVE LOADING: HL 93

SEISMIC ZONE: 2

MATERIALS AND STRENGTHS:

Class S Concrete (substructure) $f'_c = 3,500$ psi
Class S(AE) Concrete (superstructure) $f'_c = 4,000$ psi
Reinforcing Steel (AASHTO M31 or M53, Gr. 60) $f_y = 60,000$ psi

BORING LOGS: Boring Logs may be obtained from the Programs and Contracts Division.

CONCRETE PILING: Piles for Bents 2-6 shall be 24" square precast prestressed concrete and shall be driven to a minimum ultimate bearing capacity of 117 tons per pile. Piles for Bents 1 and 7 shall be 18" square precast prestressed concrete and shall be driven to a minimum ultimate bearing capacity of 70 tons per pile. All piles shall be driven to a tip elevation of 206.0 or lower. All piling shall be driven with an approved air, steam, or diesel hammer. Piling in end bents shall be driven after embankment to bottom of cap is in place. Lengths of piling shown are assumed for estimating quantities only. Actual lengths to be determined in the field. Drive one 68' test pile in Bent No. 3, and one 57' test pile in Bent No. 1.

The new bridge is positioned to avoid interference with the existing piling. The Contractor shall verify measurements before driving any piling. Any adjustments necessary to fit the new bridge to the existing conditions shall be submitted to the Engineer for approval.

PREBORING: Preboring is required in Bents 1 and 7 to a depth of 10' below bottom of cap. Prebored holes shall be 6" greater than the pile and shall be backfilled with sand or pea gravel after piles have been driven. The Contractor shall be responsible for keeping prebored holes free of debris prior to backfilling which may require the use of temporary casings or other methods. The required preboring will be paid for at the unit price bid for the pay item "Preboring". The method used to keep the prebored hole free from debris will not be paid for directly but will be considered subsidiary to the pay item "Preboring".

Preboring or other methods approved by the Engineer may be required in Bents 1-7 to achieve minimum penetration requirements. The Contractor shall be responsible for keeping prebored holes free of debris which may require the use of temporary casings or other methods. Preboring in Bents 2-6, preboring below the required preboring in Bents 1 and 7, and the method used to keep the prebored hole free from debris will not be paid for directly but will be considered subsidiary to the item "Concrete Piling (24" Sq.)" and "Concrete Piling (18" Sq.)".

DRIVING SYSTEM: The driving system approval and ultimate bearing capacity determination for piling shall be based on the requirements of subsection 805.09(b) "Method B - Wave Equation Analysis (WEAP)". It is estimated that a minimum rated hammer energy of 27,000 foot pounds per blow for the end bents and 40,000 foot pounds per blow for the intermediate bents will be required to obtain the ultimate bearing capacity.

BRIDGE DECK: The concrete bridge deck shall be given a fine finish as specified for final finishing in Subsection 802.19 for Class 5 Tined Bridge Roadway Surface Finish.

CLASS I PROTECTIVE SURFACE TREATMENT: Class I Protective Surface Treatment shall be applied to the roadway surface and the face and top of parapet rail.

EXISTING BRIDGES: Existing Bridge A3327 is 46'-8" wide and 180' long. Existing Bridge B3327 is 33'-6" wide and 180' long. Both consist of reinforced concrete slab span superstructures and precast concrete pile substructures. Existing Bridges A3327 and B3327 are located at the same location as the proposed bridges.

REMOVE AND SALVAGE: Existing bridges shall be removed in accordance with Section 205. All materials from the removed existing bridges shall become the property of the Contractor.

For description of boring material, see sheets 53227 - 53228.

MAINTENANCE OF TRAFFIC: See Roadway Plans.

HYDRAULIC DATA

FLOOD DESCRIPTION	FREQUENCY	DISCHARGE	*NATURAL WATER SURFACE ELEV.	WATER SURFACE ELEV. WITH BACKWATER
	YEARS	CFS	FEET	FEET
DESIGN	50	11000	246.25	246.73
BASE	100	12400	246.45	246.98
EXTREME	500	14500	246.73	247.06
OVERTOPPING	>500	-	-	-

* Unconstricted Water Surface without Structure or Roadway Approaches.
Drainage Area = 31.6 Square Miles.
Historical H.W. Elev. = 242.2 Ft.
Proposed low bridge chord elevation = 247.73 Ft.(A)
247.89 Ft.(B)
Exist. 100 Yr. Elev. = 247.93 Ft.

LAYOUT OF BRIDGES OVER LORANCE CREEK
BINGHAM RD. - GRANT CO. LINE (S)
PULASKI COUNTY

ROUTE 530 SEC. 1

ARKANSAS STATE HIGHWAY COMMISSION
LITTLE ROCK, ARK.

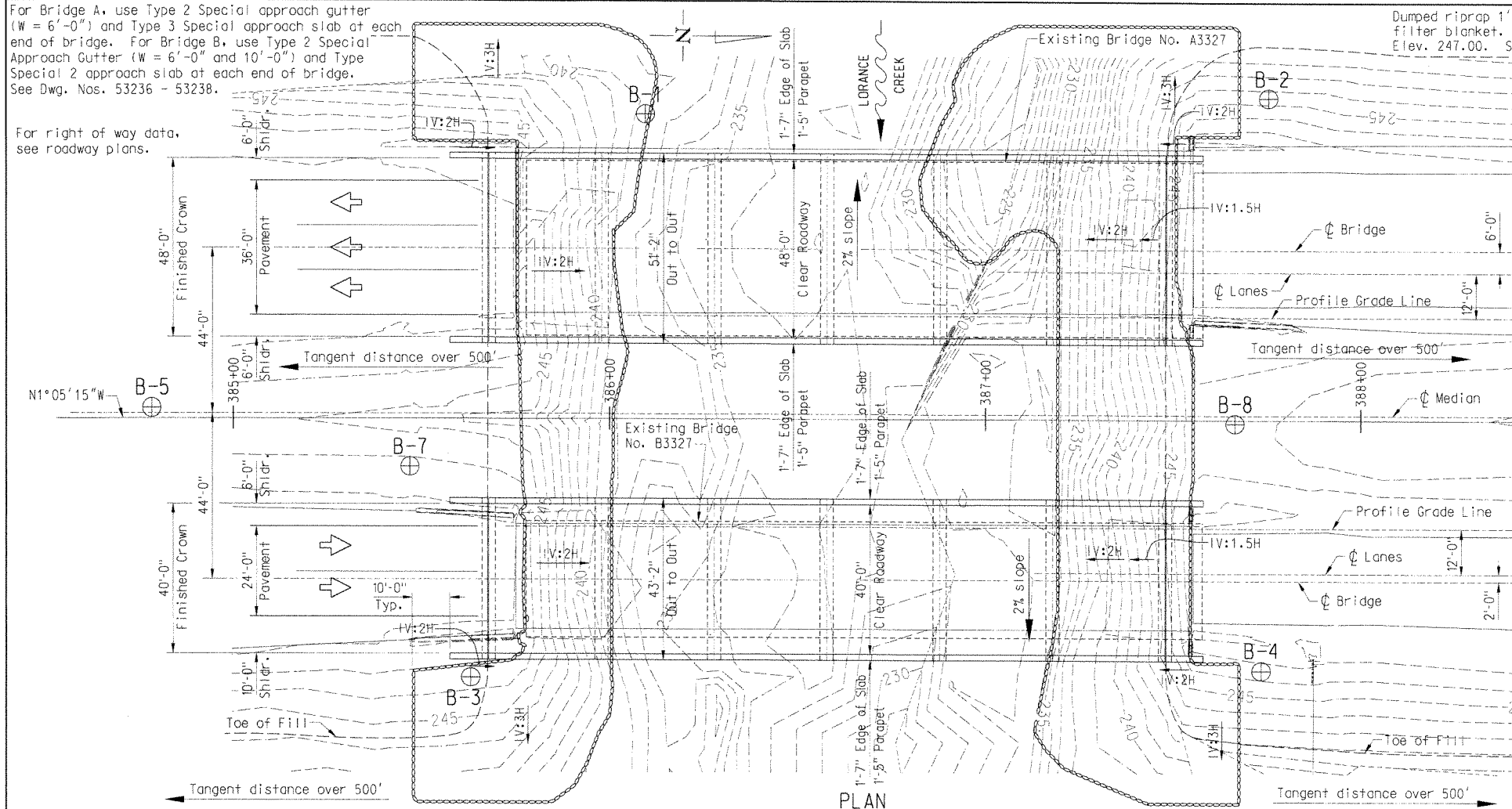
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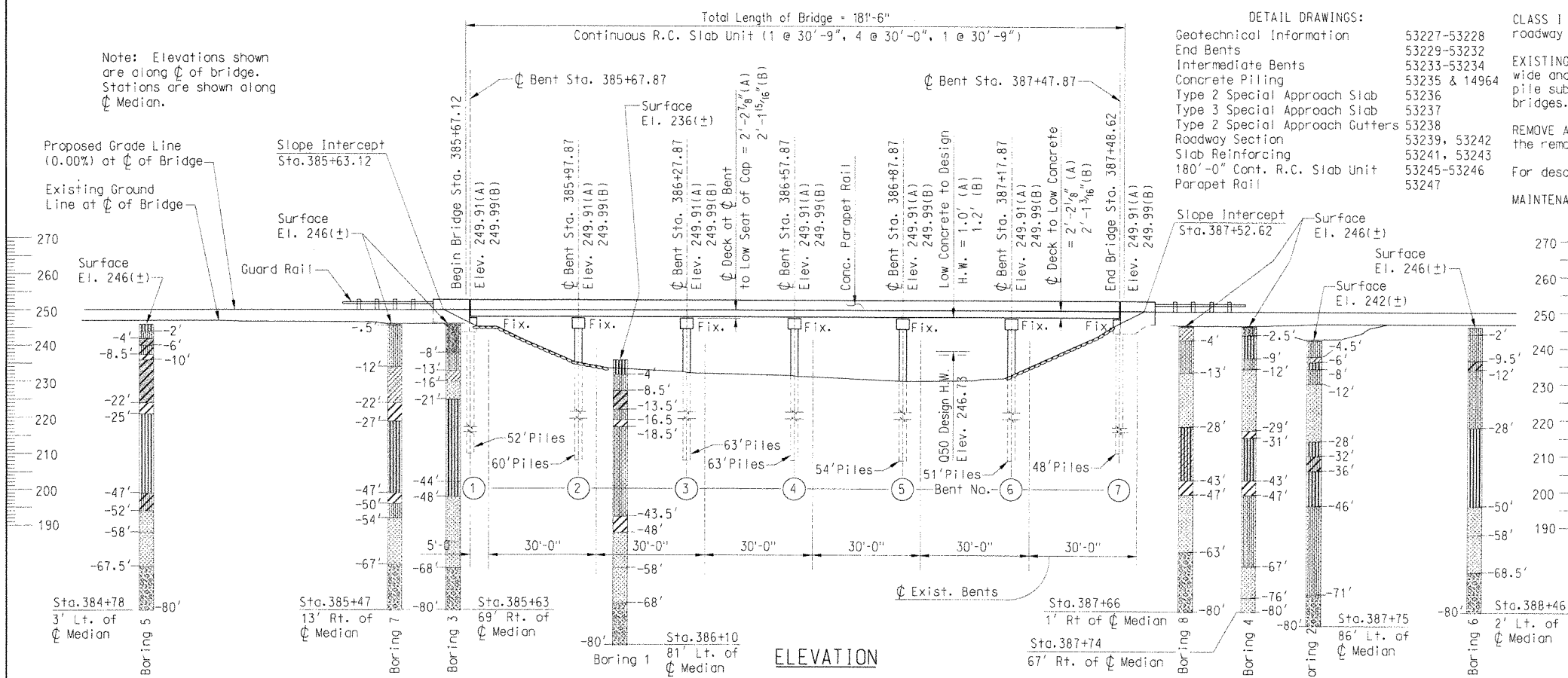
CHECKED BY: SPC DATE: 9-25-12 SCALE: 1" = 20'-0"

DESIGNED BY: SPC DATE: 9-25-12

BRIDGE NO. A7267 & B7267 DRAWING NO. 53226



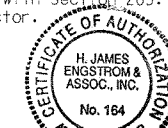
PLAN



ELEVATION

DETAIL DRAWINGS:

Geotechnical Information	53227-53228
End Bents	53229-53232
Intermediate Bents	53233-53234
Concrete Piling	53235 & 14964
Type 2 Special Approach Slab	53236
Type 3 Special Approach Slab	53237
Type 2 Special Approach Gutters	53238
Roadway Section	53239, 53242
Slab Reinforcing	53241, 53243
180'-0" Cont. R.C. Slab Unit	53245-53246
Parapet Rail	53247



DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. ROAD NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
					ARK.			
				JOB NO.	BB0614		145	219
B7266, A7267, B7267 BOR.LEG'D 53227								

BORING LEGEND

LORANCE CREEK BRIDGE

BORING 1

- 1 Very loose to loose tan fine sandy silt (temporary fill for access)
- 2 Loose tan and brown silty fine sand w/ organics
- 3 Very soft reddish tan, tan and light gray fine sandy clay w/ ferrous stains
- 4 Loose gray silty fine sand w/ occasional clay seams
- 5 Firm gray clay w/ organic stains
- 6 Loose gray silty fine sand w/ occasional clay laminations and seams
- 7 Stiff brown clay w/ organic inclusions
- 8 Medium dense reddish tan and tan fine sand, slightly silty w/ some clay laminations and seams and ferrous stains
- 9 Dense to very dense reddish tan fine to medium sand, slightly silty
- 10 Dense to very dense tan fine to medium sand, slightly silty w/ fine to coarse gravel

BORING 3

- 1 Medium dense tan silty fine sand w/ some crushed stone and surface organics (fill)
- 2 Medium dense tan and gray silty fine sand w/ numerous decayed organics
- 3 Medium dense gray clayey fine sand, silty w/ decayed organics and fine sand seams
- 4 Medium dense gray fine sand, slightly silty w/ interbedded fine sand seams and layers
- 5 Medium dense gray fine sandy silt, clayey w/ occasional clay laminations and seams and organic inclusions
- 6 Dense brown fine sandy silt
- 7 Dense tan and reddish tan fine sand, slightly silty
- 8 Dense reddish tan and tan fine to coarse sand w/ some fine to coarse gravel

BORING 5

- 1 Medium dense brown and tan fine sandy silt w/ organics (fill)
- 2 Medium dense brown and gray silty fine sand (fill)
- 3 Firm reddish tan and gray fine sandy clay w/ occasional ferrous nodules and stains (fill)
- 4 Stiff gray with tan silty clay w/ trace fine sand and ferrous stains (fill)
- 5 Stiff tan clay w/ fine sand partings and pockets and ferrous stains
- 6 Very stiff gray fine sandy clay w/ occasional clay laminations
- 7 Firm gray clay w/ fine sand partings and seams
- 8 Loose gray silt, slightly sandy w/ interbedded silty clay laminations and seams
- 9 Stiff gray silty clay w/ fine sand partings and seams and organic inclusions
- 10 Dense tan fine sand, slightly silty
- 11 Dense reddish tan fine sand w/ occasional clay laminations and seams
- 12 Dense reddish tan fine to coarse sand, slightly silty w/ fine to coarse gravel

BORING 7

- 1 Loose dark brown fine sandy silt w/ trace organics (fill)
- 2 Medium dense tan silty fine sand (fill)
- 3 Loose light gray clayey fine sand
- 4 Stiff light gray clay w/ occasional silt pockets
- 5 Loose light gray fine sandy silt, clayey w/ interbedded silty clay seams and layers
- 6 Very stiff gray and brown clay w/ silt partings
- 7 Medium dense light gray and tan silty fine sand
- 8 Dense tan and reddish tan fine to medium sand, slightly silty w/ ferrous stains
- 9 Dense tan and reddish tan fine to coarse sand w/ fine to coarse gravel and occasional clay seams

BORING 2

- 1 Medium dense tan silty fine sand w/ concrete debris (fill)
- 2 Medium dense dark brown clayey fine sand (fill)
- 3 Medium dense light gray fine sandy silt, clayey (fill)
- 4 Medium dense tan and brown silty fine sand (probable fill)
- 5 Medium dense gray fine sand, slightly silty w/ trace decayed organics
- 6 Loose to medium dense gray fine sandy silt, slightly clayey
- 7 Firm to stiff gray silty clay w/ organic inclusions
- 8 Medium dense gray fine sandy silt w/ occasional clay laminations and seams
- 9 Dense reddish tan silty fine sand
- 10 Dense to very dense reddish tan and tan fine to coarse sand w/ some fine to coarse gravel

BORING 4

- 1 Dense tan silty fine sand w/ crushed stone and surface organics (fill shoulder)
- 2 Medium dense tan and gray fine sandy silt, clayey (fill)
- 3 Very soft brown silty fine sand with trace clay and decayed organics
- 4 Loose gray fine sand, slightly silty w/ some organic inclusions
- 5 Stiff brown and gray clay w/ interbedded fine sand seams
- 6 Medium dense gray fine sandy silt w/ occasional clay laminations and seams
- 7 Stiff gray clay w/ organic inclusions
- 8 Dense reddish tan silty fine sand w/ occasional clay laminations
- 9 Dense to very dense reddish tan fine to medium sand, slightly silty w/ a little fine to coarse gravel
- 10 Dense to very dense tan and gray fine sand

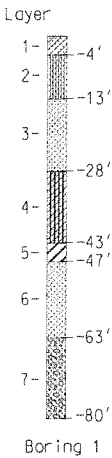
BORING 6

- 1 Medium dense reddish tan and tan silty fine sand w/ ferrous stains and organics (fill)
- 2 Dense reddish tan and gray silty fine sand, clayey w/ ferrous stains (fill)
- 3 Firm tan and gray silty clay, sandy
- 4 Loose gray and brown silty fine sand w/ decayed organics
- 5 Medium dense gray silt, sandy w/ clay seams and organic inclusions
- 6 Dense tan fine sand
- 7 Dense to very dense reddish tan fine to medium sand, slightly silty
- 8 Dense to very dense reddish tan and tan fine to coarse sand w/ fine to coarse gravel

BORING 8

- 1 Medium dense reddish tan and gray clayey fine sand (fill)
- 2 Medium dense gray silty fine sand w/ occasional clay pockets and seams (fill)
- 3 Dense light gray fine sand, slightly silty
- 4 Soft gray clayey silt w/ clay pockets
- 5 Firm brown clay
- 6 Dense to very dense reddish tan fine sand, slightly silty
- 7 Dense to very dense reddish tan and tan fine to medium sand, slightly silty w/ fine to coarse gravel

Example



CLEAR CREEK BRIDGE

BORING A

- 1 Firm reddish tan and gray silty clay, sandy (fill)
- 2 Stiff gray clay
- 3 Soft gray fine sandy clay, silty
- 4 Very soft gray fine sandy clay w/ organic inclusions (wood)
- 5 Medium dense gray silty fine sand
- 6 Firm gray clayey silt w/ fine sand partings and seams
- 7 Medium dense gray silty fine sand w/ interbedded clay seams and layers
- 8 Stiff grayish brown clay w/ organic inclusions

BORING C

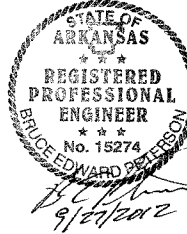
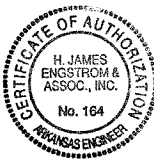
- 1 Loose brown fine sandy silt w/ organics (fill)
- 2 Very stiff reddish tan and gray fine sandy clay and silty clay (fill)
- 3 Firm reddish tan and gray fine sandy clay, silty w/ organic inclusions
- 4 Medium dense gray and tan silty fine sand
- 5 Dense gray silty fine sand
- 6 Dense to very dense tan fine sand, slightly silty w/ some organic inclusions
- 7 Very stiff grayish brown clay

BORING B

- 1 Dense brown fine sandy silt w/ crushed stone and organics (fill)
- 2 Medium dense tan fine sandy silt
- 3 Medium dense gray silt, sandy
- 4 Stiff light gray fine sandy clay
- 5 Stiff grayish brown clay

BORING D

- 1 Crushed stone base w/ silty clay pockets (fill)
- 2 Medium dense brown fine sandy silt and silty clay w/ occasional organic inclusions (fill)
- 3 Medium dense brown fine sandy silt
- 4 Medium dense gray clayey fine sand, silty
- 5 Very stiff tan and gray clay, slightly sandy w/ ferrous nodules
- 6 Stiff grayish brown clay w/ silt pockets and occasional organic inclusions



BORING LEGEND
CLEAR CREEK BRIDGE AND
LORANCE CREEK BRIDGES
BINGHAM RD. - GRANT CO. LINE (S)
PULASKI COUNTY

ROUTE 530 SEC. 1 & 3
ARKANSAS STATE HIGHWAY COMMISSION
LITTLE ROCK, ARK.

Engstrom / Modjeski and Masters

DRAWN BY: PMR DATE: 9-25-12 FILENAME: BBB0614M_SB1.DGN
CHECKED BY: SPC DATE: 9-25-12 SCALE: 1" = 20'-0"
DESIGNED BY: SPC DATE: 9-25-12
BRIDGE NO. B7266, A7267 & B7267 DRAWING NO. 53227

BRIDGE ENGINEER

BORING "N" VALUES
LORANCE CREEK BRIDGE

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. ROAD NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
					ARK.			
				JOB NO.	BB0614	146	219	
B7266, A7267, B7267 "N" VALUES 53228								

Sta. 386+10 - 81' Left of Centerline of Median

4 - 5, N=9	39 - 40, N=34
6 - 7, N=5	44 - 45, N=19
7.5 - 8.5, tube	49 - 50, N=18
10.5 - 11.5, N=7	54 - 55, N=27
12.5 - 13.5, tube	59 - 60, N=55
14 - 15, N=5	64 - 65, N=50/7"
19 - 20, N=6	69 - 70, N=50/8"
24 - 25, N=8	74 - 75, N=50/8"
29 - 30, N=11	79 - 80, N=50/9"
34 - 35, N=23	

Sta. 384+78 - 3' Lt. of Centerline of Median

0.5 - 1.5, N=17	39 - 40, N=19
2.5 - 3.5, N=18	44 - 45, N=18
4.5 - 5.5, N=7	49 - 50, N=16
6.5 - 7.5, N=15	54 - 55, N=40
9 - 10, N=18	59 - 60, N=50
14 - 15, N=44	64 - 65, N=38
19 - 20, N=9	69 - 70, N=43
24 - 25, N=9	74 - 75, N=50/9"
29 - 30, N=8	79 - 80, N=50
34 - 35, N=11	

Sta. 387+75 - 86' Left of Centerline of Median

0.5 - 1.5, N=16	39 - 40, N=14
2.5 - 3.5, N=15	44 - 45, N=14
4.5 - 5.5, N=10	49 - 50, N=52
6.5 - 7.5, N=11	54 - 55, N=50/11"
9 - 10, N=12	59 - 60, N=46
14 - 15, N=11	64 - 65, N=50/10"
19 - 20, N=18	69 - 70, N=50/8"
24 - 25, N=18	74 - 75, N=50/8"
29 - 30, N=10	79 - 80, N=50/7"
34 - 35, N=10	

Sta. 388+46 - 2' Left of Centerline of Median

0.5 - 1.5, N=27	39 - 40, N=6
2.5 - 3.5, N=30	44 - 45, N=9
4.5 - 5.5, N=32	49 - 50, N=13
6.5 - 7.5, N=7	54 - 55, N=50
9 - 10, N=9	59 - 60, N=50/9"
14 - 15, N=9	64 - 65, N=50/11"
19 - 20, N=24	69 - 70, N=50/10"
24 - 25, N=26	74 - 75, N=50/8"
29 - 30, N=12	79 - 80, N=50/8"
34 - 35, N=16	

Sta. 385+63 - 69' Right of Centerline of Median

0.5 - 1.5, N=13	39 - 40, N=8
2.5 - 3.5, N=17	44 - 45, N=47
4.5 - 5.5, N=8	49 - 50, N=41
6.5 - 7.5, N=14	54 - 55, N=50/11"
9 - 10, N=13	59 - 60, N=50/11"
14 - 15, N=20	64 - 65, N=50/10"
19 - 20, N=19	69 - 70, N=49
24 - 25, N=1	74 - 75, N=50/7"
29 - 30, N=15	79 - 80, N=50/4"
34 - 35, N=6	

Sta. 385+47 - 13' Left of Centerline of Median

0.5 - 1.5, N=13	39 - 40, N=11
2.5 - 3.5, N=16	44 - 45, N=16
4.5 - 5.5, N=18	49 - 50, N=30
6.5 - 7.5, N=17	54 - 55, N=35
9 - 10, N=13	59 - 60, N=50/11"
14 - 15, N=4	64 - 65, N=50
19 - 20, tube	69 - 70, N=50
24 - 25, N=14	74 - 75, N=50/9"
29 - 30, tube	79 - 80, N=50/5"
34 - 35, N=4	

Sta. 387+74 - 67' Right of Centerline of Median

0.5 - 1.5, N=50/9"	39 - 40, N=14
2.5 - 3.5, N=18	44 - 45, N=11
4.5 - 5.5, N=20	49 - 50, N=34
6.5 - 7.5, N=25	54 - 55, N=50/6"
9 - 10, N=4	59 - 60, N=50/11"
14 - 15, N=6	64 - 65, N=50/9"
19 - 20, N=12	69 - 70, N=50/9"
24 - 25, N=21	74 - 75, N=50/10"
29 - 30, N=15	79 - 80, N=50/7"
34 - 35, N=21	

Sta. 387+66 - 1' Rt. of Centerline of Median

0.5 - 1.5, N=18	39 - 40, N=9
2.5 - 3.5, N=16	44 - 45, N=9
4.5 - 5.5, N=12	49 - 50, N=50/8"
6.5 - 7.5, N=13	54 - 55, N=50/10"
9 - 10, N=9	59 - 60, N=50/9"
14 - 15, N=36	64 - 65, N=50/8"
19 - 20, N=21	69 - 70, N=50/10"
24 - 25, N=22	74 - 75, N=50/8"
29 - 30, N=6	79 - 80, N=50/8"
34 - 35, N=3	

CLEAR CREEK BRIDGE

Sta. 174+95 - 1' Rt. of Centerline of Median

0.5 - 1.5, N=7	39 - 40, N=24
2.5 - 3.5, N=11	44 - 45, N=20
4.5 - 5.5, N=13	49 - 50, N=23
6.5 - 7.5, N=16	54 - 55, N=50
9 - 10, N=17	59 - 60, N=45
14 - 15, N=4	64 - 65, N=55
19 - 20, N=2	69 - 70, N=53
24 - 25, N=13	74 - 75, N=49
29 - 30, N=11	79 - 80, N=50/10"
34 - 35, N=7	84 - 85, N=50/9"

Sta. 174+05 - 71' Right of Centerline of Median

0.5 - 1.5, N=9	39 - 40, N=50/11"
2.5 - 3.5, N=27	44 - 45, N=50/11"
4.5 - 5.5, N=23	49 - 50, N=32
6.5 - 7.5, N=28	54 - 55, N=29
9 - 10, N=31	59 - 60, N=34
14 - 15, N=8	64 - 65, N=41
19 - 20, N=12	69 - 70, N=39
24 - 25, N=30	74 - 75, N=44
29 - 30, N=7	79 - 80, N=44
34 - 35, N=50/10"	84 - 85, N=50/2"

Sta. 176+93 - 14' Right of Centerline of Median

0.5 - 1.5, N=30	39 - 40, N=30
2.5 - 3.5, N=20	44 - 45, N=34
4.5 - 5.5, N=12	49 - 50, N=33
6.5 - 7.5, N=17	54 - 55, N=34
9 - 10, N=11	59 - 60, N=50/7"
14 - 15, N=11	64 - 65, N=41
19 - 20, N=17	69 - 70, N=50/6"
24 - 25, N=9	74 - 75, N=45
29 - 30, N=17	79 - 80, N=38
34 - 35, N=32	84 - 85, N=42

Sta. 177+84 - 64' Right of Centerline of Median

0.5 - 1.5, N=45	39 - 40, tube
2.5 - 3.5, N=22	44 - 45, N=32
4.5 - 5.5, N=11	49 - 50, tube
6.5 - 7.5, N=11	54 - 55, N=30
9 - 10, N=14	59 - 60, N=50
14 - 15, N=30	64 - 65, N=50
19 - 20, N=25	69 - 70, N=50/10"
24 - 25, N=10	74 - 75, N=44
29 - 30, N=15	79 - 80, N=50
34 - 35, N=31	84 - 85, N=50/4"



BORING "N" VALUES
CLEAR CREEK BRIDGE AND
LORANCE CREEK BRIDGES
BINGHAM RD. - GRANT CO. LINE (S)
PULASKI COUNTY

ROUTE 530 SEC. 1 & 3
ARKANSAS STATE HIGHWAY COMMISSION
LITTLE ROCK, ARK.

Engstrom / Modjeski and Masters

DRAWN BY: PMR DATE: 9-25-12 FILENAME: BBB0614M_SB2.DGN
CHECKED BY: SPC DATE: 9-25-12 SCALE: 1" = 20'-0"
DESIGNED BY: SPC DATE: 9-25-12
BRIDGE NO. B7266, A7267 & B7267 DRAWING NO. 53228

BRIDGE ENGINEER

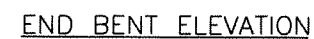
Plan view of a bridge structure showing dimensions, materials, and layout. The diagram includes a centerline, bridge centerline, and various dimensions for the bridge deck, abutments, and approach. Key features include 1/2 inch joint filler, 1/2 inch x 2 inch poured joint sealer, and 1/2 inch x 2 inch poured joint sealer. The bridge is labeled "PLAN".

Dimensions and Materials:

- 1/2" Jt. Filler (AASHTO M153 Type I) with 1/2" X 2" Poured Joint Sealer (Subsection 501.02(h) Type 3,4, or 6)
- 1/2" Jt. Filler (AASHTO M153 Type I) with 1/2" X 2" Poured Joint Sealer (Subsection 501.02(h) Type 3,4, or 6)

Labels and Dimensions:

- Centerline
- Bridge
- Cap & Piles
- Slope Intercept for IV:2H slope
- Begin or End of Bridge
- 44'-0"
- 21'-7" (typ.)
- 6'-0"
- 4'-8"
- 1'-0"
- 1'-4"
- 3'-6"
- 1'-9"
- 5"
- 4'-0"
- 1'-0"
- 9"
- 43'-6"
- 2'-10"
- 2'-10"

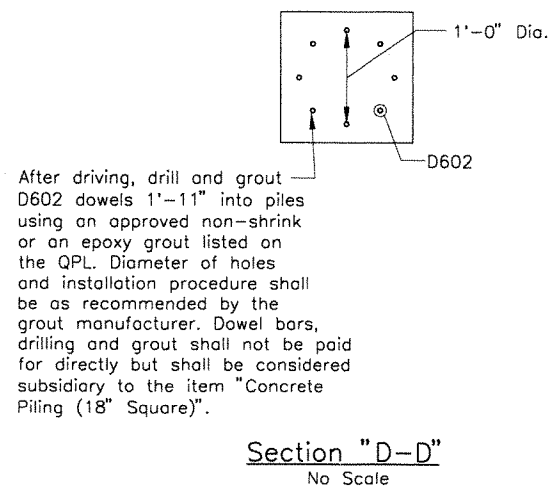
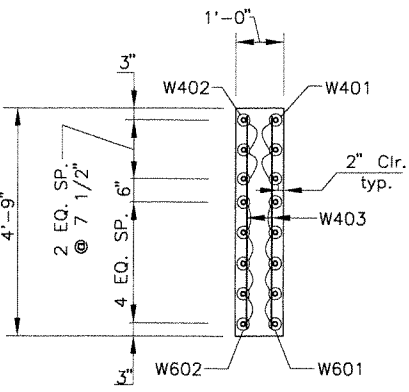
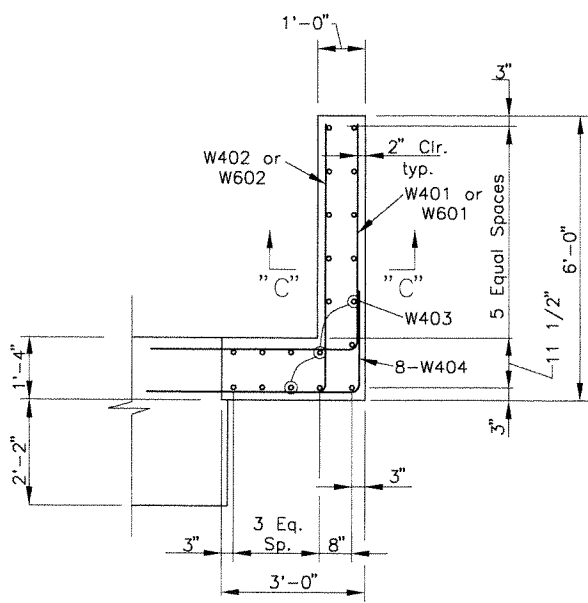
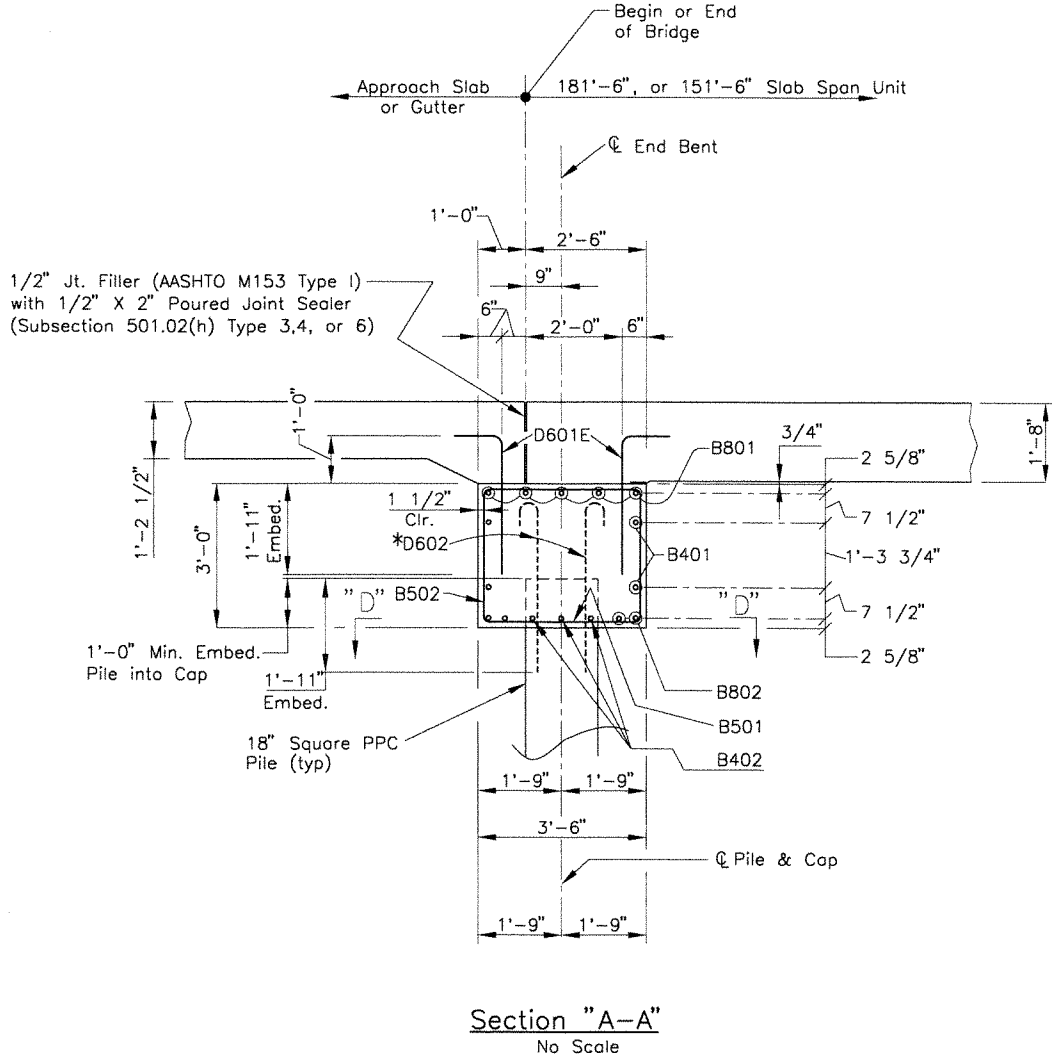


FOR SECTIONS SEE DRAWING NO.53230

Engstrom/Modjeski and Masters, Inc.

DRAWN BY: DSK DATE: 9-25-12 FILENAME: BBB0614M_B11.DGN
 CHECKED BY: SPC DATE: 9-25-12 SCALE: 1/2" = 1'-0"
 DESIGNED BY: ERD/JAM DATE: 9-25-12
 BRIDGE NO. B7266 & B7267 DRAWING NO. 53229

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. ROAD NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
					ARK.			
						JOB NO. BBO614	148	219
						B7266 & B7267	INT. BENT	53230

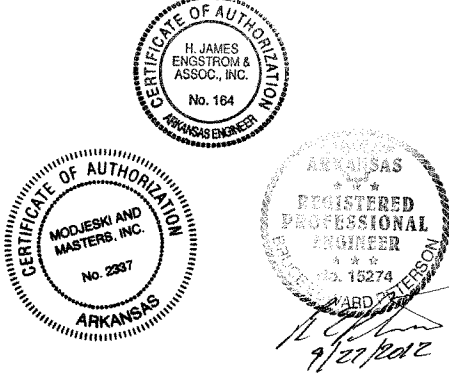


After driving, drill and grout D602 dowels 1'-11" into piles using an approved non-shrink or an epoxy grout listed on the QPL. Diameter of holes and installation procedure shall be as recommended by the grout manufacturer. Dowel bars, drilling and grout shall not be paid for directly but shall be considered subsidiary to the item "Concrete Piling (18" Square)".

BAR LIST-PER BENT

Mark	No. Req'd	Length	P.D.	Bending Diagrams
B501	51	13'-0"	2 1/2"	Dimensions are out to out of bars.
B502	18	8'-5"	2 1/2"	
B401	4	43'-3"	Str.	
B402	15	6'-11"	3	
B801	5	44'-10"	6"	
B802	4	43'-3"	Str.	
D601E	84	3'-9"	4 1/2"	
*D602	48	4'-2"	4 1/2"	
W401	6	7'-4"	2	
W402	6	7'-8"	2	
W403	40	4'-5"	Str.	
W404	16	4'-8"	2	
W601	10	8'-11"	4 1/2"	
W602	10	9'-3"	4 1/2"	

- NOTES:
- All concrete shall be Class "S" with minimum 28-day Compressive Strength, $f'_c=3,500$ psi. All exposed corners to be chamfered 3/4" unless otherwise noted. All concrete shall be poured in the dry.
 - All reinforcing steel shall conform to AASHTO M31 or M53, Gr.60.
 - For additional information, see Layout.
 - Bars with "E" suffix shall be Epoxy Coated and shall be measured and paid for as "Epoxy Coated Reinforcing Steel (Grade 60)".
- * Bar D602 is a Non-Pay item and is subsidiary to item "Concrete Piling(18" Square)".



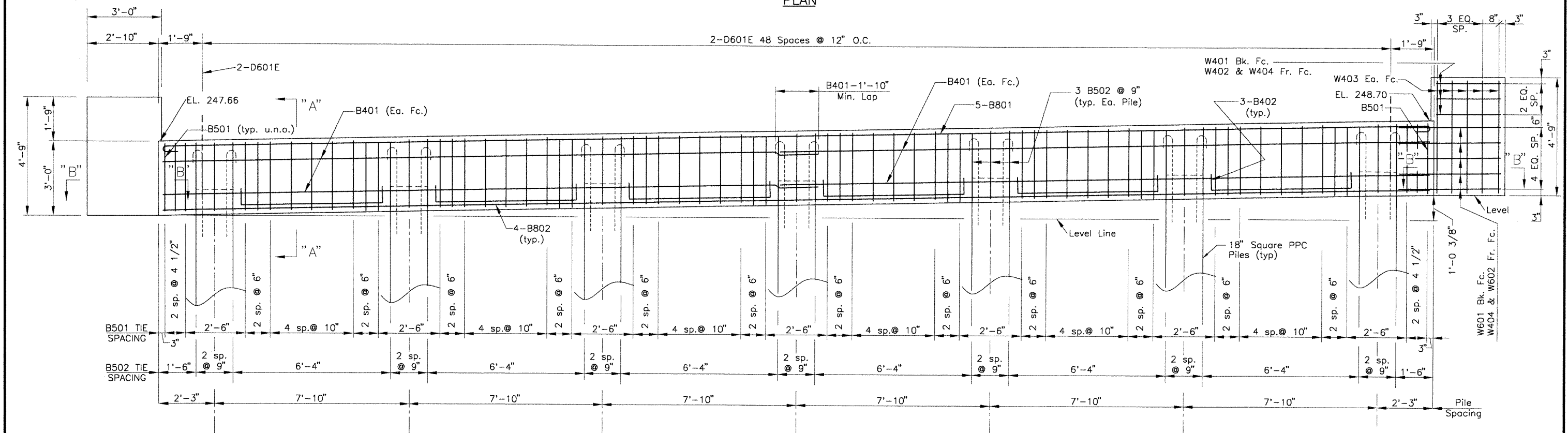
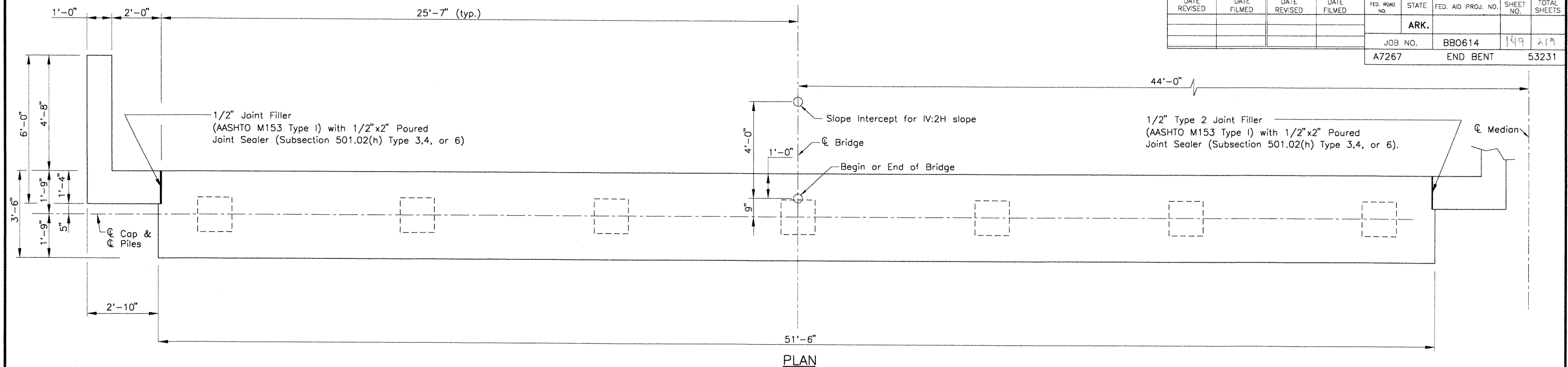
SHEET 2 OF 2
DETAILS OF END BENTS
CLEAR CREEK BRIDGE
AND LORANCE CREEK BRIDGE B
BINGHAM RD. - GRANT CO. LINE (S)
PULASKI COUNTY

ROUTE SEC.
ARKANSAS STATE HIGHWAY COMMISSION
LITTLE ROCK, ARK.

Engstrom/Modjeski and Masters, Inc.

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CHECKED BY: SPC DATE: 9-25-12 SCALE: 1/2" = 1'-0"
DESIGNED BY: ERD/JAM DATE: 9-25-12
BRIDGE NO. B7266 & B7267 DRAWING NO. 53230

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. ROAD NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
					ARK.			
				JOB NO.		BB0614	149	219
				A7267	END BENT		53231	



END BENT ELEVATION
(Looking Back-Bent No. 1)
(Bent No. 7 - Similar)

FOR SECTIONS SEE DRAWING NO.53232

SHEET 1 OF 2
DETAILS OF END BENTS
LORANCE CREEK BRIDGE A
BINGHAM RD. - GRANT CO. LINE (S)
PULASKI COUNTY

ROUTE

SEC.

ARKANSAS

STATE HIGHWAY COMMISSION

LITTLE ROCK, ARK.

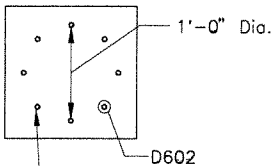
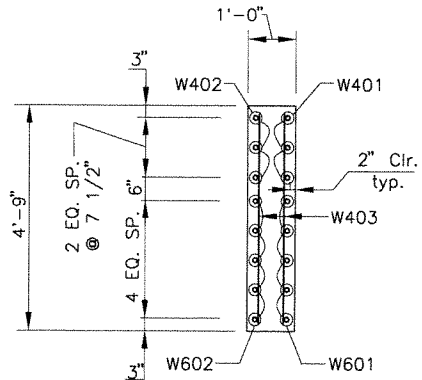
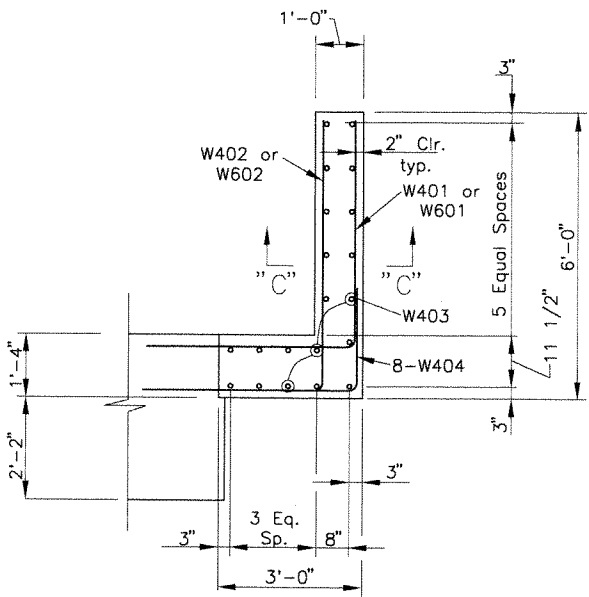
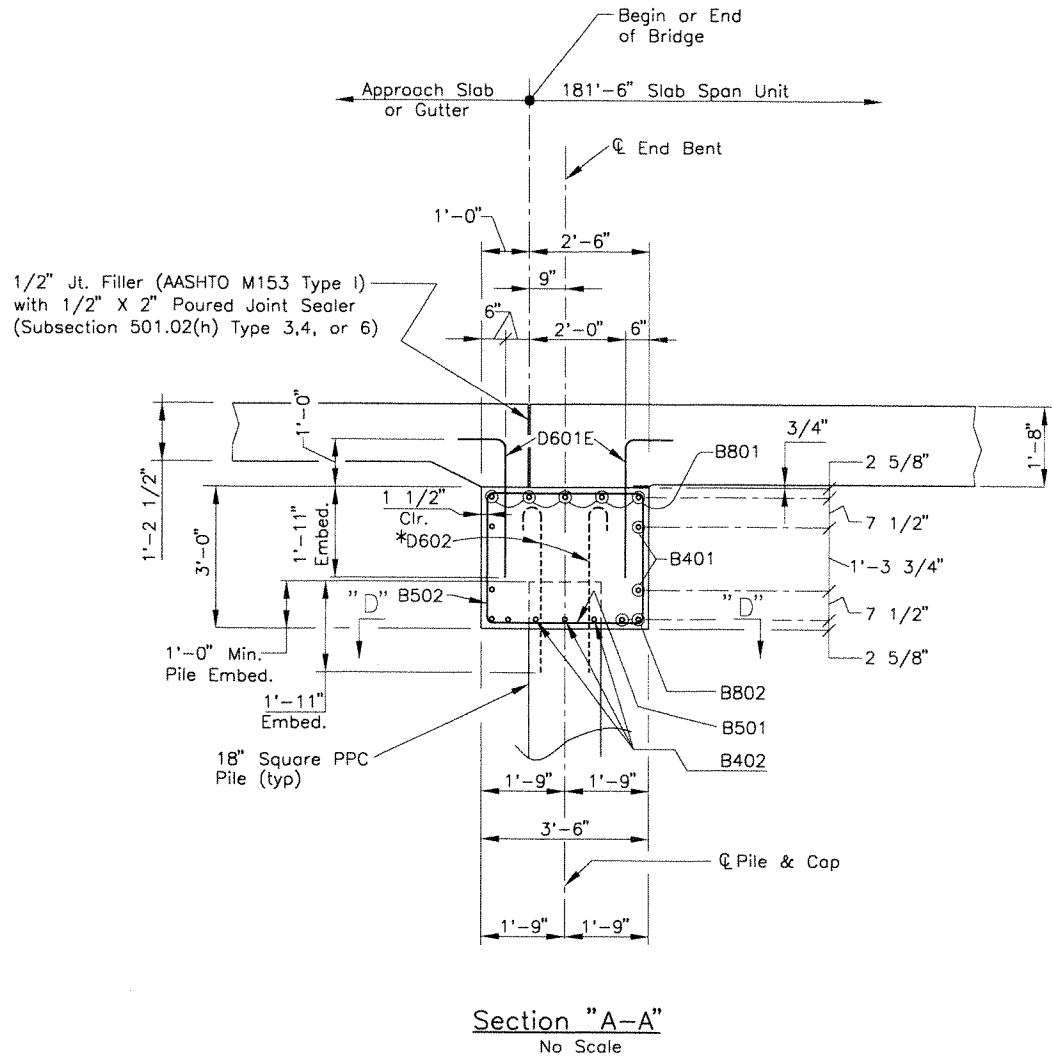
Engstrom/Modjeski and Masters, Inc.

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 CHECKED BY: SPC DATE: 9-25-12 SCALE: 1/2" = 1'-0"
 DESIGNED BY: ERD/JAM DATE: 9-25-12
 BRIDGE NO. A7267 DRAWING NO. 53231



BRIDGE ENGINEER

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. ROAD NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
					ARK.			
				JOB NO.		BBO614	150	219
				A7267		END BENT		53232



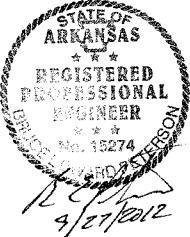
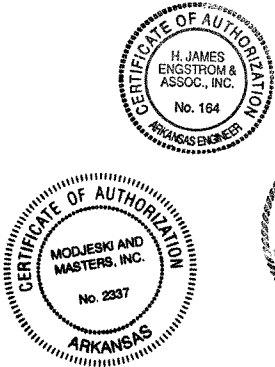
After driving, drill and grout D602 dowels 1'-11" into piles using an approved non-shrink or an epoxy grout listed on the QPL. Diameter of holes and installation procedure shall be as recommended by the grout manufacturer. Dowel bars, drilling and grout shall not be paid for directly but shall be considered subsidiary to the item "Concrete Piling (18" Square)".

BAR LIST-PER BENT

Mark	No.	Req'd	Length	P.D.	Bending Diagrams
					Dimensions are out to out of bars.
B501	60	13'-0"	2 1/2"		
B502	21	8'-5"	2 1/2"		
B401	8	26'-7"	Str.		
B402	18	6'-10"	3		
B801	5	53'-1"	6"		
B802	4	51'-3"	Str.		
D601E	98	3'-9"	4 1/2"		
*D602	56	4'-2"	4 1/2"		
W401	6	7'-4"	2		
W402	6	7'-8"	2		
W403	40	4'-5"	Str.		
W404	16	4'-8"	2		
W601	10	8'-11"	4 1/2"		
W602	10	9'-3"	4 1/2"		

- NOTES:
- All concrete shall be Class "S" with minimum 28-day Compressive Strength, $f'_c=3,500$ psi. All exposed corners to be chamfered $3/4"$ unless otherwise noted. All concrete shall be poured in the dry.
 - All reinforcing steel shall conform to AASHTO M31 or M53, Gr.60.
 - For additional information, see Layout.
 - Bars with "E" suffix shall be Epoxy Coated and shall be measured and paid for as "Epoxy Coated Reinforcing Steel (Grade 60)".

*Bar D602 is a Non-Pay item and is subsidiary to item "Concrete Piling(18" Square)".

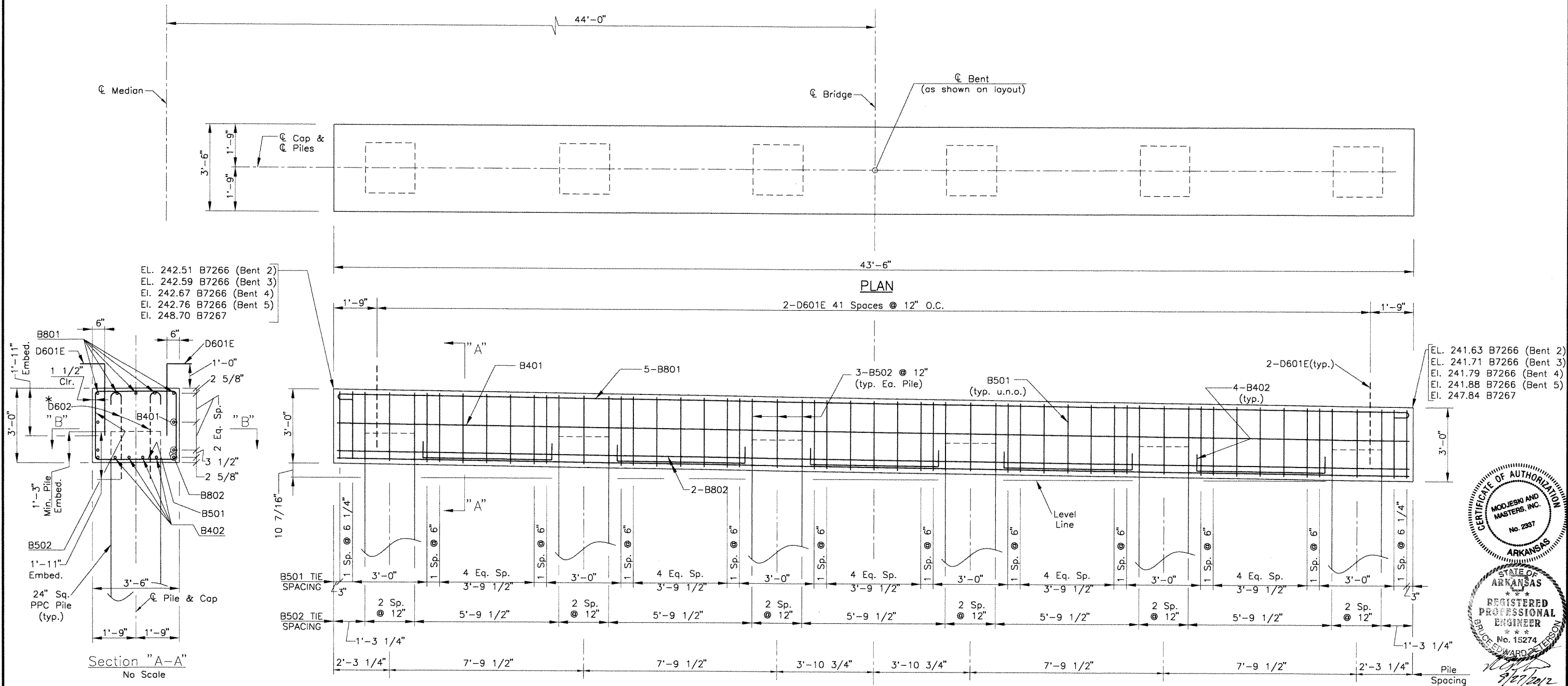


SHEET 2 OF 2
DETAILS OF END BENTS
LORANCE CREEK BRIDGE A
BINGHAM RD. - GRANT CO. LINE (S)
PULASKI COUNTY

ROUTE SEC.
ARKANSAS STATE HIGHWAY COMMISSION
LITTLE ROCK, ARK.

Engstrom/Modjeski and Masters, Inc.

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CHECKED BY: SPC DATE: 9-25-12 SCALE: 1/2" = 1'-0"
DESIGNED BY: ERD/JAM DATE: 9-25-12
BRIDGE NO. A7267 DRAWING NO. 53232



BAR LIST-PER BENT

Mark	No. Req'd	Length	P.D.	Bending Diagrams
B501	39	13'-0"	2 1/2"	Dimensions are out to out of bars.
B502	18	8'-5"	2 1/2"	
B401	2	43'-3"	Str.	
B402	20	6'-4"	3	
B801	5	44'-10"	6"	
B802	4	43'-3"	Str.	
D601E	84	3'-9"	4 1/2"	
*D602	48	4'-2"	4 1/2"	

NOTES:

- All concrete shall be Class "S" with Minimum 28-day Compressive Strength, $F'_c=3,500$ psi. All exposed corners to be chamfered $3/4"$ unless otherwise noted. All concrete shall be poured in the dry.
- All reinforcing steel shall conform to AASHTO M31 or M53, Gr.60.
- For additional information, see Layout.
- Bars with "E" suffix shall be Epoxy Coated and shall be measured and paid for as "Epoxy Coated Reinforcing Steel (Grade 60)".

*Bar D602 is a Non-Pay item and is subsidiary to item "Concrete Piling(24" Square)".

After driving, drill and grout D602 dowels 1'-11" into piles using an approved non-shrink or an epoxy grout listed on the QPL. Diameter of holes and installation procedure shall be as recommended by the grout manufacturer. Dowel bars, drilling and grout shall not be paid for directly but shall be considered subsidiary to the item "Concrete Piling (24" Square)".

Section "B-B"

No Scale

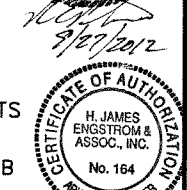
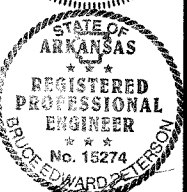
DETAILS OF INTERMEDIATE BENTS
CLEAR CREEK BRIDGE
AND LORANCE CREEK BRIDGE B
BINGHAM ROAD - GRANT CO. LINE (S)
PULASKI COUNTY

ROUTE SEC.

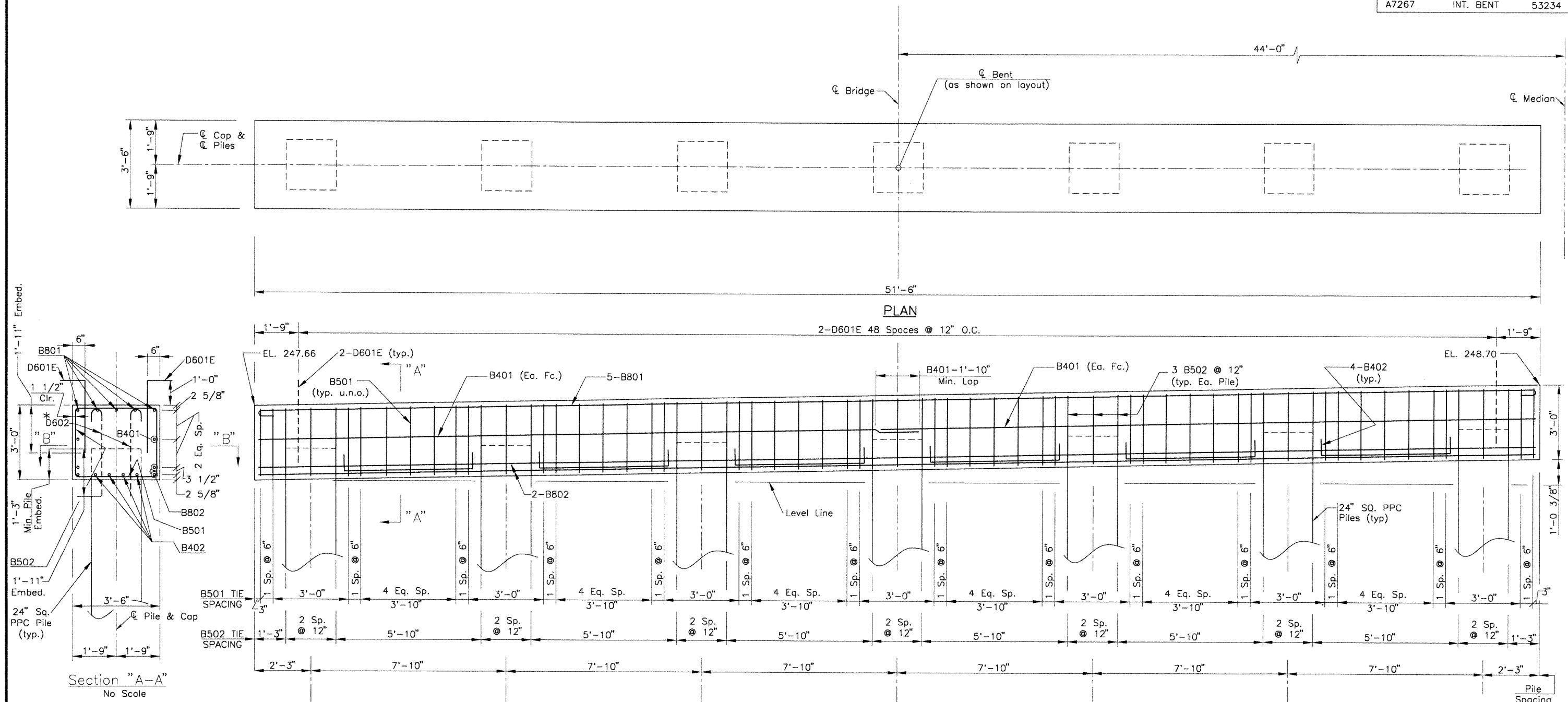
ARKANSAS STATE HIGHWAY COMMISSION
LITTLE ROCK, ARK.

Engstrom/Modjeski and Masters, Inc.

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CHECKED BY: SPC DATE: 9-25-12 SCALE: 1/2" = 1'-0"
DESIGNED BY: ERD/JAM DATE: 9-25-12
BRIDGE NO. B7266 & B7267 DRAWING NO. 53233



DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. ROAD NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
					ARK.			
				JOB NO.		BB0614	152	219
				A7267		INT. BENT		53234



BAR LIST-PER BENT					Bending Diagrams	
Mark	No. Req'd	Length	P.D.		Dimensions are out to out of bars.	
B501	46	13'-0"	2 1/2"			
B502	21	8'-5"	2 1/2"			
B401	4	26'-7"	Str.			
B402	24	6'-4"	3			
B801	5	53'-1"	6"			
B802	4	51'-3"	Str.			
D601E	98	3'-9"	4 1/2			
*D602	56	4'-2"	4 1/2			

ELEVATION
BRIDGE A7267
(LOOKING AHEAD)

SECTION "B-B"
No Scale

After driving, drill and grout D602 dowels 1'-11" into piles using an approved non-shrink or an epoxy grout listed on the QPL. Diameter of holes and installation procedure shall be as recommended by the grout manufacturer. Dowel bars, drilling and grout shall not be paid for directly but shall be considered subsidiary to the item "Concrete Piling (24" Square)".

DETAILS OF INTERMEDIATE BENTS
LORANCE CREEK BRIDGE A
BINGHAM RD. - GRANT CO. LINE (S)
PULASKI COUNTY

ROUTE SEC.
ARKANSAS STATE HIGHWAY COMMISSION
LITTLE ROCK, ARK.

Engstrom/Modjeski and Masters, Inc.

DRAWN BY: DSK DATE: 9-25-12 FILENAME: BBB0614A4_B2.DGN
CHECKED BY: SPC DATE: 9-25-12 SCALE: 1/2" = 1'-0"
DESIGNED BY: ERD/JAM DATE: 9-25-12
BRIDGE NO. A7267 DRAWING NO. 53234

NOTES:

- All concrete shall be Class "S" with minimum 28-day Compressive Strength, f'c=3,500 psi. All exposed corners to be chamfered 3/4" unless otherwise noted. All concrete shall be poured in the dry.
- All reinforcing steel shall conform to AASHTO M31 or M53, Gr.60.
- For additional information, see Layout.
- Bars with "E" suffix shall be Epoxy Coated and shall be measured and paid for as "Epoxy Coated Reinforcing Steel (Grade 60)".

*Bar D602 is a Non-Pay item and is subsidiary to item "Concrete Piling(24" Square)".

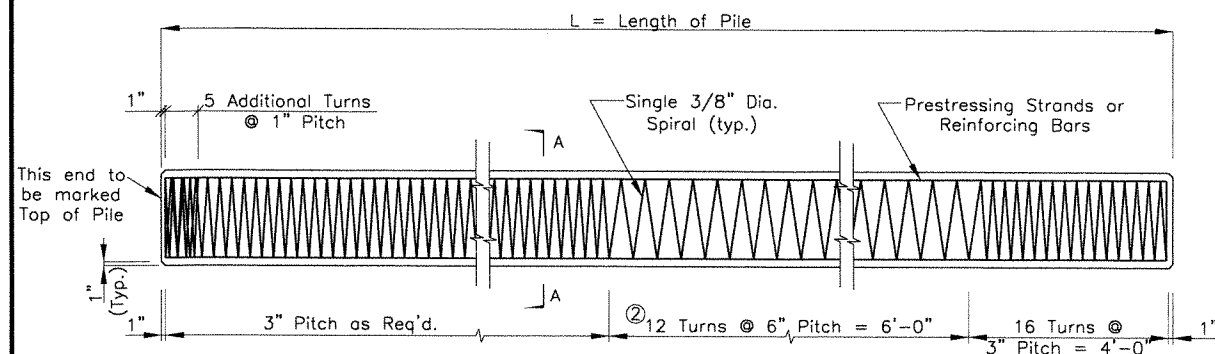
CERTIFICATE OF AUTHORIZATION
H. JAMES ENGSTROM & ASSOC., INC.
No. 164
ARKANSAS ENGINEER

CERTIFICATE OF AUTHORIZATION
MODJESKI AND MASTERS, INC.
No. 2337
ARKANSAS

STATE OF ARKANSAS
REGISTERED PROFESSIONAL ENGINEER
No. 15274
BRUCE D. JAMES

Spirals terminate with 1 1/2" turns.

② Omit this section and use 3" pitch throughout for pile lengths of 20'-0" or less.



For anchorage of Pile to Bent, see Bent Details.

PLAN OF PILE SHOWING SPIRAL TIE SPACING

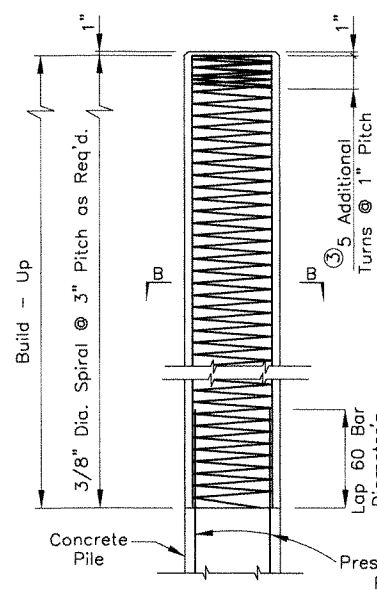
Strand location shall be symmetrical about the axis of the Pile. Circular spiral ties are required for all Piles.

Prestressed Concrete Pile Properties

	Grade	Strand Diameter	No. of Strands per Size "D" 24" Sq.	Minimum Ultimate Tensile Strength Per Strand (Lbs.)	Initial Prestressing Force Per Strand (Lbs.)
Low relaxation	250	7/16"	24	27,000	20,300
		1/2"	18	36,000	27,000
	270	7/16"	21	31,000	23,300
		1/2"	16	41,300	31,000

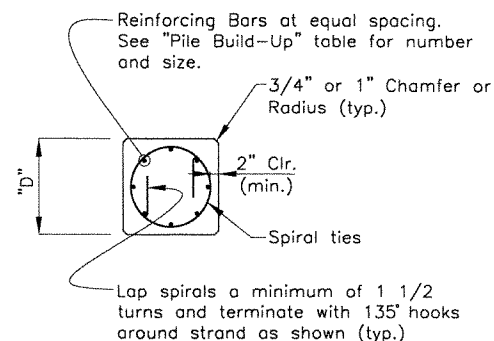
① Number based on initial pre-stress force of "B" x Ultimate Tensile Stress, Prestress Losses and min. 700 psi Unit Prestress on concrete after Losses.

"B" - 0.75 Low Relaxation



BUILD - UP

③ The five additional turns of spiral reinforcing may be omitted for build-up without additional driving.

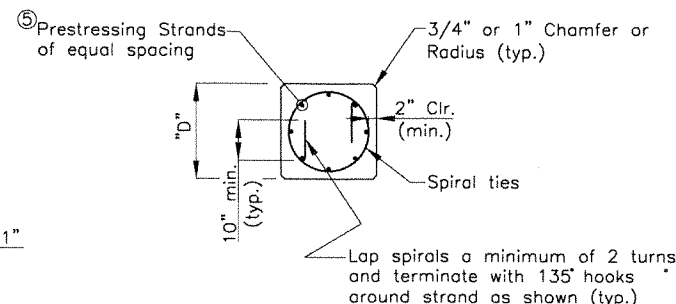


SECTION B-B

BUILD-UP PILE REINFORCING

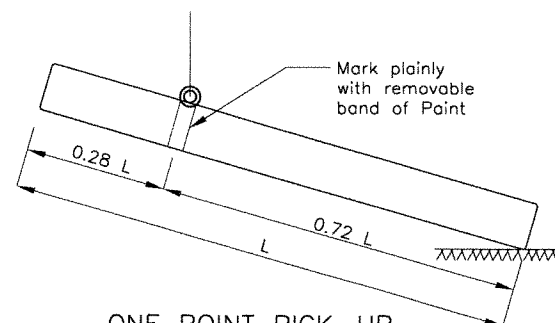
Pile Size	No. Req'd.	Bar Size
24" Sq.	12	#9

PILE BUILD-UP

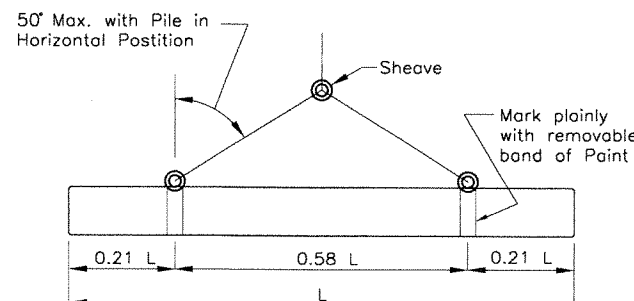


SECTION A-A

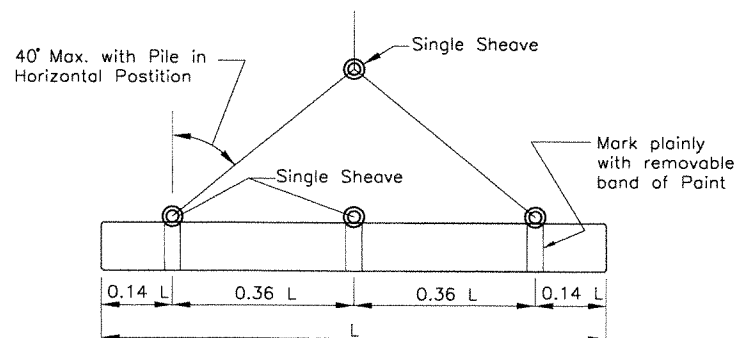
⑤ See table "Prestressed Concrete Pile Properties" for actual number of strands per pile size.



ONE POINT PICK-UP



TWO POINT PICK-UP



THREE POINT PICK-UP

MAXIMUM PICKUP LENGTHS "L"

Type of Pick-Up	Prestressed 24" Sq.
One Point	69'
Two Point	104'
Three Point	138'

GENERAL NOTES:

CONSTRUCTION SPECIFICATIONS: Arkansas State Highway and Transportation Department Standard Specifications for Highway Construction (2003 Edition) with applicable Supplemental Specifications and Special Provisions.

DESIGN SPECIFICATIONS: AASHTO LRFD Bridge Design Specifications, Sixth Edition (2012).

SEISMIC PERFORMANCE ZONES: 1 & 2

The Contractor shall use prestressed piles only. Piles will be measured and paid for at the contract unit price bid for "Concrete Piling".

CONCRETE: Concrete in prestressed piles shall be Class S(AE) and shall have a minimum compressive strength (f'c) of 5,000 psi at 28 days. Compressive strength at transfer of the prestressing force shall be not less than 4,000 psi. Concrete in build-ups shall have a minimum compressive strength of 4,000 psi and shall be cured for a minimum of 10 days.

COARSE AGGREGATE: Maximum size of coarse aggregate shall be 3/4".

PRESTRESSING REINFORCING: Seven-wire low relaxation strands shall conform to the general requirements of AASHTO M 203. Broken wires within individual strands will be permitted up to 2% of the total number of wires in each pile, providing that there is not more than one broken wire per strand. Two or more broken wires per strand will be cause for replacement of the strand, even though the two broken wires are within the 2% limitation.

SPIRAL REINFORCING: Spiral reinforcing shall be steel wire meeting the requirements of AASHTO M 32 or M 225 or shall be plain round steel bars meeting the requirements of AASHTO M 31 or M 53, Gr. 60.

REINFORCING STEEL: Reinforcing Steel for pile build-up shall conform to AASHTO M 31 or M 53.

MANUFACTURE, TRANSPORTATION AND STORAGE: Shipment of piles from the plant site or pile driving will not be permitted until the required minimum compressive strength is reached, and in no case less than 10 days after pouring the concrete. Prestressed piles may be removed from the casting bed to nearby storage any time after transfer of stress. See Section 802 "Concrete for Structures" of the Standard Specifications for additional information.

Unless otherwise approved by the Engineer, all protruding or exposed pile lifting or transporting devices above the finished ground shall be removed after pile driving is complete. Removal shall be a minimum of 1" below the surface of the pile and the cavity shall be filled with a non-shrink grout listed on the Department's OPL.

FORMS: For forming exterior of piles, the use of steel forms on concrete-founded casting beds is required unless otherwise approved by the Engineer. Side forms may have a maximum drift on each side not exceeding 1/4" per foot.

TOLERANCES: Pile ends shall be plane surfaces perpendicular to the longitudinal axis of pile with a maximum tolerance of 1/8" per foot transversely.

The maximum sweep (deviation from straightness measured from end to end of the pile, while not subject to bending forces) shall not exceed 1/8" in 10 feet.

BUILD-UPS: To provide for build-ups of piles where authorized by the Engineer, concrete shall be cut back to expose the reinforcing steel for a distance sufficient to provide a lap of 60 diameters of the reinforcing bars required for build-up.

INSTALLATION, MEASUREMENT AND PAYMENT: See Section 805 "Piling" of the Standard Specifications.



DETAILS OF 24" SQUARE PRESTRESSED CONCRETE PILES

CLEAR CREEK BRIDGE AND LORANCE CREEK BRIDGES

BINGHAM RD. - GRANT CO. LINE (S)
PULASKI COUNTY

ROUTE 530 SEC. 1 & 3

ARKANSAS STATE HIGHWAY COMMISSION
LITTLE ROCK, ARK.

Engstrom/Modjeski and Masters

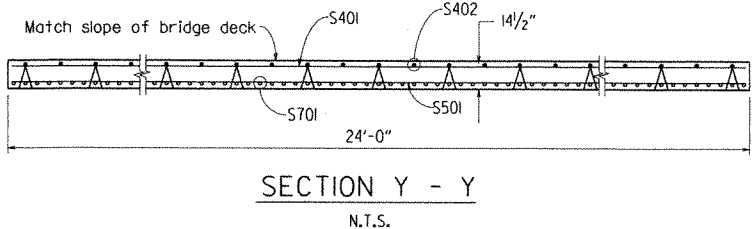
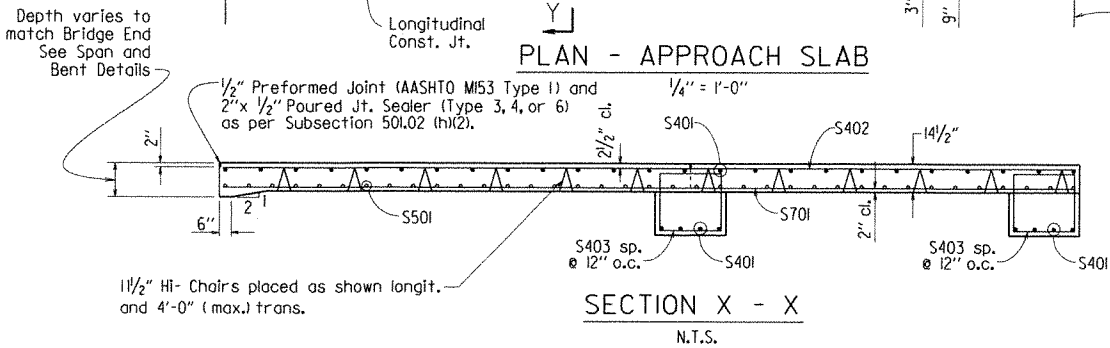
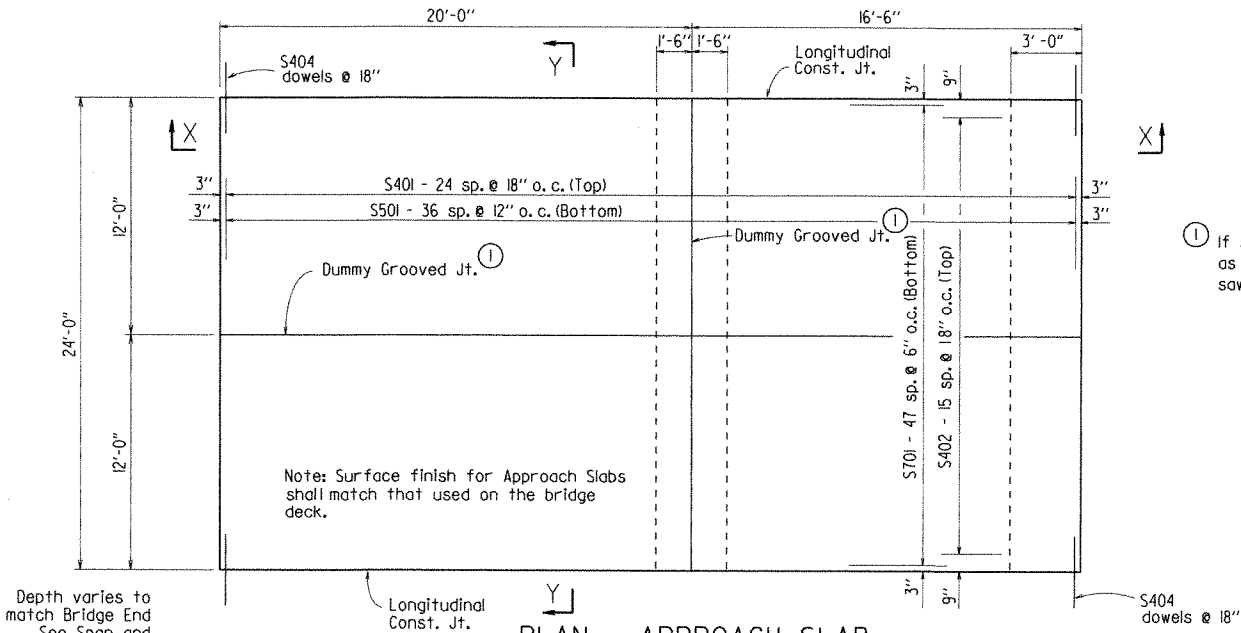
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CHECKED BY: SPC DATE: 9-25-12 SCALE: 1/2" = 1'-0"
DESIGNED BY: BEP DATE: 9-25-12

BRIDGE ENGINEER

BRIDGE NO. B7266, A7267 & B7267 DRAWING NO. 53235

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB NO.		BB0614	134	219
				B7266&B7267 - APPR. SLAB - 53236				

① If slab joints are to be sawed, they shall be sawed as soon as the concrete has set enough to allow sawing of the joints without damaging the slab.



BAR LIST

Mark	No. Req'd.	Length	Bending Diagrams
S401	33	23'-8"	
S402	16	36'-2"	
S403	48	10'-4"	
S404	50	3'-0"	
S501	37	23'-8"	
S701	48	36'-2"	

GENERAL NOTES
Concrete shall be Class S (AE) (f'c = 4,000 psi).
Reinforcement Steel shall conform to AASHTO M31 or M53, Grade 60 (fy = 60,000 psi).
Approach Slabs will be measured and paid for in accordance with Section 504.

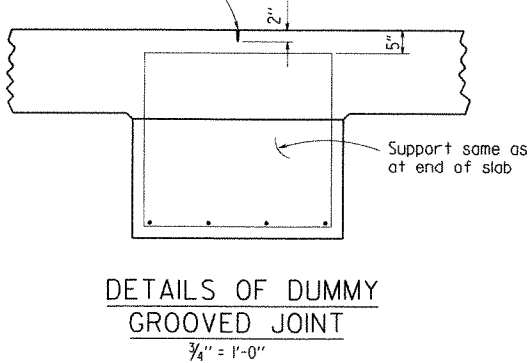
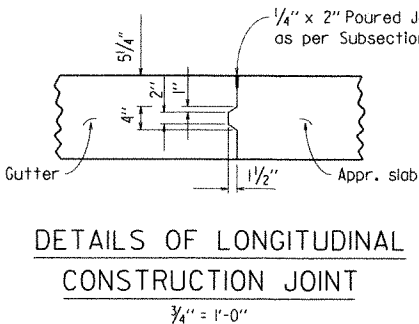
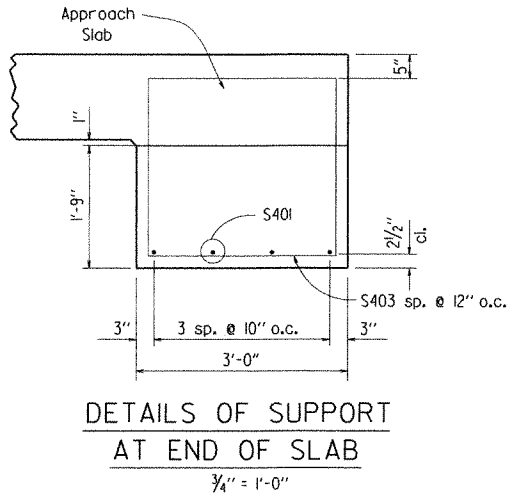
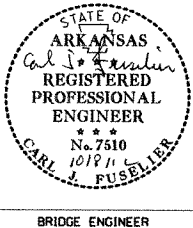


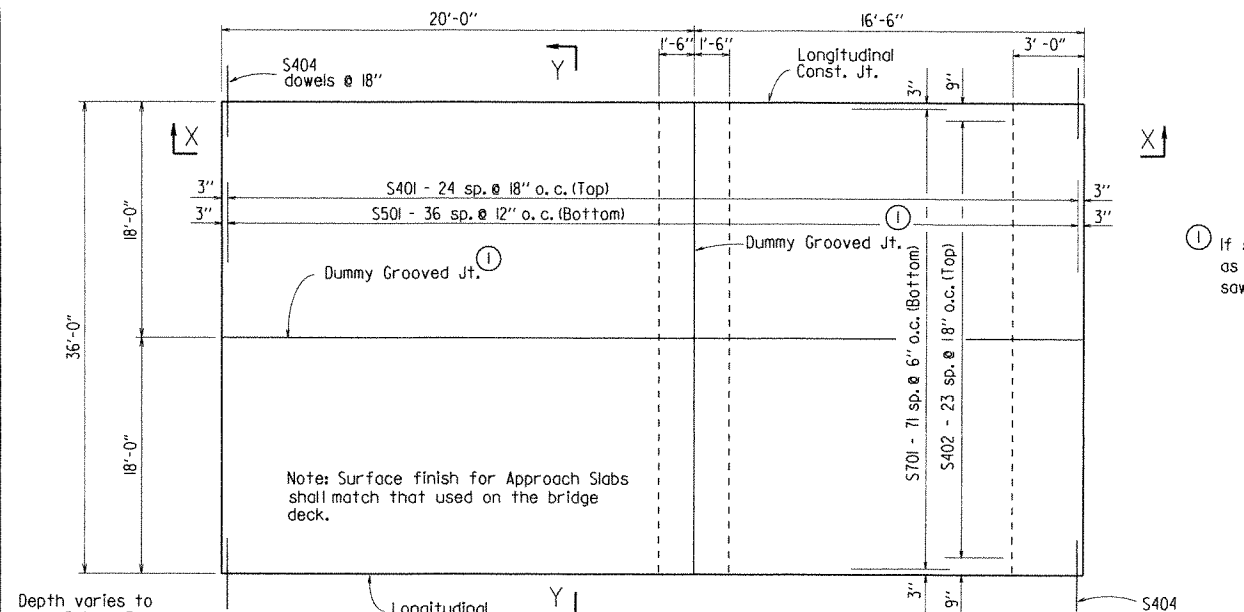
TABLE OF QUANTITIES FOR ONE APPROACH SLAB

Slab Width	Reinforcing Steel	Concrete (Cu. Yds.)
24'-0"	5,802 lbs.	49.40

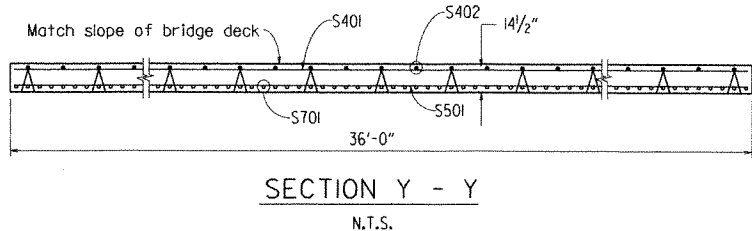
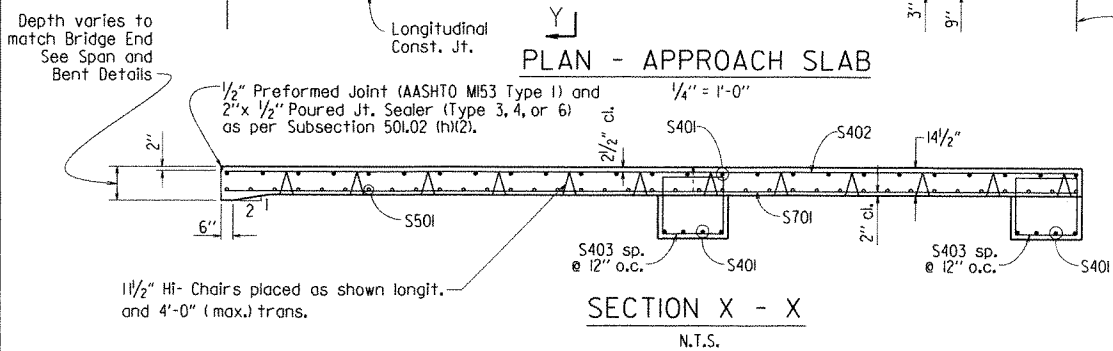


DETAILS OF TYPE SPECIAL 2
APPROACH SLAB
ROUTE SEC.
ARKANSAS STATE HIGHWAY COMMISSION
LITTLE ROCK, ARK.
DRAWN BY: LJB DATE: 8/24/12 FILENAME: bb0614_asl.dgn
CHECKED BY: TMC DATE: 9/28/12 SCALE: AS SHOWN
DESIGNED BY: DATE:
BRIDGE NO. B7266&B7267 DRAWING NO. 53236

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB NO.		BB0614	155	219
				AT267 -	APPR. SLAB.	-	53237	



① If slab joints are to be sawed, they shall be sawed as soon as the concrete has set enough to allow sawing of the joints without damaging the slab.



BAR LIST

Mark	No. Req'd.	Length	Bending Diagrams
S401	33	35'-8"	
S402	24	36'-2"	
S403	72	10'-4"	
S404	50	3'-0"	
S501	37	35'-8"	
S701	72	36'-2"	

GENERAL NOTES

Concrete shall be Class S (AE) (f'c = 4,000 psi).
Reinforcement Steel shall conform to AASHTO M31 or M53, Grade 60 (fy = 60,000 psi).
Approach Slabs will be measured and paid for in accordance with Section 504.

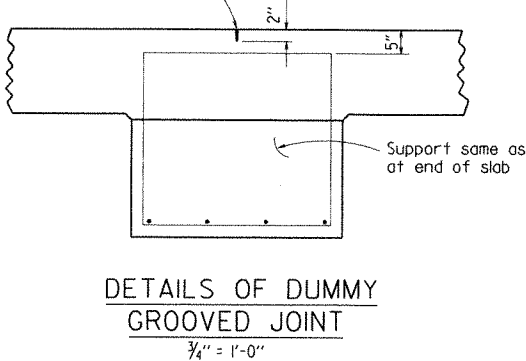
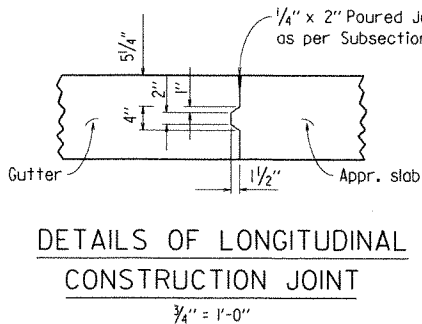
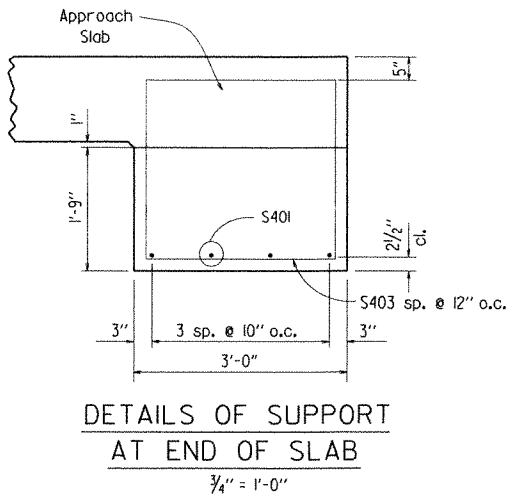
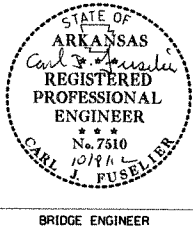


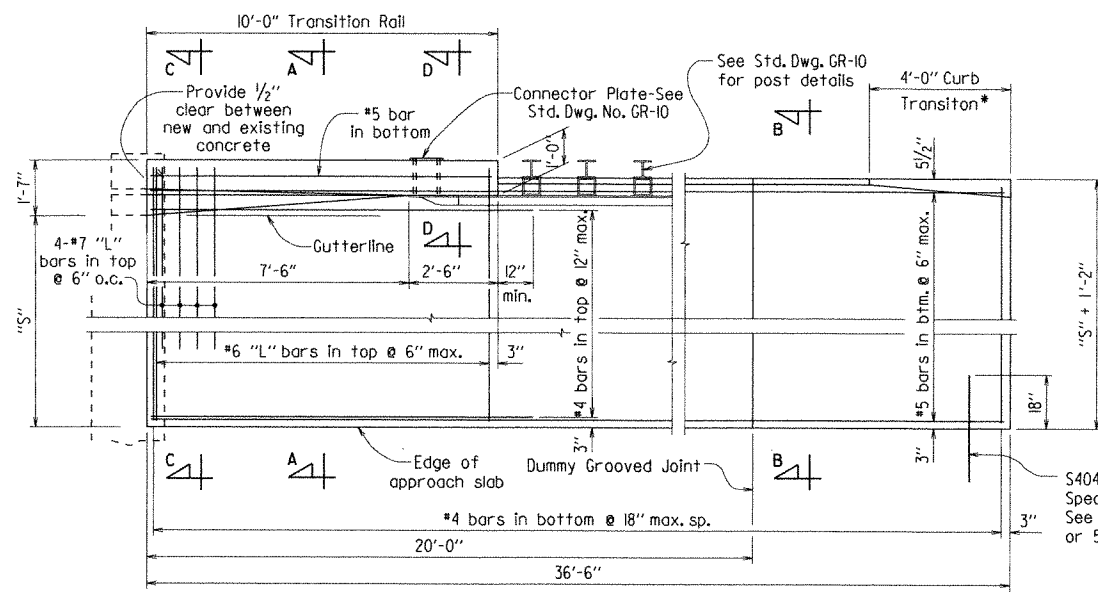
TABLE OF QUANTITIES FOR ONE APPROACH SLAB

Slab Width	Reinforcing Steel	Concrete (Cu. Yds.)
36'-0"	8663 lbs.	74.10



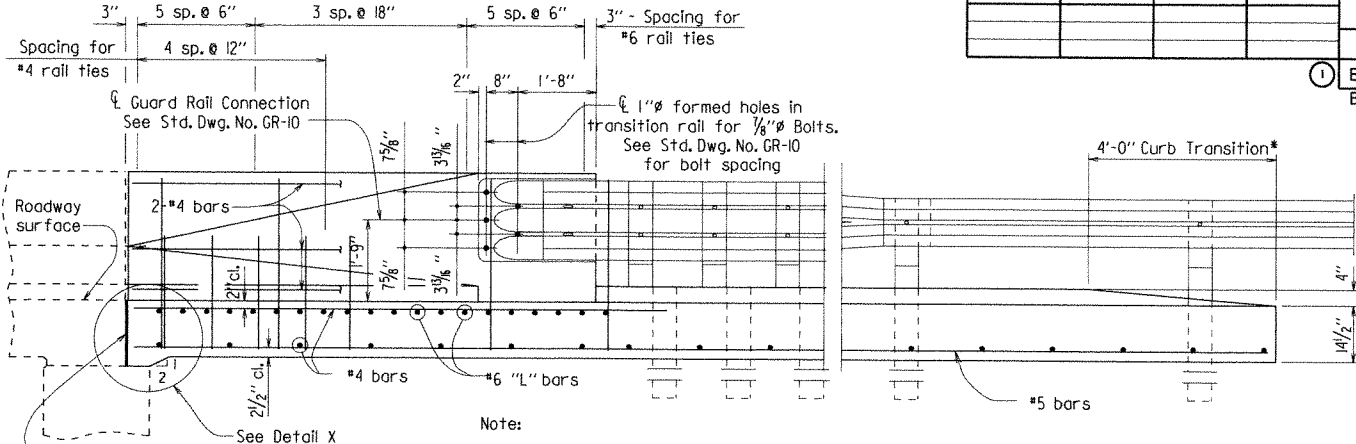
DETAILS OF TYPE SPECIAL 3
APPROACH SLAB
ROUTE SEC.
ARKANSAS STATE HIGHWAY COMMISSION
LITTLE ROCK, ARK.
DRAWN BY: LJB DATE: 8/24/12 FILENAME: bb0614.asldgn
CHECKED BY: TMG DATE: 9/28/12 SCALE: AS SHOWN
DESIGNED BY: DATE:
BRIDGE NO. A7267 DRAWING NO. 53237

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB NO.		BB0614	156	219
						B7266, A7267 - APPR. GUTTER - 53238		
						B7267		



PLAN
Scale: 3/8" = 1'-0"

*S' = Distance from gutterline to edge of approach slab.

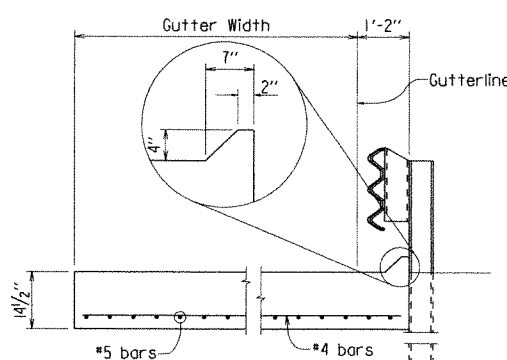
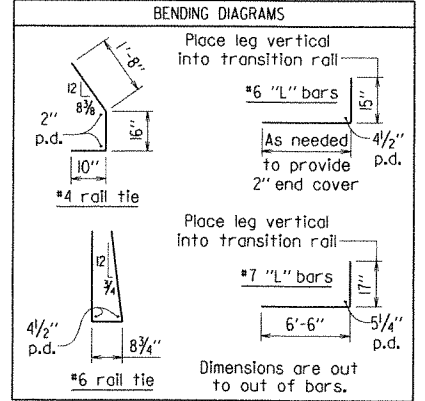


LONGITUDINAL SECTION THRU GUTTER
Scale: 1/2" = 1'-0"

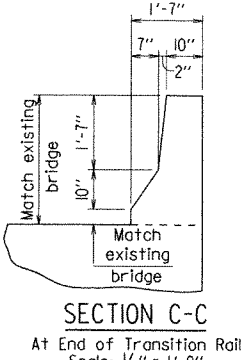
* Construct curb with height-transition as shown if drop inlet is not used at end of gutter.
Construct curb full height (no height-transition) if drop inlet is used at end of gutter. Curb height transition placed on drop inlet. See drop inlet details.

GENERAL NOTES

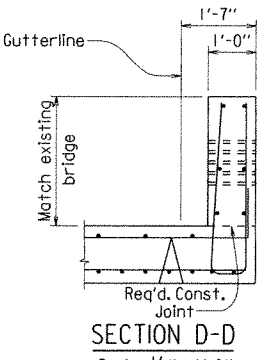
Concrete shall be Class S(AE) (f'c = 4,000 psi.)
Reinforcing steel shall conform to AASHTO M31 or M53, Grade 60 (fy = 60,000 psi.) Fabricate bar lengths to provide 2" minimum cover at each end.
Approach gutters will be measured and paid for in accordance with Section 504 of the Standard Specifications.



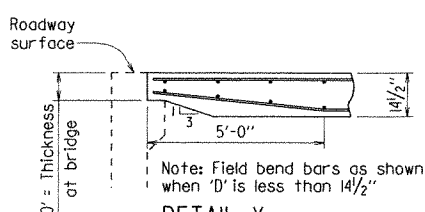
SECTION B-B
Scale: 1/2" = 1'-0"



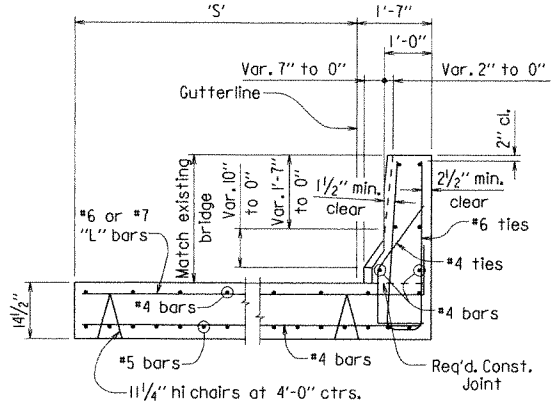
SECTION C-C
At End of Transition Rail
Scale: 1/2" = 1'-0"



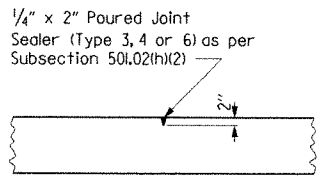
SECTION D-D
Scale: 1/2" = 1'-0"



DETAIL X
Scale: 3/8" = 1'-0"



SECTION A-A
Scale: 1/2" = 1'-0"



DETAILS OF DUMMY GROOVED JOINT
Scale: 1/2" = 1'-0"

QUANTITIES FOR ONE APPROACH GUTTER

S'	Concrete	Reinforcing Steel
6'-0"	13.19 cu.yd.	1238 lb.
10'-0"	19.79 cu.yd.	1758 lb.



DETAILS OF TYPE SPECIAL 2 APPROACH GUTTERS

ROUTE 100
SEC. 1
ARKANSAS STATE HIGHWAY COMMISSION
LITTLE ROCK, ARK.

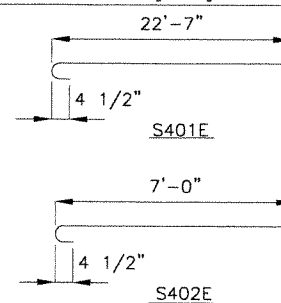
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CHECKED BY: TMG DATE: 9/28/12 SCALE: AS NOTED
DESIGNED BY: DATE: -
BRIDGE NO. B7266, B7267 DRAWING NO. 53238
B7267

BRIDGE ENGINEER

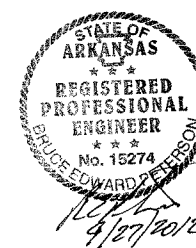
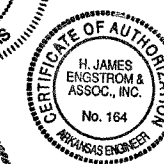
BAR LIST

	Mark	No. Reqd.	Length	Pin Dia.
TOP	S401E	518	23'-1"	3"
	S402E	516	7'-6"	3"
	S801E	178	48'-5"	Str.
	S802E	178	37'-3"	Str.
BOTTOM	S403E	518	26'-7"	Str.
	S803E	178	33'-1"	Str.
	S804E	267	36'-5"	Str.
RAILS	P401E	568	5'-6"	3"
	P402E	40	4'-11"	3"
	P403E	80	5'-6"	Str.
	P404E	112	7'-8"	Str.
	P405E	70	13'-8"	Str.
	P406E	28	8'-5"	Str.
	P501E	568	6'-11"	3 3/4"

Bending Diagrams



For Bending Diagrams not shown,
see Drawing Number 53247.
Dimensions are out to out of bars.
Bar Designations using "E" suffix are
epoxy coated.

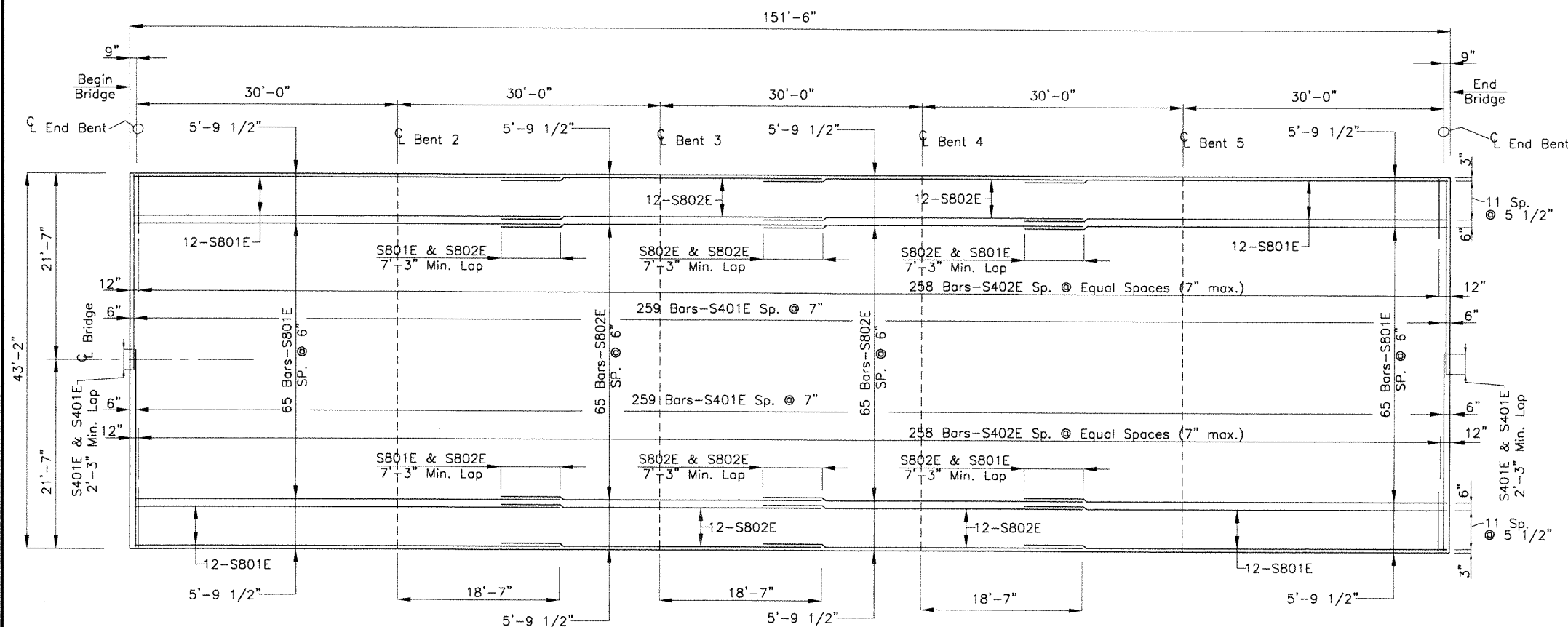


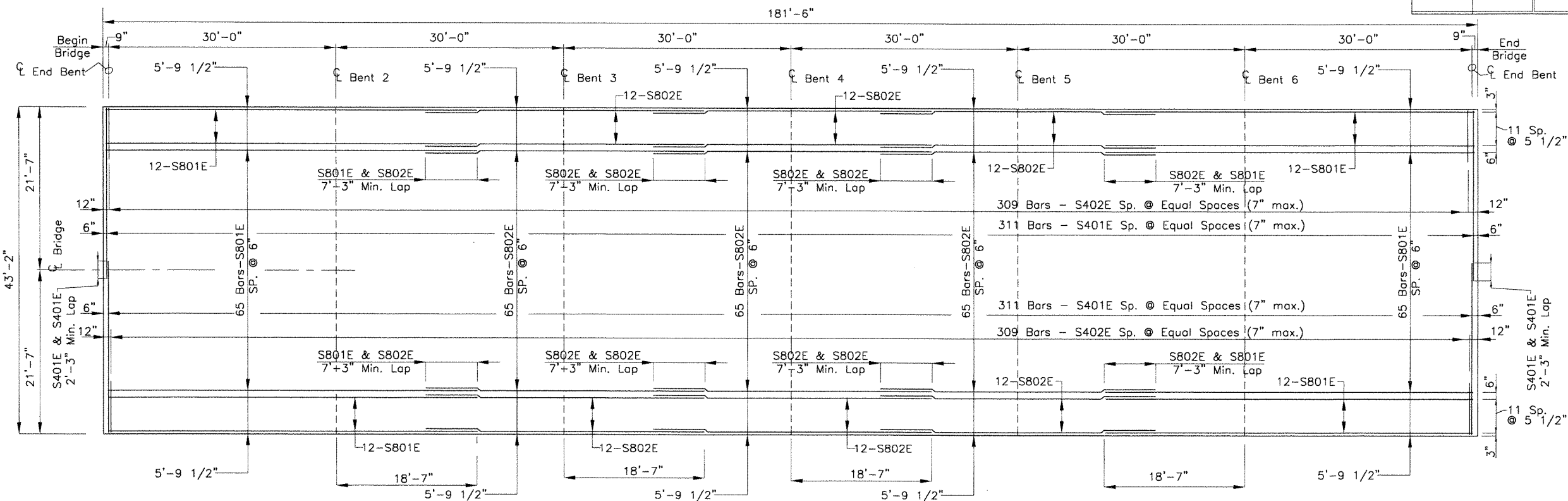
SLAB REINFORCING PLAN
151'-6" CONTINUOUS R. C. SLAB UNIT
CLEAR CREEK BRIDGE
BINGHAM RD. - GRANT CO. LINE (S)
PULASKI COUNTY

ROUTE SEC.
ARKANSAS STATE HIGHWAY COMMISSION
LITTLE ROCK, ARK.

Engstrom/Modjeski and Masters

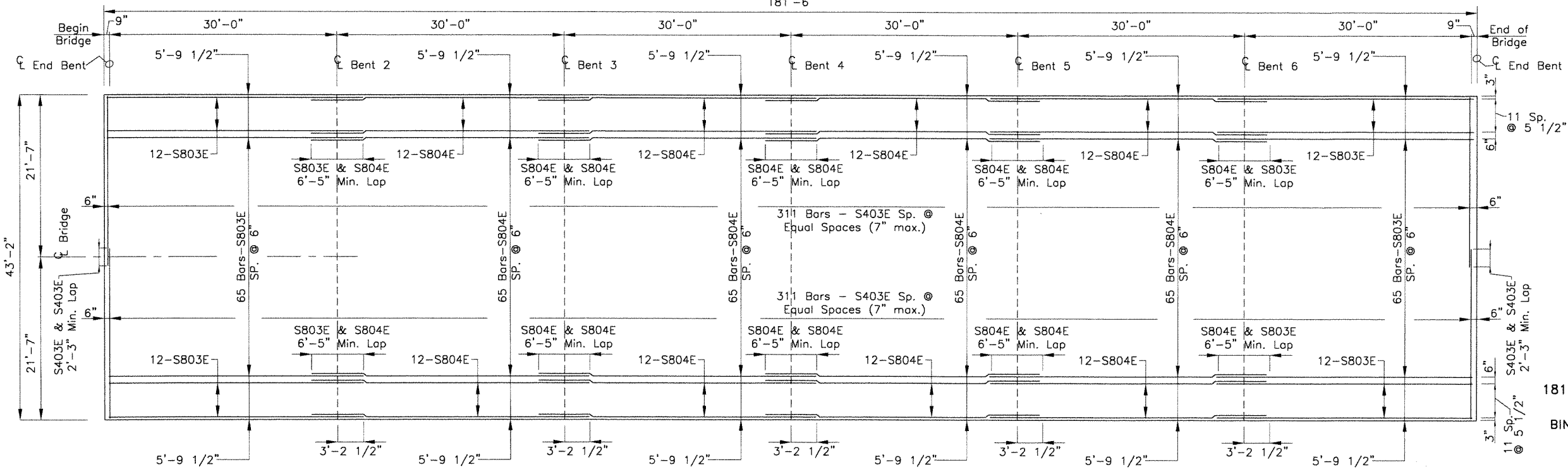
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CHECKED BY: SPC DATE: 9-25-12 SCALE: As Noted
DESIGNED BY: JAM/SPC DATE: 9-25-12
BRIDGE NO. B7266 DRAWING NO. 53240





REINFORCING PLAN - TOP OF SLAB

Scale: 1/8" = 1'-0"
Lorance Creek Bridge "B"
181'-6"



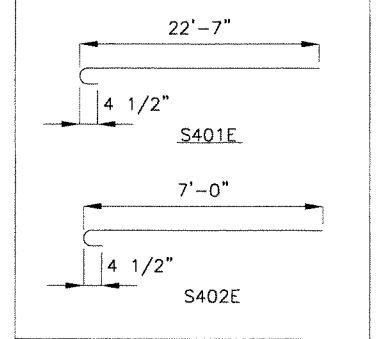
REINFORCING PLAN - BOTTOM OF SLAB

Scale: 1/8" = 1'-0"
Lorance Creek Bridge "B"

BAR LIST

Mark	No. Reqd.	Length	Pin Dia.
S401E	622	23'-1"	3"
S402E	618	7'-6"	3"
S801E	178	48'-5"	Str.
S802E	267	37'-3"	Str.
TOP			
S403E	622	22'-7"	Str.
S803E	178	33'-1"	Str.
S804E	356	36'-5"	Str.
BOTTOM			
P401E	680	5'-6"	3"
P402E	48	4'-11"	3"
P403E	96	5'-6"	Str.
P404E	140	7'-8"	Str.
P405E	84	13'-8"	Str.
P406E	28	8'-5"	Str.
P501E	680	6'-11"	3 3/4"

Bending Diagrams



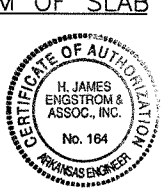
For Bending Diagrams not shown, see Drawing Number 53247. Dimensions are out to out of bars. Bar Designations using "E" suffix are epoxy coated.

SLAB REINFORCING PLAN
181'-6" CONTINUOUS R. C. SLAB UNIT
LORANCE CREEK BRIDGE B
BINGHAM RD. - GRANT CO. LINE (S)
PULASKI COUNTY

ROUTE SEC.
ARKANSAS STATE HIGHWAY COMMISSION
LITTLE ROCK, ARK.

Engstrom/Modjeski and Masters

DRAWN BY: DSK DATE: 9-25-12 FILENAME: BBB0614B4_S1.DGN
CHECKED BY: SPC DATE: 9-25-12 SCALE: As Noted
DESIGNED BY: JAM/SPC DATE: 9-25-12
BRIDGE NO. B7267 DRAWING NO. 53241



BRIDGE ENGINEER

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. ROAD NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
					ARK.			
				JOB NO.	BB0614	160	219	

Technical drawing of a bridge deck cross-section. The drawing shows a symmetrical layout with a central roadway and shoulders. Key dimensions include a total width of 51'-2" Out to Out, a 48'-0" Clear Roadway, and 24'-0" lane widths. Reinforcement details specify 81 Bars-S801E or S802E at 6" max. Sp. Top and 81 Bars-S803E or S804E at 6" max. Sp. Bot. The drawing also indicates a 2% slope, a 3/4" riser at bents, and required construction joints at the level.

Dimensions and Labels:

- 51'-2" Out to Out
- 44'-0"
- 48'-0" Clear Roadway
- 24'-0"
- 24'-0"
- 1'-5"
- 2"
- 6'-0" Shoulder
- 12'-0" Lane
- 12'-0" Lane
- 12'-0" Lane
- 6'-0" Shoulder
- CL Bridge
- CL Lanes
- 6'-0"
- CL Median
- Level Line
- 2% Slope
- S401E (typ.)
- 2'-3" Min. Lap #4's
- 1'-8"
- 2'-9"
- Req'd. Const. Jt. (Level)
- S402E
- CL 3/4" Drip Groove
- 3"
- Req'd. Const. Jt. (Level)
- 6"
- 81 Bars-S801E, or S802E @ 6" max. Sp. Top
- 81 Bars-S803E, or S804E @ 6" max. Sp. Bot
- 12 Bars-S801E, or S802E @ 5 1/2" Sp. Top
- 12 Bars-S803E, or S804E @ 5 1/2" Sp. Bot.
- 3/4" Riser @ Bents
- CL 3/4" Drip Groove
- 3"

LORANCE CREEK BRIDGE "A"
Scale: 1/2" = 1'-0"

C.L. Const. Jt. →

1/2" x 1" Type 3, 4, or 6
Joint sealer

1/2" 5" 1 1/2"

7 1/4"

1'-8"

2"

Not to Scale

DRAWN BY: GAE DATE: 9-25-12 FILENAME: BBB0614A4_S1.DGN
 CHECKED BY: SPC DATE: 9-25-12 SCALE: As Noted
 DESIGNED BY: ERD/JAM DATE: 9-25-12
 BRIDGE NO. A7267 DRAWING NO. 53242

Not to Scale

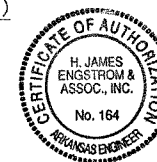
Diagram illustrating the assembly of a concrete cap and bearing pad:

- 12"**: Dimension indicating the width of the concrete cap.
- 1/8" x 4" Bearing Pad (Glue to Cap)**: Label for the bearing pad.
- 1/2" Rounding**: Dimension indicating the radius of the top corner of the cap.
- Note: After Deck forms are removed, Face of Pad shall be cleaned of any Laitant Concrete (Typ.)**: Instruction for post-pouring cleanup.
- 3/4"**: Dimension indicating the thickness of the bearing pad.
- Top of Cap**: Label for the top surface of the concrete cap.
- 4" (Typ.)**: Dimension indicating the typical width of the base of the cap.

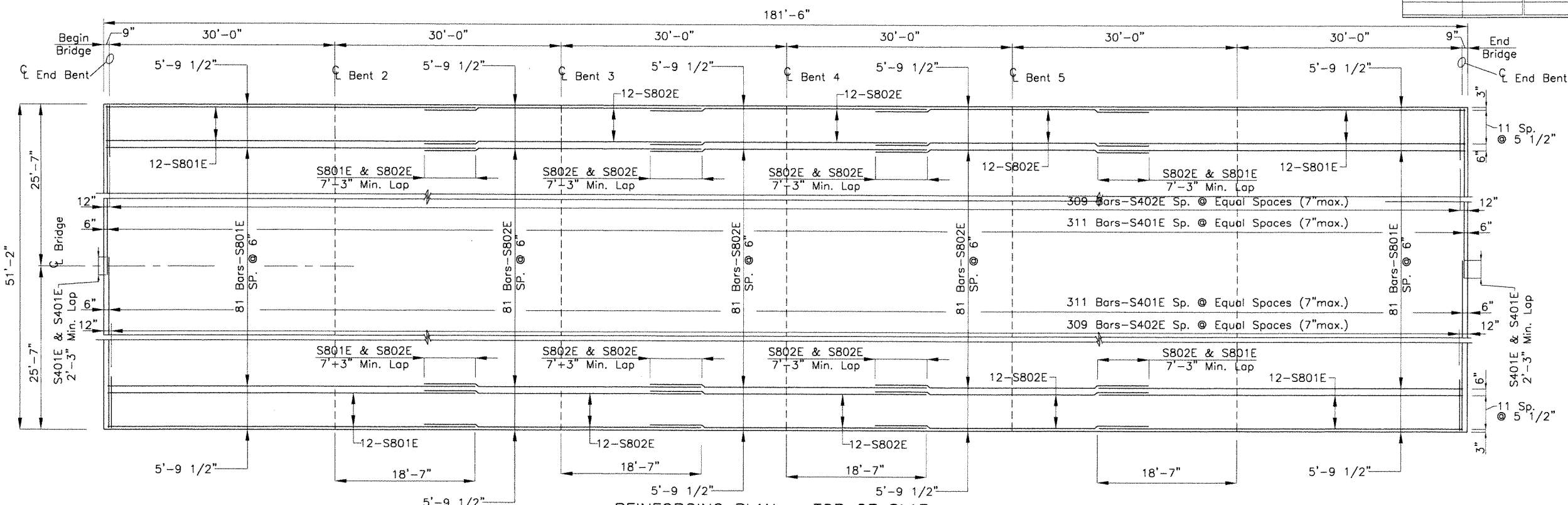
Note: Joint to be cleaned by sandblasting or other approved method before pouring joint sealer.

Not to Scale

For General Notes, See Drawing No.53246

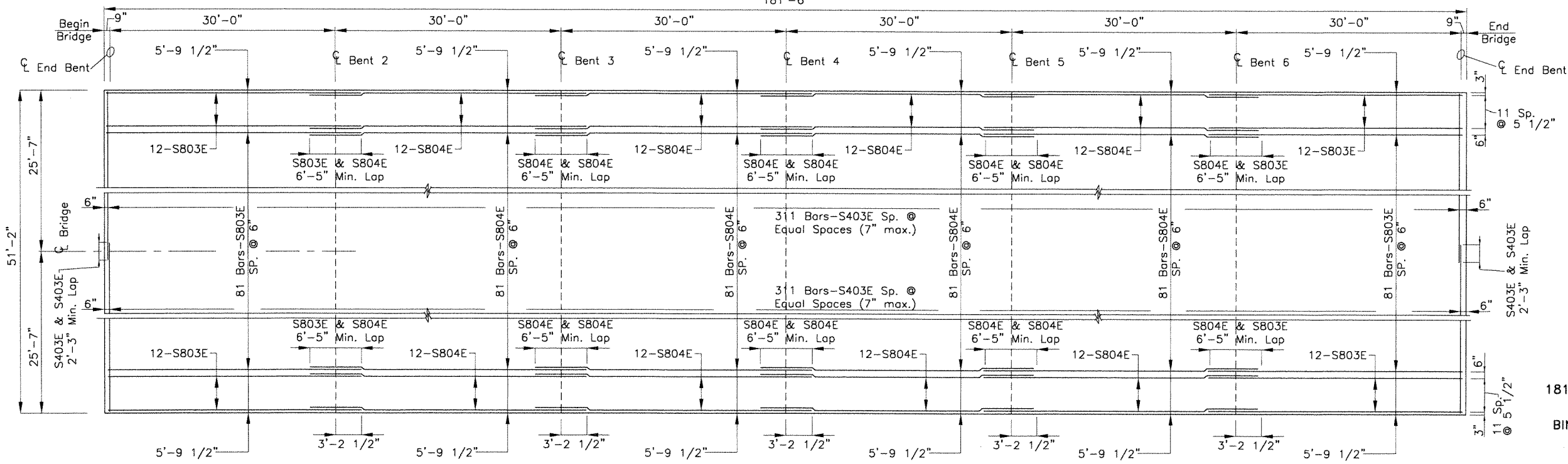


BRIDGE ENGINEER



REINFORCING PLAN - TOP OF SLAB

Scale: 1/8" = 1'-0"
Lorance Creek Bridge "A"
181'-6"



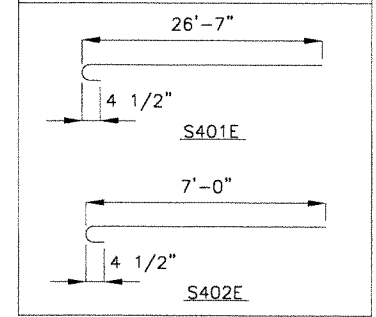
REINFORCING PLAN - BOTTOM OF SLAB

Scale: 1/8" = 1'-0"
Lorance Creek Bridge "A"

BAR LIST

Mark	No. Req'd.	Length	Pin Dia.
S401E	622	27'-1"	3"
S402E	618	7'-6"	3"
S801E	210	48'-5"	Str.
S802E	315	37'-3"	Str.
TOP			
S403E	622	26'-7"	Str.
S803E	210	33'-1"	Str.
S804E	420	36'-5"	Str.
BOTTOM			
P401E	680	5'-6"	3"
P402E	48	4'-11"	3"
P403E	96	5'-6"	Str.
P404E	140	7'-8"	Str.
P405E	84	13'-8"	Str.
P406E	28	8'-5"	Str.
P501E	680	6'-11"	3 3/4"

Bending Diagrams

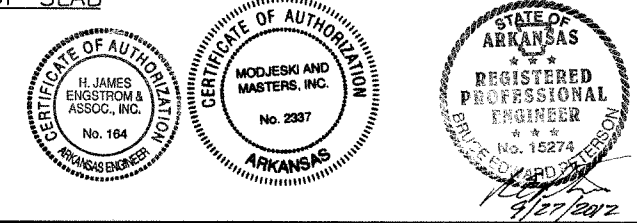


For Bending Diagrams not shown, see Drawing Number 53247. Dimensions are out to out of bars. Bar Designations using "E" suffix are epoxy coated.

SLAB REINFORCING PLAN
181'-6" CONTINUOUS R. C. SLAB UNIT
LORANCE CREEK BRIDGE A
BINGHAM RD. - GRANT CO. LINE (S)
PULASKI COUNTY

ROUTE SEC.
ARKANSAS STATE HIGHWAY COMMISSION
LITTLE ROCK, ARK.

Engstrom/Modjeski and Masters



DRAWN BY: DSK DATE: 9-25-12 FILENAME: BBB0614A4_S2.DGN
CHECKED BY: SPC DATE: 9-25-12 SCALE: As Noted
DESIGNED BY: JAM/SPC DATE: 9-25-12
BRIDGE NO. A7267 DRAWING NO. 53243

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. ROAD NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
11-16-12					ARK.			
						JOB NO. BB0614	162	219
				B7266		R.C. SLAB DETAILS		53244

GENERAL NOTES

CONSTRUCTION SPECIFICATIONS: Arkansas State Highway and Transportation Department Standard Specifications for Highway Construction, 2003 Edition, with applicable supplemental specifications and special provisions. Unless otherwise noted in the plans Section and Subsection refer to the Standard Construction Specifications.

DESIGN SPECIFICATIONS: AASHTO LRFD Bridge Design Specifications Sixth Ed. (2012).

LIVE LOADING: HL-93

MATERIALS AND STRENGTHS

Class S(AE) Concrete
Reinforcing Steel (AASHTO M31 or M53, Gr. 60)
Structural Steel (AASHTO M270 Gr. 36)

$f'_c=4,000$ psi
 $f_y=60,000$ psi
 $f_y=36,000$ psi

CONCRETE: Concrete shall be poured in the dry and all exposed corners are to be chamfered $3/4"$ unless otherwise noted. All concrete shall be Class S(AE) Concrete with a minimum 28 Day Compressive strength of $f'_c=4,000$ psi.

Concrete in bridge superstructure shall be placed, consolidated, and screeded off for the entire pour before any concrete has taken its initial set. This may require the use of a retarding agent.

The concrete deck shall be given a fine finish in accordance with subsection 802.19 for Class 5 Tined Bridge Roadway Surface Finish. Movement of the finishing machine across new concrete shall be on planks placed on the surface and shall be prohibited for 72 hours after finishing the pour.

BEARING PAD: The $1/8"$ bearing pad shall be an unreinforced pad meeting the requirements of Section 808, or shall be nylon reinforced neoprene meeting the requirements of subsection 807.20. The pad shall be furnished in one piece for the required width and full length of the bearing and glued to the bent cap with an adhesive approved by the Engineer. Pad and Adhesive will not be paid for directly, but will be considered subsidiary to the item "Class S(AE) Concrete".

REINFORCING STEEL: All reinforcing steel shall conform to AASHTO M31 or M53, Gr. 60. The reinforcing steel is to be accurately located in the forms and firmly held in place by steel wire supports, sufficient in number and size to prevent displacement during the course of construction. The wire supports will not be paid for directly, but will be considered subsidiary to the item "Epoxy Coated Reinforcing Steel (GR.60)".

STRUCTURAL STEEL: All structural steel shall be AASHTO M270, Gr.36 unless otherwise noted and will not be paid for directly, but will be considered subsidiary to the item "Class S(AE) Concrete". All exposed surfaces shall be cleaned in accordance with subsection 807.84(e) unless otherwise noted.

Drawings show general features of design only. Shop drawings shall be made in accordance with the specifications, submitted and approval secured before fabrication is begun.

Requests for substitution of structural steel shapes shown with shapes of greater size must be submitted by the Contractor to the Engineer for approval. Steels of equal or greater strengths will be accepted only when shown on the approved shop drawings. Payment will be based on the basis of shapes and materials shown in the plans, and no additional compensation will be made for any adjustments due to substitutions.

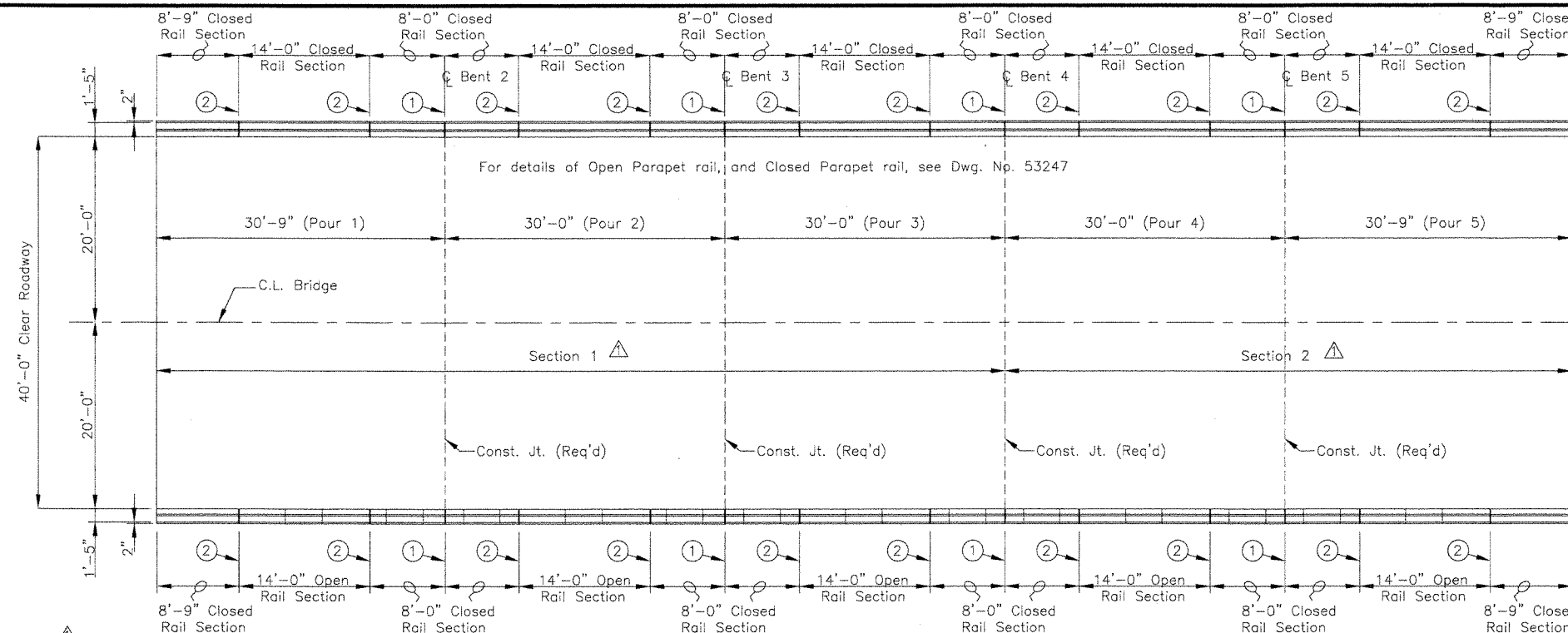
All welding that is done during the fabrication of the structural steel, including temporary welds, shall be detailed on the shop drawings with detailed drawings and shall be submitted to the Engineer for approval. If additional welds are required, whether permanent or temporary, a formal request with detailed drawings shall be submitted to the Engineer for approval. All welding shall conform to subsection 807.26.

DETAILS OF 151'-6" CONTINUOUS
R.C. SLAB UNIT
CLEAR CREEK BRIDGE
BINGHAM RD. - GRANT CO. LINE (S)
PULASKI COUNTY

ROUTE SEC.
ARKANSAS STATE HIGHWAY COMMISSION
LITTLE ROCK, ARK.

Engstrom/Modjeski and Masters

DRAWN BY: DSK DATE: 9-25-12 FILENAME: BBB0614B2_S1.DGN
CHECKED BY: SPC DATE: 9-25-12 SCALE: As Noted
DESIGNED BY: ERD/JAM DATE: 9-25-12
BRIDGE NO. B7266 DRAWING NO. 53244



Note: Contractor shall pour the slab unit in sections according to the pouring sequence shown. No part of the falsework for the entire unit shall be removed until all of the concrete in the complete bridge deck slab has been placed and cured. All pours shall begin at one end of the bridge and proceed in sequence to the other end of the bridge. 72 hours shall elapse between the end of a pour and the start of the next pour.

PLAN OF RAILING AND DECK

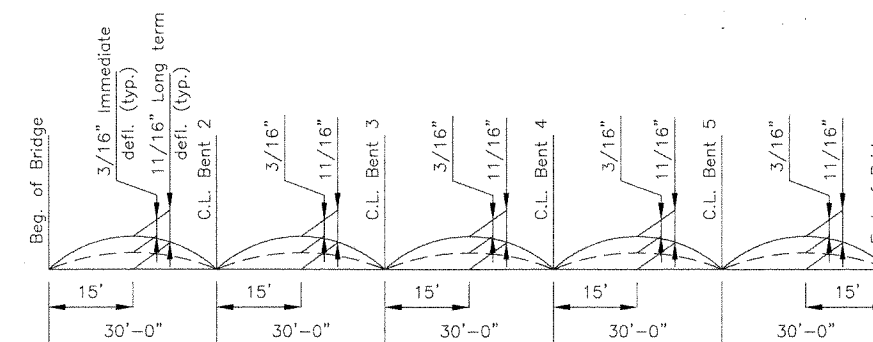
Scale: $1/8" = 1'-0"$
Clear Creek Bridge "B"

Concrete in the bridge superstructure shall be placed and consolidated for the entire pour before any concrete has taken its initial set. This may require the use of a retarding agent. The Contractor must obtain approval from the Engineer for any deviation from the pouring sequence shown and for any railing pours made before the entire bridge slab has been poured.

POURING SEQUENCE FOR CONT. R.C. SLAB UNIT

1. C.L. Full-Depth Parapet Joint (1/4" to 1" max.) Stop 4" from top of slab.
2. C.L. Partial-Depth Parapet Joint (1/4" to 1" max.) Stop 1'-2" from top of slab.

Note: Contractor shall pour the slab unit according to the pouring sequence shown. No part of the falsework for the entire section shall be removed until all of the concrete in that section of bridge deck has been placed and cured. Falsework for span 3 shall remain in place until after section 2 is placed and cured. All pours shall begin at one end of the bridge and proceed in sequence to the other end of the bridge. 72 hours shall elapse between the end of a pour and the start of the next pour.



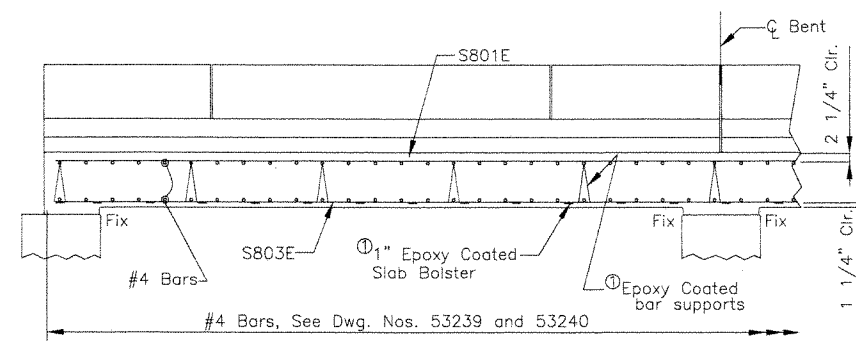
DEAD LOAD CAMBER DIAGRAM

Tolerance $+ 1/4"$ for camber and vertical alignment. Camber shown is from a chord from centerline bent to centerline bent. Vertical alignment corrections are not included.



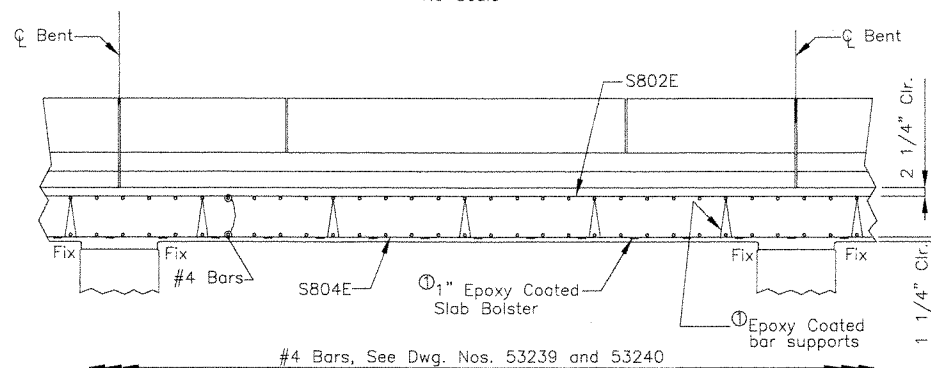
Revised Slab Pouring Notes
11-16-12.

BRIDGE ENGINEER



LONGITUDINAL SECTION ALONG C.L. BRIDGE - END SPAN

No Scale

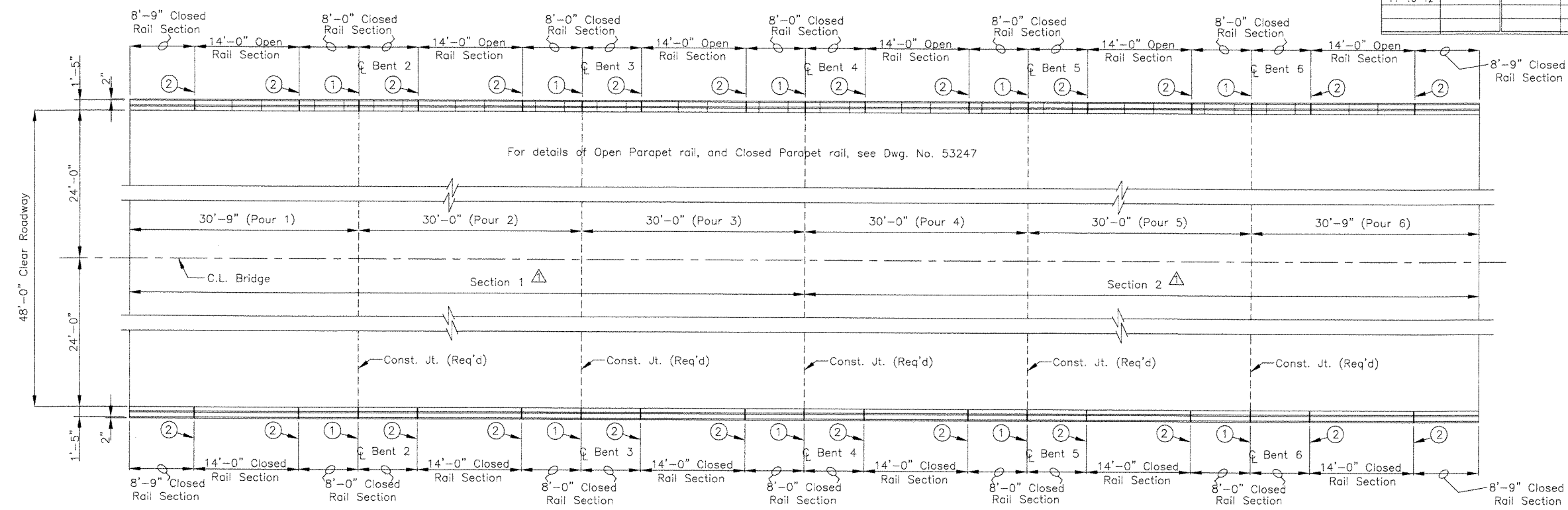


LONGITUDINAL SECTION ALONG C.L. BRIDGE BETWEEN BENTS

No Scale

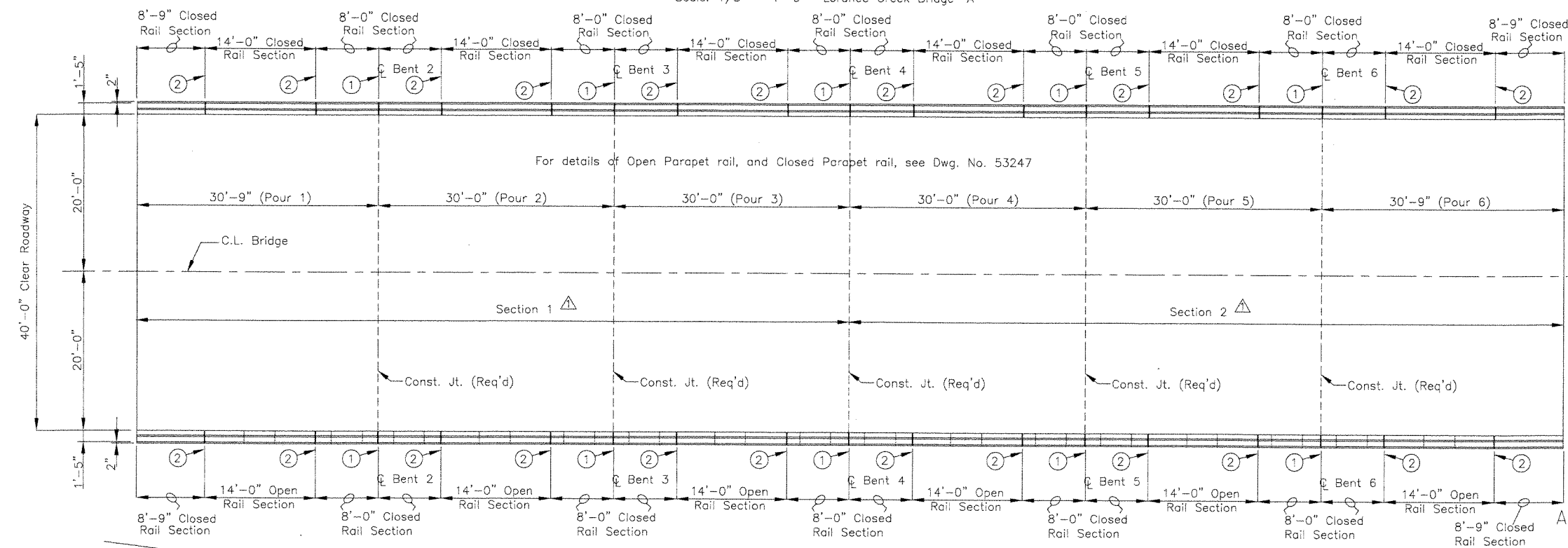
Size and Spacing shall be sufficient to support reinforcing under all construction activities without deforming.

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. ROAD NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
11-16-12					ARK.			
				JOB NO.	BB0614	163	219	
				A7267 & B7267 R.C. SLAB DTLS 53245				



PLAN OF RAILING AND DECK
POURING SEQUENCE FOR CONT. R.C. SLAB UNIT

Scale: 1/8" = 1'-0" Lorraine Creek Bridge "A"



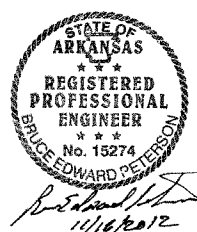
PLAN OF RAILING AND DECK
POURING SEQUENCE FOR CONT. R.C. SLAB UNIT

Scale: 1/8" = 1'-0" Lorraine Creek Bridge "B"

Note: 1. Refer to Dwg. No. 53246 for Dead Load Camber information.

- ① C.L. Full-Depth Parapet Joint (1/4" to 1" max.) Stop 4" from top of slab.
- ② C.L. Partial-Depth Parapet Joint (1/4" to 1" max.) Stop 1'-2" from top of slab.

△ Revised Slab Pouring Notes 11-16-12.



△ Note: Contractor shall pour the slab unit according to the pouring sequence shown. No part of the falsework for the entire section shall be removed until all of the concrete in that section of bridge deck has been placed and cured. All pours shall begin at one end of the bridge and proceed in sequence to the other end of the bridge. 72 hours shall elapse between the end of a pour and the start of the next pour.

SHEET 1 OF 2
DETAILS OF 181'-6" CONTINUOUS
R.C. SLAB UNITS
LORRAINE CREEK BRIDGE
BINGHAM RD. - GRANT CO. LINE (S)
PULASKI COUNTY

ROUTE SEC.
ARKANSAS STATE HIGHWAY COMMISSION
LITTLE ROCK, ARK.

Engstrom/Modjeski and Masters

DRAWN BY: DSK DATE: 9-25-12 FILENAME: BBB0614M4_S11.DGN
CHECKED BY: SPC DATE: 9-25-12 SCALE: As Noted
DESIGNED BY: ERD/JAM DATE: 9-25-12
BRIDGE NO. A7267 & B7267 DRAWING NO. 53245

Concrete in the bridge superstructure shall be placed and consolidated for the entire pour before any concrete has taken its initial set. This may require the use of a retarding agent. The Contractor must obtain approval from the Engineer for any deviation from the pouring sequence shown and for any railing pours made before the entire bridge slab has been poured.

BRIDGE ENGINEER

△ Note: Contractor shall pour the slab unit in sections according to the pouring sequence shown. No part of the falsework for the entire unit shall be removed until all of the concrete in the complete bridge deck slab has been placed and cured. All pours shall begin at one end of the bridge and proceed in sequence to the other end of the bridge. 72 hours shall elapse between the end of a pour and the start of the next pour.

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. ROAD NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
					ARK.			
				JOB NO.		BB0614	164	219
A7267 & B7267 R.C. SLAB DTLS. 53246								

GENERAL NOTES

CONSTRUCTION SPECIFICATIONS: Arkansas State Highway and Transportation Department Standard Specifications for Highway Construction, 2003 Edition, with applicable supplemental specifications and special provisions. Unless otherwise noted in the plans Section and Subsection refer to the Standard Construction Specifications.

DESIGN SPECIFICATIONS: AASHTO LRFD Bridge Design Specifications Sixth Ed. (2012).

LIVE LOADING: HL-93

MATERIALS AND STRENGTHS

Class S(AE) Concrete $f'_c=4,000$ psi
Reinforcing Steel (AASHTO M31 or M53, Gr. 60) $f_y=60,000$ psi
Structural Steel (AASHTO M270 Gr. 36) $f_y=36,000$ psi

CONCRETE: Concrete shall be poured in the dry and all exposed corners are to be chamfered $3/4"$ unless otherwise noted. All concrete shall be Class S(AE) Concrete with a minimum 28 Day Compressive strength of $f'_c=4,000$ psi.

Concrete in bridge superstructure shall be placed, consolidated, and screeded off for the entire pour before any concrete has taken its initial set. This may require the use of a retarding agent.

The concrete deck shall be given a tine finish in accordance with subsection 802.19 for Class 5 Tined Bridge Roadway Surface Finish. Movement of the finishing machine across new concrete shall be on planks placed on the surface and shall be prohibited for 72 hours after finishing the pour.

BEARING PAD: The $1/8"$ bearing pad shall be an unreinforced pad meeting the requirements of Section 808, or shall be nylon reinforced neoprene meeting the requirements of subsection 807.20. The pad shall be furnished in one piece for the required width and full length of the bearing and glued to the bent cap with an adhesive approved by the Engineer. Pad and Adhesive will not be paid for directly, but will be considered subsidiary to the item "Class S(AE) Concrete".

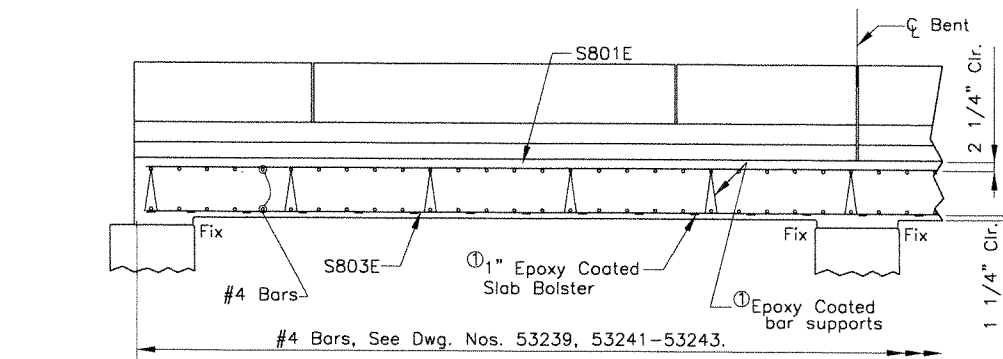
REINFORCING STEEL: All reinforcing steel shall conform to AASHTO M31 or M53, Gr. 60. The reinforcing steel is to be accurately located in the forms and firmly held in place by steel wire supports, sufficient in number and size to prevent displacement during the course of construction. The wire supports will not be paid for directly, but will be considered subsidiary to the item "Epoxy Coated Reinforcing Steel (GR.60)."

STRUCTURAL STEEL: All structural steel shall be AASHTO M270, Gr.36 unless otherwise noted and will not be paid for directly, but will be considered subsidiary to the item "Class S(AE) Concrete". All exposed surfaces shall be cleaned in accordance with subsection 807.84(e) unless otherwise noted.

Drawings show general features of design only. Shop drawings shall be made in accordance with the specifications, submitted and approval secured before fabrication is begun.

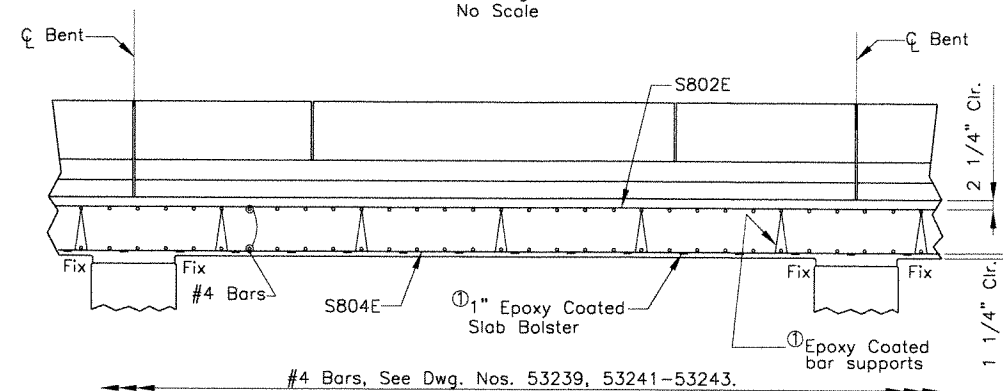
Requests for substitution of structural steel shapes shown with shapes of greater size must be submitted by the Contractor to the Engineer for approval. Steels of equal or greater strengths will be accepted only when shown on the approved shop drawings. Payment will be based on the basis of shapes and materials shown in the plans, and no additional compensation will be made for any adjustments due to substitutions.

All welding that is done during the fabrication of the structural steel, including temporary welds, shall be detailed on the shop drawings with detailed drawings and shall be submitted to the Engineer for approval. If additional welds are required, whether permanent or temporary, a formal request with detailed drawings shall be submitted to the Engineer for approval. All welding shall conform to subsection 807.26.



LONGITUDINAL SECTION ALONG C.L. BRIDGE - END SPAN

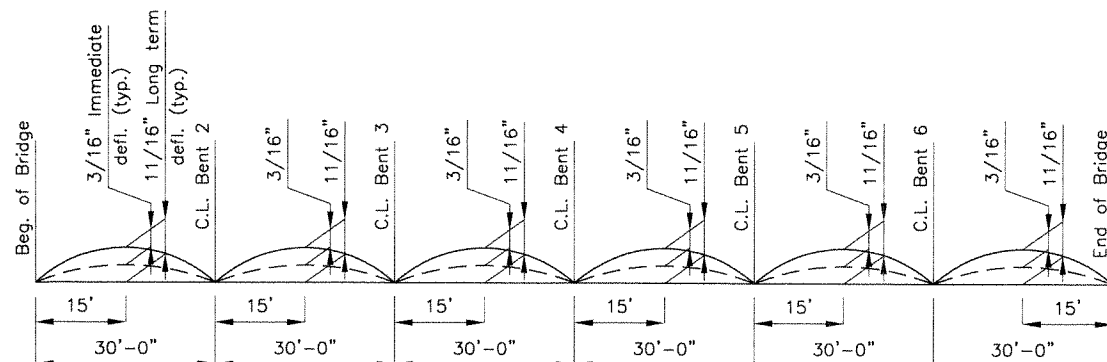
Lorance Creek Bridges "A" & "B"
No Scale



LONGITUDINAL SECTION ALONG C.L. BRIDGE BETWEEN BENTS

Lorance Creek Bridges "A" & "B"
No Scale

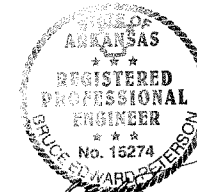
① Size and Spacing shall be sufficient to support reinforcing under all construction activities without deforming.



DEAD LOAD CAMBER DIAGRAM

Lorance Creek Bridges "A" & "B"

Tolerance $+ 1/4$ for camber and vertical alignment. Camber shown is from a chord from centerline bent to centerline bent. Vertical alignment corrections are not included.



SHEET 2 OF 2
DETAILS OF 181'-6" CONTINUOUS
R.C. SLAB UNITS
LORANCE CREEK BRIDGES
BINGHAM RD. - GRANT CO. LINE (S)
PULASKI COUNTY

ROUTE SEC.
ARKANSAS STATE HIGHWAY COMMISSION
LITTLE ROCK, ARK.

Engstrom/Modjeski and Masters

DRAWN BY: DSK DATE: 9-25-12 FILENAME: BB0614M4_S12.DGN
CHECKED BY: SPC DATE: 9-25-12 SCALE: As Noted
DESIGNED BY: ERD/JAM DATE: 9-25-12

BRIDGE ENGINEER

BRIDGE NO. A7267 & B7267 DRAWING NO. 53246

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB NO.		BB0614	106	249
B3323,B3326 - LMC OVERLAY - 53249								

GENERAL NOTES:

CONSTRUCTION SPECIFICATIONS: Arkansas State Highway Commission Standard Specifications for Highway Construction, Edition of 2003, with applicable special provisions and Supplemental Specifications. Unless otherwise noted in the plans Section and Subsection refer to the Standard Specifications.

Drawing shows details and dimensions of existing structures based on the original bridge plans. The Contractor shall make check measurements in the field and make any adjustments necessary to meet the required clearances and fit the new work to the existing structure.

The operation or placement of equipment and/or materials on the subject bridges necessary for the completion of this work shall be subject to the provisions of SS-105-2 "Equipment and Material Storage on Bridge Structures". Certifications of the adequacy of all components for the anticipated loads shall address the capacity of the existing structure at all phases of this work.

HYDRODEMOLITION: The designated area of the existing bridge deck shall receive hydrodemolition in accordance with the Job Special Provision "Hydrodemolition" to a planned depth of 1/2" below the existing bridge deck surface. Deteriorated concrete below this depth shall be removed up to the limits detailed and at the direction of the Engineer. Cold milling of the concrete deck prior to hydrodemolition will be allowed to a maximum depth of 1" unless there will be a conflict with existing reinforcing steel. These areas shall be measured by the square yard and shall be paid for at the unit price bid for the item SP Job BB0614 "Hydrodemolition."

LATEX MODIFIED CONCRETE OVERLAY: The designated area of the existing bridge deck shall receive a Latex Modified Concrete (LMC) Overlay to a planned depth of 1/2" below the existing bridge deck surface, in accordance with the Job Special Provision "Latex Modified Concrete Overlay". These areas shall be measured by the square yard and shall be paid for at the unit price bid for the item SP Job BB0614 "Latex Modified Concrete Overlay (1/2" Thick)". Areas of the existing bridge deck removed at the direction of the Engineer to a depth greater than 1/2" below the existing bridge deck surface shall be filled with LMC concurrent to the placement of the 1/2" LMC Overlay. This area shall be measured and paid for as SP Job BB0614 "Latex Modified Concrete (Variable Depth)" at the unit price bid for the item.

BRIDGE DECK: The LMC Overlay surface shall be given a grooved finish as specified for final finishing in Subsection 802.19 for Class 7 Grooved Bridge Roadway Surface Finish.

PROTECTIVE SURFACE TREATMENT: Longitudinal Construction Joints as shown on the plans and transverse and longitudinal construction joints constructed as directed by the Engineer shall be given a Class 3 Protective Surface Treatment as specified in Section 803 and in accordance with Job Special Provision "Latex Modified Concrete Overlay".

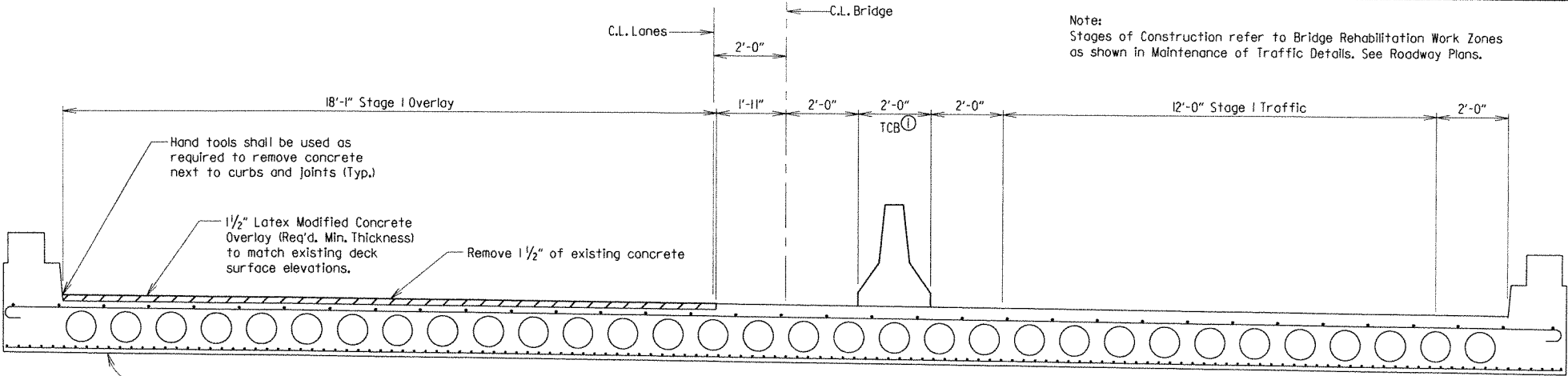
The roadway surface of the completed LMC Overlay shall be given a Class 1 Protective Surface Treatment as specified in Section 803.

TRANSVERSE JOINT REHABILITATION: After the placement of the LMC Overlay, the existing transverse slab joints at the intermediate bents shall be given a Type A Joint Rehabilitation as specified in Section 509.

DETAILS OF LATEX MODIFIED CONCRETE OVERLAY

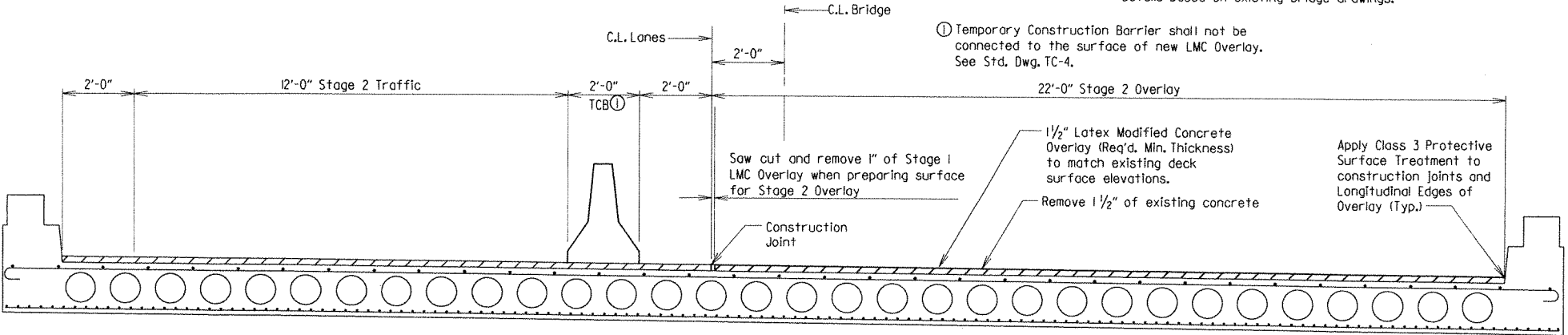
ROUTE 530 SEC. 2 & 3
 ARKANSAS STATE HIGHWAY COMMISSION
 LITTLE ROCK, ARK.

DRAWN BY: LJB DATE: 9/13/12 FILENAME: bb0614_concoverlay.dgn
 CHECKED BY: GWP DATE: 10/5/12 SCALE: NO SCALE
 DESIGNED BY: DATE:
 BRIDGE NO. B3323,B3326 DRAWING NO. 53249

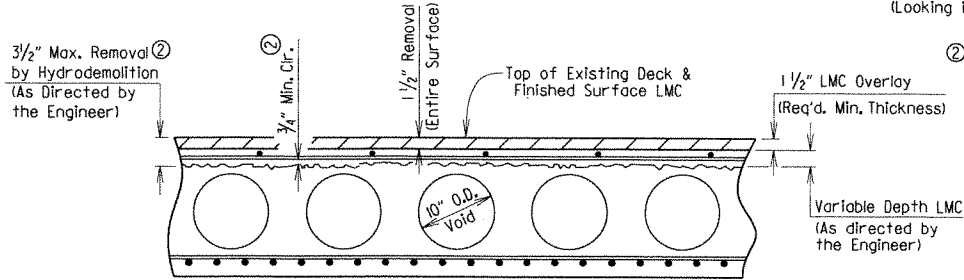


STAGE 1 LATEX MODIFIED CONCRETE OVERLAY
(Looking in direction of traffic)

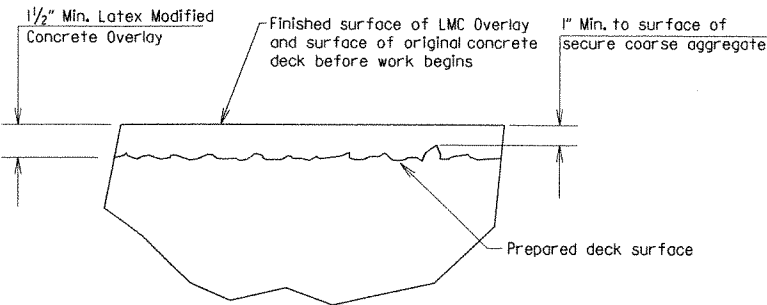
NOTE:
The minimum pour length shall be from joint to joint.
Details based on existing bridge drawings.



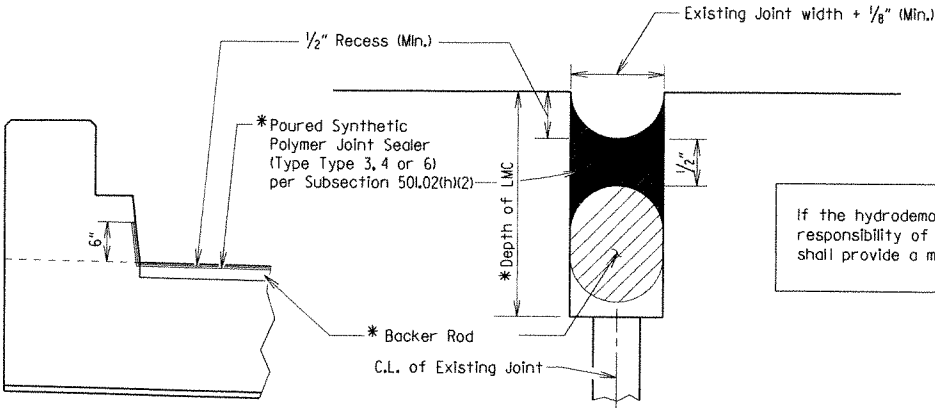
STAGE 2 LATEX MODIFIED CONCRETE OVERLAY
(Looking in direction of traffic)



DETAILS OF HYDRODEMOLITION AND LATEX MODIFIED CONCRETE OVERLAY



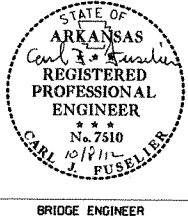
LMC OVERLAY TOLERANCE

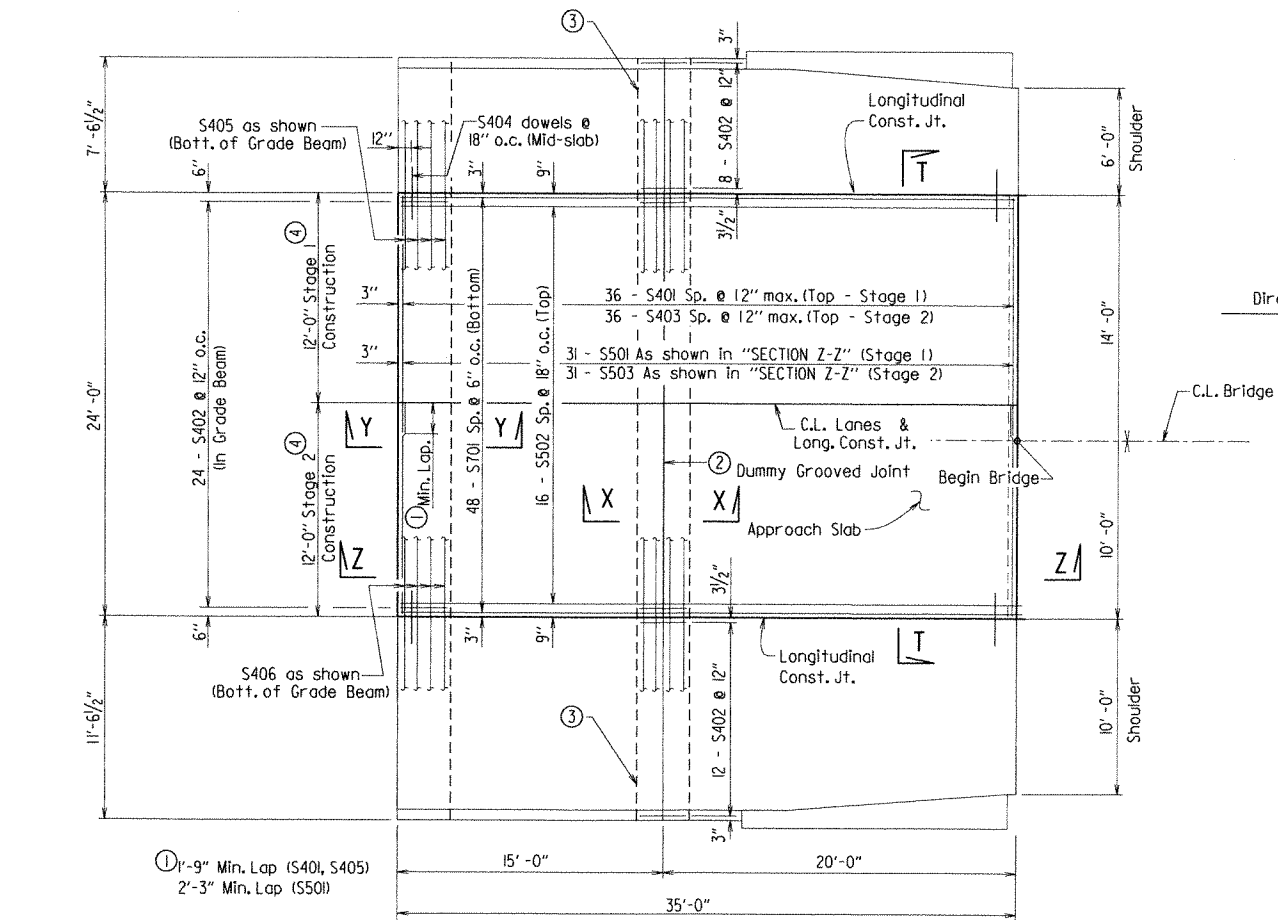


DETAILS OF TYPE A JOINT REHABILITATION

NOTE: Saw, cut or router ends of slab to achieve joint width as shown. Sawing beyond the face of curb is not required. See Section 509 for additional information & payment.

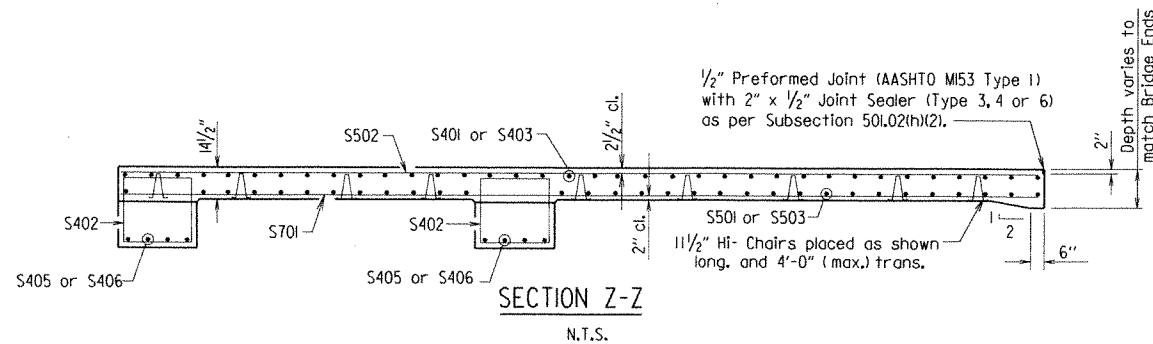
* NOTE: Depth of joint and joint installation shall be in accordance with manufacturer's recommendations.





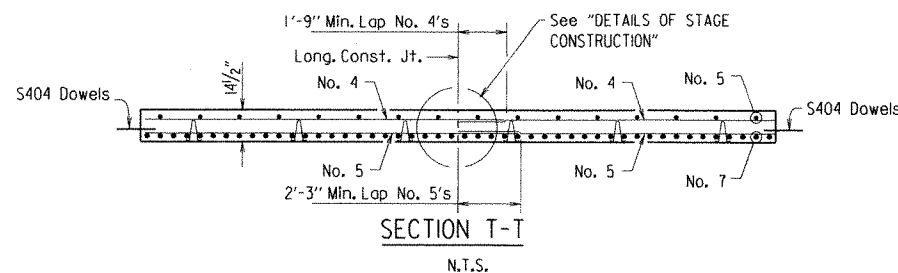
PLAN VIEW - APPROACH SLAB

3/16" = 1'-0"



SECTION Z-Z

N.T.S.



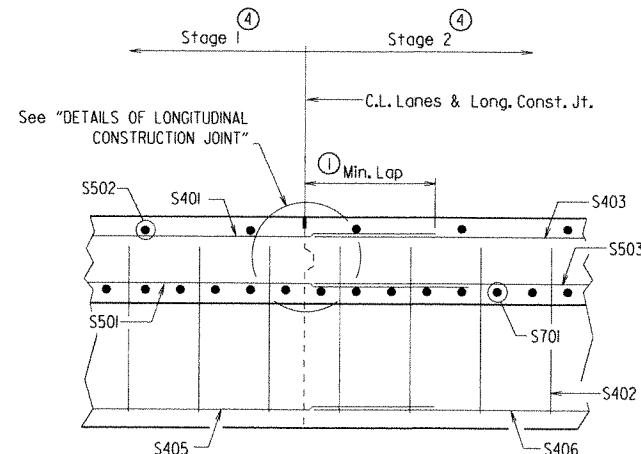
SECTION T-T

N.T.S.

② If slab joints are to be sawed, they shall be sawed as soon as the concrete has sufficiently set to allow sawing of the joint without damage to the slab.

③ Extend Grade Beam under proposed Approach Gutters. Quantities included in Approach Slab Quantities.

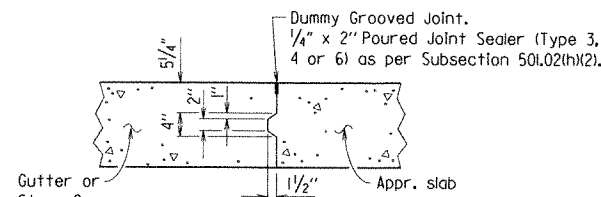
④ Stages of Construction shown refer to Bridge Rehabilitation Work Zones as shown in Maintenance of Traffic Details, sheet no. XXXXX of Roadway Plans.



DETAILS OF STAGE CONSTRUCTION

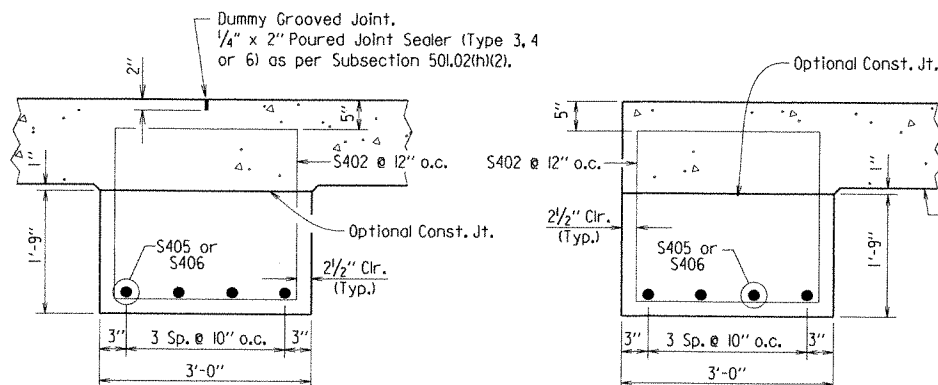
LOOKING AHEAD

3/4" = 1'-0"



DETAILS OF LONGITUDINAL CONSTRUCTION JOINT

3/4" = 1'-0"



SECTION X-X

3/4" = 1'-0"

SECTION Y-Y

3/4" = 1'-0"



BRIDGE ENGINEER

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			

JOB NO. BB0614 167 219

B3323&B3326 - APPR. SLABS - 53250

BAR LIST

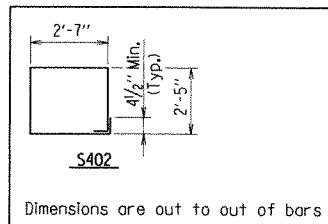
Mark	No. Req'd.		Length	Pin Dia.
	Stage 1	Stage 2		
S401	36	—	13'-7"	Str.
S402	40	48	10'-4"	2"
S403	—	36	11'-10"	Str.
S404	23	23	3'-0"	Str.
S405	8	—	21'-8"	Str.
S406	—	8	23'-4"	Str.
S501	31	—	14'-1"	Str.
S502	8	8	34'-8"	Str.
S503	—	31	11'-10"	Str.
S701	24	24	34'-8"	Str.

TABLE OF QUANTITIES FOR ONE APPROACH SLAB

Slab Width	Reinforcing Steel	Concrete
24'-0"	6,400 lbs.	55.44 Cu. Yds.

Note: Quantities for Grade Beam under Approach Gutters Included.

BENDING DIAGRAM



Dimensions are out to out of bars

GENERAL NOTES

Concrete shall be Class S (AE) (f'c = 4,000 psi).
Reinforcement Steel shall conform to AASHTO M31 or M53, Grade 60 (fy = 60,000 psi).
Approach Slabs will be measured and paid for in accordance with Section 504 of the Standard Specifications.
Approach Slab will be given a tined surface finish as specified in Subsection 802.19 for Class 5 Tined Bridge Roadway Finish.
See Roadway Plans for Maintenance of Traffic and additional information.

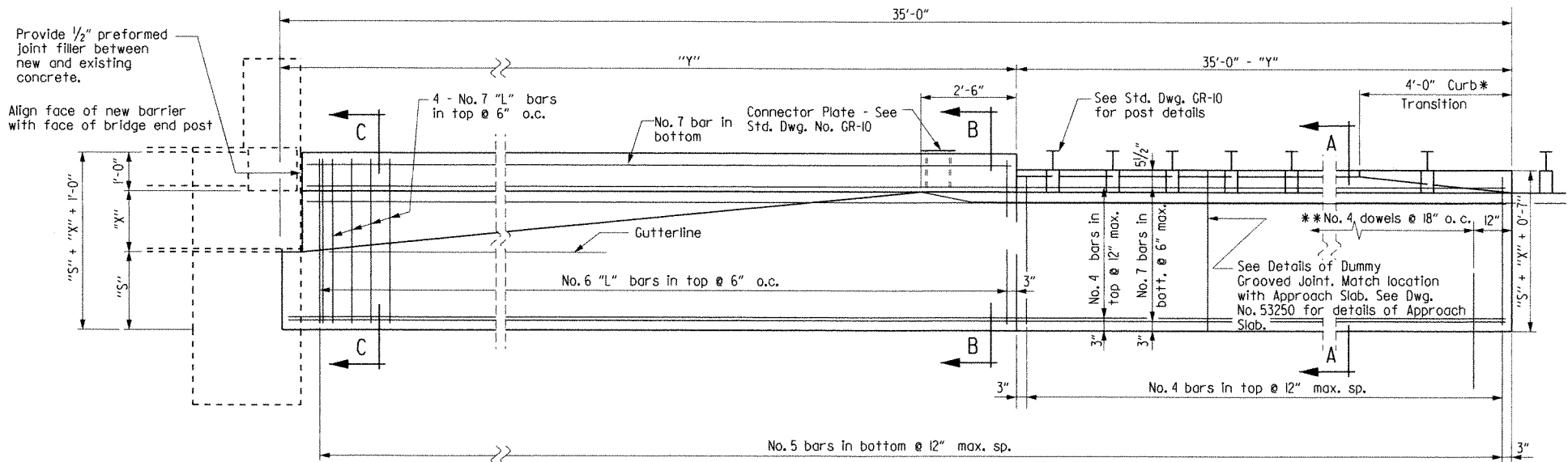
DETAILS OF TYPE SPECIAL I APPROACH SLAB

ROUTE 167
SEC. 219
ARKANSAS STATE HIGHWAY COMMISSION
LITTLE ROCK, ARK.

DRAWN BY: LJB DATE: 8/22/12 FILENAME: bb0614_typed.as.dgn
CHECKED BY: TMA DATE: 9/25/12 SCALE: AS NOTED
DESIGNED BY: — DATE: —
BRIDGE NO. B3323&B3326 DRAWING NO. 53250

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB NO.		BBO614	108	219

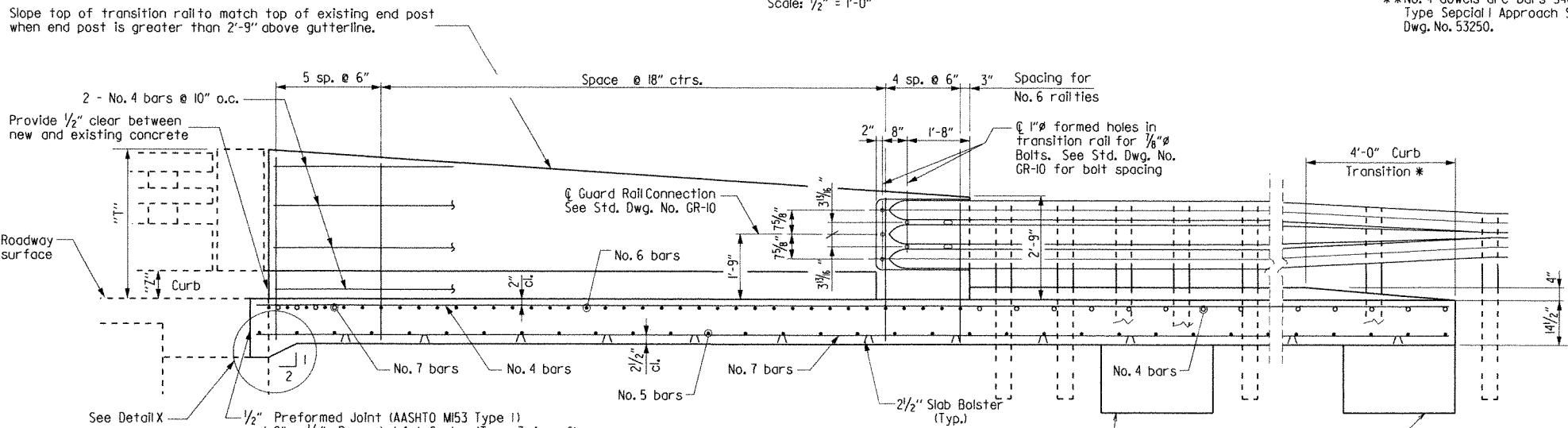
1 B3323&B3326 - APPR. GUTTERS - 53251



PLAN - SQUARE BRIDGES

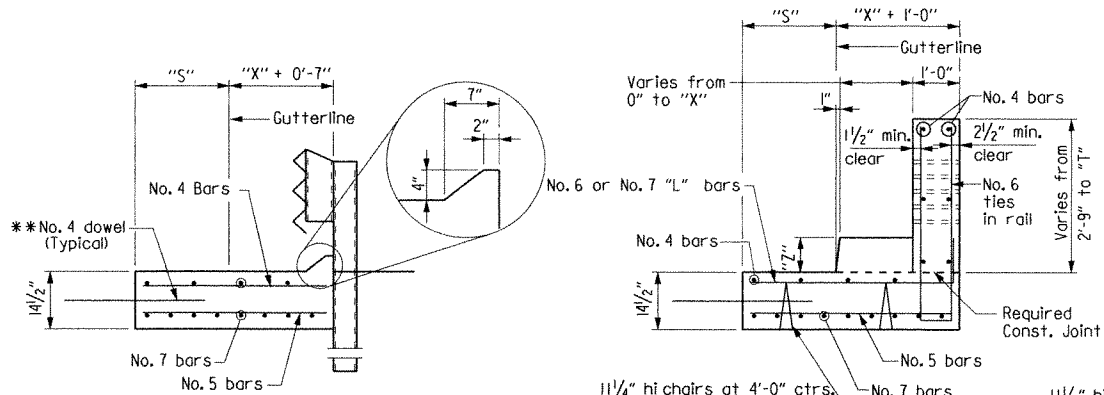
Scale: 1/2" = 1'-0"

**No. 4 dowels are bars S404 from Type Special I Approach Slab, Dwg. No. 53250.



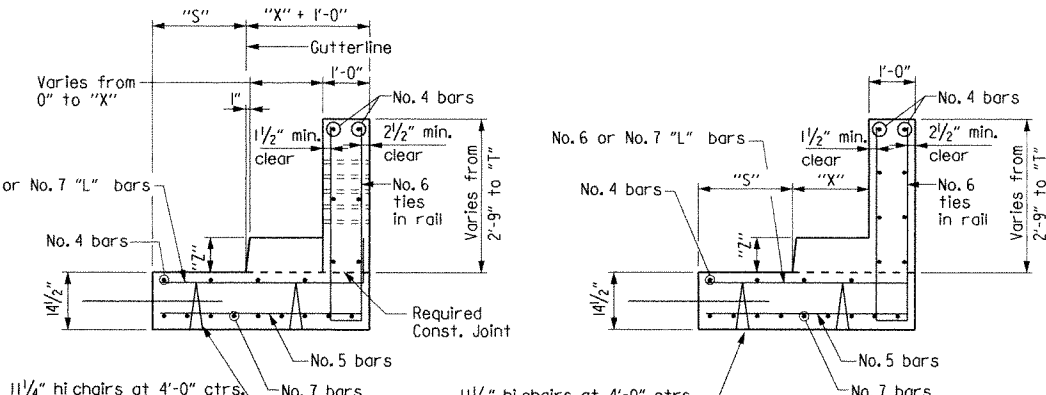
LONGITUDINAL SECTION THRU GUTTER

Scale: 1/2" = 1'-0"



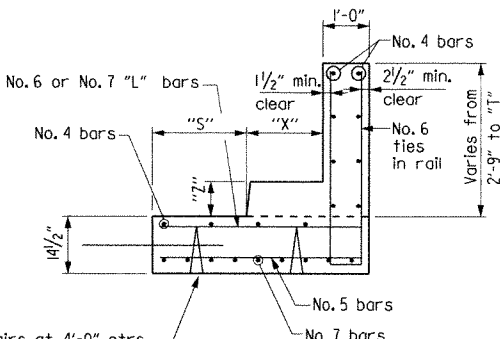
SECTION A-A

Scale: 1/2" = 1'-0"



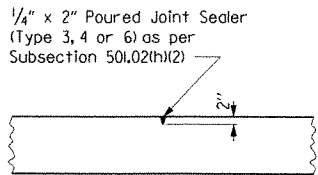
SECTION B-B

Scale: 1/2" = 1'-0"



SECTION C-C

At end of Transition Rail
Scale: 1/2" = 1'-0"



DETAILS OF DUMMY GROOVED JOINT

1/2" = 1'-0"

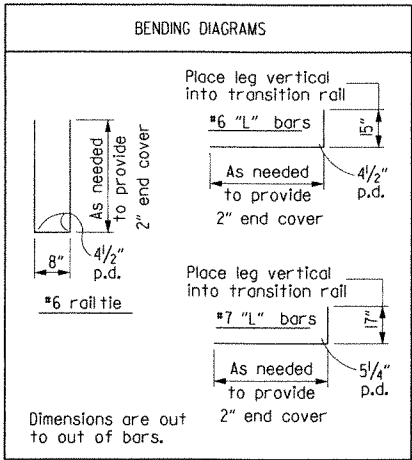


TABLE OF VARIABLES

"S"	"T"	"X"	"Y"	"Z"	APPROX. QUANTITIES FOR INFORMATION ONLY	
					CONCRETE	REINFORCING STEEL
					CU. YD.	LB.
6'-0"	3'-5"	11 1/2"	15'-0"	12"	14.05	2362
10'-0"	3'-5"	11 1/2"	15'-0"	12"	20.32	3427

NOTE: "S" = Distance from gutterline to edge of shoulder or edge of approach slab, if present, but in no case less than 3'-0".
"X" = Distance from gutterline to face of existing end post.
"Z" = Height of bridge curb.
"T" = Distance from top of end post to top of deck.

GENERAL NOTES

Concrete shall be Class (SAE) (f'c = 4,000 psi).

Reinforcing steel shall conform to AASHTO M31 or M53, Grade 60 (fy = 60,000 psi.) Fabricate bar lengths to provide 2" minimum cover at each end.

Approach gutters will be measured and paid for in accordance with Section 504 of the Standard Specifications.

Remove existing guard rail connection post. Work and material will not be paid for separately but will be considered subsidiary to other items in the contract.

Bridge end may vary from that shown. Adjust gutter details as required to provide similar rail transition.

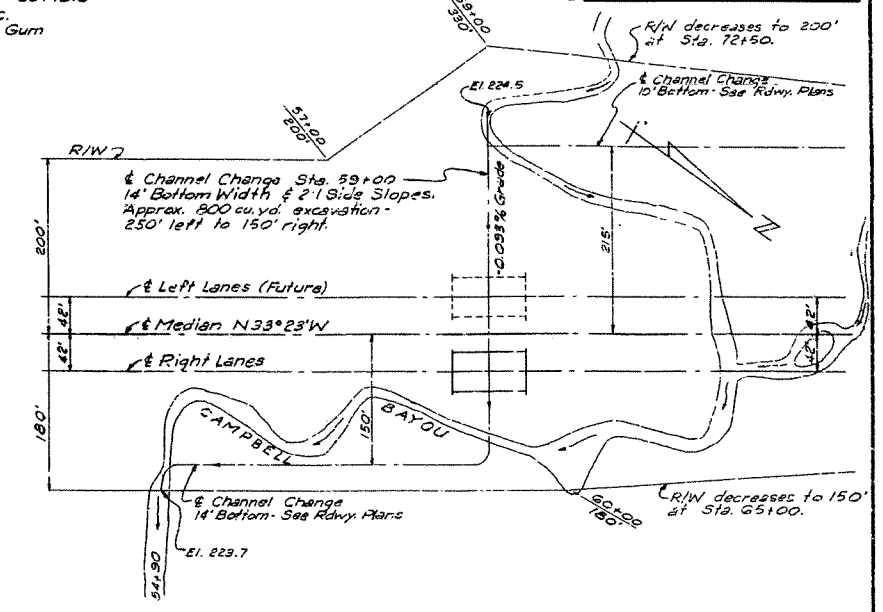
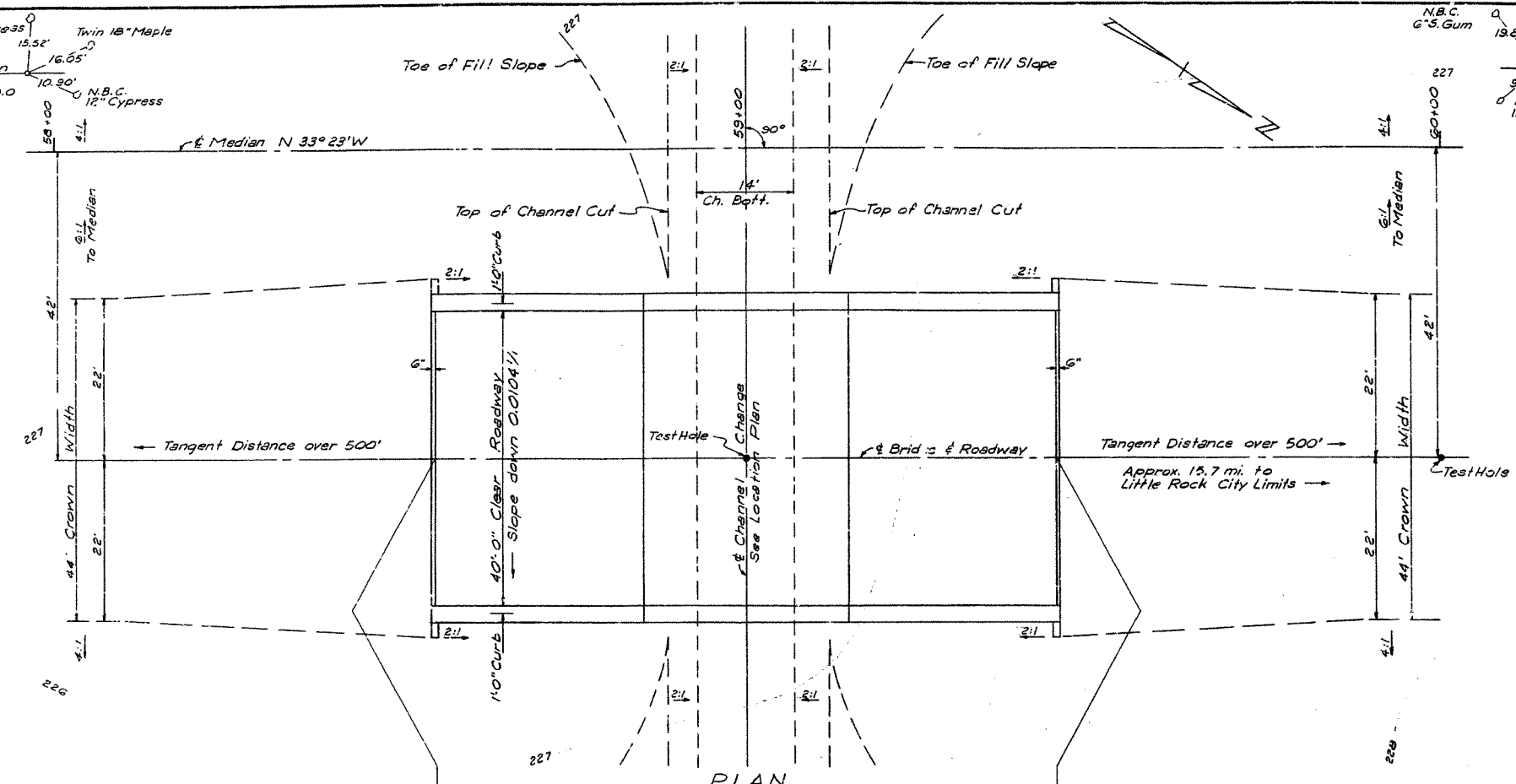
DETAILS OF TYPE SPECIAL I APPROACH GUTTERS

ROUTE 66
SEC. 1
ARKANSAS STATE HIGHWAY COMMISSION
LITTLE ROCK, ARK.

DRAWN BY: LJB DATE: 9/11/12 FILENAME: bb0614_type1gutter.dgn
CHECKED BY: JMB DATE: 9/18/12 SCALE: AS NOTED
DESIGNED BY: — DATE: —
BRIDGE NO. B3323&B3326 DRAWING NO. 53251



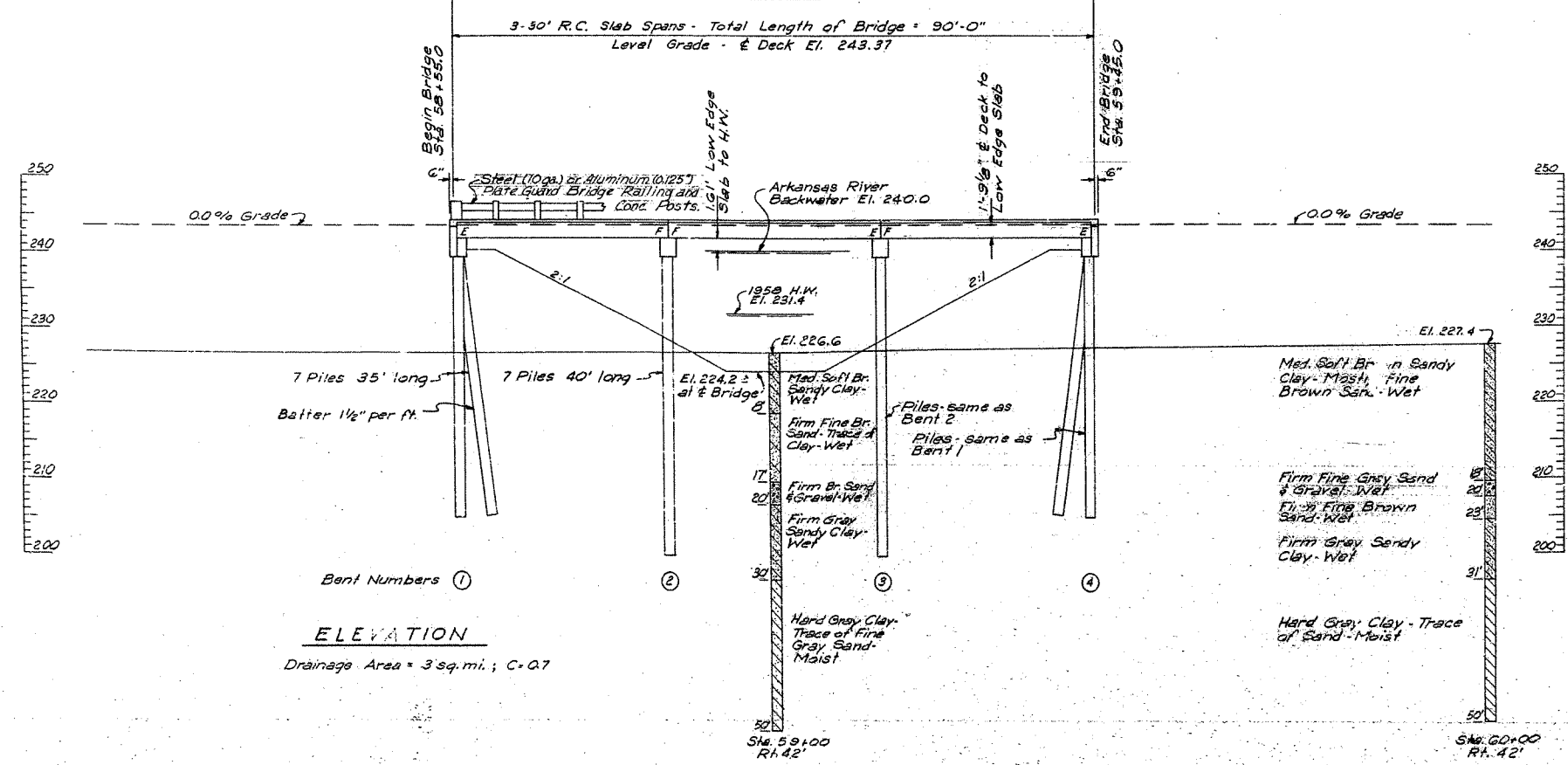
FEED NO.	DATE	FEED AD. PROJ. NO.	SHEET NO.	TOTAL SHEETS
6	APR.	BB0614	10	21
JOB NO.		B3323 - LAYOUT -	53252	



LOCATION PLAN
Scale: 1" = 100'

GENERAL NOTES

B.M. Stake in side 18" Cypress 6' left Sta 59+86; El. 229.56.
All piling to be 16" octagonal precast concrete piles driven to a minimum bearing of 35 tons per pile. Lengths of piling shown are assumed for estimating quantities only. Actual lengths to be determined in the field. Drive one 45' test pile in bent number 2. Drive and bent piles after embankment is in place.
For details of superstructure see Dwg. No. 5431B.
For details of substructure see Dwg. No. 5431A.
For details of precast piles see Dwg. No. 2382.
Specifications: Arkansas State Highway Commission Standard Specifications for Highway Construction, adopted Dec. 9, 1959.
Design Specifications: A.A.S.H.O. 1957
Design Live Loading: H20-SIG
Unit Stresses: Class S Concrete (n=10) 1200 p.s.i.
Reinforcing Steel 20,000 p.s.i.

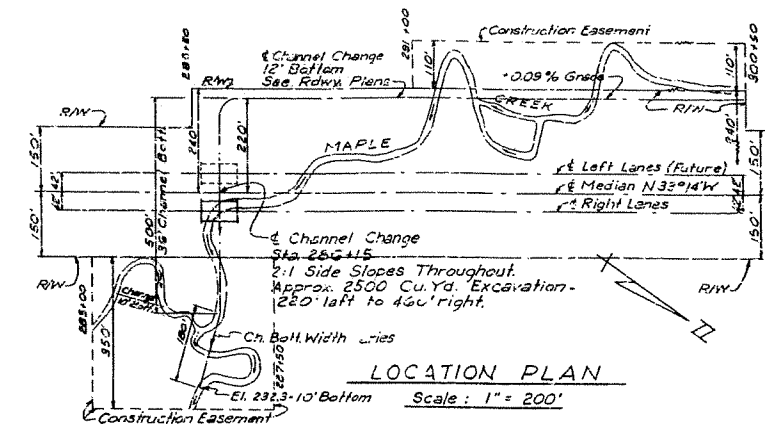
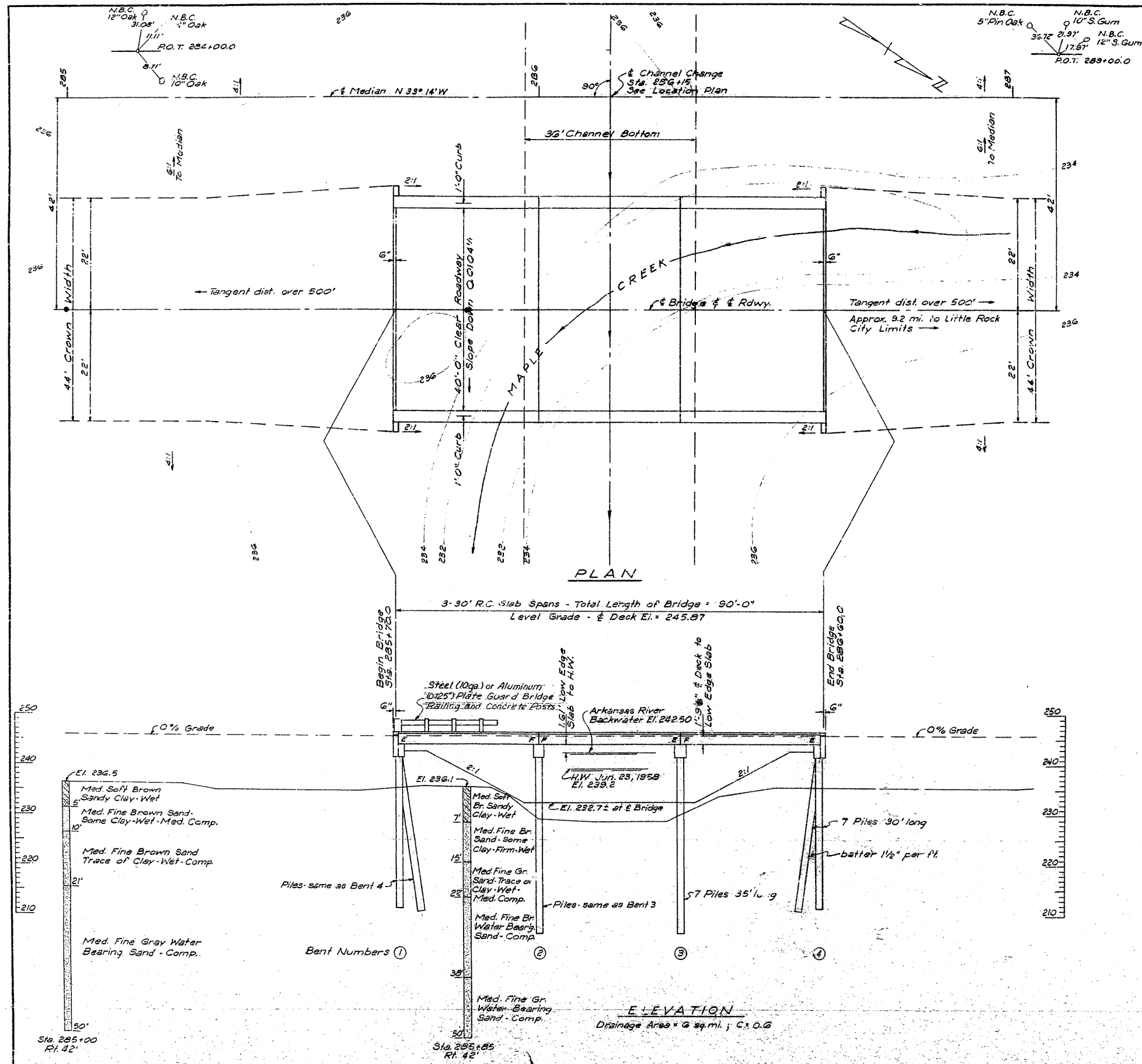


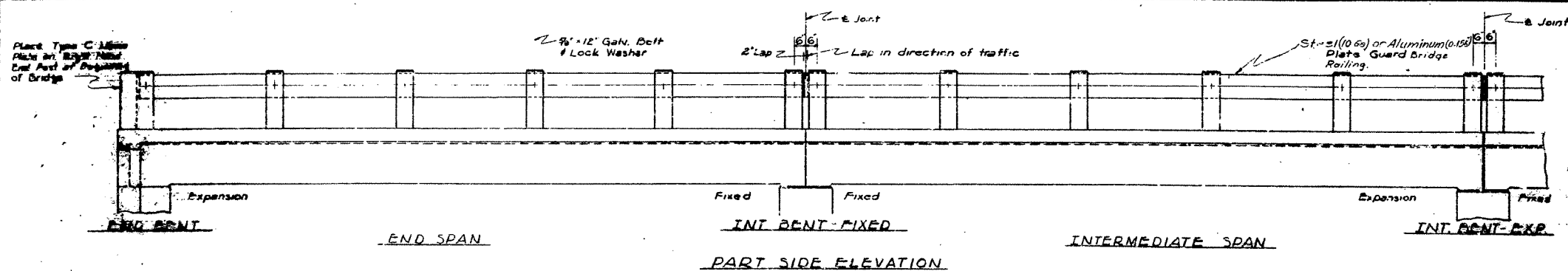
ELEVATION
Drainage Area = 3 sq. mi.; C = 0.7

FOR INFORMATION ONLY

LAYOUT OF BRIDGE
OVER CAMPBELL BAYOU
GRANT CO. LINE - BINGHAM ROAD
PULASKI COUNTY
ROUTE 65 SEC. 12
ARKANSAS STATE HIGHWAY COMMISSION

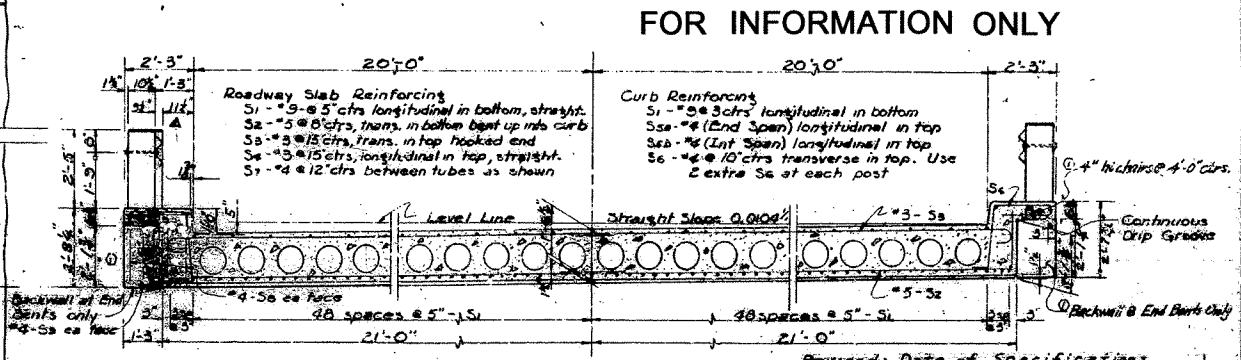
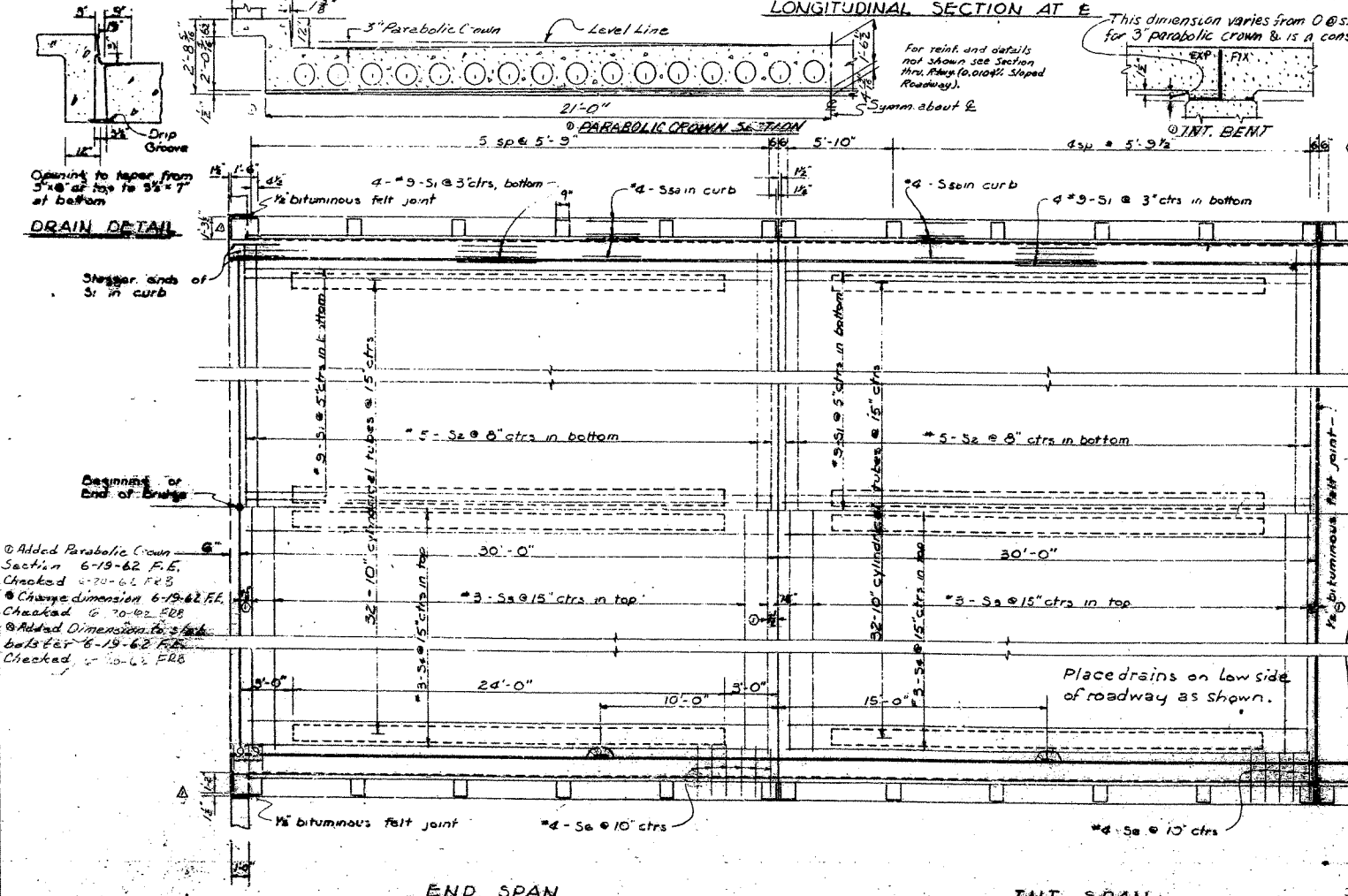
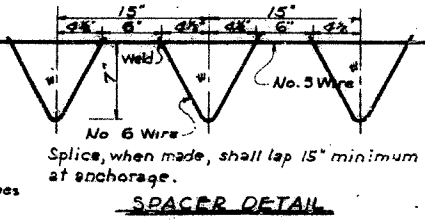
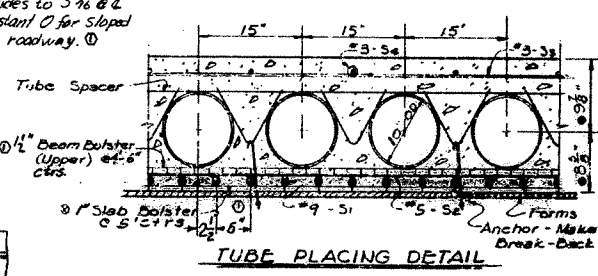
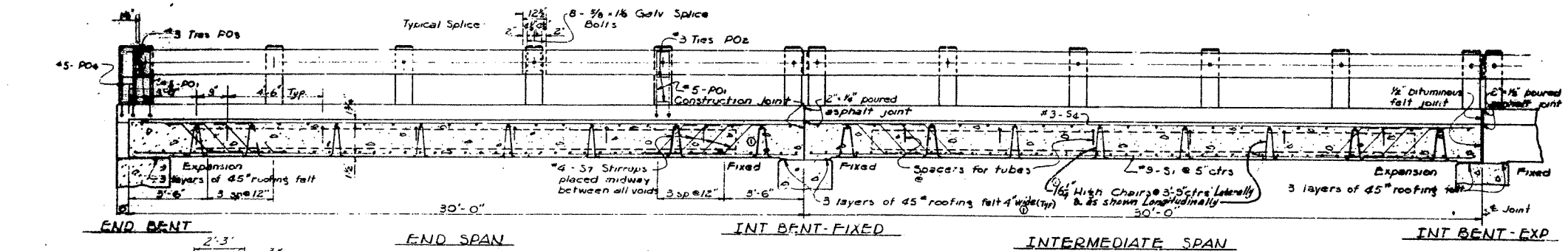
LITTLE ROCK, ARK.
DRAWN BY: R.L.G. DATE: 1-20-59
CHECKED BY: E.D.N. DATE: 1-22-59
BRIDGE NO. B3323
DRAWING NO. 53252





JOB NO. BB0614
 SHEET NO. 19
 B3323, B3326 - SLAB SPANS - 53254
DESIGN SPECIFICATIONS
 Design Live Loading: H20-S16, and Special Interchange Loading of K20
 Load distribution to slab: 24,000 lbs. 4' on Centers
 Dead Load: 182 lb./sq. ft.
 Live Load: 0.774 times width with 50% impact
 Unit Stresses: Class 5 Concrete (n=10)
 Reinforcing Steel: 120,000 psi
 24,000 psi

GENERAL NOTES
 All concrete to be Class 5. All exposed corners to be chamfered 1/4" unless otherwise noted.
 Reinforcing steel to be determined bars of intermediate or end spans. Shop lists and bending diagrams must be submitted and approved before fabrication is begun.
 All cylindrical tubes used to form voids shall be moisture protected, standard type construction, minimum thickness 0.025, and shall be furnished complete with end closures.
 All reinforcing steel and fiber tubes shall be accurately located in the forms and firmly held in place by means of steel wire supports and spacers for tubes of sufficient size and number to prevent displacement during the course of construction, but in no case of lesser design than that shown.
 Wire supports for reinforcing bars will not be paid for directly but will be considered subsidiary to the item of Reinforcing Steel.
 Tubes for forming voids and wire supports and spacers for tubes will not be paid for directly but will be considered subsidiary to the item of Class 5 Concrete.
 Shop lists and diagrams of wire supports and spacers for tubes shall be submitted for approval before fabrication is begun.
 Rebar, bituminous felt and poured asphalt joints shall be measured and paid for as Class 5 Concrete.
 The bridge railing shall be of the type shown or an equivalent rigid type as approved by the Engineer. The plate guard railing including all concrete posts and fastenings shall be paid for at the unit price bid per linear foot for steel or aluminum plate guard railing.
 SPECIFICATIONS: Arkansas State Highway Commission Standard Specifications for Highway Construction, Edition of 1959.
 A Revised End Post 11-2-61 RWM

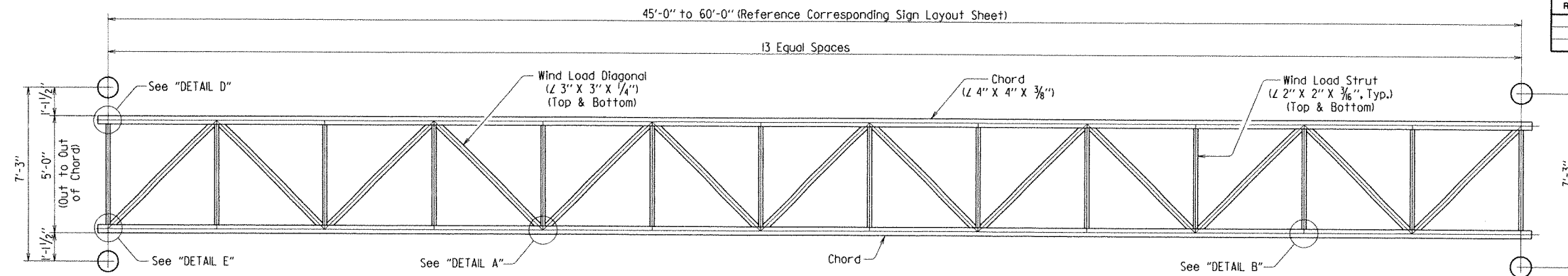


BAR LIST PER SPAN

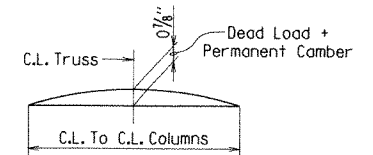
Bar No.	Size	Length	Bending Diagram
S1	#5	103	25'-7"
S2	#5	90	24'-0"
S3	#5	48	21'-11"
S4	#5	33	20'-7"
S5	#5	18	10'-1"
S6	#5	8	2'-0"
S7	#5	24	25'-0"
S8	#5	22	23'-0"
S9	#5	30	22'-8"
S10	#5	8	2'-0"

Revisions:
 High chair spacing - Aug. 5, 58 G.W.B.
 Title Slab Specs. 6-1-60 H.B.
 Plate Guard Rail 6-23-60 H.B.
 Curb Height to 12" 7-11-61 H.B.
DETAILS OF STANDARD 30'-0" R.C. SLAB SPANS
 40'-0" CLEAR ROADWAY 2 CURBS @ 1'-0"
 ROUTE SEC.
 ARKANSAS STATE HIGHWAY COMMISSION
 B3323, B3326 - DRAWING NO. 53254

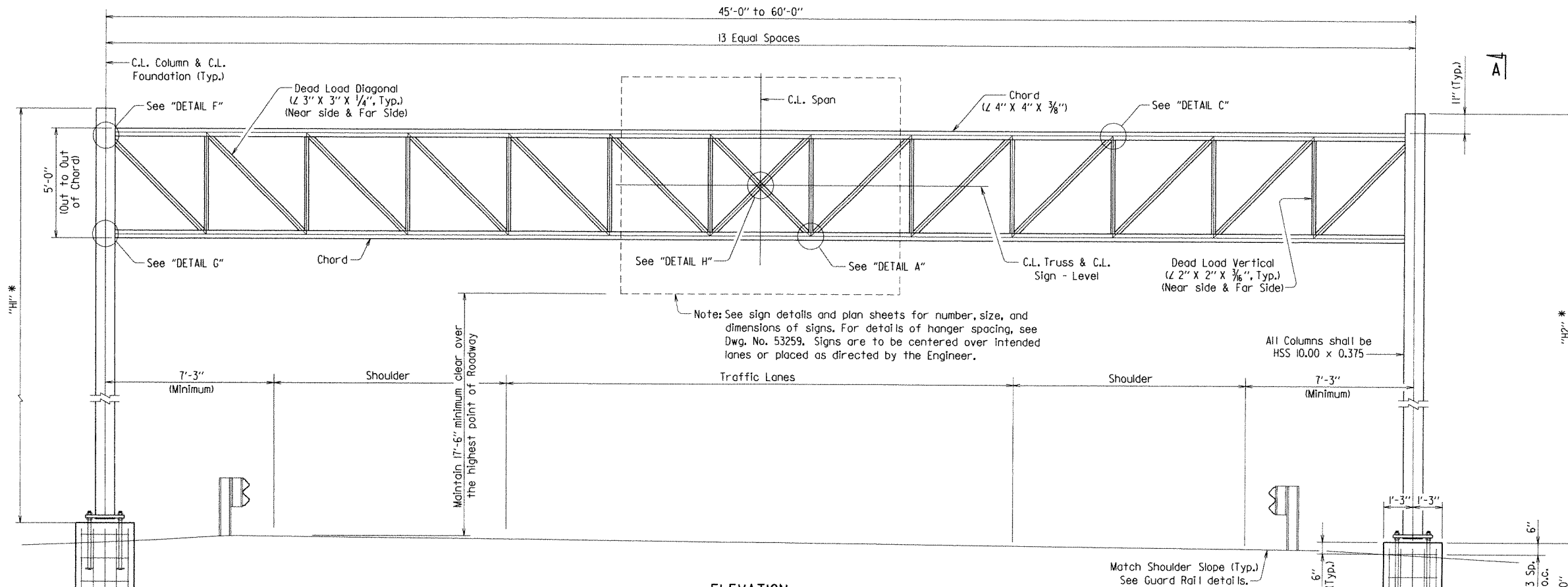
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				6	ARK.			
				JOB NO.	BB0614		172	219
				OH-530-60-03 SIGN STRUCTURE		53255		



PLAN



TRUSS CAMBER DIAGRAM



ELEVATION

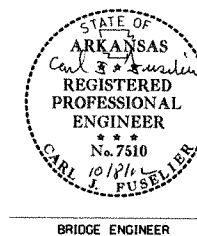
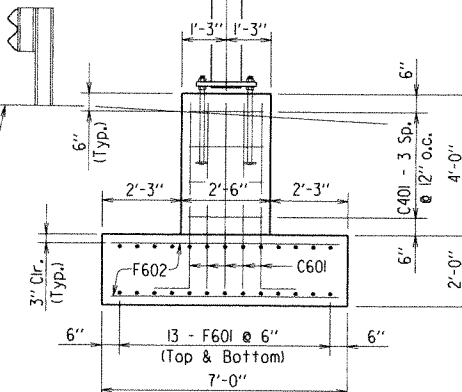
* Note: The Contractor shall make field measurements to determine the Column heights "H1" and "H2" that are required to maintain the minimum vertical clearance with the centerline of the sign located at the centerline of the truss. These column heights shall be shown on the shop drawings with a note stating that the Contractor has made the required field measurements. If the structure height ("H1" or "H2") exceeds 30'-0" contact the Engineer. The Contractor shall also verify that the variable span length (45'-0" to 60'-0") is sufficient to meet the minimum clearances and to fit the new structure to the existing and/or proposed conditions.

APPROXIMATE QUANTITIES FOR FOUNDATION
PER FOOTING - (FOR INFORMATION ONLY)

STRUCTURE NUMBER	CLASS 5 CONCRETE (Cu. Yds.)	REINFORCING STEEL (Lbs.)	EXCAVATION (Cu. Yds.)
EACH STRUCTURE	12.00	1,742	39

BAR LIST-FOUNDATION - PER FOOTING

MARK	NO. REQ'D	LENGTH	P.D.	BENDING DIAGRAMS
C40I	8	17'-6"	2"	
C60I	46	6'-4"	4 1/2"	
F60I	26	15'-6"	Str.	
F602	62	6'-6"	Str.	



SHEET 1 OF 5
DETAILS FOR 45' TO 60'
STEEL OVERHEAD SIGN STRUCTURES
BINGHAM RD. - GRANT COUNTY LINE (S)
PULASKI AND SALINE COUNTIES

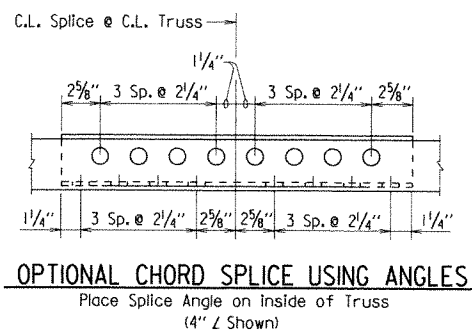
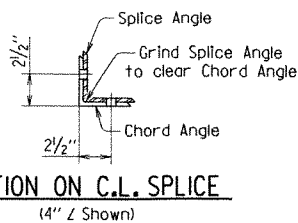
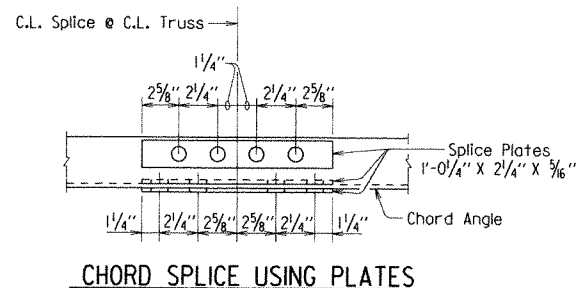
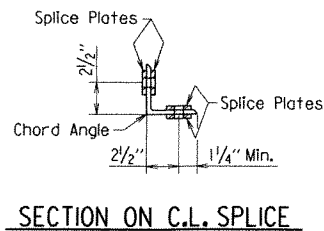
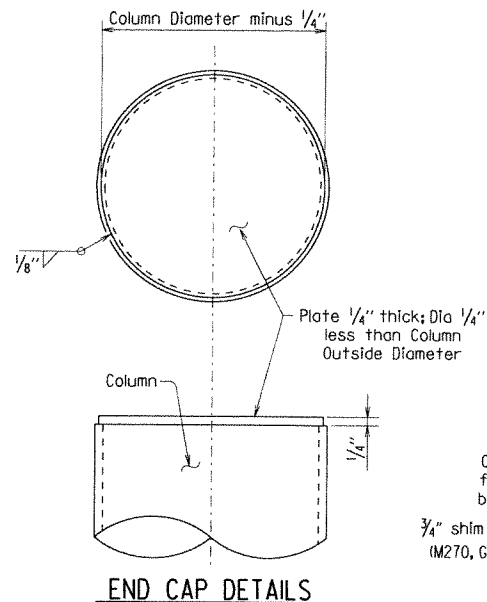
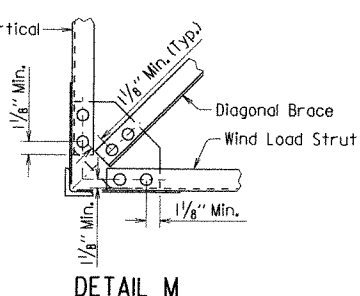
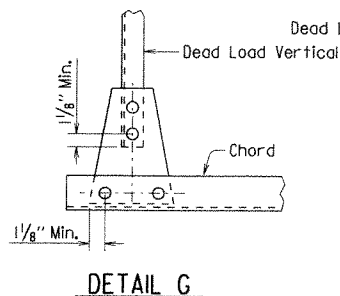
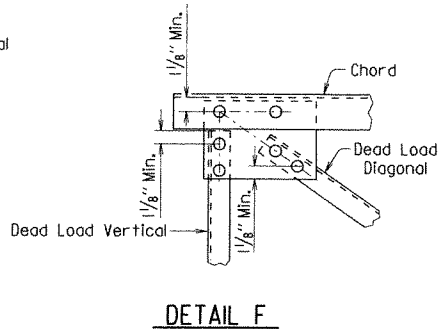
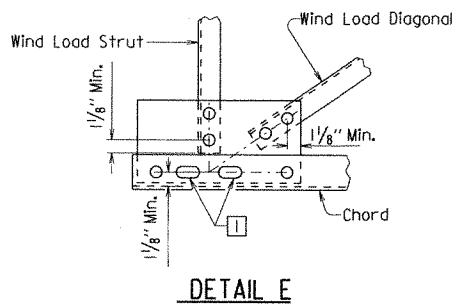
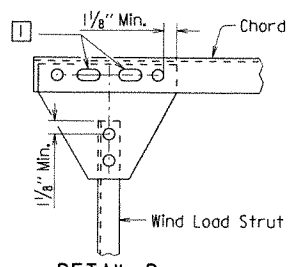
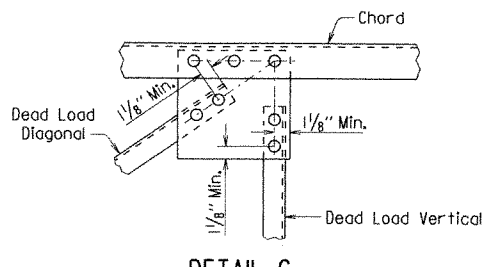
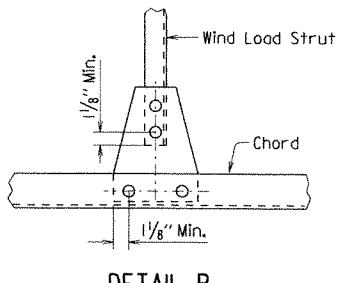
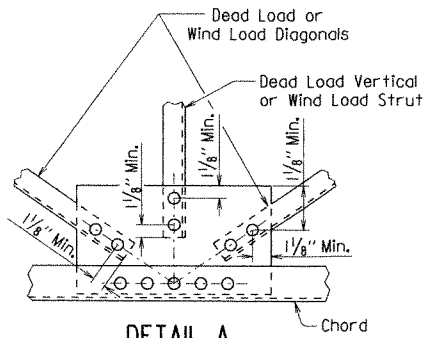
ROUTE 530 SEC. 1
ARKANSAS STATE HIGHWAY COMMISSION
LITTLE ROCK, ARK.

DRAWN BY: LJB DATE: 8/15/12 FILENAME: bb0614signdetails.dgn
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DESIGNED BY: STD DATE:
BRIDGE NO. OH-530-60-03 DRAWING NO. 53255

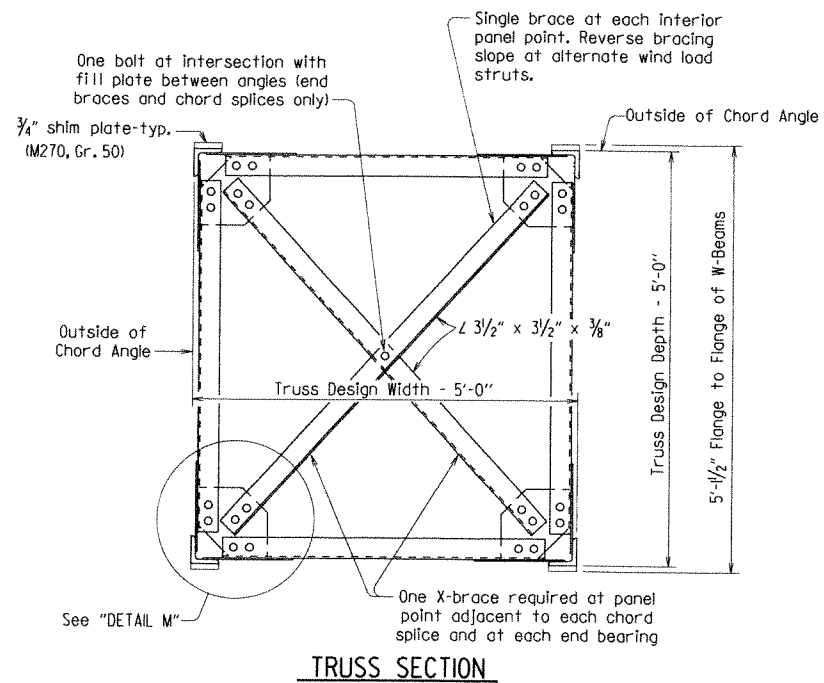
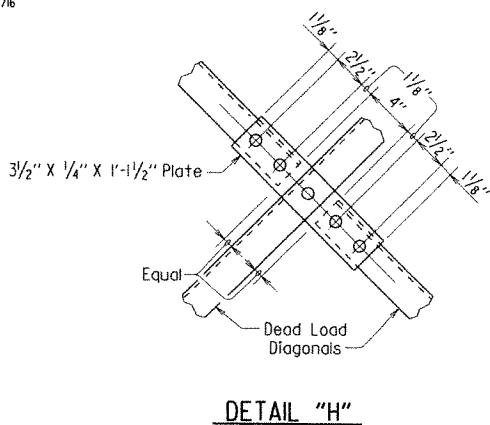
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				6	ARK.			
				JOB NO.		BBO6I4	174	219
				OH-530-60-03 SIGN STRUCTURE 53257				

Note: All Gusset Plate thicknesses shall be $\frac{3}{8}$ ".

1 Slotted Hole in Gusset Plate and Chord Angle $\frac{1}{8}$ " X 2". Use plate washer on Gusset plate side. $\frac{1}{8}$ " holes in $\frac{3}{4}$ " shim plate and beam flange.



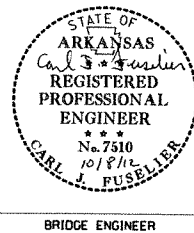
Note: Chord angles may be spliced in convenient lengths for galvanizing and sign placement.



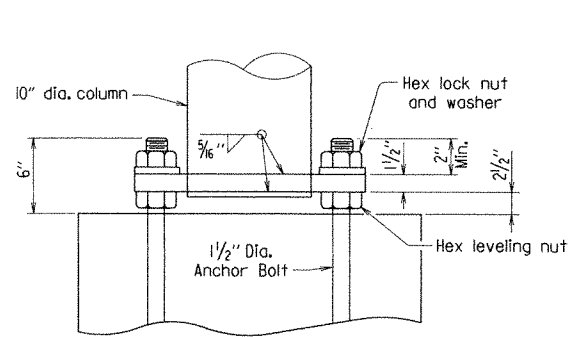
SHEET 3 OF 5
DETAILS FOR 45' TO 60'
STEEL OVERHEAD SIGN STRUCTURES
BINGHAM RD. - GRANT COUNTY LINE (S)
PULASKI AND SALINE COUNTIES

ROUTE 530 SEC. 1
ARKANSAS STATE HIGHWAY COMMISSION
LITTLE ROCK, ARK.

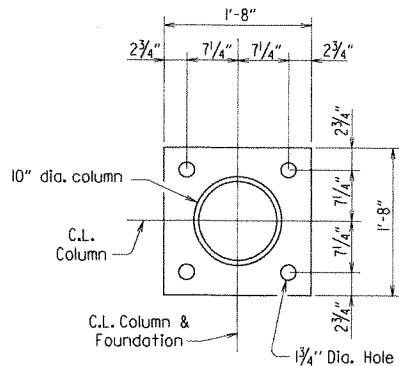
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BRIDGE NO. OH-530-60-03 DRAWING NO. 53257



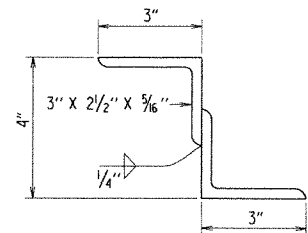
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				6	ARK.			
				JOB NO.		BB0614	176	219
				OH-530-60-03 SIGN STRUCTURE				
				53258				



ELEVATION - COLUMN BASE

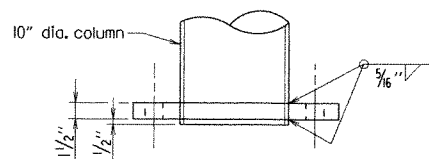


PLAN - COLUMN BASE



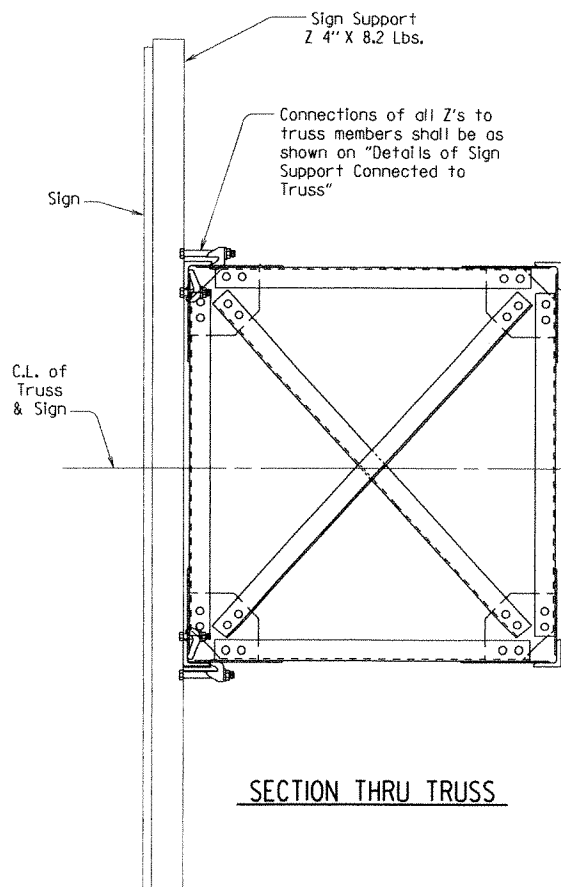
NOTE: Structural Z support may be fabricated from angles as shown.

DETAILS OF ALTERNATE Z SUPPORT

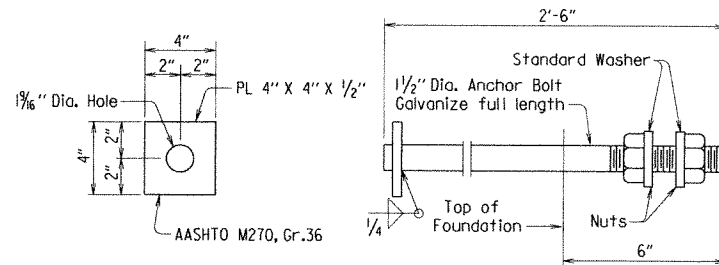


Note: Diameter of hole in base plate to be 1/8" larger than column diameter.

DETAIL OF COLUMN CONNECTION TO BASE PLATE

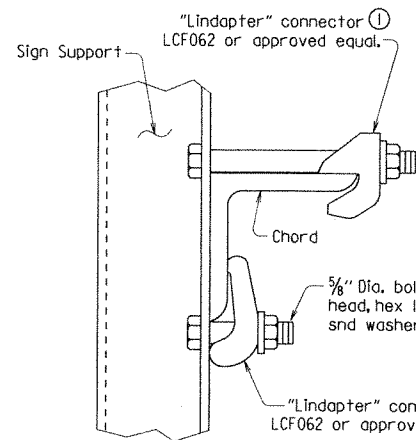


SECTION THRU TRUSS

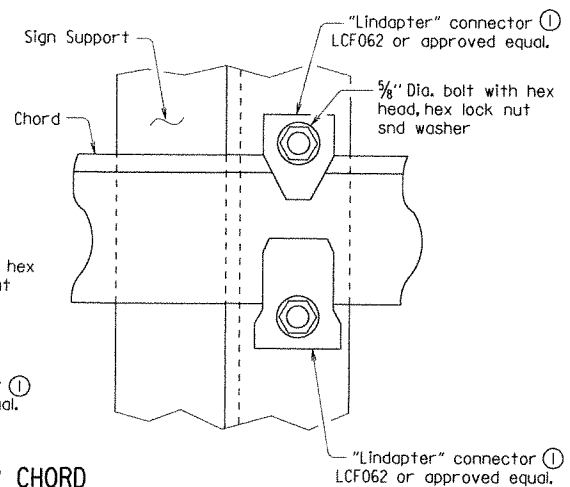


Anchor bolts shall comply with AASHTO M314, Grade 55, with Supplementary Requirement S1, and galvanized according to Section 807.07. Nuts for bolts shall be as specified in Section 807.07.

ANCHOR BOLT DETAIL



TOP CHORD



BOTTOM CHORD

① Note: All "Lindapter" connectors or approved equal shall be installed according to manufacturer's recommendations. All connectors, bolts, nuts and washers shall be galvanized.

Note: Install all support connectors clear of the gusset plates and splice locations.

DETAIL OF SIGN SUPPORT CONNECTED TO TRUSS



BRIDGE ENGINEER

SHEET 4 OF 5
DETAILS FOR 45' TO 60'
STEEL OVERHEAD SIGN STRUCTURES
BINGHAM RD. - GRANT COUNTY LINE (S)
PULASKI AND SALINE COUNTIES

ROUTE 530 SEC. 1
ARKANSAS STATE HIGHWAY COMMISSION
LITTLE ROCK, ARK.

DRAWN BY: LJB DATE: 8/15/12 FILENAME: bb0614signdetails.dgn
CHECKED BY: CMW DATE: 9/19/12 SCALE: NOT TO SCALE
DESIGNED BY: STD DATE: BRIDGE NO. OH-530-60-03 DRAWING NO. 53258

GENERAL NOTES:
Construction Specifications: Arkansas State Highway Commission Standard Specifications for Highway Construction, Edition of 2003, with applicable special provisions and Supplemental Specifications.

Design Specifications: Standard Specifications for Structural supports for Highway Signs, Luminaries and Traffic Signals, AASHTO Fifth Edition (2009) with 2011 Interims.

Basic Wind Speed = 90 m.p.h.

This structure is approved for a sign area equivalent to 75% of the span at 15' tall. Use of additional sign area must be approved by the Engineer. If the structure height ("H1" or "H2") exceeds 30'-0" contact the engineer.

FOUNDATION MATERIALS AND STRENGTHS:

Class S Concrete $f'c = 3,500$ psi.
Reinforcing Steel (AASHTO M31 or M53, Grade 60) $f_y = 60,000$ psi.

Structural steel sign support members shall comply with the following specifications:

Angles: AASHTO M270, Grade 36 ($F_y = 36,000$ psi.)
Plate, W-Section, WT-Section: AASHTO M270, Grade 50 ($F_y = 50,000$ psi.)
Pipe: ASTM A500 Grade B ($F_y = 42,000$ psi.)
ASTM A501 Grade B ($F_y = 50,000$ psi.)
ASTM A714, Class 2, Grade II, Type E or S ($F_y = 50,000$ psi.)
All grades of pipe require Heat traceability back to the original melting mill
Z-Shapes: AASHTO M270, Grade 36 ($F_y = 36,000$ psi.)
Shim Plates: ASTM A1011, SS, Grade 36, Type 2, or Grade 40.
Bolts: AASHTO M164, Type I.
Locknuts - Approved Type: Meeting or exceeding AASHTO M292
Washers: AASHTO M293
Nuts: AASHTO M291 or M292, Grade 2H or Grade DH (Grade IOS)

All steel shall be Galvanized according to section 807.19. Steel completely encased in concrete may not be Galvanized.

Drawings show general features of design only. Shop drawings shall be made in accordance with section 807.04, submitted, and approval secured before fabrication is begun.

No circumferential butt welds will be allowed in any pipe sections.

Requests for substitution of structural steel shapes shown with shapes of greater size must be submitted by the Contractor to the Engineer for approval. Steels of equal or greater strengths will be accepted only when shown on the approved shop drawings. Shapes and materials shown in the plans will be the basis of payment and no additional compensation will be made for any adjustments due to substitutions.

Connections shall be bolted with high-strength bolts. Unless noted otherwise, bolts shall be 5/8" diameter and open holes shall be 1 1/16". Bolt spacing shall be 2 1/4" for 5/8" diameter bolts unless otherwise noted. Bolts shall be placed with heads on the outside face of all members.

All welding that is to be done during fabrication of structural steel, including temporary welds, shall be detailed on the shop drawings and submitted for approval. If additional welds are required, whether temporary or permanent, a formal request with detailed drawings shall be submitted to the Engineer for approval. All welding shall conform to subsection 807.26.

Anchor bolts shall comply with AASHTO M314, Grade 55, with Supplementary Requirement SI, and galvanized according to Section 807.07. Nuts and washers for anchor bolts shall be furnished and galvanized in accordance with subsection 807.07. Anchor Bolts shall be pretensioned in accordance with Special Provision BB0614 "STEEL OVERHEAD SIGN STRUCTURES".

All truss frame bolts shall comply with AASHTO M164 Type I, galvanized according to Subsection 807.06. Nuts and washers for AASHTO M164 Type I bolts shall be furnished and galvanized in accordance with subsection 807.06.

Field splices shall be located in order to avoid sign panel connections. There shall be a maximum of two field splices and they shall be spaced a minimum of 15 ft. apart.

All main load carrying tension members greater than 1/2" in thickness shall conform to the requirements of the Longitudinal Charpy V-notch test specified for Zone I minimum service temperature. This work and materials shall be paid for in accordance with Special Provision Job BB0614 "STEEL OVERHEAD SIGN STRUCTURES".

All fillet welds of critical members shall be tested according to AWS D1.1 Structural Welding Code - Steel, using the magnetic particle method. Critical welds shall include: column to base plate and truss bottom support to column.

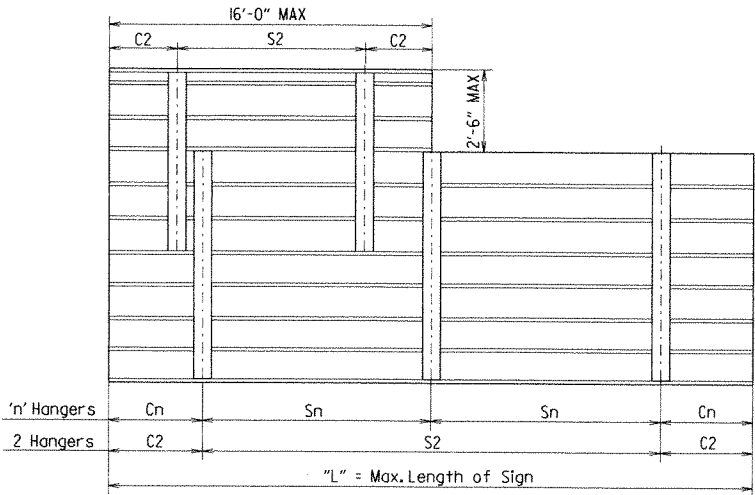
Truss field sections shall be shop assembled. Entire truss shall be fully assembled and lifted into place as one unit on to tower supports. All truss member connections shall be bolted connections.

Lock nuts to be equipped with nylon locking inserts or other approved type locking system. Locknuts to be installed according to manufacturer's recommendations.

The excavations for the footings shall be back-filled before the structure is attached to the foundations.

Galvanized Coating damaged during transport, handling, or erection shall be field repaired in accordance with section 807.88.

The Contractor shall make check measurements in the field and make any adjustments necessary to meet the required clearances and to fit the new structures to the existing conditions.



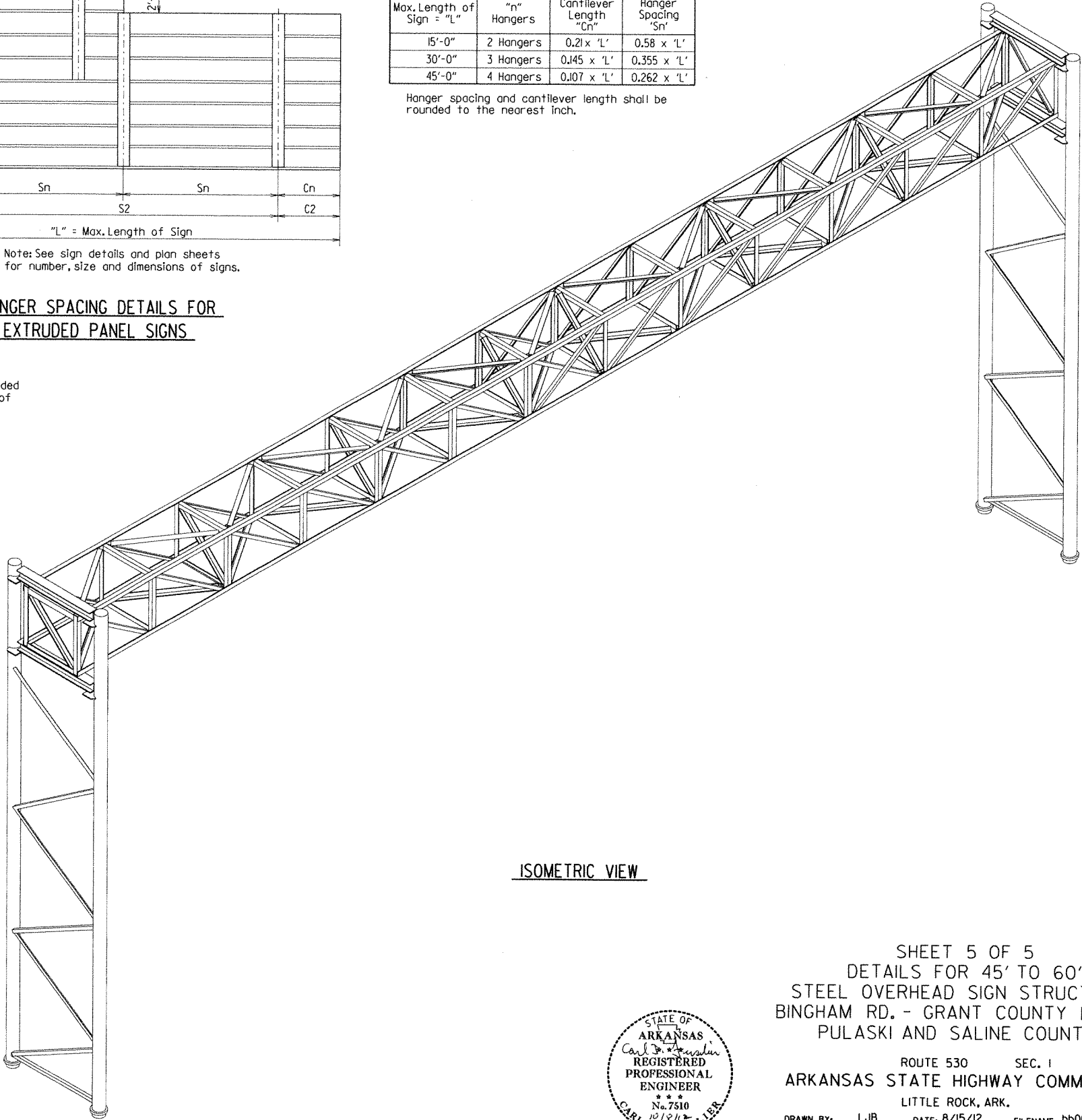
Note: See sign details and plan sheets for number, size and dimensions of signs.

HANGER SPACING DETAILS FOR
EXTRUDED PANEL SIGNS

In addition to material requirements, all pipe used for welded applications shall have a maximum carbon equivalency (CE) of 0.4 using the following equation: $CE = \frac{1}{2}C + \frac{Mn}{6} + \frac{Cu}{40} + \frac{Ni}{20} + \frac{Cr}{10} - \frac{Mo}{50} - \frac{V}{10}$

HANGER VARIABLES			
Max. Length of Sign = "L"	"n" Hangers	Cantilever Length "Cn"	Hanger Spacing "Sn"
15'-0"	2 Hangers	0.21 x 'L'	0.58 x 'L'
30'-0"	3 Hangers	0.145 x 'L'	0.355 x 'L'
45'-0"	4 Hangers	0.107 x 'L'	0.262 x 'L'

Hanger spacing and cantilever length shall be rounded to the nearest inch.

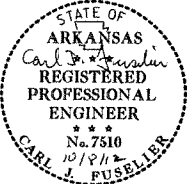


ISOMETRIC VIEW

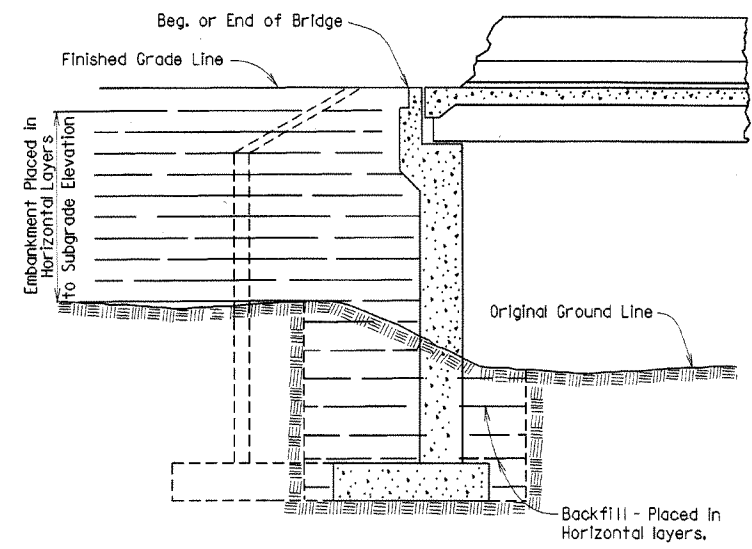
SHEET 5 OF 5
DETAILS FOR 45' TO 60'
STEEL OVERHEAD SIGN STRUCTURES
BINGHAM RD. - GRANT COUNTY LINE (S)
PULASKI AND SALINE COUNTIES

ROUTE 530 SEC. 1
ARKANSAS STATE HIGHWAY COMMISSION
LITTLE ROCK, ARK.

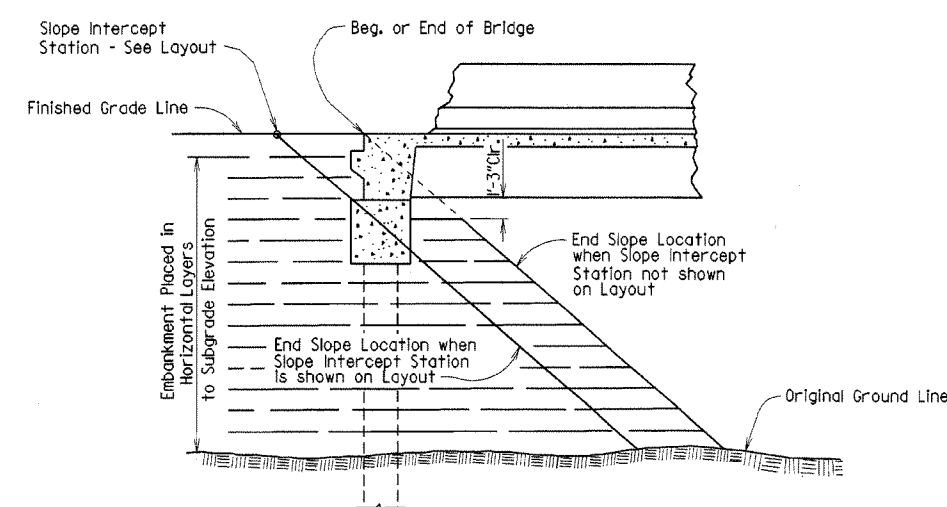
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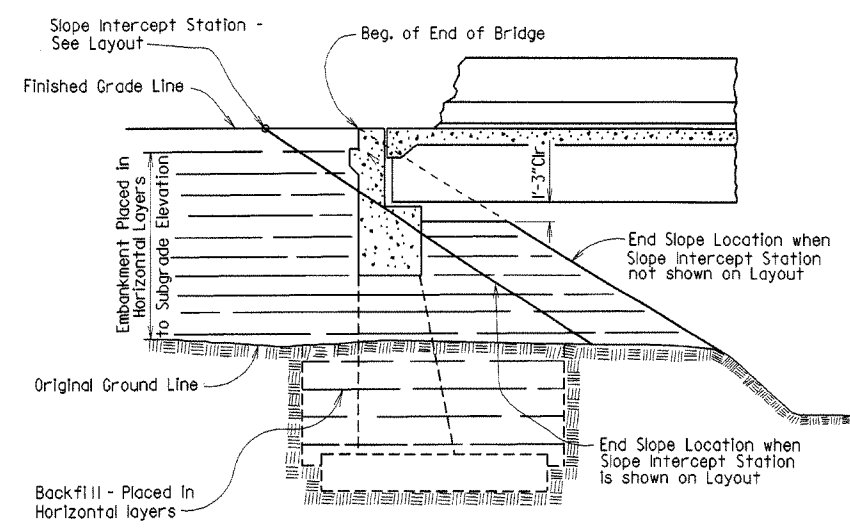
BRIDGE ENGINEER



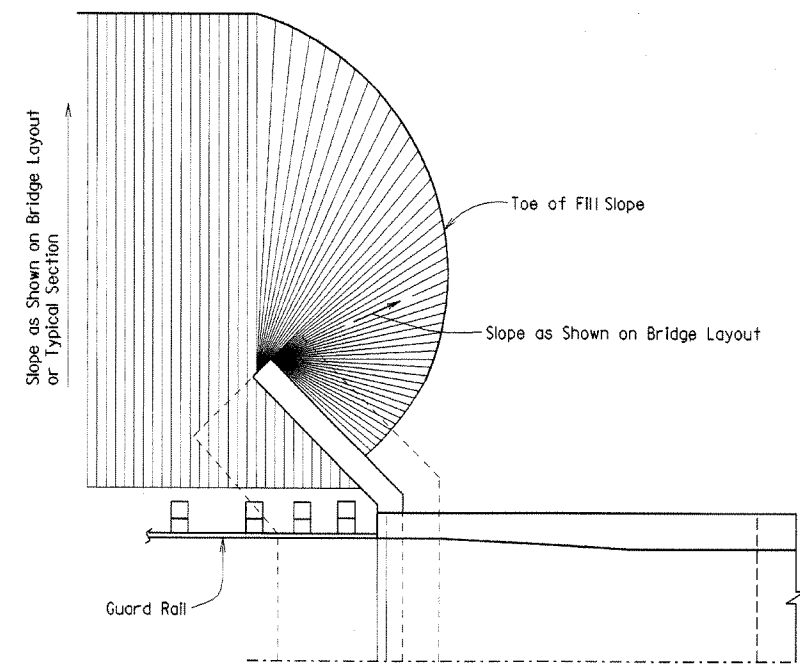
EMBANKMENT CONSTRUCTION AND FOOTING BACKFILL AT VERTICAL WALL ABUTMENTS



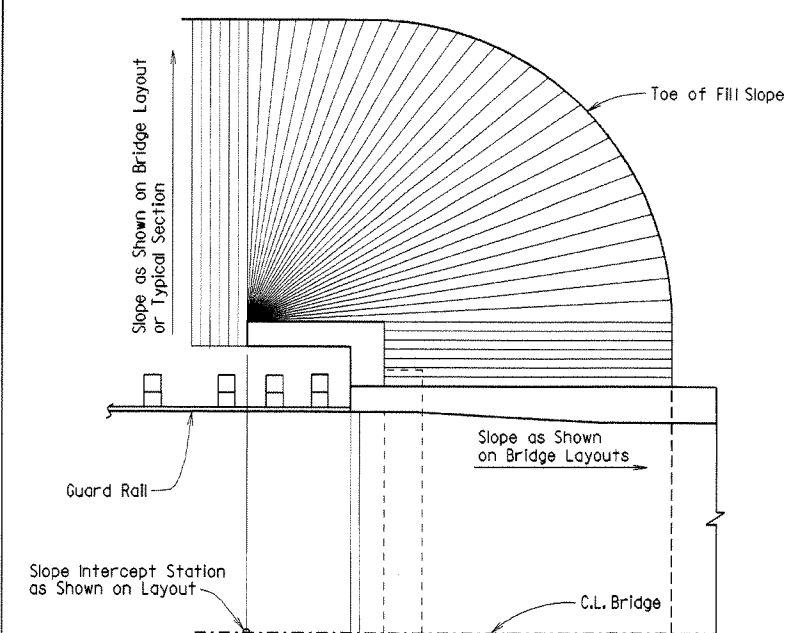
EMBANKMENT CONSTRUCTION AT SPILL-THROUGH PILE END BENTS



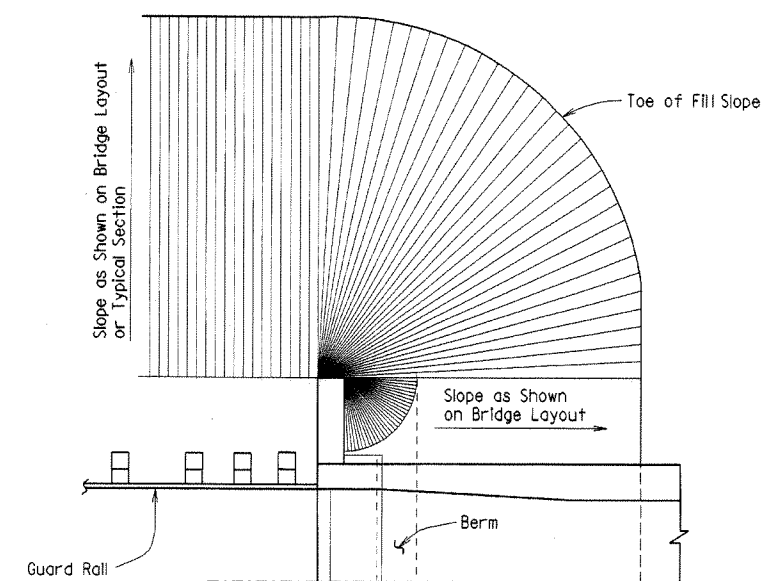
EMBANKMENT CONSTRUCTION AND FOOTING BACKFILL AT SPILL-THROUGH END BENTS



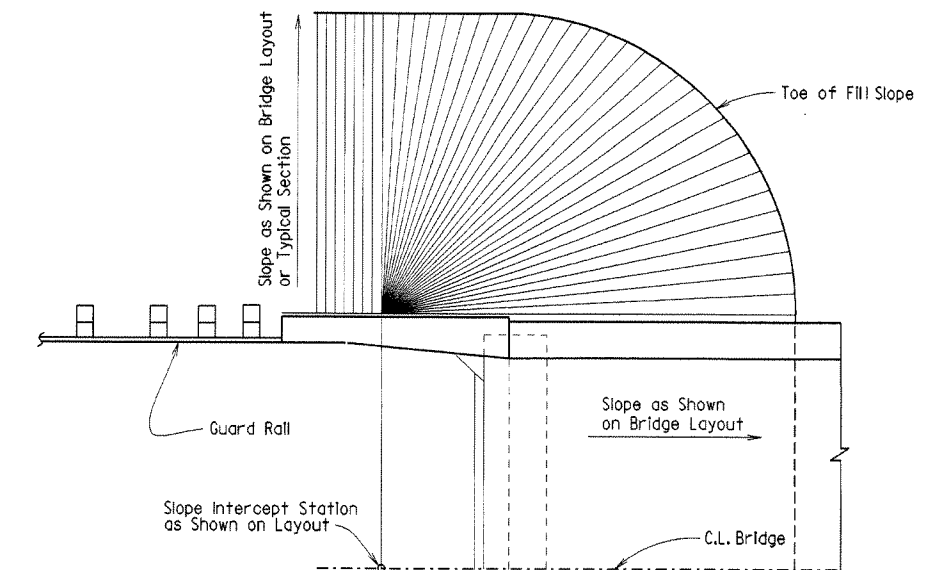
VERTICAL WALL ABUTMENTS



SPILL-THROUGH END BENTS WITH TURNBACK WING



SPILL-THROUGH END BENTS WITH STUB WING



SPILL-THROUGH END BENTS WITH TRANSITION WING

METHOD OF DETERMINING FILL SLOPE LOCATION AT BRIDGE ENDS

GENERAL NOTES

The Bridge End Embankment shall be defined as a section of embankment, not less than 20 feet long adjacent to the bridge end, together with the side slopes and slopes under the bridge end including around the end of wingwalls. Embankment adjacent to structures shall be constructed in 4 inch horizontal layers (loose measure) and compacted by the use of mechanical equipment to the satisfaction of the Engineer. Refer to subsections 210.09, 210.10 and 801.08 of the Specifications for construction requirements.

Revised and redrawn MJT 04-10-2003
Chk'd. By: csf 04-10-2003



BRIDGE ENGINEER

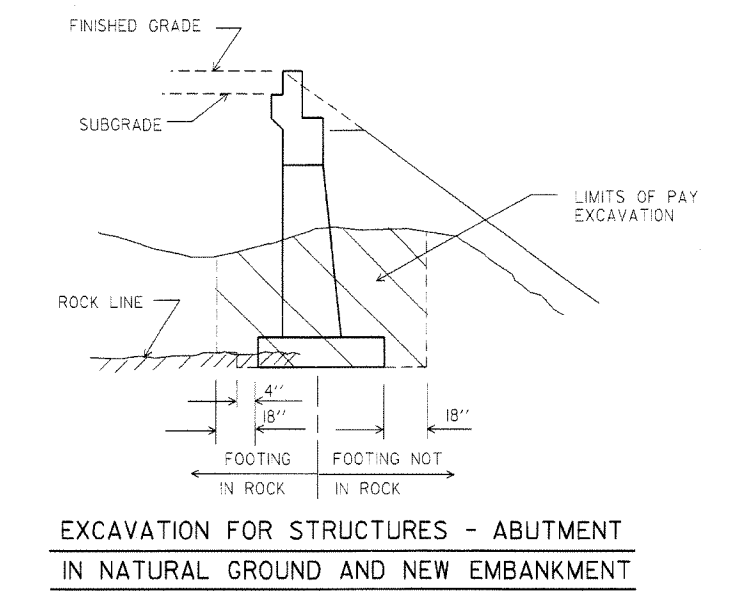
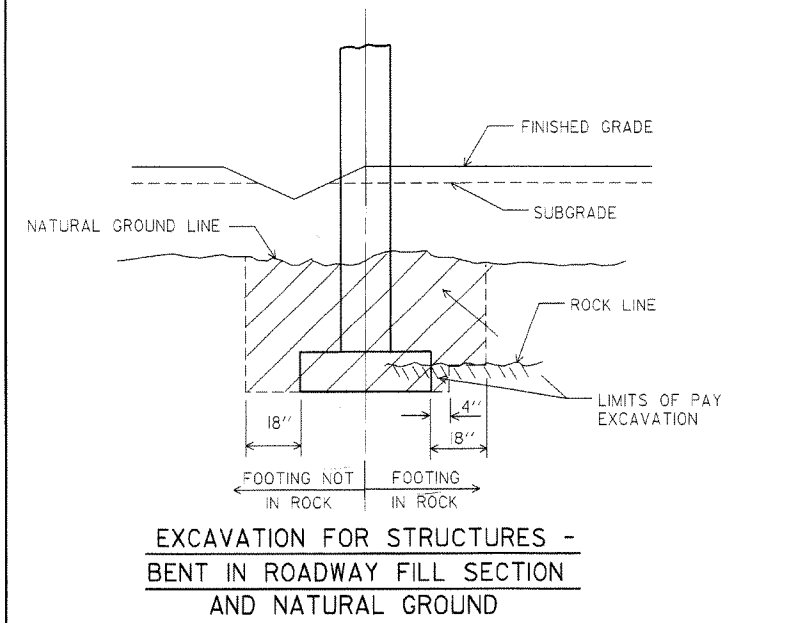
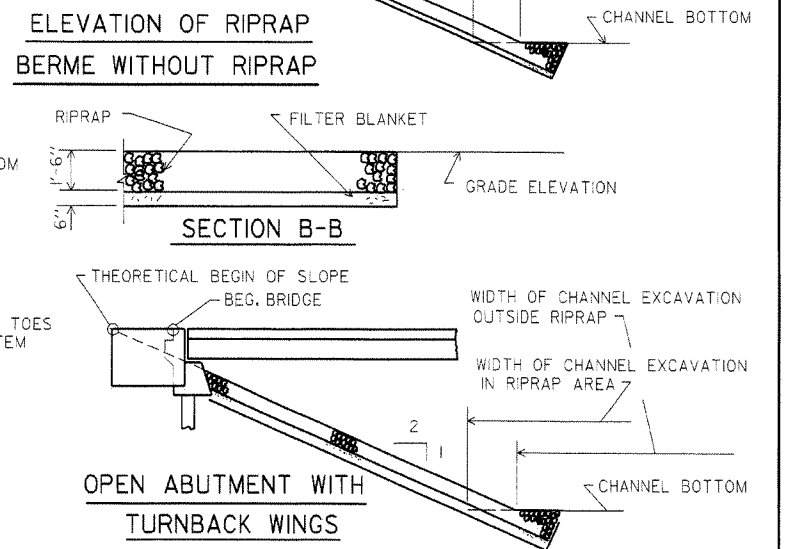
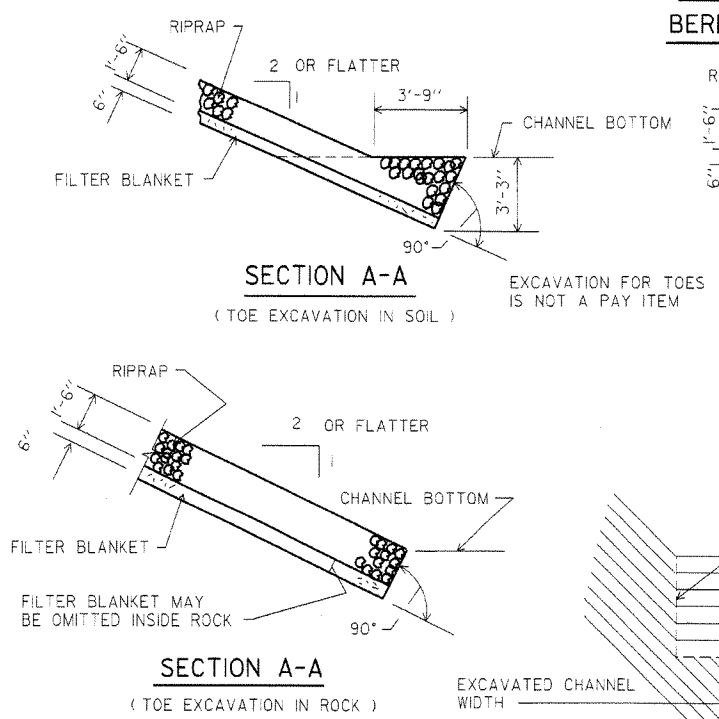
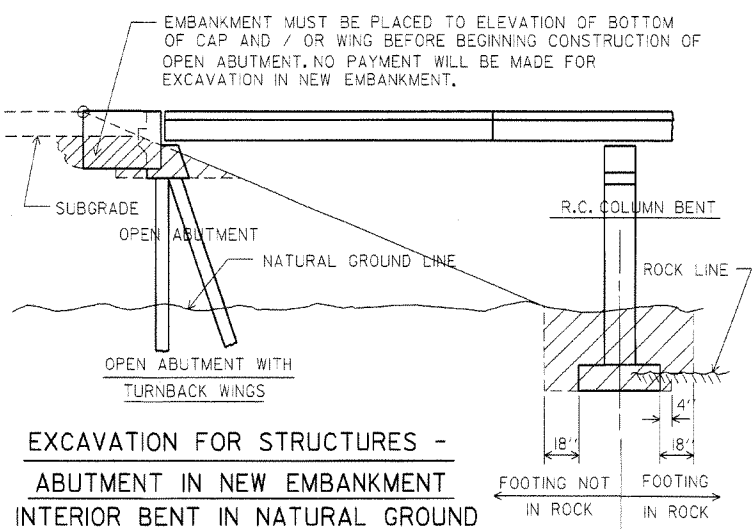
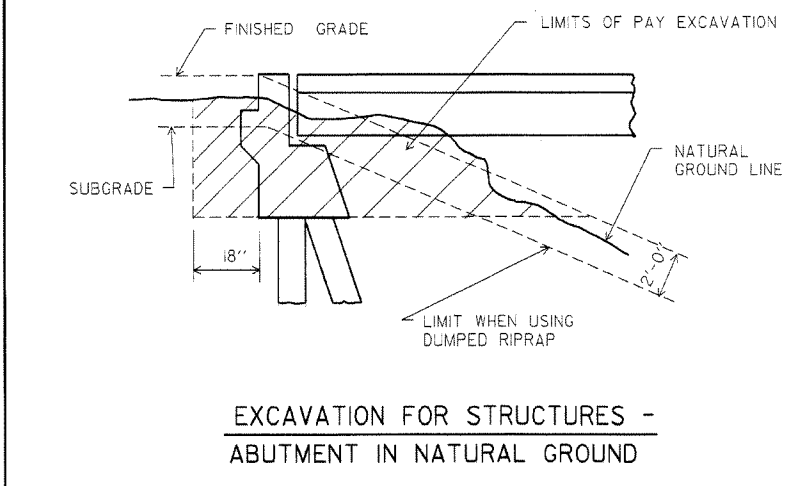
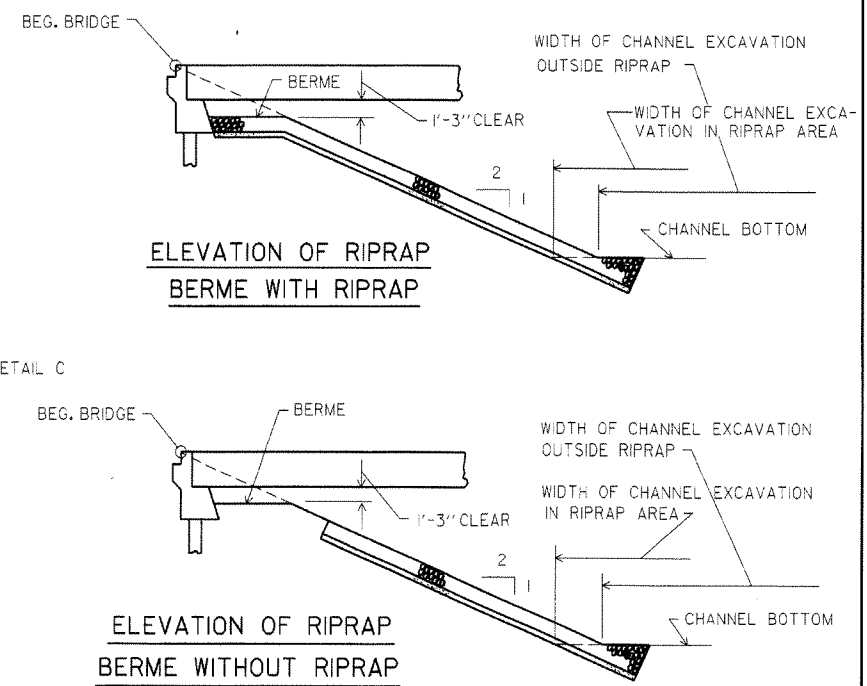
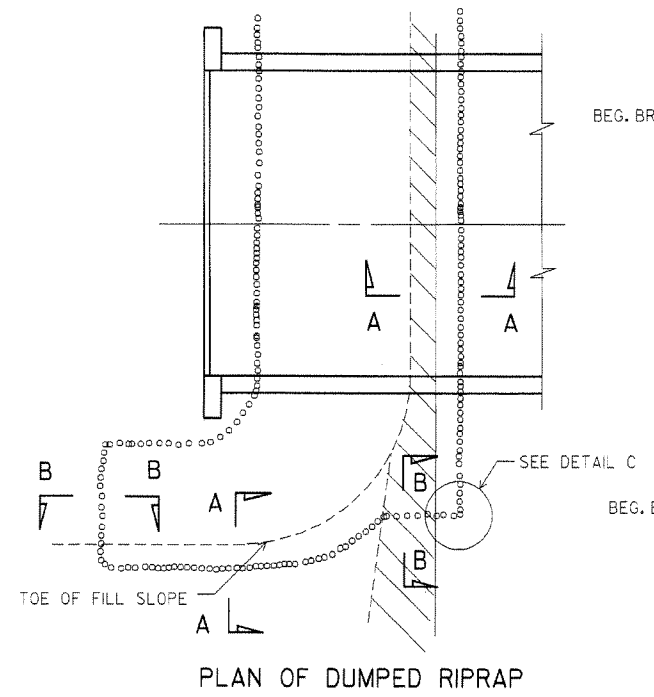
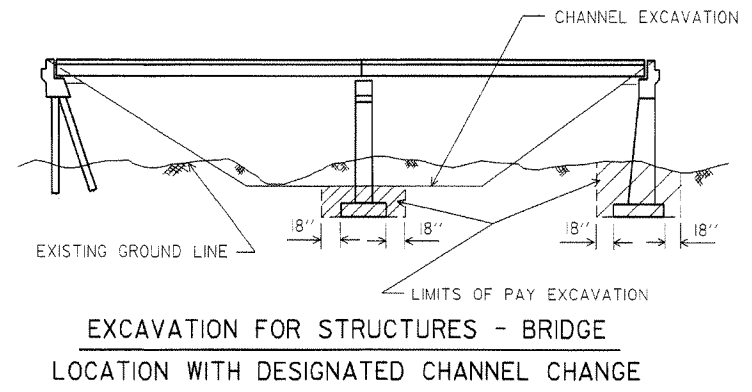
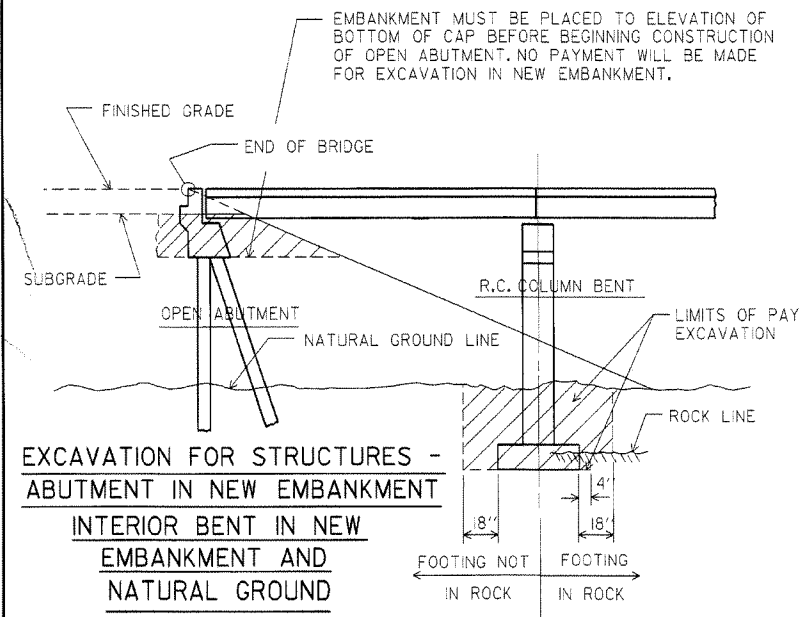
EMBANKMENT CONSTRUCTION AND BACKFILL AT BRIDGE ENDS

ROUTE SEC.
ARKANSAS STATE HIGHWAY COMMISSION
LITTLE ROCK, ARK.

DRAWN BY: MJT DATE: 04-10-2003 FILENAME: B1888A.STD
CHECKED BY: C.J.F. DATE: 04-10-2003 SCALE: NO SCALE
DESIGNED BY: STD. DATE:
BRIDGE NO. DRAWING NO. 1888A

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
04-10-2003				6	ARK.		177	
				JOB NO.				
				EMBANKMENT & BACKFILL			1888A	

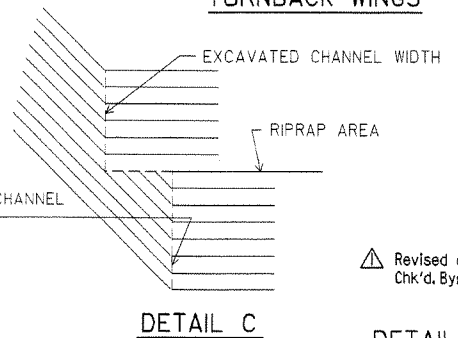
DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
04-10-2003				6	ARK.		178	
				JOB NO.				
				1 RIP. & EXCAV.			189IF	



NOTE : USE THIS TYPE OF TOE WHEN ROCK IS ENCOUNTERED WHICH IS IN A STABLE CONDITION.

NOTE : IN LIEU OF AN AGGREGATE FILTER BLANKET, A SYNTHETIC FIBER GEOTEXTILE FABRIC COMPLYING WITH THE REQUIREMENTS OF SUBSECTION 816.02(e) MAY BE USED.

NOTE : DETAILS FOR COMPUTING EXCAVATION FOR STRUCTURES ARE INCLUDED FOR INFORMATION AS TO HOW PLAN QUANTITIES WERE CALCULATED AND FOR USE WHEN ADJUSTING QUANTITIES WHEN CHANGING FOOTING ELEVATION.



STATE OF ARKANSAS
REGISTERED PROFESSIONAL ENGINEER
No. 4337
CHARLES P. BRAND
BRIDGE ENGINEER

Revised and redrawn MJT 04-10-2003
Chk'd. By: CJF 04-10-2003

DETAILS FOR DUMPED RIPRAP AND FILTER BLANKET AND DETAILS FOR COMPUTING EXCAVATION FOR STRUCTURES

ROUTE SEC.
ARKANSAS STATE HIGHWAY COMMISSION
LITTLE ROCK, ARK.

DRAWN BY: MJT DATE: 04-10-2003 FILENAME: B189IF.STD
CHECKED BY: CJF DATE: 04-10-2003 SCALE: NO SCALE
DESIGNED BY: STD DATE: BRIDGE NO. DRAWING NO. 189IF

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
9-8-11				6	ARK.		179	
				JOB NO.		NAME PLATE 2387		

The name of the bridge as shown on the plans shall be placed on Lines 1 - 3 using $\frac{1}{8}$ " raised letters and numerals $\frac{3}{8}$ " high.

Example 1	Example 2	Example 3	Example 4
Line 1 Red River	Line 1 Southern	Line 1 Saline	Line 1 Highway 5
Line 2 Relief	Line 2 Railroad	Line 2 River	
Line 3	Line 3 Overpass	Line 3 Relief	

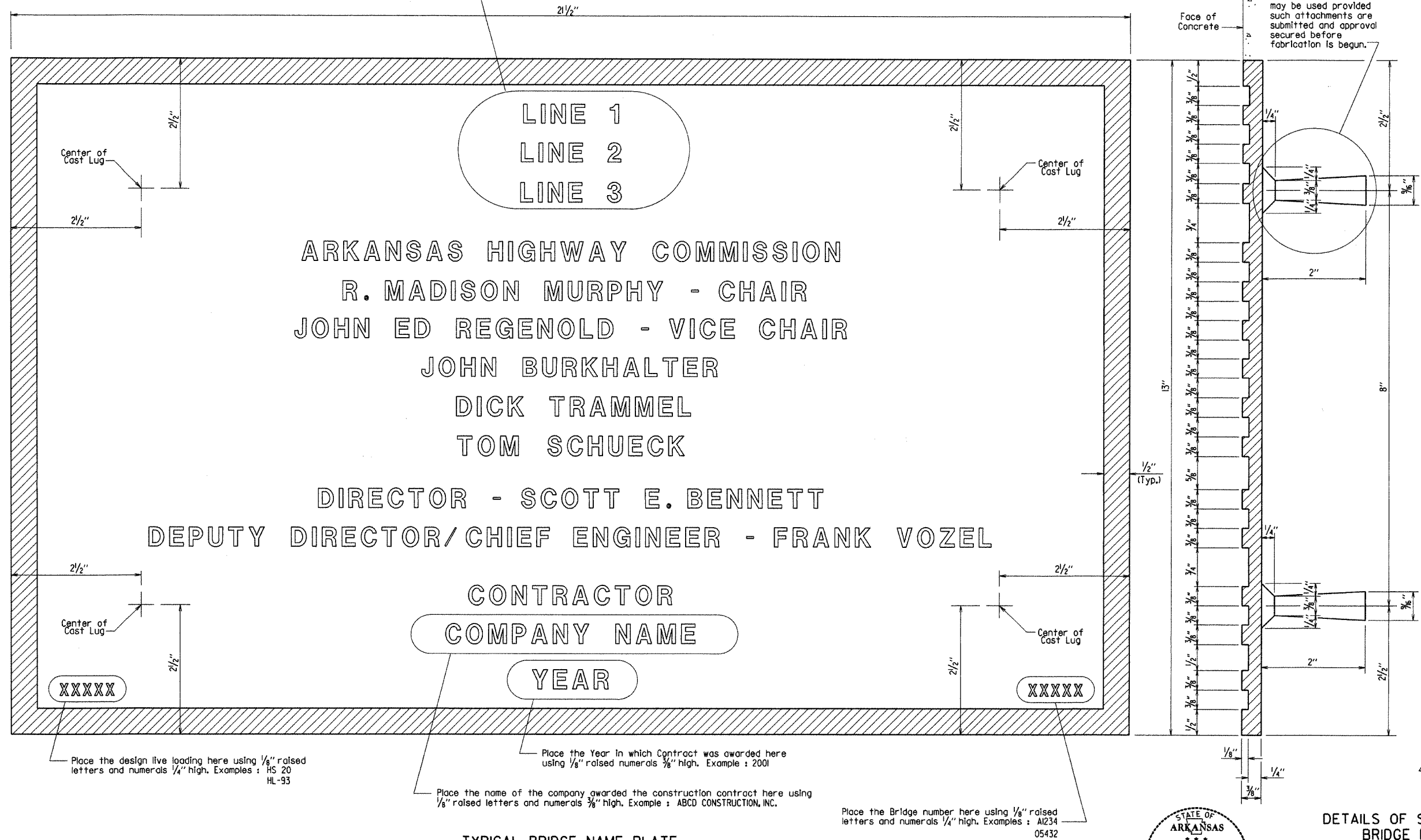
GENERAL NOTES

Specifications: Arkansas State Highway and Transportation Department Standard Specifications for Highway Construction, (2003 Edition) with applicable Supplemental Specifications and Special Provisions.

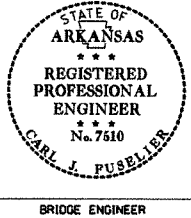
Name plates shall be cast bronze and shall meet the material requirements as specified in Section 812 of the Standard Specifications.

Body of plate shall be $\frac{1}{4}$ " thick and shall include four tapering cone lugs $\frac{3}{8}$ " to $\frac{9}{16}$ " x 2" long. The border and all lettering shall be raised $\frac{1}{8}$ " above the face of plate and shall be polished.

All lettering shall be plain gothic, square cut and not tapered. The number of plates required and the location and name on the plate for each bridge shall be as designated on the plans.



TYPICAL BRIDGE NAME PLATE



**DETAILS OF STANDARD TYPE D
BRIDGE NAME PLATE**

ROUTE SEC.
ARKANSAS STATE HIGHWAY COMMISSION
LITTLE ROCK, ARK.

DRAWN BY: KDH DATE: 9-8-11 FILENAME: B2387.STD
CHECKED BY: CRE DATE: 9-8-11 SCALE: 1'-0" = 1'-0"
DESIGNED BY: STD. DATE: OR AS NOTED
BRIDGE NO. DRAWING NO. 2387

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
12-10-2009				6	ARK.		180	
				JOB NO.		CONCRETE PILES		
						14964		

MAXIMUM PICKUP LENGTHS "L"

Type of Pick-Up	Prestressed		Non-Prestressed		Prestressed			Non-Prestressed		
	16" Oct.	18" Oct.	16" or 18" Oct.	④ 14" Sq.	16" Sq.	18" Sq.	④ 14" Sq.	16" Sq.	18" Sq.	
One Point	52'	55'	46'	55'	59'	63'	52'	51'	55'	
Two Point	75'	80'	67'	79'	84'	90'	75'	74'	79'	
Three Point	105'	112'	93'	110'	117'	126'	104'	103'	111'	

GENERAL NOTES:

CONSTRUCTION SPECIFICATIONS: Arkansas State Highway and Transportation Department Standard Specifications for Highway Construction (2003 Edition) with applicable Supplemental Specifications and Special Provisions.

DESIGN SPECIFICATIONS: AASHTO LRFD Bridge Design Specifications, Fourth Edition.

SEISMIC PERFORMANCE ZONES: 1 & 2

The Contractor may use prestressed piles or non-prestressed piles. Either type will be measured and paid for at the contract unit price bid for "Concrete Piling".

SPIRAL REINFORCING: Spiral reinforcing shall be steel wire meeting the requirements of AASHTO M 32 or M 225 or shall be plain round steel bars meeting the requirements of AASHTO M 31 or M 53, Gr. 60.

MANUFACTURE, TRANSPORTATION AND STORAGE: Shipment of piles from the plant site or pile driving will not be permitted until the required minimum compressive strength is reached, and in no case less than 10 days after pouring the concrete. Prestressed piles may be removed from the casting bed to nearby storage any time after transfer of stress. See Section 802 "Concrete for Structures" of the Standard Specifications for additional information.

⚠ Unless otherwise approved by the Engineer, all protruding or exposed pile lifting or transporting devices above the finished ground shall be removed after pile driving is complete. Removal shall be a minimum of 1" below the surface of the pile and the cavity shall be filled with a non-shrink grout listed on the Department's QPL.

FORMS: For forming exterior of piles, the use of steel forms on concrete-founded casting beds is required unless otherwise approved by the Engineer. Side forms may have a maximum drift on each side not exceeding 1/4" per foot.

TOLERANCES: Pile ends shall be plane surfaces perpendicular to the longitudinal axis of pile with a maximum tolerance of 1/8" per foot transversely.

The maximum sweep (deviation from straightness measured from end to end of the pile, while not subject to bending forces) shall not exceed 1/8" in 10 feet.

BUILD-UPS: To provide for build-ups of piles where authorized by the Engineer, concrete shall be cut back to expose the reinforcing steel for a distance sufficient to provide a lap of 60 diameters of the reinforcing bars required for build-up. Reinforcing for build-ups shall be the reinforcing shown for non-prestressed piles.

INSTALLATION, MEASUREMENT AND PAYMENT: See Section 805 "Piling" of the Standard Specifications.

ADDITIONAL NOTES FOR PRESTRESSED PILES ONLY:

CONCRETE: Concrete in prestressed piles shall be Class S(AE) and shall have a minimum compressive strength (f'c) of 5,000 psi at 28 days. Compressive strength at transfer of the prestressing force shall be not less than 4,000 psi. Concrete in build-ups shall have a minimum compressive strength of 4,000 psi and shall be cured for a minimum of 10 days.

PRESTRESSING REINFORCING: Seven-wire stress-relieved or low relaxation strands shall conform to the general requirements of AASHTO M 203. Broken wires within individual strands will be permitted up to 2% of the total number of wires in each pile, providing that there is not more than one broken wire per strand. Two or more broken wires per strand will be cause for replacement of the strand, even though the two broken wires are within the 2% limitation.

ADDITIONAL NOTES FOR NON-PRESTRESSED PILES ONLY:

All concrete shall be Class S(AE) and shall have a minimum compressive strength (f'c) of 4,000 psi at 28 days.

All longitudinal reinforcing bars shall be deformed bars and shall conform to the requirements of AASHTO M 31 or M 53, Gr. 60.

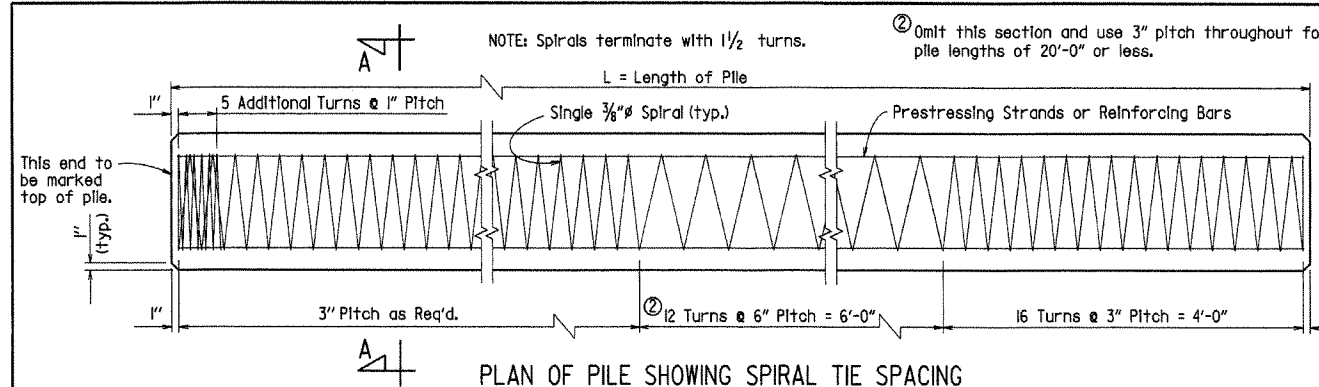
⚠ Added paragraph to General Notes KOH 12-10-2009
Checked by: cdf Date: 12-10-2009



STANDARD DETAILS OF CONCRETE PILES IN SEISMIC PERFORMANCE ZONES 1 & 2

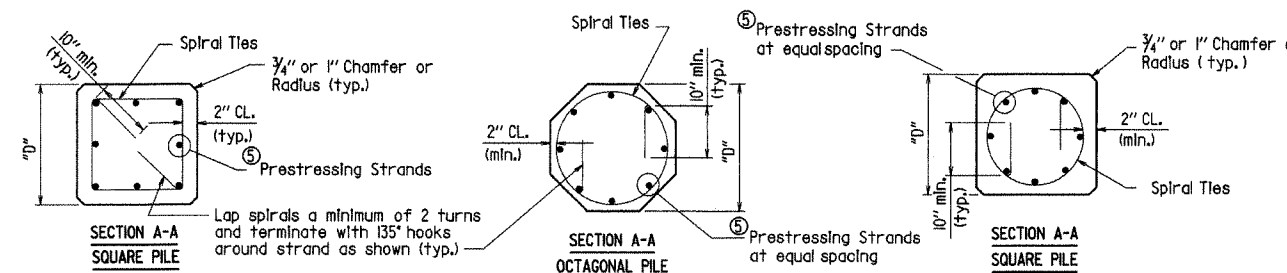
ROUTE SEC.
ARKANSAS STATE HIGHWAY COMMISSION
LITTLE ROCK, ARK.

DRAWN BY: KMY DATE: 1-17-08 FILENAME: b4964_std.dgn
CHECKED BY: JAC DATE: 1-17-08 SCALE: no scale
DESIGNED BY: Std DATE: BRIDGE NO. DRAWING NO. 14964



For anchorage of pile to bent, see Bent Details.

NOTE: Strand location shall be symmetrical about the axis of the pile with no more than one strand difference between any two adjacent sides. Circular spiral ties are required for odd number of strands.



① Number based on Initial prestress force of "B" x Ultimate Tensile Stress, Prestress Losses and min. 700 psi Unit Prestress on concrete after Losses.

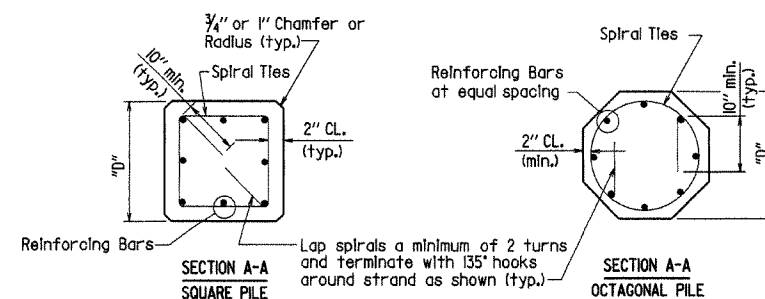
"B" 0.75 Low Relaxation
0.70 Stress-Relieved

⑤ See table "Prestressed Concrete Pile Properties" for actual number of strands per pile size.

PRESTRESSED CONCRETE PILES

PRESTRESSED CONCRETE PILE PROPERTIES

	Grade	Strand Diameter	① Number of Strands per Size "D"					Minimum Ultimate Tensile Strength Per Strand (Lbs.)	Initial Prestressing Force Per Strand (Lbs.)
			16" Oct.	18" Oct.	④ 14" Sq.	16" Sq.	18" Sq.		
Stress-Relieved	250	7/16"	11	13	10	13	16	27,000	18,900
		1/2"	8	10	8	10	12	36,000	25,200
	270	7/16"	9	11	8	12	14	34,000	21,700
		1/2"	7	9	6	8	10	41,300	28,900
Low Relaxation	250	7/16"	9	11	8	11	14	27,000	20,200
		1/2"	7	9	6	8	10	36,000	27,000
	270	7/16"	8	10	7	9	12	31,000	23,300
		1/2"	6	8	6	7	9	41,300	31,000

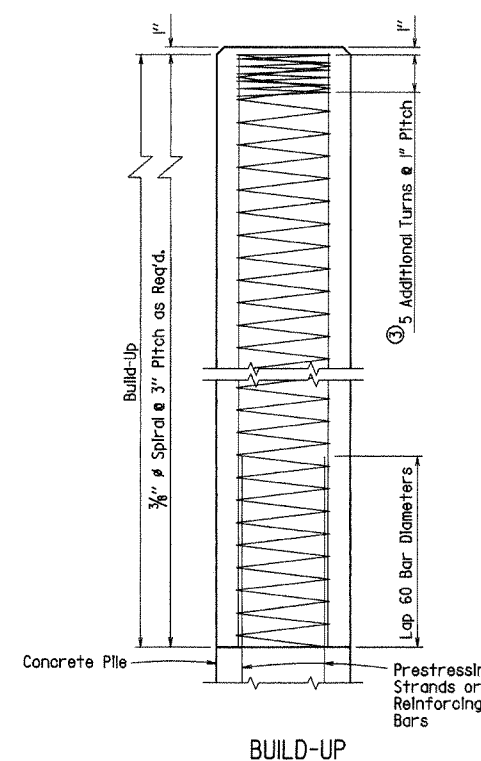


NON-PRESTRESSED CONCRETE PILES

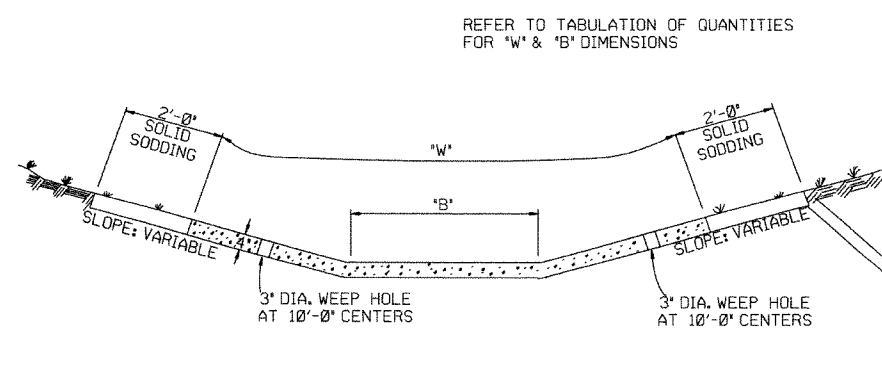
NON-PRESTRESSED PILE REINFORCING

Pile Size	No. Req'd.	Bar Size
16" Oct.	8	# 7
18" Oct.	8	# 7
④ 14" Sq.	8	# 7
16" Sq.	8	# 7
18" Sq.	8	# 8

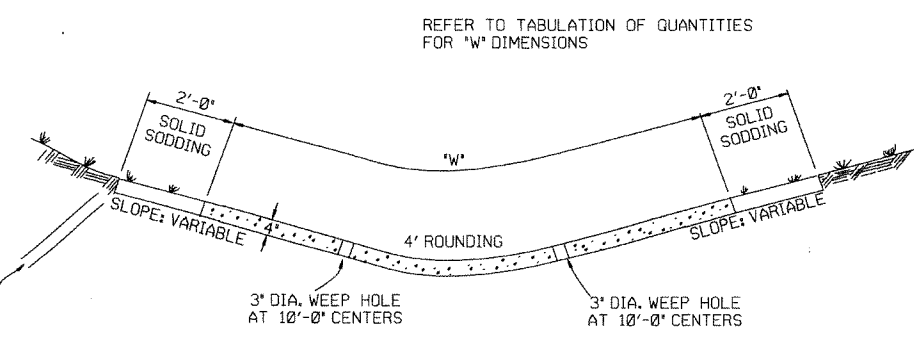
④ 14" sq. piles to be used in Seismic Performance Zone 1 only.



③ The five additional turns of spiral reinforcing may be omitted for build-up without additional driving.

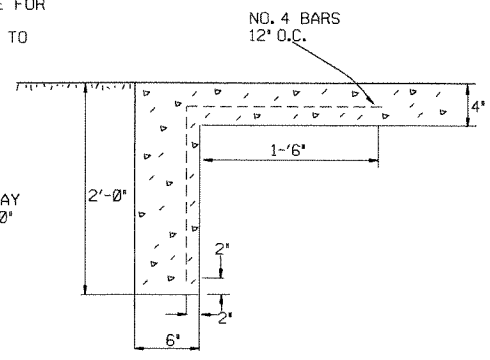


TYPE A

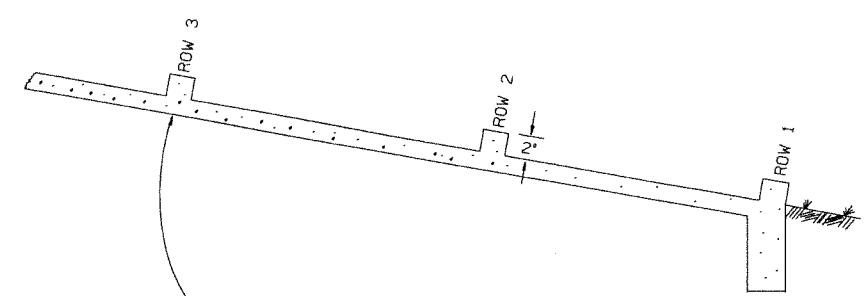


TYPE B

THE STEEL AND ADDITIONAL CONCRETE FOR THE WALLS SHALL NOT BE PAID FOR DIRECTLY, BUT SHALL BE CONSIDERED TO BE INCLUDED IN THE PRICE BID FOR 'CONCRETE DITCH PAVING.'

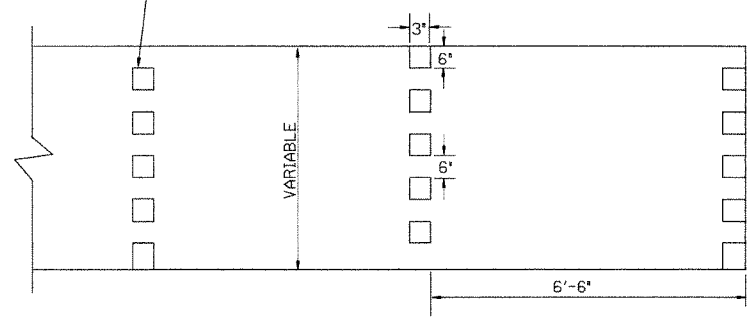


TOE WALL DETAIL FOR CONCRETE DITCH PAVING



NUMBER OF ELEMENTS PER ROW VARIES WITH WIDTH OF PAVING SPECIFIED

ENERGY DISSIPATORS TO BE USED FOR THE ENTIRE LENGTH OF DITCH WHEN SLOPE OF DITCH PAVING EXCEEDS 7%. THE DISSIPATORS WILL NOT BE PAID FOR DIRECTLY, BUT SHALL BE CONSIDERED TO BE INCLUDED IN THE PRICE BID FOR CONCRETE DITCH PAVING.

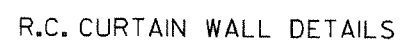
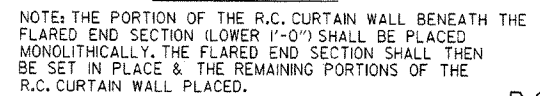
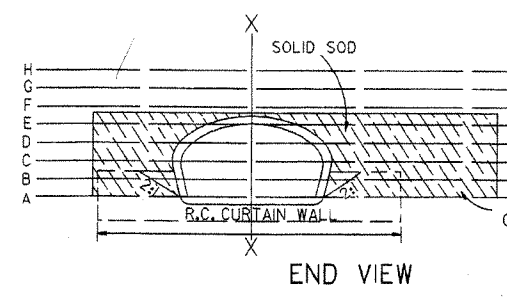
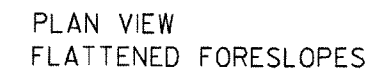
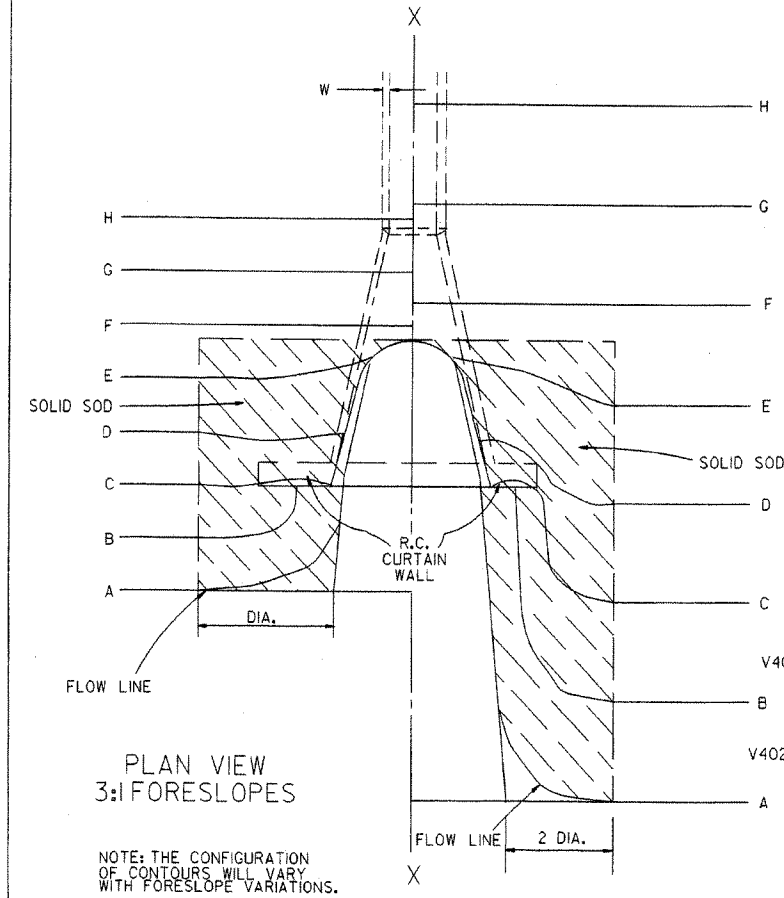
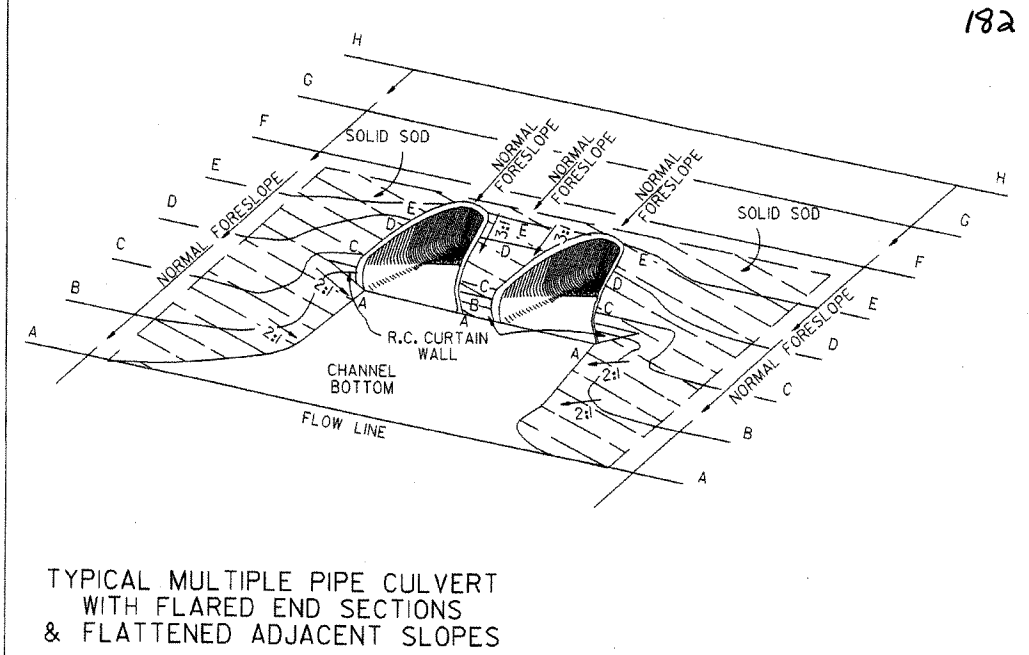
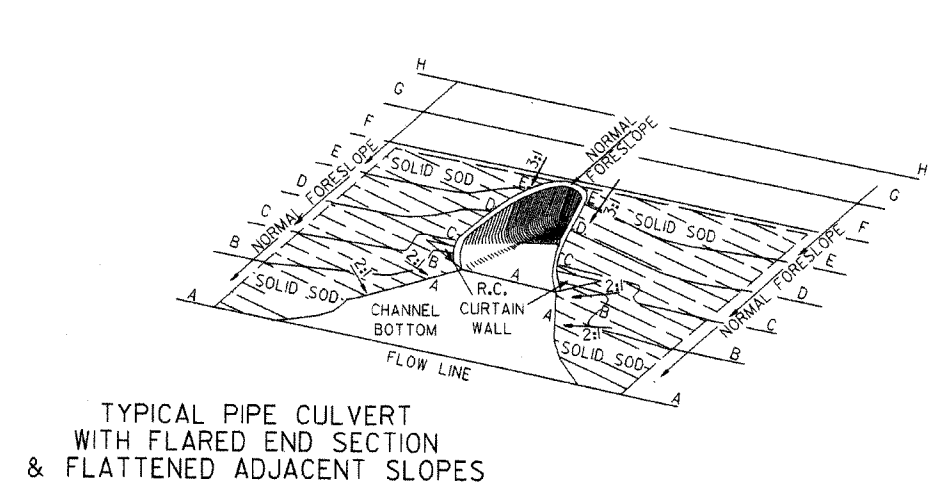
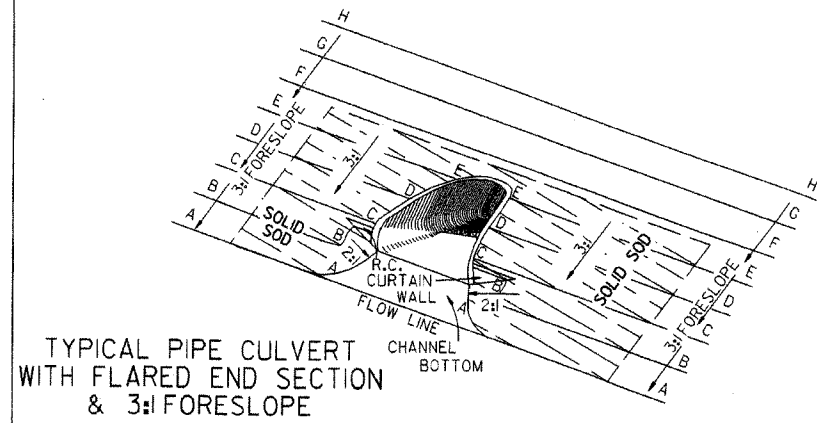


ENERGY DISSIPATORS
(NO SCALE)

GENERAL NOTES:

- THE FULL WIDTH OF EACH SECTION SHALL BE POURED MONOLITHICALLY.
- TOE WALLS TO BE CONSTRUCTED FULL WIDTH AT EACH END OF DITCH PAVING, AND POURED MONOLITHICALLY.
- SOLID SOD ALONG DITCH PAVING TO BE PLACED WITHIN 14 DAYS OF DITCH PAVING CONSTRUCTION.
- 1' WIDE TRANSVERSE EXPANSION JOINTS SHALL BE PLACED IN CONCRETE DITCH PAVING AT 45' INTERVALS. THE SPACE SHALL BE FILLED WITH APPROVED JOINT FILLER COMPLYING WITH AASHTO M213.

ARKANSAS STATE HIGHWAY COMMISSION		
CONCRETE DITCH PAVING		
STANDARD DRAWING CDP-1		
11-17-10	ADDED GENERAL NOTE	
6-2-94	ADDED GENERAL NOTE ABOUT SOLID SODDING	
11-30-8	ELIMINATED MIN. ROWS OF ELEMENTS	111-30-89
7-15-88	REVISED DISSIPATOR NOTE	653-7-15-88
4-3-87	REVISED ENERGY DISSIPATOR	671-4-3-87
1-9-87	MODIFIED NOTE ON ENERGY DISS.	532-1-9-87
11-3-86	ADDED NOTE TO ENERGY DISS.	599-12-1-86
11-1-84	ENERGY DISSIPATOR DETAILS	508-11-1-84
11-1-84	ADDED	
11-1-84	EXCAVATION DETAILS ADDED	
10-2-72	TYPED A & B	
10-2-72	REVISED AND REDRAWN	508-10-2-72
DATE	REVISION	DATE FILM'D



R.C. CURTAIN WALL

DIMENSIONS & QUANTITIES

PIPE DIA.	H ₁	L ₁	L	L (DBL.) 2	SINGLE R.C.P.C.		DOUBLE R.C.P.C.	
					CONC.	REINF. STEEL	CONC.	REINF. STEEL
					CU. YDS.	LBS.	CU. YDS.	LBS.
18"	11½"	3'-5"	8'-0"	6'-3"	0.31	27.7	0.45	39.5
24"	1'-0½"	4'-6"	9'-6"	7'-6"	0.37	33.4	0.53	48.0
30"	1'-3½"	5'-7"	11'-0"	9'-0"	0.45	39.0	0.67	59.0
36"	1'-7"	6'-8"	13'-0"	10'-6"	0.58	52.6	0.83	73.9
42"	2'-1½"	7'-3"	15'-6"	12'-0"	0.82	77.1	1.10	100.7
48"	2'-5"	7'-10"	17'-0"	13'-0"	0.98	94.9	1.27	120.4
54"	2'-9½"	8'-5"	18'-6"	14'-0"	1.16	115.8	1.47	143.7
60"	3'-4"	9'-0"	20'-6"	15'-6"	1.47	149.7	1.84	180.3
72"	4'-5"	10'-2"	25'-6"	18'-6"	2.31	232.6	2.73	271.0

NOTE: QUANTITIES SHOWN ARE FOR ONE "U" CURTAIN WALL.

PIPE DIA.	SINGLE R.C. PIPE CULVERT								DOUBLE R.C. PIPE CULVERT									
	H401		H402		V401		V402		H401		H402		H403		V401		V402	
	L	NO.	L	NO.	L	NO.	L	NO.	L	NO.	L	NO.	L	NO.	L	NO.	L	NO.
18"	7'-8"	2	1'-11 1/2"	4	1'-7 1/2"	8	8"	8	12'-2"	2	1'-11 1/2"	4	8"	2	1'-7 1/2"	10	8"	14
24"	9'-2"	2	2'-2"	4	1'-8 1/2"	10	8"	9	14'-8"	2	2'-2"	4	8"	2	1'-8 1/2"	12	8"	18
30"	10'-8"	2	2'-4 1/2"	4	1'-11 1/2"	10	8"	12	17'-8"	2	2'-4 1/2"	4	8"	2	1'-11 1/2"	14	8"	22
36"	12'-8"	2	2'-10"	6	2'-3"	12	8"	14	20'-8"	2	2'-10"	6	8"	3	2'-3"	14	8"	28
42"	15'-2"	2	3'-9 1/2"	8	2'-9 1/2"	16	8"	15	23'-8"	2	3'-9 1/2"	8	8"	4	2'-9 1/2"	18	8"	30
48"	16'-8"	2	4'-3"	10	3'-1"	18	8"	16	25'-8"	2	4'-3"	10	8"	5	3'-1"	20	8"	32
54"	18'-2"	2	4'-8 1/2"	12	3'-5 1/2"	20	8"	17	27'-8"	2	4'-9"	12	8"	6	3'-5 1/2"	22	8"	34
60"	20'-2"	2	5'-5"	14	4'-0"	24	8"	18	30'-8"	2	5'-5"	14	8"	7	4'-0"	26	8"	36
72"	25'-2"	2	7'-4"	18	5'-1"	30	8"	20	36'-8"	2	7'-4"	18	8"	9	5'-1"	33	8"	40

SOLID SODDING

PIPE DIA.	SINGLE R.C.P.C.			DOUBLE R.C.P.C.		
	3ft	4ft	6ft	3ft	4ft	6ft
	SO. YDS.			SO. YDS.		
18"	5	7	12	6	8	13
24"	8	12	19	9	13	20
30"	13	18	29	14	19	30
36"	17	26	41	18	28	43
42"	23	35	55	25	37	57
48"	29	46	68	31	48	70
54"	35	57	85	37	59	87
60"	44	62	104	48	65	107
72"	64	92	156	67	95	159

GENERAL NOTES

1. A CAST-IN-PLACE OR PRECAST CURTAIN WALL MAY BE USED. PAYMENT FOR THE CURTAIN WALL SHALL BE CONSIDERED TO BE INCLUDED IN THE UNIT PRICE BID EACH FOR FLARED END SECTIONS OF THE SEVERAL SIZES, WHICH PRICE SHALL BE FULL COMPENSATION FOR FURNISHING ALL MATERIALS INCLUDING REINFORCING STEEL AND CONCRETE FOR FORMS, MIXING AND PLACING; FOR EXCAVATION AND BACKFILL, AND FOR ALL LABOR, TOOLS, EQUIPMENT AND INCIDENTALS NECESSARY TO COMPLETE THE WORK.
2. ALL EXPOSED EDGES SHALL BE CHAMFERED $\frac{3}{4}$ ".
3. CONCRETE FOR CURTAIN WALL SHALL MEET THE REQUIREMENTS FOR CLASS A OR S CONCRETE AS PROVIDED IN SECTION 802 OF THE STANDARD SPECIFICATIONS OR FOR PAVING CONCRETE AS PROVIDED IN SECTION 501 OF THE STANDARD SPECIFICATIONS.
4. WELDED WIRE MESH 3×3 W/10 $\times$ W/10 MAY BE USED IN LIEU OF REINFORCING BARS.

10-18-96	ADDED NOTE TO SOLID SODDING	10-18-96	ARKANSAS STATE HIGHWAY COMMISSION
10-12-96	CORRECTED SPELLING		FLARED END SECTION
7-94	ADDED GENERAL NOTE NO. 4		
8-15-91	REV. CURTAIN WALL QUANT. STEEL SCH. & SOLID SOD QUANT.		
3-2-81	ALLOW PRECAST IN 2 OR MORE PIECES CHAMFER EDGES		
5-15-80	ADDED PRECAST WALL & GENERAL NOTES		
10-2-72	REVISED AND REDRAWN		
DATE	REVISION	FILED	STANDARD DRAWING FES-1

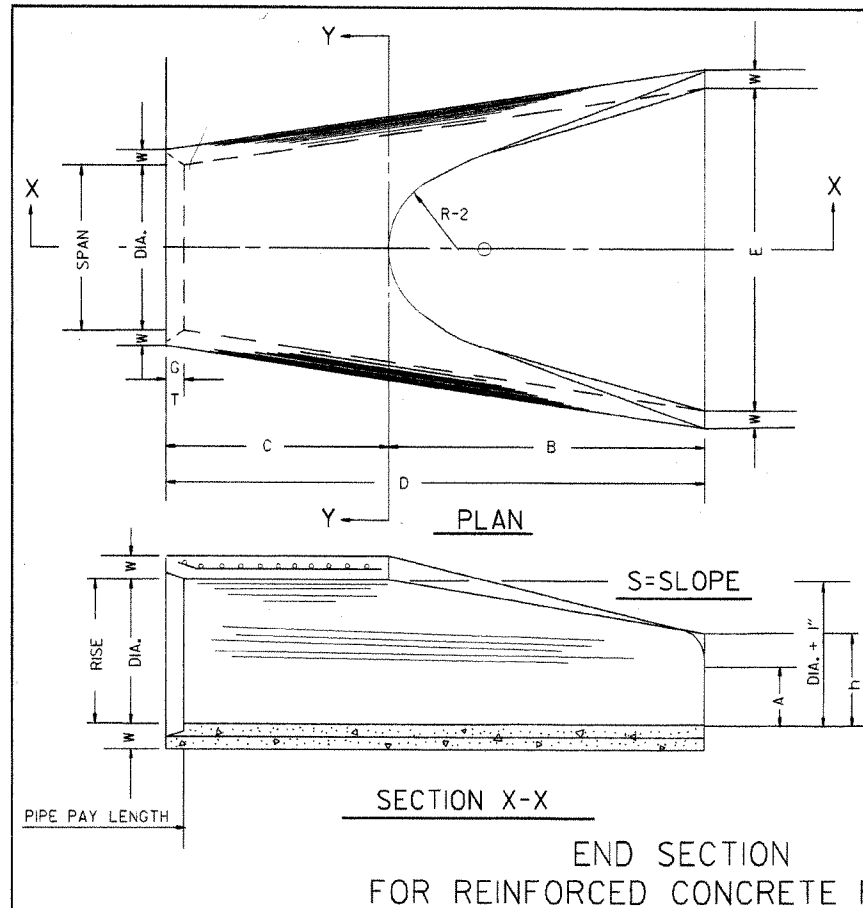
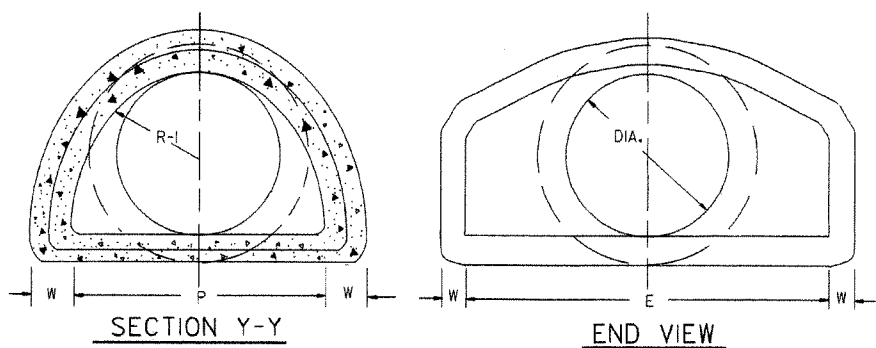


TABLE OF DIMENSIONS															
DIA.	WALL	A	B	C	D	E	S	DIA. + 1"	P	R-1	R-2	G-T	WT.	h	
18"	2 1/2"	9"	2'-3"	3'-10"	6'-1"	3'-0"	3:1	19"	29"	15 1/2"	12"	2"	1000	1'-0 1/2"	
24"	3"	9 1/2"	3'-7 1/2"	2'-6"	6'-1 1/2"	4'-0"	3:1	25"	33 3/8"	16 1/8"	14"	2 1/2"	1600	1'-1 1/2"	
30"	3 1/2"	1'-0"	4'-6"	1'-7 1/4"	6'-1 3/4"	5'-0"	3:1	31"	37"	18 1/2"	15"	3 1/4"	1940	1'-4 5/8"	
36"	4"	1'-3"	5'-3"	2'-10 3/4"	8'-1 3/4"	6'-0"	3:1	37"	47 1/8"	24 1/8"	20"	3 1/2"	4100	1'-8"	
42"	4 1/2"	1'-9"	5'-3"	2'-11"	8'-2"	6'-6"	3:1	43"	53 1/4"	27 1/2"	22"	3 1/2"	5380	2'-2 1/2"	
48"	5"	2'-0"	6'-0"	2'-2"	8'-2"	7'-0"	3:1	49"	56 1/2"	28 1/2"	22"	3 1/2"	6550	2'-6"	
54"	5 1/2"	2'-4"	6'-6"	1'-10"	8'-4"	7'-6"	3:1	55"	65 1/2"	33 1/8"	24"	4"	8750	2'-10 1/2"	
60"	6"	2'-10"	6'-6"	1'-10"	8'-4"	8'-0"	3:1	61"	72 1/2"	36 1/8"	24"	4"	9270	3'-5"	
72"	7"	3'-10"	6'-6"	1'-10"	8'-4"	9'-0"	3:1	73"	77 1/8"	38 1/8"	24"	5"	13250	4'-6"	

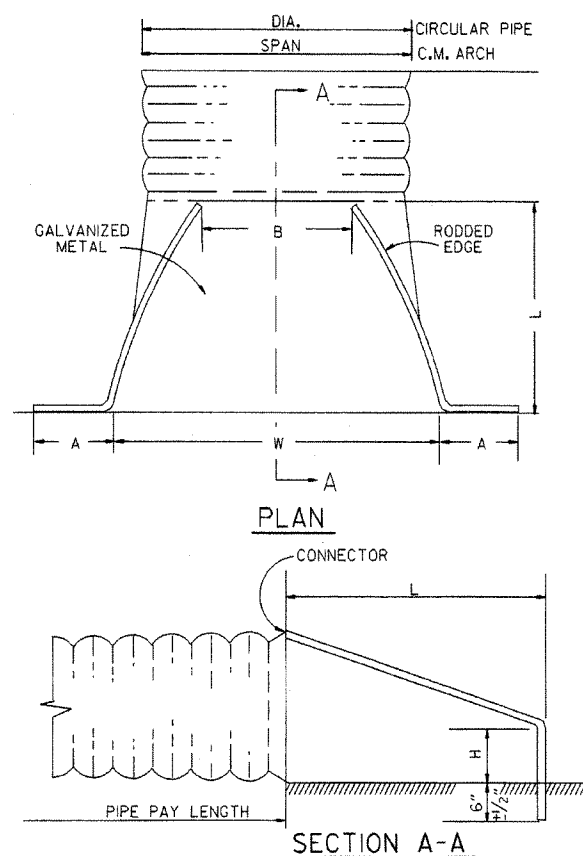
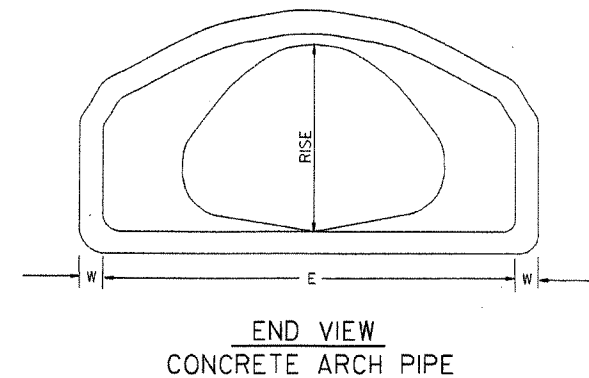


NOTE: TONGUE END ON UPSTREAM SECTION
GROOVE END ON DOWNSTREAM SECTION

ARCH PIPE

EQUIV. DIA.	• SPAN		• RISE		W	A	B	C	D	E	P	R2	G-T	S
	AASHTO M 206	AHD NOMINAL	AASHTO M 206	AHD NOMINAL										
	INCHES													
15	18	18	11	11	2"	4"	2'-0"	4'-0"	6'-0"	3'-0"	29"	12"	1 1/2"	2 1/2"
18	22	22	13 1/2	14	2 1/2"	5"	2'-0"	4'-1"	6'-1"	3'-6"	32 1/8"	13"	2 1/2"	2 1/2"
21	26	26	15 1/2	16	2 3/4"	7"	2'-3"	3'-10"	6'-1"	4'-0"	34 1/8"	14"	2 1/2"	2 1/2"
24	28 1/2	29	18	18	3"	9"	2'-3"	3'-10"	6'-1"	5'-0"	36 1/8"	15"	2 1/2"	2 1/2"
30	36 1/4	36	22 1/2	23	3 1/2"	10"	3'-1"	3'-0 1/2"	6'-1 1/2"	6'-0"	47 1/8"	20"	3"	2 1/2"
36	43 3/4	44	26 5/8	27	4"	10 1/2"	4'-0"	2'-1 1/2"	6'-1 1/2"	6'-6"	54 1/8"	22"	3 1/2"	2 1/2"
42	51 5/8	51	31 1/8	31	4 1/2"	11 1/2"	4'-7"	1'-10 1/4"	6'-5 1/4"	7'-2"	59 1/2"	23"	3 3/4"	2 1/2"
48	58 1/2	59	36	36	5"	1'-3"	5'-3"	2'-10 3/4"	8'-1 3/4"	7'-10"	70 3/8"	24"	4 1/4"	2 1/2"
54	65	65	40	40	5 1/2"	1'-7"	5'-3"	2'-11"	8'-2"	8'-6"	72 1/8"	24"	4 3/4"	2 1/4"
60	73	73	45	45	6"	1'-10"	5'-6"	2'-8"	8'-2"	9'-0"	77 3/8"	24"	5"	2 1/4"

* THE MEASURED SPAN AND RISE SHALL NOT VARY MORE THAN ± 2 PER CENT FROM THE VALUES SPECIFIED BY AASHTO M 206.

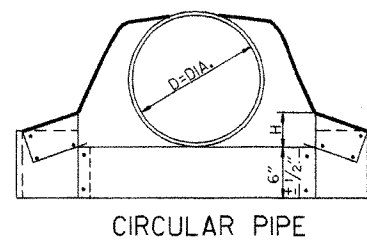


NOTE: ALTERNATE CONNECTIONS TO THE PIPE CULVERTS, IN ACCORDANCE WITH MANUFACTURER'S STANDARD PRACTICES, MAY BE MADE SUBJECT TO THE APPROVAL OF THE ENGINEER.

END SECTIONS FOR CORRUGATED METAL PIPE CULVERTS

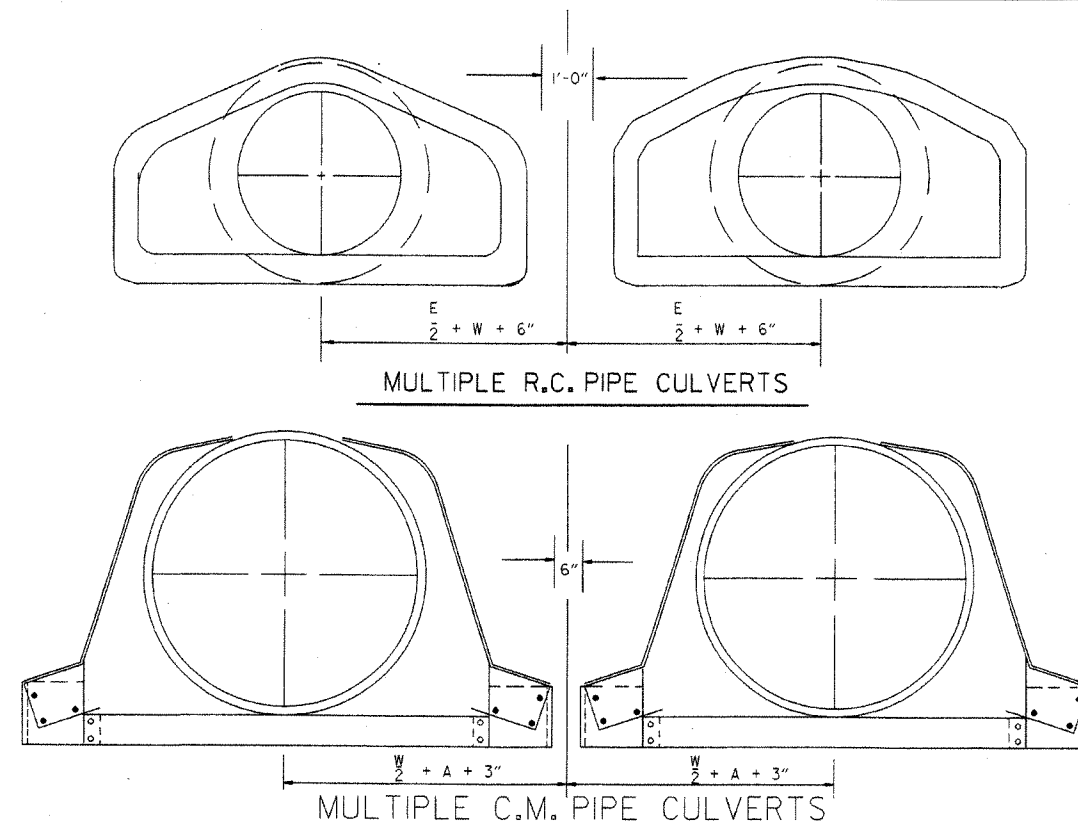
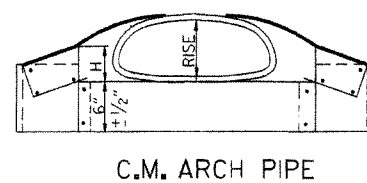
CIRCULAR PIPE

D. DIA.	GAUGE	A 1" $\pm$	B. MAX. 1" $\pm$	H 1" $\pm$	L 1 1/2" $\pm$	W 2" $\pm$	S
12	16	6	6	6	21	24	2 1/2:1
15	16	7	8	6	26	30	2 1/2:1
18	16	8	10	6	31	36	2 1/2:1
21	16	9	12	6	36	42	2 1/2:1
24	16	10	13	6	41	48	2 1/2:1
30	14	12	16	8	51	60	2 1/2:1
36	14	14	19	9	60	72	2 1/2:1
42	12	16	22	11	69	84	2 1/2:1
48	12	18	27	12	78	90	2 1/2:1
54	12	18	30	12	84	102	2:1
60	12	18	33	12	87	114	1 1/2:1
66	2	18	36	12	87	120	1 1/2:1
72	12	18	39	12	87	126	1 1/3:1



C.M. ARCH PIPE

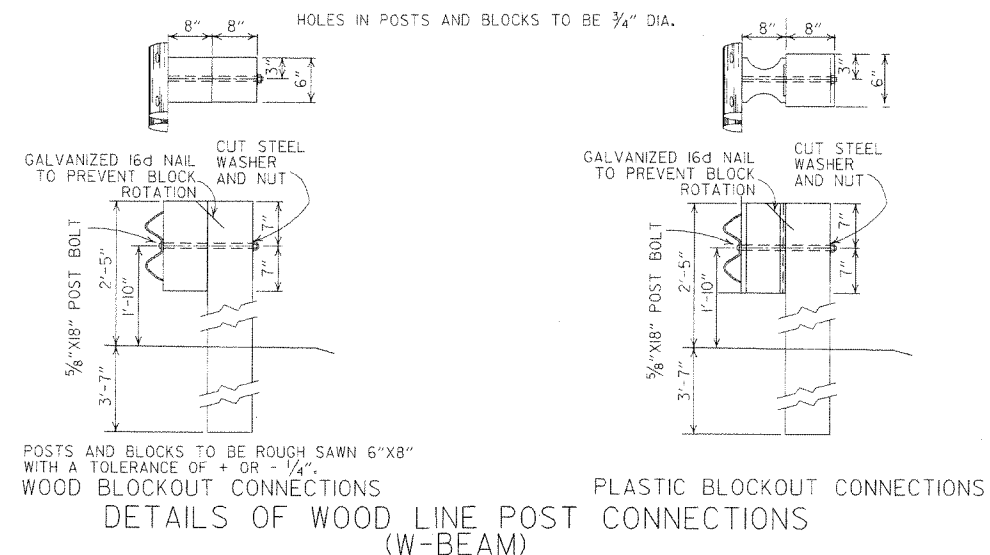
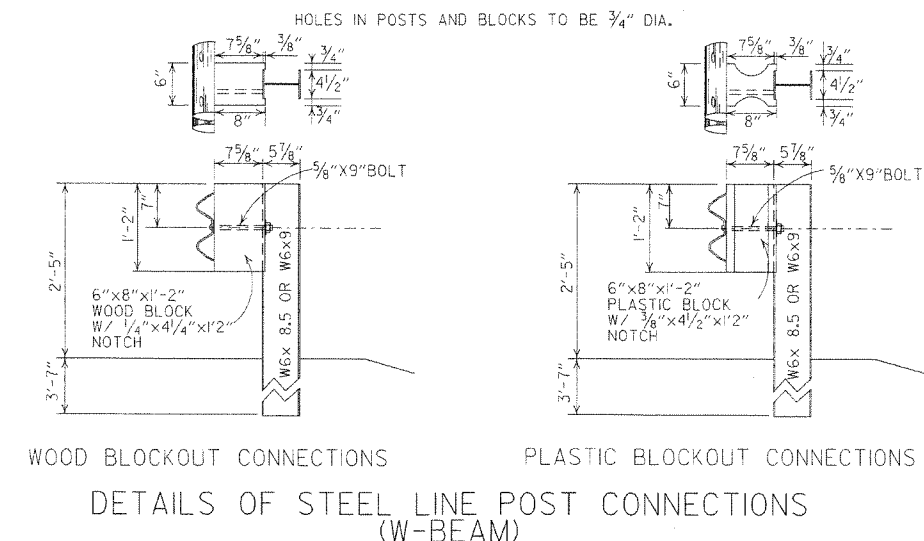
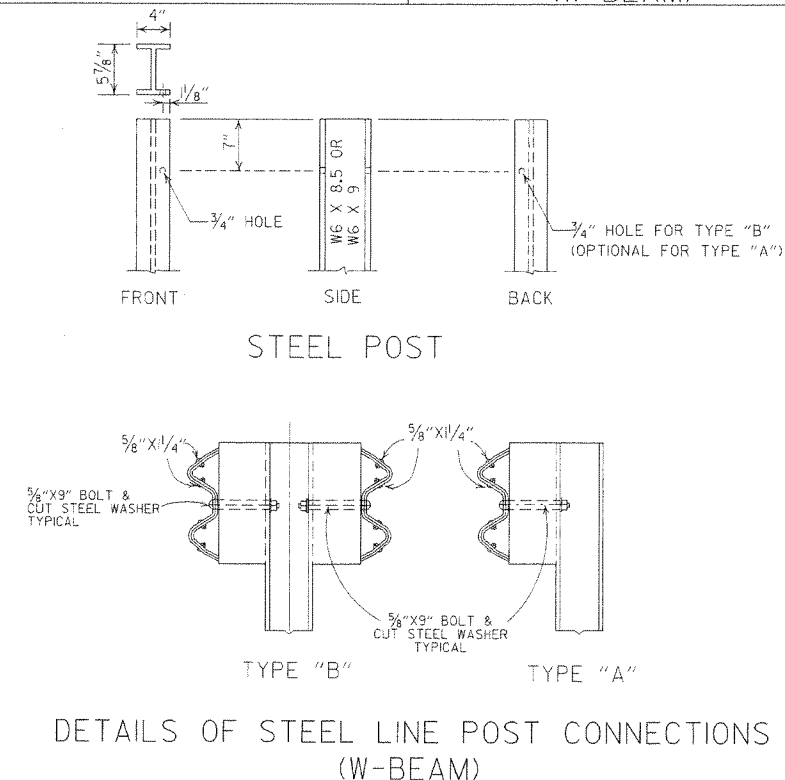
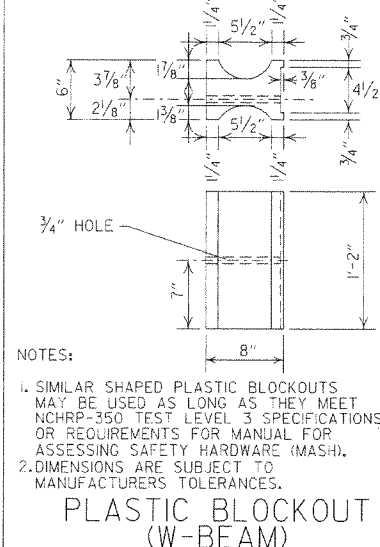
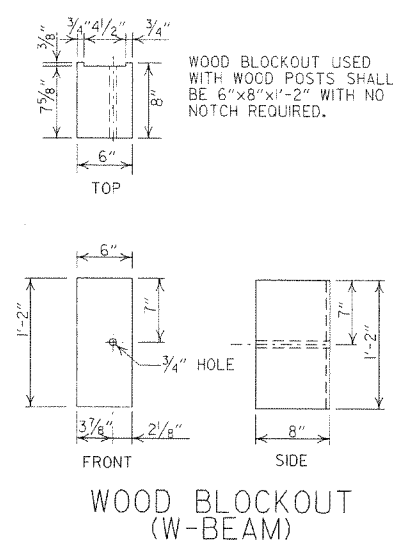
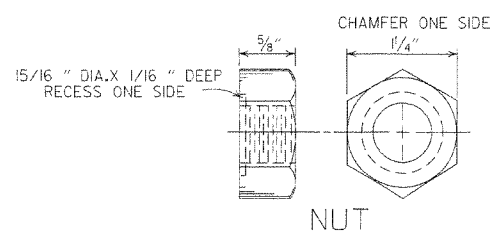
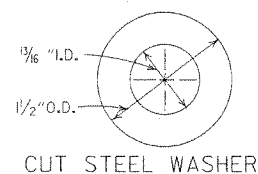
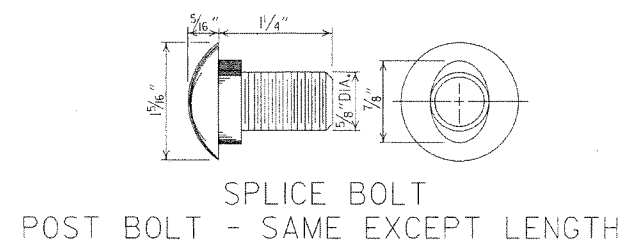
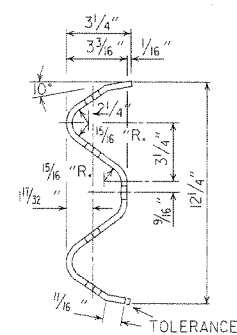
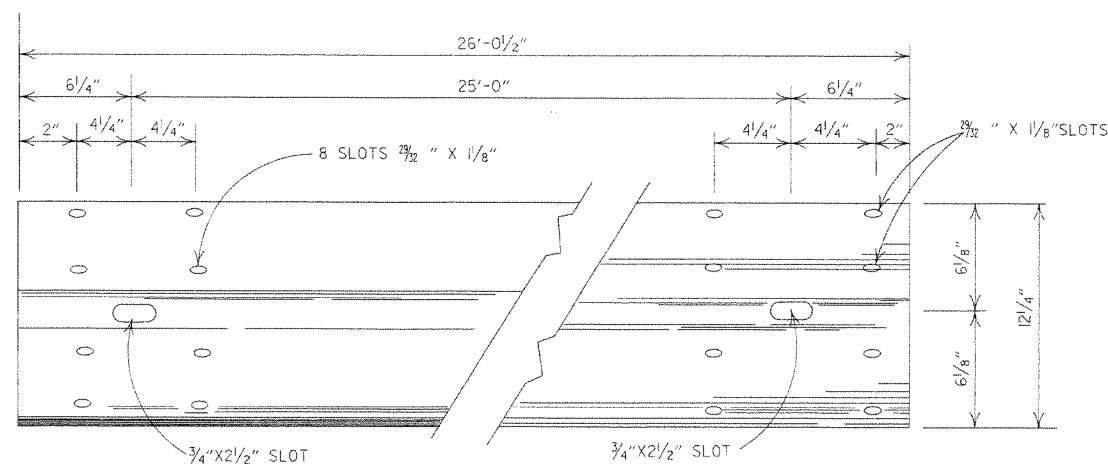
EQUIV. DIA.	SPAN	RISE	A 1" $\pm$	B. MAX. 1" $\pm$	H 1" $\pm$	L 1 1/2" $\pm$	W 2" $\pm$	S	GAUGE
15"	17	13	7	9	6	19	30	2 1/2:1	16
18"	21	15	7	10	6	23	36	2 1/2:1	16
21"	24	18	8	12	6	28	42	2 1/2:1	16
24"	28	20	9	14	6	32	48	2 1/2:1	16
30"	35	24	10	16	6	39	60	2 1/2:1	14
36"	42	29	12	18	8	46	75	2 1/2:1	14
42"	49	33	13	21	9	53	85	2 1/2:1	12
48"	57	38	18	25	12	63	90	2 1/2:1	12
54"	64	43	18	30	12	70	102	2 1/2:1	12
60"	71	47	18	33	12	77	114	2 1/2:1	12



10-18-96	REVISED ASTM REF. TO AASHTO	10-18-96	ARKANSAS STATE HIGHWAY COMMISSION
5-15-80	REVISED DISTANCE BETWEEN MULTIPLE R.C.P. F.E.S.	664-5-15-80	
7-14-78	C.M. ARCH SIZES TO CONFORM WITH AASHTO SIZES	752-7-14-78	
8-22-75	ADDED MULTIPLE PIPE CULVERTS	517-8-22-75	
12-5-74	REMOVED NOTE RE REINF. FOR R.C. F.E.S.	500-12-5-74	
5-24-73	CMP END SECTION, SHOW PIPE PAY LENGTH	627-5-24-73	
10-2-72	REVISED AND REDRAWN	760-10-2-72	
DATE	REVISION	FILMED	

FLARED END SECTION

STANDARD DRAWING FES-2



-GENERAL NOTES-

ALL BOLTS SHALL BE SUFFICIENT LENGTH TO EXTEND THROUGH THE FULL THICKNESS OF THE NUT AND NO MORE THAN $\frac{3}{4}$ " BEYOND IT.

WHERE W-BEAM GUARD RAIL CONTINUES, THE INTERMEDIATE SECTIONS SHALL HAVE A POST SPACING OF 6'-3" UNLESS OTHERWISE NOTED.

W-BEAM GUARD RAIL REPRESENTING INTERMEDIATE SECTIONS
WILL BE MEASURED ALONG THE ROADWAY FACE FROM CENTERLINE OF
POST TO CENTERLINE OF POST.

USE W-BEAM GUARD RAIL COMPONENTS OF SAME MATERIAL FOR ENTIRE JOB.
FOR EXTENSIONS OR MODIFICATION OF EXISTING GUARD RAIL, W-BEAM GUARD RAIL
COMPONENTS OF THE SAME TYPE AS THOSE EXISTING SHALL BE USED.

ANY BACKFILLING UNDER OR AROUND POST SHALL BE DAMP SAND THOROUGHLY TAMPED IN PLACE.

WOOD POSTS & WOOD BLOCKS SHALL BE EITHER DENSE NO.1 STRUCTURAL OR BETTER 9.7f (1400 f) OR NO.1 1350 f SOUTHERN PINE.

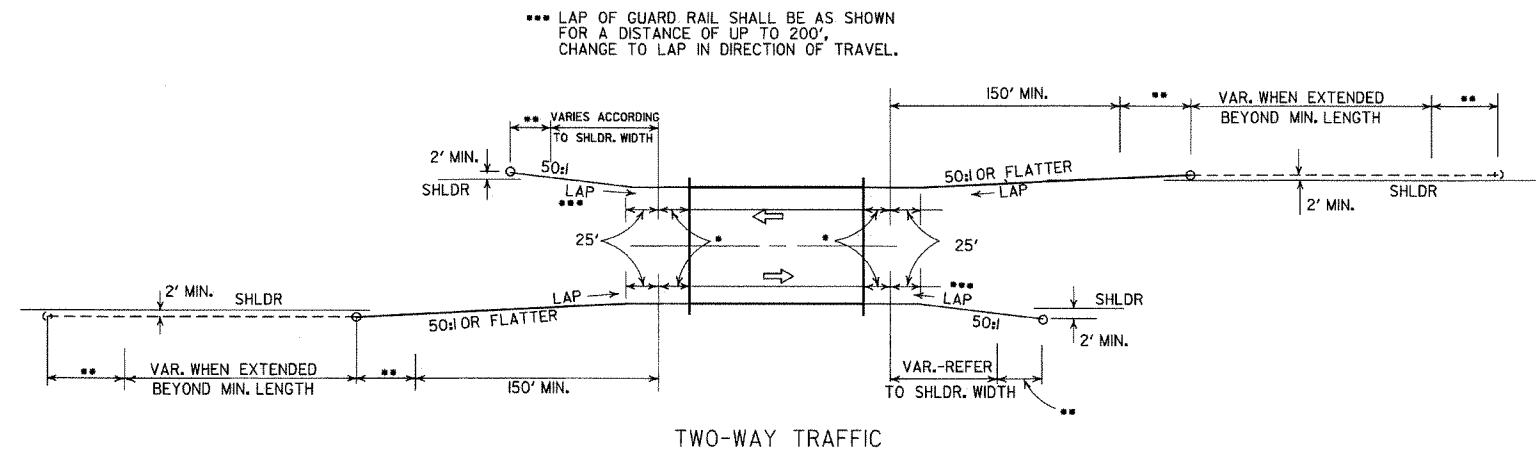
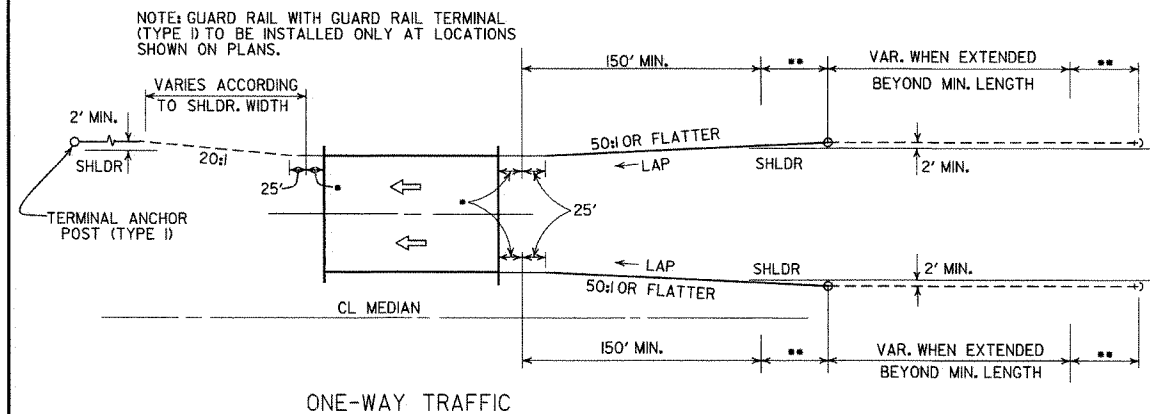
CONTRACTOR SHALL HAVE THE OPTION OF USING WOOD BLOCKOUTS FOR W-BEAM GUARD RAIL OR PLASTIC BLOCKOUTS, AS LONG AS BLOCKOUT USED MEETS NCHRP-350 TEST LEVEL 3 SPECIFICATIONS OR REQUIREMENTS FOR MANUAL FOR ASSESSING SAFETY HARDWARE (MASH) FOR W-BEAM GUARD RAIL.

7-4-10	RAISED HEIGHT OF GUARD RAIL 1"	
10-15-09	ADDED REFERENCE TO MASH	
4-10-03	REVISED GENERAL NOTES	
8-22-02	REVISED DIMENSION ON WOOD & PLASTIC BLOCKOUT CONNECTIONS & ON STEEL POST	
11-16-01	REVISED WOOD BLOCKOUT & DETAILS OF WOOD LINE POST CONNECTIONS	
3-30-00	REMOVED GUARD RAIL AT BRIDGE ENDS	
11-2-00	ADDED PLASTIC BLOCKOUT	
8-12-98	REV. BLOCKOUTS TO WOOD, DELETED CONC. POST & REV. GENERAL NOTES, DELETED DET. OF GUARD RAIL REPLACED BEHIND CURB & DET. OF POST PLATE, IN SOLID ROCK, & ADDED DETAILS OF STEEL LINE POST CONN. REMOVED BACK-UP PLATE, REVISED HOLES IN STEEL POLES	
4-3-97	REMOVED "LAP IN DIRECTION OF TRAFFIC" NOTE & PLACED ARROWS ON WASHERS	
10-18-96	REVISED WOOD POST NOTE	
6-2-94	ADDED ALT. STEEL POST SIZE	
8-5-93	REVISED STEEL POST SIZE	8-5-93
10-1-92	REDRAWN & REVISED	10-1-92
8-15-91	REVISED WASHER NOTE	8-15-91
8-2-90	REV. GEN. NOTE & DEPTH OF ANC. POST IN ROCK	8-2-90
7-15-88	REVISED SECTION 3 & GENERAL NOTES	
3-4-88	REV. ANCHOR POST, ELEV. NOTES, & POST IN ROCK	780-3-4-88
10-30-87	REVISED WOOD LINE POST DETAIL	546-10-30-87
10-9-87	REDRAWN & REVISED	802-10-9-87
DATE	REVISION	DATE FILED

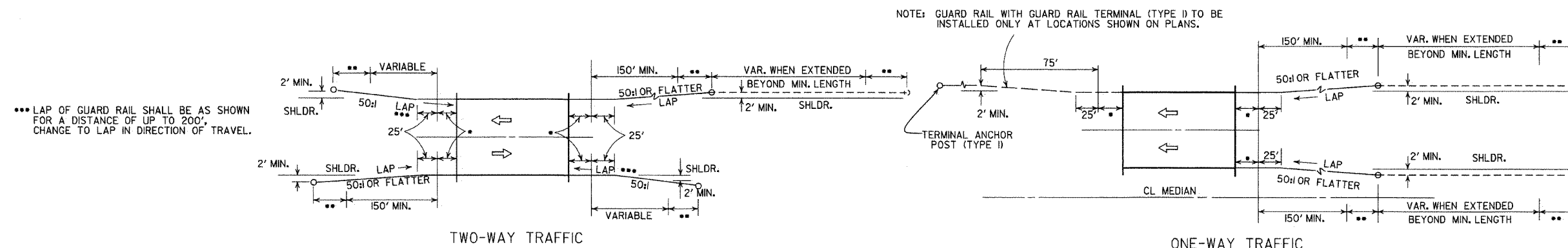
ARKANSAS STATE HIGHWAY COMMISSION

GUARD RAIL DETAILS

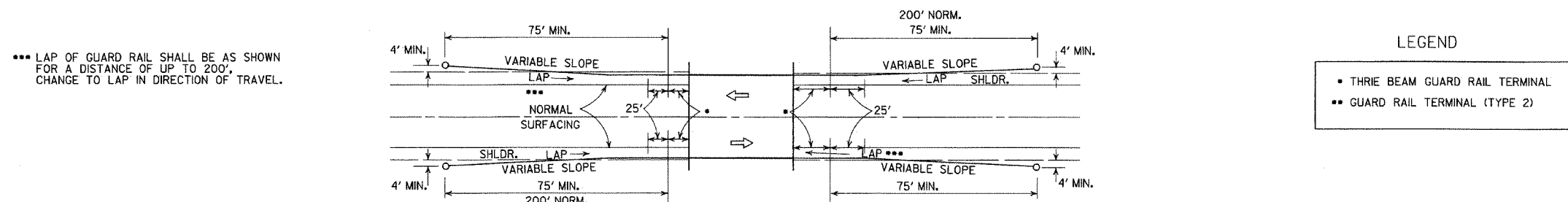
STANDARD DRAWING GR-8



METHODS OF INSTALLATION OF GUARD RAIL AT LESS THAN FULL SHOULDER WIDTH BRIDGES USING GUARD RAIL TERMINAL (TYPE 2)

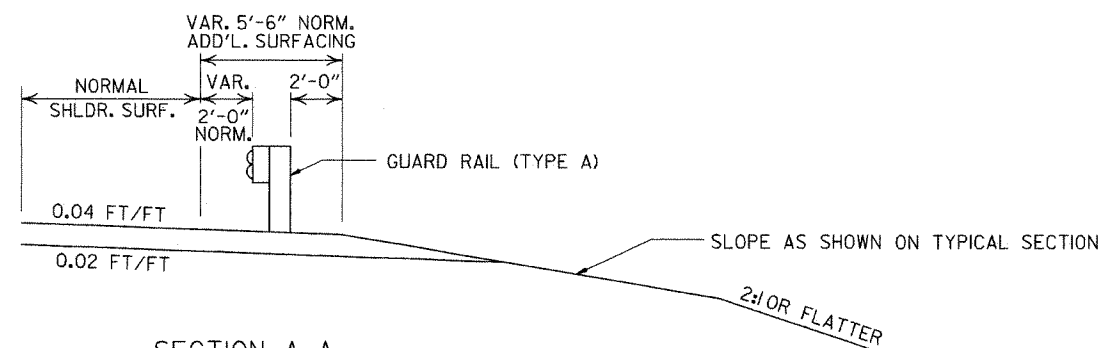
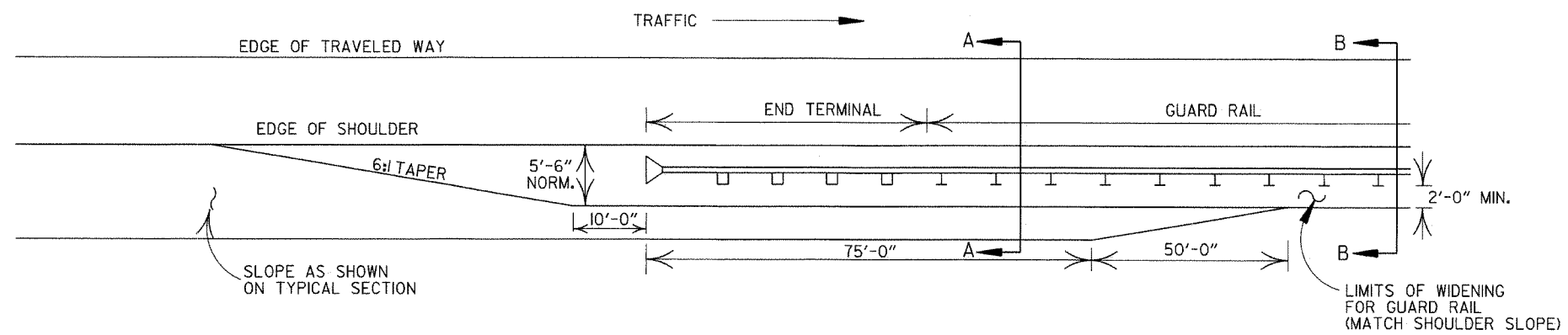


METHOD OF INSTALLATION OF GUARD RAIL AT FULL SHOULDER WIDTH BRIDGES USING GUARD RAIL TERMINAL (TYPE 2)

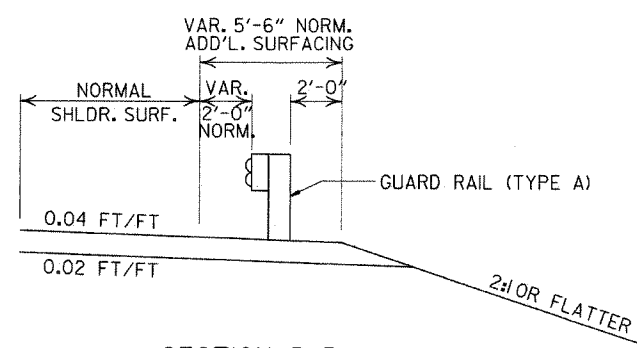


METHOD OF INSTALLATION OF GUARD RAIL USING GUARD RAIL TERMINAL (TYPE I) (FULL SHOULDER WIDTH OR LESS BRIDGES)

ARKANSAS STATE HIGHWAY COMMISSION		
GUARD RAIL DETAILS		
STANDARD DRAWING GR-9		
4-17-08	REVISED LAYOUTS	
11-10-05	REMOVED GUARD RAIL NOTES AND DETAILS	
11-16-01	DELETED NOTE-METHOD OF INSTALLATION OF GUARD RAIL USING GUARD RAIL TERM. (TY. I)	
1-12-00	ADDED CONSTRUCTION NOTE	1-12-00
6-26-97	REVISED LAYOUT	
10-1-92	REDRAWN & REVISED	10-1-92
10-9-87	ADDED NOTE	
10-9-87	REDRAWN & REVISED	
DATE	REVISION	DATE FILM



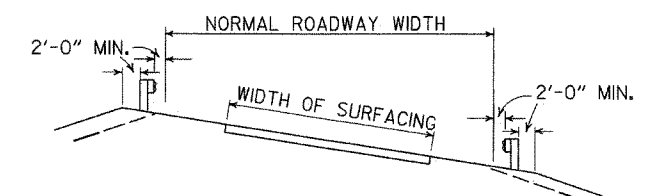
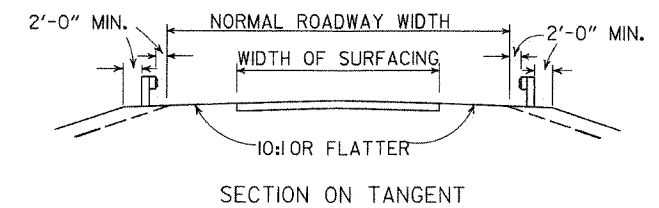
SECTION A-A



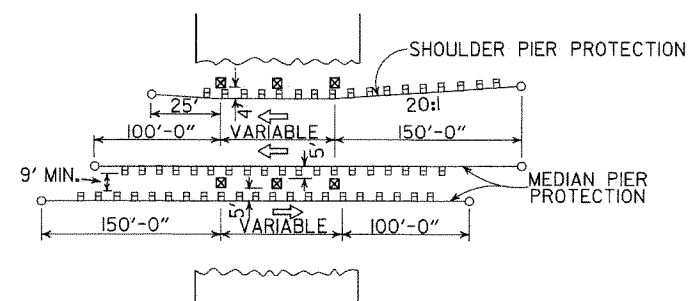
SECTION B-B

DETAILS OF WIDENING FOR GUARD RAIL

NOTE: NORMAL SECTION TO BE WIDENED APPROX. 5'-6" EACH SIDE TO SUPPORT GUARD RAIL.

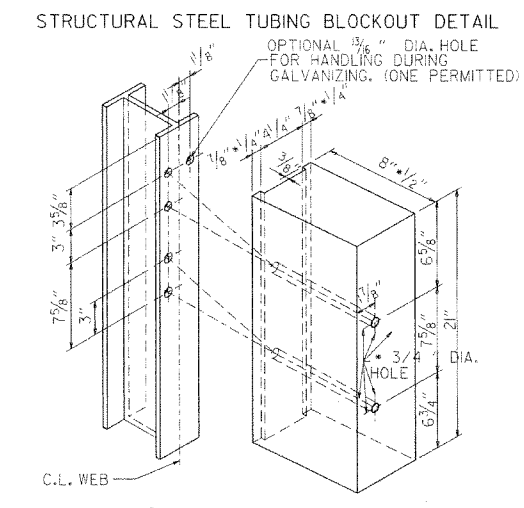
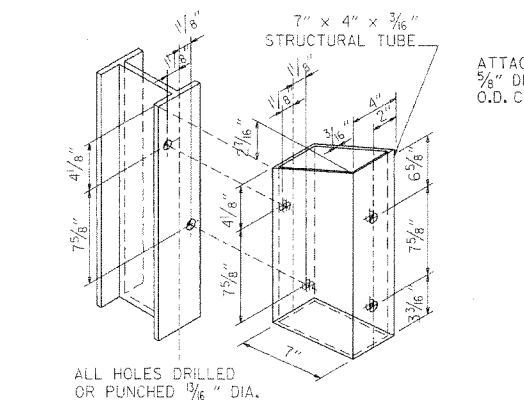
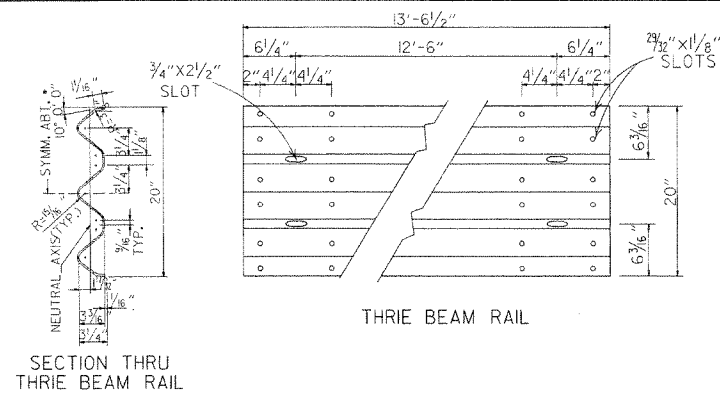


DETAILS SHOWING POSITION OF GUARD RAIL ON HIGHWAY

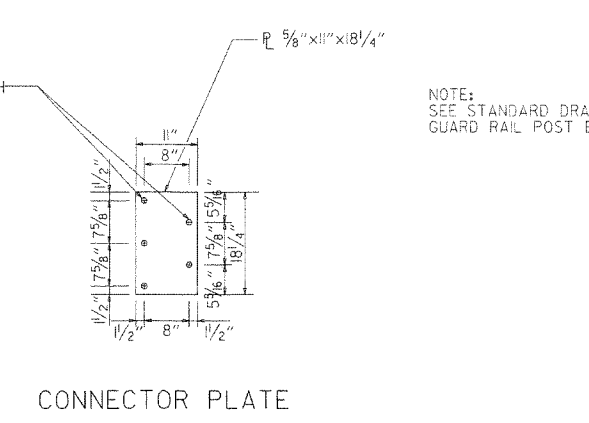
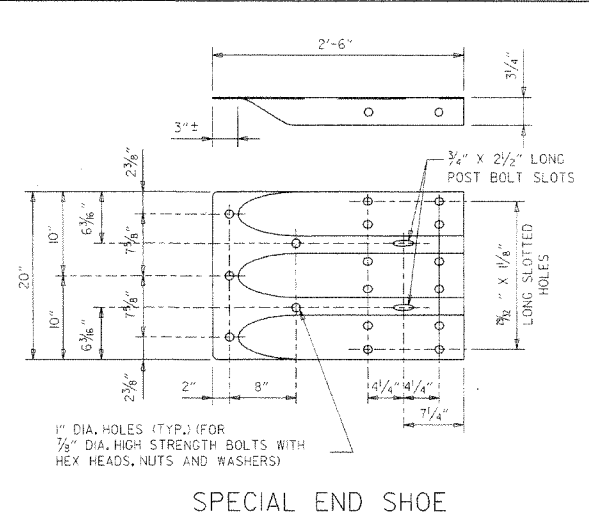


METHOD OF INSTALLATION OF GUARD RAIL AT FIXED OBSTACLE

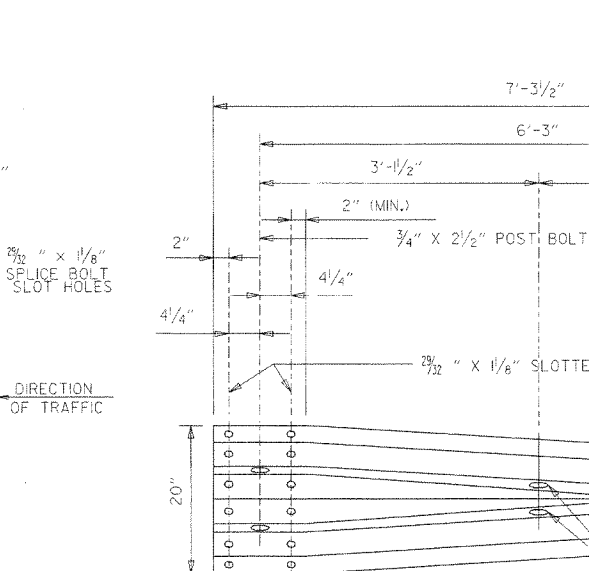
			ARKANSAS STATE HIGHWAY COMMISSION
			GUARD RAIL DETAILS
			STANDARD DRAWING GR-9A
4-17-08	MINOR REVISION		
11-10-05	DRAWN		
DATE	REVISION	DATE	FILM



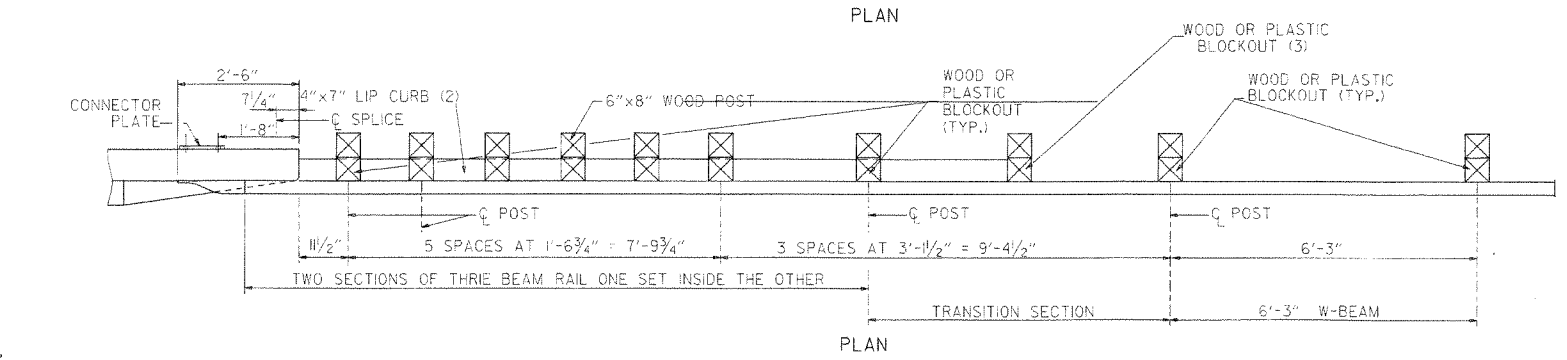
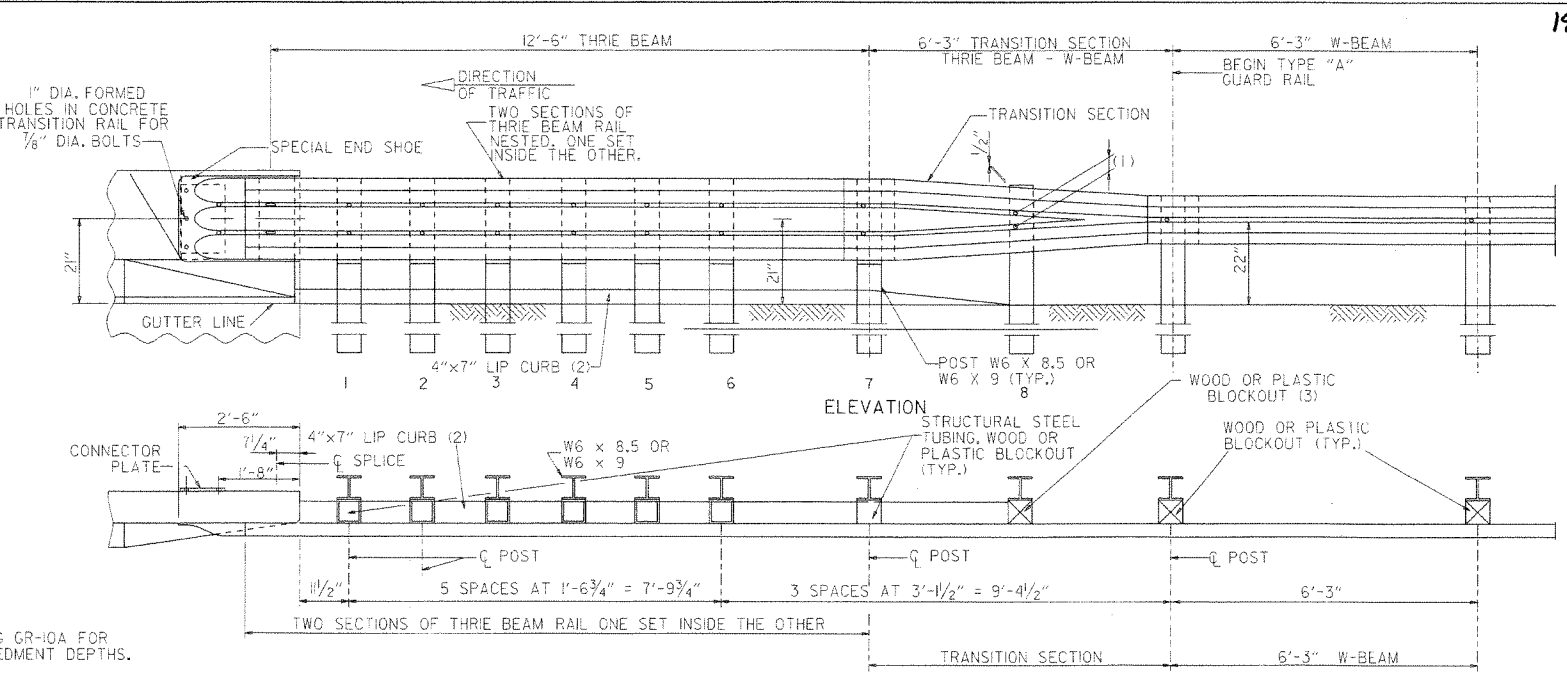
NOTE: BLOCKS SHALL BE THE SAME TYPE THROUGHOUT THE PROJECT LIMITS.



CONNECTOR PLATE SHALL BE AASHTO M270, GR. 36 AND SHALL BE GALVANIZED AFTER FABRICATION. GALVANIZING SHALL CONFORM TO SUBSECTION 807.19 OF THE STANDARD SPECIFICATIONS. CONNECTOR PLATE TO BE BOLTED TO SPECIAL END SHOE USING 7/8" DIA. HIGH STRENGTH BOLTS, WITH THE HEADS PLACED ON THE TRAFFIC FACE. WASHERS SHALL BE USED UNDER THE HEAD AND NUT. BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED AND SHALL CONFORM TO SUBSECTION 807.06.



THRIE BEAM RAIL SPLICE AT POST



(1) VERIFY BOLT SPACING FROM RAIL TRANSITION PRODUCER.
(2) REFER TO APPROACH GUTTER DETAILS.
(3) LENGTH OF BLOCKOUT ON POST 8 TO BE MODIFIED TO FIT RAIL WIDTH.

THRIE BEAM GUARD RAIL CONNECTION AT BRIDGE ENDS

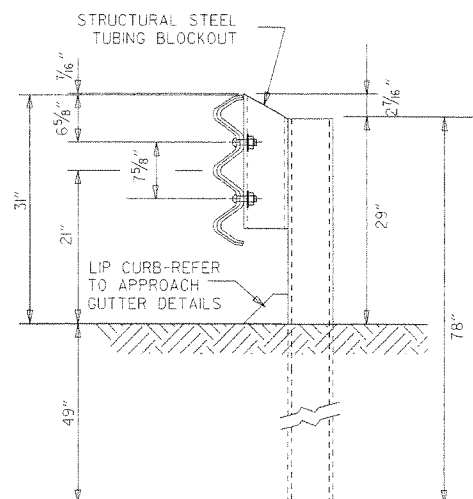
GENERAL NOTES:

- THE THRIE BEAM RAIL, SPECIAL END SHOE, AND THE TRANSITION SECTION SHALL BE MADE OF STEEL AND SHALL BE 12 GAGE. ZINC COATING SHALL BE TYPE I.
- RAIL POSTS SHALL BE SET PERPENDICULAR TO THE ROADWAY PROFILE GRADE AND VERTICALLY IN CROSS SECTION.
- ALL BOLTS SHALL BE SUFFICIENT LENGTH TO EXTEND THROUGH THE FULL THICKNESS OF THE NUT AND NO MORE THAN 3/4" BEYOND IT.
- ALL LAP SPLICES, INCLUDING SPECIAL END SHOES, SHALL BE MADE IN THE DIRECTION SHOWN ON STANDARD DRAWINGS GR-9 & GR-11.
- WOOD POSTS & WOOD BLOCKS SHALL BE EITHER DENSE NO. 1 STRUCTURAL OR BETTER 9.7f (1400 f) OR NO. 1 350 f SOUTHERN PINE.
- REFER TO STD. DRWG. GR-10A FOR POST DETAILS.
- USE THRIE BEAM GUARD RAIL COMPONENTS OF SAME MATERIAL FOR ENTIRE JOB.
- THRIE BEAM POSTS SHALL BE SAME MATERIAL AS W-BEAM POSTS FOR ENTIRE JOB.

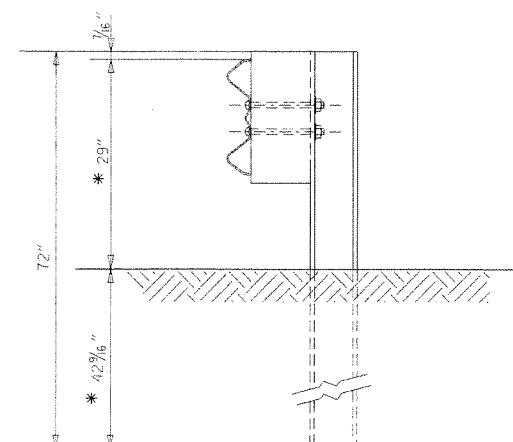
7-14-10	RAISED HEIGHT OF W-BEAM 1"		ARKANSAS STATE HIGHWAY COMMISSION
11-29-07	ADDED PLASTIC BLOCKOUTS		
11-10-05	ADDED NOTE FOR ATTACHING STEEL BLOCKOUT		
11-18-04	REVISED GENERAL NOTES		
10-9-03	REVISED GENERAL NOTES		
4-10-03	REVISED GENERAL NOTES		
8-22-02	REVISED NOTE (2)		
6-29-00	MOVED DIMENSION LINES		
5-18-00	ADDED NOTE		
3-30-00	DRAWN & ISSUED		
DATE	REVISION	DATE FILM	

GUARD RAIL DETAILS

STANDARD DRAWING GR-10

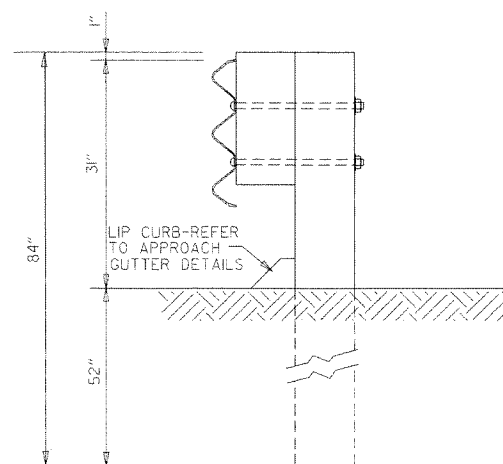


THRIE BEAM RAIL WITH STEEL TUBING BLOCKOUT
AND STEEL POST
POSTS 1-7

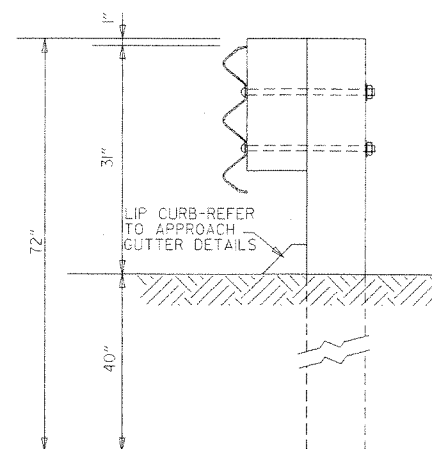


W-BEAM TO THRIE BEAM TRANSITION RAIL
WITH WOOD OR PLASTIC BLOCKOUT AND STEEL POST
POST 8

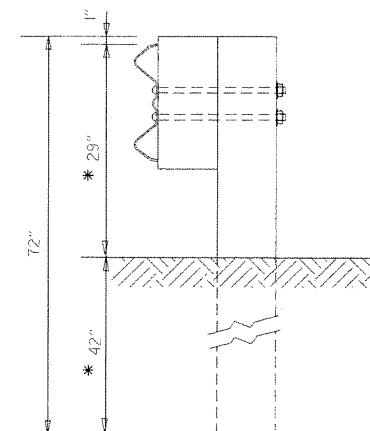
* NOTE:
THESE DIMENSIONS WILL NEED TO BE ADJUSTED
IN THE FIELD TO MAKE THE TRANSITION FROM
21" MID POINT OF THRIE BEAM TO 22" MID POINT
OF W-BEAM.



THRIE BEAM RAIL
WITH WOOD OR PLASTIC
BLOCKOUTS & WOOD POSTS
POSTS 1-6



THRIE BEAM RAIL
WITH WOOD OR PLASTIC
BLOCKOUT & WOOD POST
POST 7



W-BEAM TO THRIE BEAM
TRANSITION RAIL WITH WOOD OR
PLASTIC BLOCKOUT & WOOD POST
POST 8

GENERAL NOTES:

RAIL POSTS SHALL BE SET PERPENDICULAR TO THE ROADWAY PROFILE GRADE AND
VERTICALLY IN CROSS SECTION.

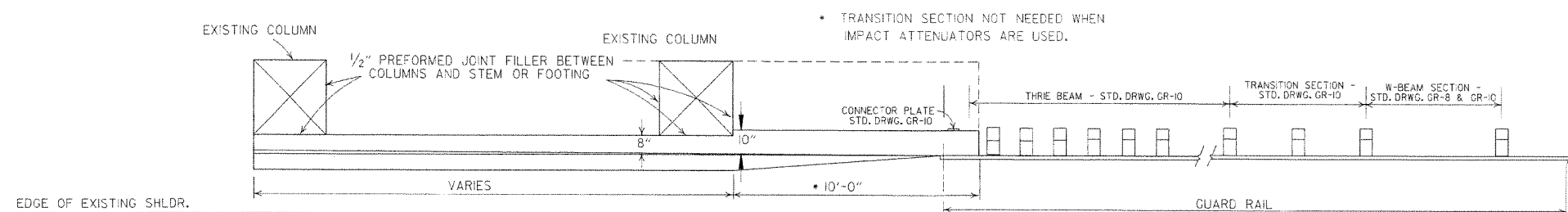
WOOD POSTS & WOOD BLOCKS SHALL BE EITHER DENSE NO. 1 STRUCTURAL OR
BETTER 9.7f (1400 f) OR NO. 1 (1350 f) SOUTHERN PINE.

ARKANSAS STATE HIGHWAY COMMISSION

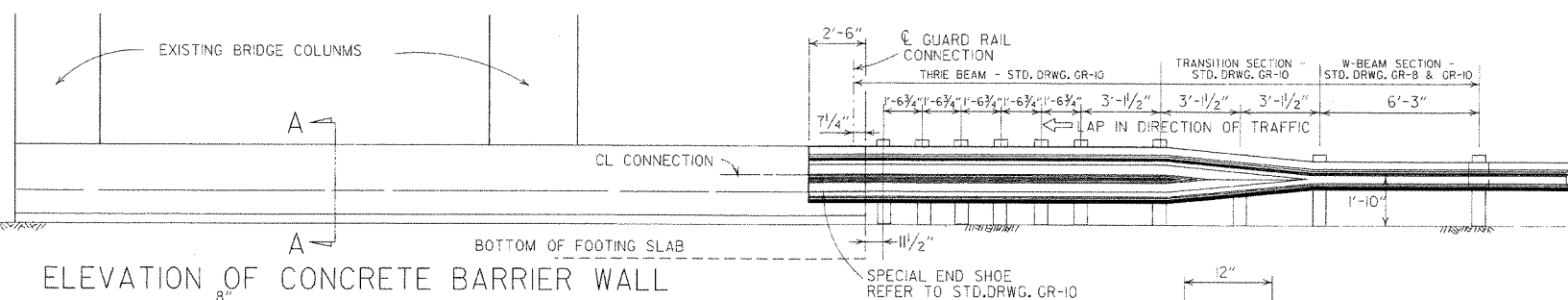
GUARD RAIL DETAILS

STANDARD DRAWING GR-10A

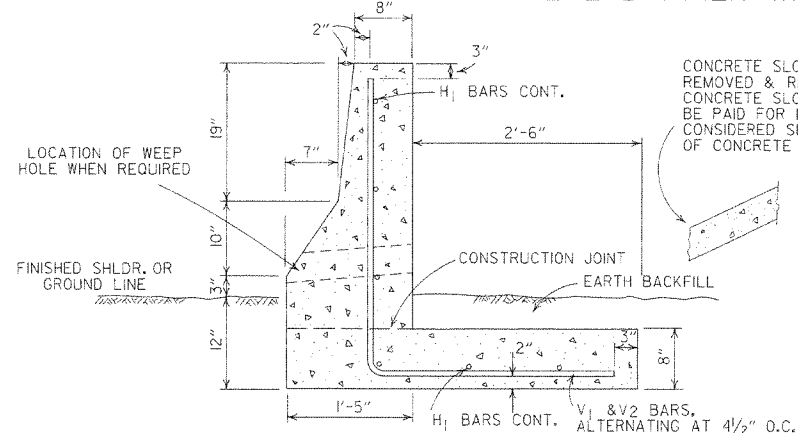
DATE	REVISION	DATE	FILM
7-14-10	REVISED POST 8 DIMENSIONS		
11-29-07	ADDED PLASTIC BLOCKOUTS		
8-22-02	REVISED LIP CURB NOTE		
3-30-00	DRAWN & ISSUED		



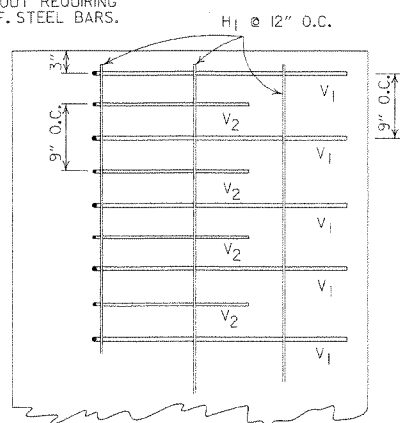
AT LEAST ONE 1/2" JOINT SHALL BE CONSTRUCTED
IN THE CONCRETE BARRIER WALL.
JOINTS SHALL BE EQUALLY SPACED AT A MAXIMUM
OF 25'-0" O.C. FILL JOINT WITH PREFORMED JOINT
FILLER.



NOTE:
ALL EXPOSED EDGES OF
THE CONCRETE BARRIER WALL
SHALL HAVE A $\frac{3}{4}$ " CHAMFER.

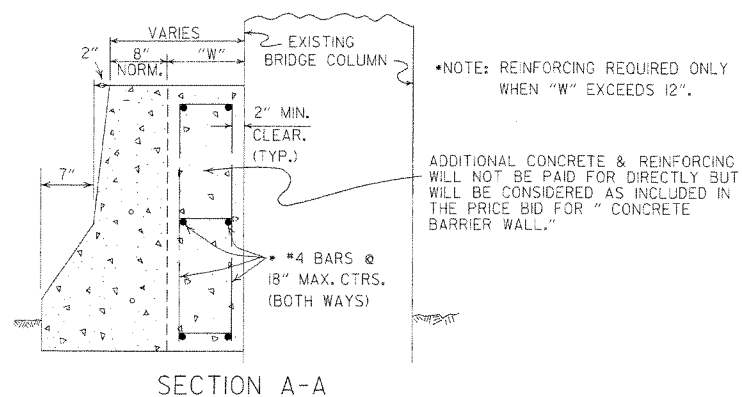
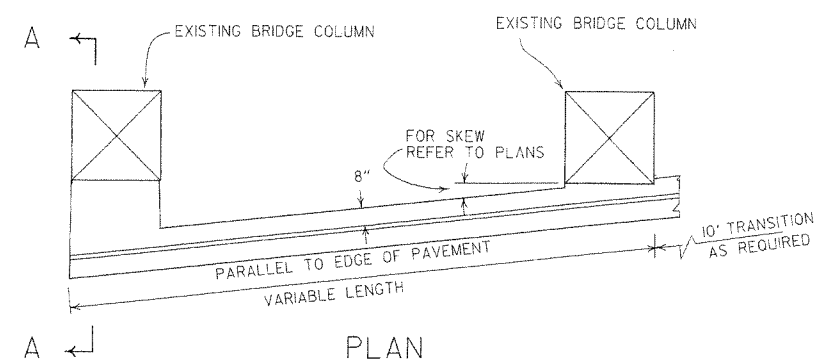
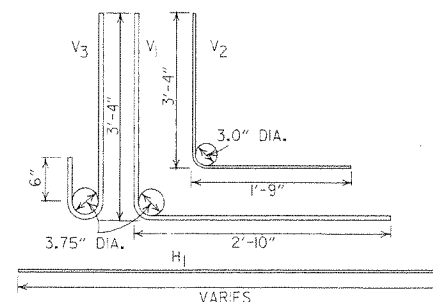
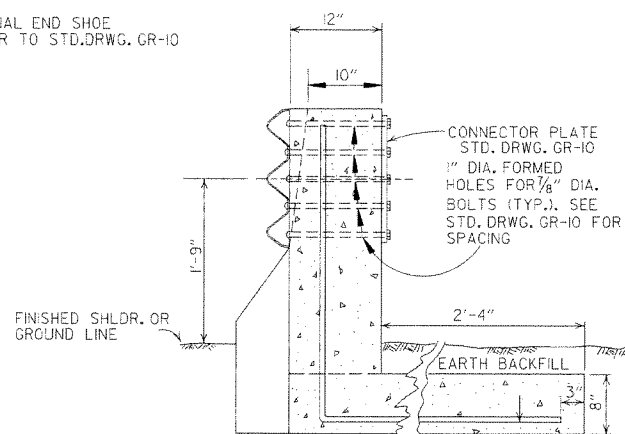


IF FOR ANY REASON IT IS
NECESSARY TO CONSTRUCT THE
FOOTING AT A LOWER ELEVATION
THAN IS SHOWN, THE STEM MAY
BE LENGTHENED 1'-0" BETWEEN
FIN. SHLDR. AND TOP OF
FOOTING WITHOUT REQUIRING
HEAVIER REINF. STEEL BARS.

BAR LIST

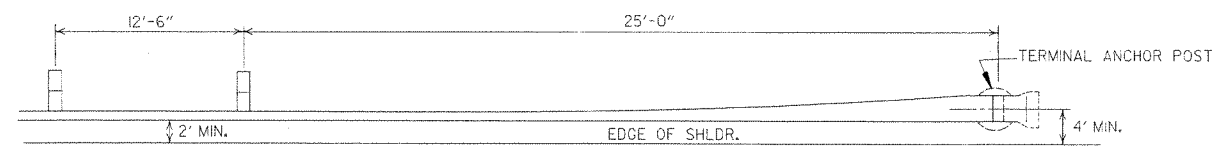
MARK	NO.	SIZE	LENGTH
V1		#5	6'-2 1/2"
V2		#4	5'-1"
V3		#5	4'-1 1/8"
H1	6	4	VAR.

THE V3 BARS SHALL BE USED IN PLACE
OF THE V1 & V2 BARS IN FRONT OF
PIERS.

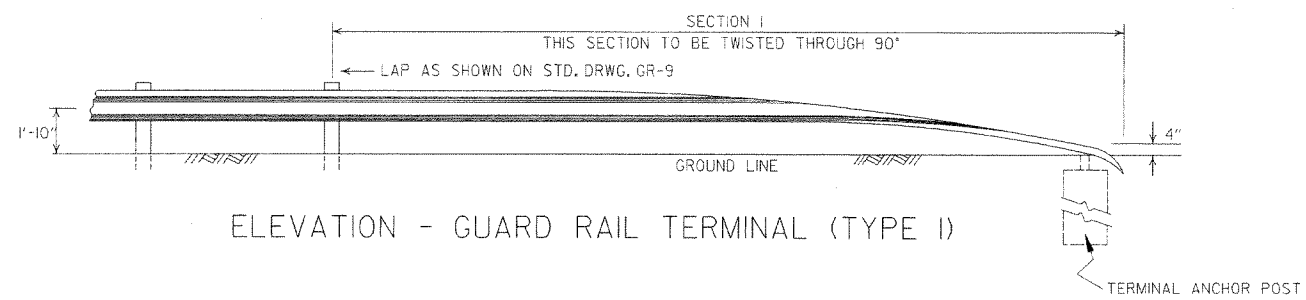


DETAILS OF CONCRETE BARRIER WALL
WHEN PIERS ARE SKEWED TO ROADWAY

7-14-10	RAISED HEIGHT OF W-BEAM 1"		ARKANSAS STATE HIGHWAY COMMISSION
8-22-02	REV. SECTION A-A OF DETAILS OF CONCRETE BARRIER WALL		
6-29-00	MOVED DIMENSION LINE		CONCRETE BARRIER WALL (PIER PROTECTION TYPE A)
5-18-00	ADDED NOTE		
3-30-00	REVISED TO INCLUDE THREE BEAM		STANDARD DRAWING GR-II
6-2-94	ADDED TRANSITION SECTION NOTE		
10-1-92	REDRAWN & REVISED	10-1-92	
8-15-91	REVISED DRAWING PLAN CONC. BARR.	8-15-91	
2-16-89	ADDED SKEWED DETAILS	594-2-16-89	
7-14-88	CHANGED TITLE		
10-9-87	REDRAWN & REVISED		
DATE	REVISION	DATE FILED	

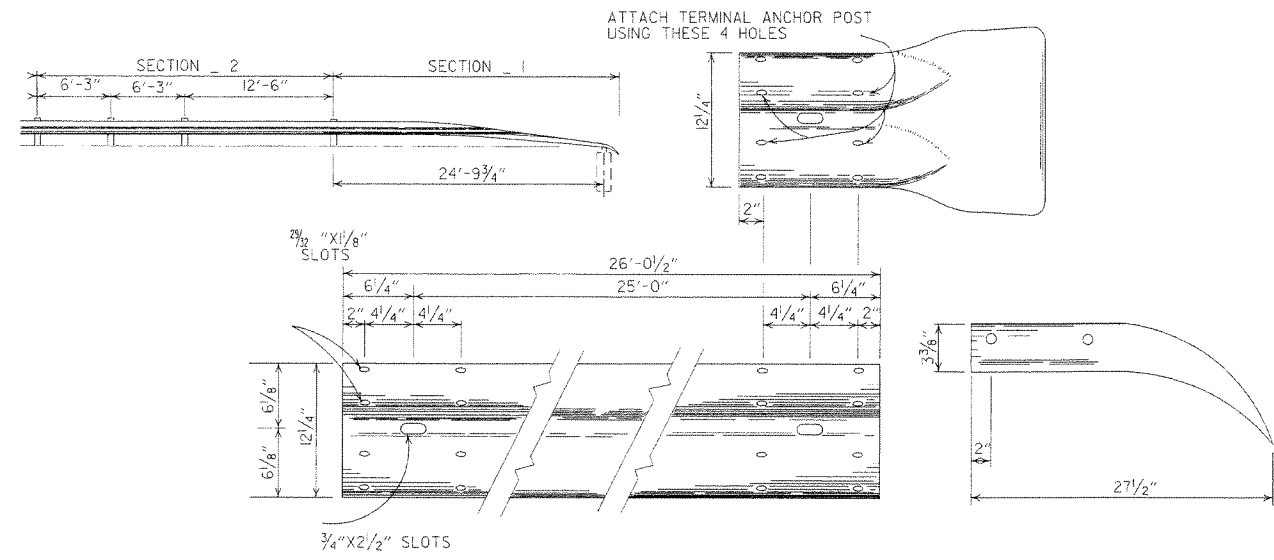


PLAN - GUARD RAIL TERMINAL (TYPE I)



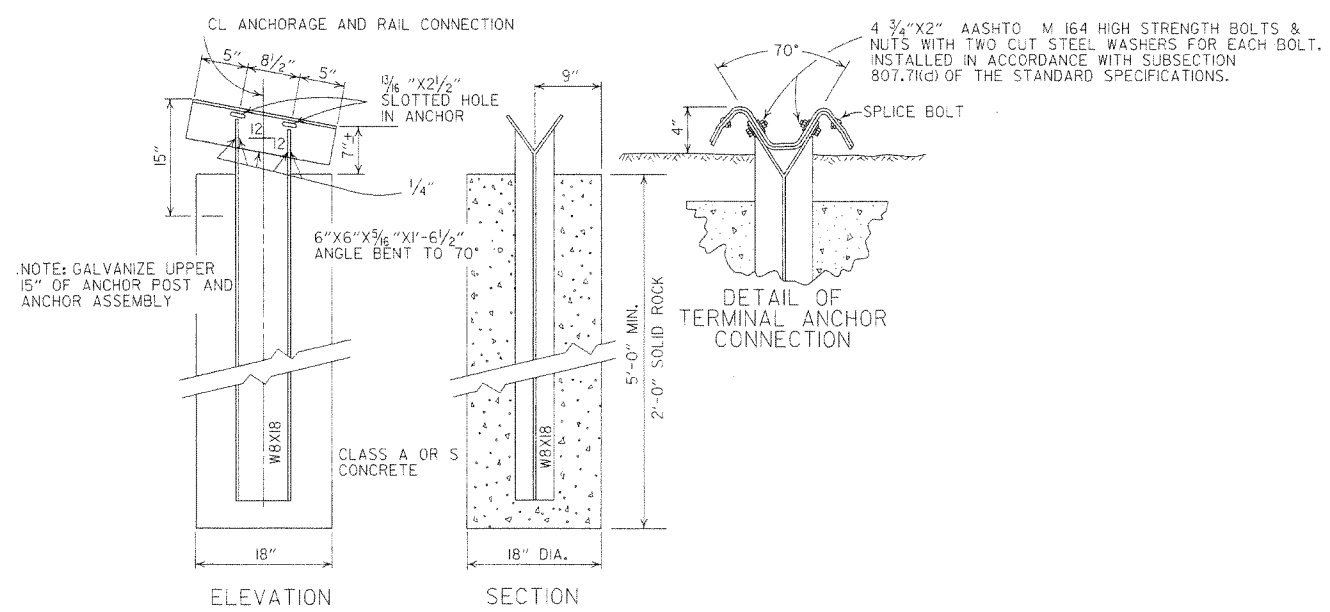
ELEVATION - GUARD RAIL TERMINAL (TYPE I)

NOTE:
SECTIONS 1 AND 2 OF GUARD RAIL TERMINAL
SHALL BE PAID FOR AT THE PRICE BID PER
LINEAR FOOT OF THE TYPE OF GUARD RAIL SPECIFIED.



SECTION 1

TERMINAL SECTION



ELEVATION

SECTION

NOTE: RAIL MEMBERS MAY BE BOLTED TO ANGLE AT TERMINAL ANCHOR AND THE TWO ASSEMBLIES POSITIONED TO PROPER ALIGNMENT PRIOR TO PLACING CONCRETE AROUND 8 WF 17 POST IF CONTRACTOR SO DESIRES.

DETAIL OF TERMINAL ANCHOR POST (TYPE I)

ARKANSAS STATE HIGHWAY COMMISSION		
GUARD RAIL DETAILS		
STANDARD DRAWING GRT-1		
7-14-10	RAISED HEIGHT OF GUARD RAIL 1"	
6-26-97	REVISED LAP NOTE	
10-18-96	REVISED ASTM REF. TO AASHTO	
11-3-94	DIMENSION TERMINAL DETAIL	
11-11-92	ADDED NOTE FOR PAYMENT	11-11-92
10-1-92	DRAWN & ISSUED	10-1-92
DATE	REVISION	DATE FILM

REINFORCED CONCRETE
ARCH PIPE DIMENSIONS

EQUIV. DIA.	SPAN		RISE	
	AASHTO M 206	AHTD NOMINAL	AASHTO M 206	AHTD NOMINAL
INCHES	INCHES			
15	18	18	11	11
18	22	22	13½	14
21	26	26	15½	16
24	28½	29	18	18
30	36¼	36	22½	23
36	43¾	44	26¾	27
42	51¼	51	31¾	31
48	58½	59	36	36
54	65	65	40	40
60	73	73	45	45
72	88	88	54	54
84	102	102	62	62
90	115	115	72	72
96	122	122	77½	77
108	138	138	87½	87
120	154	154	96¾	97
132	168¾	169	106½	107

THE MEASURED SPAN AND RISE SHALL NOT VARY MORE THAN ± 2 PERCENT FROM THE VALUES SPECIFIED BY AASHTO M206.

REINFORCED CONCRETE
HORIZONTAL ELLIPTICAL
PIPE DIMENSIONS

EQUIV. DIA.	AASHTO M 207	
	SPAN	RISE
INCHES	INCHES	
18	23	14
24	30	19
27	34	22
30	38	24
33	42	27
36	45	29
39	49	32
42	53	34
48	60	38
54	68	43
60	76	48
66	83	53
72	91	58
78	98	63
84	106	68

THE MEASURED SPAN AND RISE SHALL NOT VARY MORE THAN ± 2 PERCENT FROM THE VALUES SPECIFIED BY AASHTO M207.

CONSTRUCTION SEQUENCE

1. PLACE STRUCTURAL BEDDING MATERIAL TO GRADE. DO NOT COMPACT.
2. INSTALL PIPE TO GRADE.
3. COMPACT STRUCTURAL BEDDING OUTSIDE THE MIDDLE THIRD OF THE PIPE.
4. PLACE AND COMPACT THE HAUNCH AREA UP TO THE MIDDLE OF THE PIPE.
5. COMPLETE BACKFILL ACCORDING TO SUBSECTION 606.03.(f)(1).

NOTE: HAUNCH AND STRUCTURAL BEDDING MATERIAL WILL NOT BE PAID FOR SEPARATELY, BUT COMPENSATION WILL BE CONSIDERED TO BE INCLUDED IN THE PRICE BID PER LINEAR FOOT OF CONCRETE PIPE.

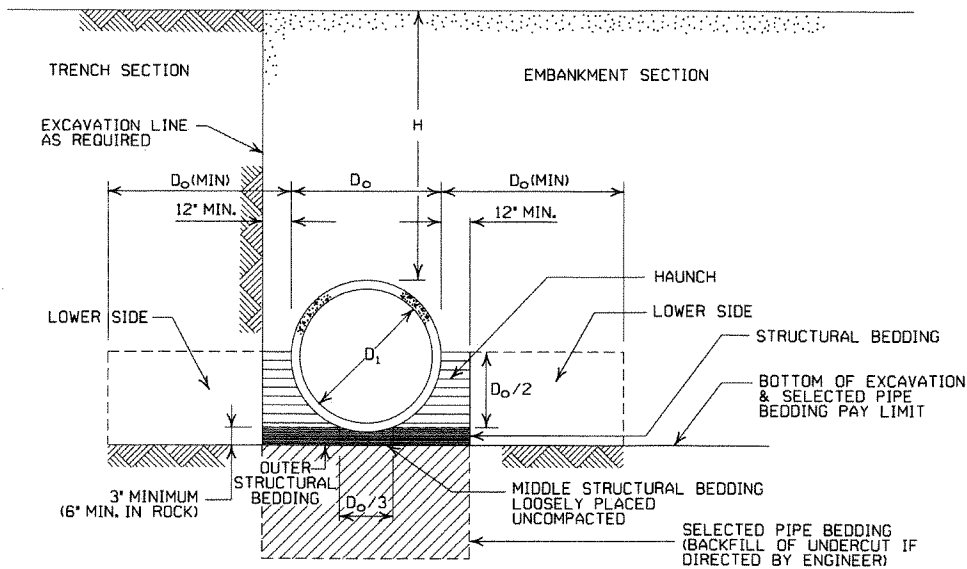
- LEGEND -

D_i = NORMAL INSIDE DIAMETER OF PIPE
D_o = OUTSIDE DIAMETER OF PIPE
H = FILL COVER HEIGHT OVER PIPE (FEET)
MIN. = MINIMUM
= UNDISTURBED SOIL

INSTALLATION TYPE	MATERIAL REQUIREMENTS FOR HAUNCH AND STRUCTURAL BEDDING
TYPE 1	AGGREGATE BASE COURSE (CLASS 5 OR CLASS 7)
TYPE 2	SELECTED MATERIALS (CLASS SM-1, SM-2, OR SM-4) OR TYPE 1 INSTALLATION MATERIAL*
TYPE 3**	AASHTO CLASSIFICATION A-1 THRU A-6 SOIL OR TYPE 1 OR 2 INSTALLATION MATERIAL

* SM-3 WILL NOT BE ALLOWED.

** MATERIALS SHALL NOT INCLUDE ORGANIC MATERIALS OR STONES LARGER THAN 3 INCHES.



EMBANKMENT AND TRENCH INSTALLATIONS

1. MATERIAL IN THE HAUNCH AND OUTER STRUCTURAL BEDDING SHALL BE COMPACTED TO 95% OF THE MAXIMUM DENSITY ACCORDING TO THE TYPE OR CLASS OF MATERIAL USED.
2. FOR TRENCHES WITH WALLS OF NATURAL SOIL, THE DENSITY OF THE SOIL IN THE LOWER SIDE ZONE SHALL BE AS FIRM AS THE 95% DENSITY REQUIRED FOR THE HAUNCH. IF THE EXISTING SOIL DOES NOT MEET THIS CRITERIA, IT SHALL BE REMOVED AND RECOMPACTED TO 95% OF THE MAXIMUM DENSITY ACCORDING TO THE TYPE OF MATERIAL USED.
3. FOR EMBANKMENTS, THE MATERIAL IN THE LOWER SIDE ZONE SHALL BE COMPACTED TO 95% OF THE MAXIMUM DENSITY ACCORDING TO THE TYPE OR CLASS OF MATERIAL USED.

GENERAL NOTES

1. CONCRETE PIPE CULVERT CONSTRUCTION SHALL CONFORM TO ARKANSAS STATE HIGHWAY AND TRANSPORTATION DEPARTMENT STANDARD SPECIFICATIONS FOR HIGHWAY CONSTRUCTION (2003 EDITION), WITH APPLICABLE SUPPLEMENTAL SPECIFICATIONS AND SPECIAL PROVISIONS. UNLESS OTHERWISE NOTED IN THE PLANS, SECTION AND SUBSECTION REFER TO THE STANDARD CONSTRUCTION SPECIFICATIONS.
2. CONCRETE PIPE CULVERT DESIGN SHALL CONFORM TO AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS, FIFTH EDITION (2010) WITH 2010 INTERIMS.
3. ALL PIPE SHALL CONFORM TO SECTION 606. CIRCULAR R.C. PIPE CULVERTS SHALL CONFORM TO AASHTO M170, R.C. ARCH PIPE CULVERTS SHALL CONFORM TO AASHTO M206 AND HORIZONTAL ELLIPTICAL PIPE CULVERTS SHALL CONFORM TO AASHTO M207.
4. ALL PIPE SHALL BE PROTECTED DURING CONSTRUCTION BY A COVER SUFFICIENT TO PREVENT DAMAGE FROM PASSAGE OF EQUIPMENT.
5. THE MINIMUM TRENCH WIDTH SHALL BE THE OUTSIDE DIAMETER OF THE PIPE PLUS 24 INCHES. THE MAXIMUM ALLOWABLE TRENCH WIDTH SHALL BE THE MINIMUM WIDTH PRACTICABLE FOR WORKING CONDITIONS.
6. MULTIPLE PIPE CULVERTS SHALL BE INSTALLED WITH A MINIMUM CLEARANCE OF 24 INCHES BETWEEN STRINGS OF PIPE. REFER TO STD. DWG. FES-2 FOR MINIMUM CLEARANCE WHERE FLARED END SECTIONS ARE USED.
7. IMPERVIOUS MATERIAL SHOULD BE PLACED AS DIRECTED BY THE ENGINEER AT THE ENDS OF THE CULVERT TO PREVENT LOSS OF STRUCTURAL BEDDING WHEN PERVIOUS MATERIAL IS USED FOR STRUCTURAL BEDDING AND/OR BACKFILL.
8. NOT MORE THAN ONE LIFTING HOLE MAY BE PROVIDED IN CONCRETE PIPE TO FACILITATE HANDLING. HOLE MAY BE CAST IN PLACE, CUT INTO THE FRESH CONCRETE AFTER FORMS ARE REMOVED, OR DRILLED. THE HOLE SHALL NOT BE MORE THAN TWO INCHES IN DIAMETER OR TWO INCHES SQUARE. CUTTING OR DISPLACEMENT OF REINFORCEMENT WILL NOT BE PERMITTED. SPALLED AREAS AROUND THE HOLE SHALL BE REPAIRED IN A WORKMANLIKE MANNER. LIFTING HOLE SHALL BE FILLED WITH MORTAR, CONCRETE, OR OTHER METHOD AS APPROVED BY THE ENGINEER.
9. WHEN DIRECTED BY THE ENGINEER, UNSUITABLE MATERIAL THAT IS ENCOUNTERED AT THE BOTTOM OF THE EXCAVATED TRENCH (BELOW THE AREA IDENTIFIED AS "STRUCTURAL BEDDING" ABOVE) WILL BE EXCAVATED AND REPLACED WITH SELECTED PIPE BEDDING. THE QUANTITY OF MATERIAL REQUIRED TO BACKFILL THE UNDERCUT AREA UP TO THE SELECTED PIPE BEDDING PAY LIMIT DESIGNATED ABOVE WILL BE MEASURED AND PAID FOR AS "SELECTED PIPE BEDDING."
10. WHEN THE EXISTING MATERIAL EXCAVATED FOR THE PIPE TRENCH IS DETERMINED BY THE ENGINEER TO BE UNSUITABLE FOR BACKFILLING THE PIPE (ABOVE THE AREA IDENTIFIED ABOVE AS THE HAUNCH), BORROW MATERIAL OR MATERIAL FROM THE ROADWAY EXCAVATION WILL BE USED TO BACKFILL THE PIPE. IF SUITABLE MATERIAL IS NOT AVAILABLE, THE ENGINEER MAY AUTHORIZE THE USE OF "SELECTED PIPE BACKFILL."

MINIMUM HEIGHT OF FILL "H"
OVER CIRCULAR R.C. PIPE CULVERTS

INSTALLATION TYPE	CLASS OF PIPE			
	TYPE 1 OR 2	TYPE 3	ALL	ALL
PIPE ID (IN.)	FEET			
12-15	2	2.5	2	1
18-24	2.5	3	2	1
27-33	3	4	2	1
36-42	3.5	5	2	1
48	4.5	5.5	2	1
54-60	5	7	2	1
66-78	6	8	2	1
84-108	7.5	8	2	1

NOTE: FOR MINIMUM COVER VALUES, "H" SHALL INCLUDE A MINIMUM OF 12" OF PAVEMENT AND/OR BASE.

MINIMUM HEIGHT OF FILL "H"
OVER R.C. ARCH & HORIZONTAL
ELLIPTICAL PIPE CULVERTS

INSTALLATION TYPE	CLASS OF PIPE	
	CLASS III	CLASS IV
FEET		
TYPE 2 OR TYPE 3	2.5	1.5

NOTE: TYPE 1 INSTALLATION WILL NOT BE ALLOWED FOR ARCH & HORIZONTAL ELLIPTICAL PIPE CULVERTS.

NOTE: FOR MINIMUM COVER VALUES, "H" SHALL INCLUDE A MINIMUM OF 12" OF PAVEMENT AND/OR BASE.

MAXIMUM HEIGHT OF
FILL "H" OVER CIRCULAR
R.C. PIPE CULVERTS

INSTALLATION TYPE	CLASS OF PIPE		
	CLASS III	CLASS IV	CLASS V
FEET			
TYPE 1	21	32	50
TYPE 2	16	25	39
TYPE 3	12	20	30

NOTE: IF FILL HEIGHT EXCEEDS 50 FEET, A SPECIAL DESIGN CONCRETE PIPE WILL BE REQUIRED USING TYPE 1 INSTALLATION.

MAXIMUM HEIGHT OF FILL "H"
OVER R.C. ARCH & HORIZONTAL
ELLIPTICAL PIPE CULVERTS

INSTALLATION TYPE	CLASS OF PIPE	
	CLASS III	CLASS IV
FEET		
TYPE 2	13	21
TYPE 3	10	16

NOTE: TYPE 1 INSTALLATION WILL NOT BE ALLOWED FOR ARCH & HORIZONTAL ELLIPTICAL PIPE CULVERTS.

DATE	REVISION	DATE FILMED
12-15-11	REVISED FOR LRFD DESIGN SPECIFICATIONS	
5-18-00	REVISED TYPE 3 BEDDING & ADDED NOTE	
3-30-00	REVISED INSTALLATIONS	
11-06-97	ISSUED	

ARKANSAS STATE HIGHWAY COMMISSION

CONCRETE PIPE CULVERT
FILL HEIGHTS & BEDDING

STANDARD DRAWING PCC-1



CORRUGATED STEEL PIPE (ROUND)

PIPE DIAMETER (INCHES)	① MINIMUM COVER TOP OF PIPE TO TOP OF GROUND "H" (FEET)	MAX. FILL HEIGHT "H" ABOVE TOP OF PIPE (FEET)				
		METAL THICKNESS (INCHES)				
		0.064	0.079	0.109	0.138	0.168
2 3/8 INCH BY 1/2 INCH CORRUGATION RIVETED, WELDED, OR HELICAL LOCK-SEAM						
12	1	84	91			
15	1	67	73			
18	1	56	61			
24	1	42	46	59		
30	2	34	36	47		
36	2		30	39	41	
42	2		43	67	70	73
48	2		37	58	61	64
② 3 INCH BY 1 INCH OR 5 INCH BY 1 INCH CORRUGATION RIVETED, WELDED, BOLTED, OR HELICAL LOCK-SEAM						
36	1	48	60	88	111	118
42	1	41	51	72	90	102
48	1	36	45	64	77	85
54	2	32	40	59	71	79
60	2	29	36	53	64	71
66	2	26	33	47	58	64
72	2	24	30	44	53	59
78	2		28	41	49	54
84	2		26	38	45	51
90	2		24	35	43	45
96	2		22	33	40	44
102	2			31	38	42
108	2			30	35	39
114	2			28	34	37
120	2			27	32	35

CONSTRUCTION SEQUENCE

1. PLACE STRUCTURAL BEDDING MATERIAL TO GRADE. DO NOT COMPACT.
2. INSTALL PIPE TO GRADE.
3. COMPACT STRUCTURAL BEDDING OUTSIDE THE MIDDLE THIRD OF THE PIPE.
4. COMPLETE STRUCTURAL BACKFILL OPERATION BY WORKING FROM SIDE TO SIDE OF THE PIPE. THE SIDE TO SIDE STRUCTURAL BACKFILL DIFFERENTIAL SHALL NOT EXCEED 24 INCHES OR 1/3 THE SIZE OF THE PIPE, WHICHEVER IS LESS.

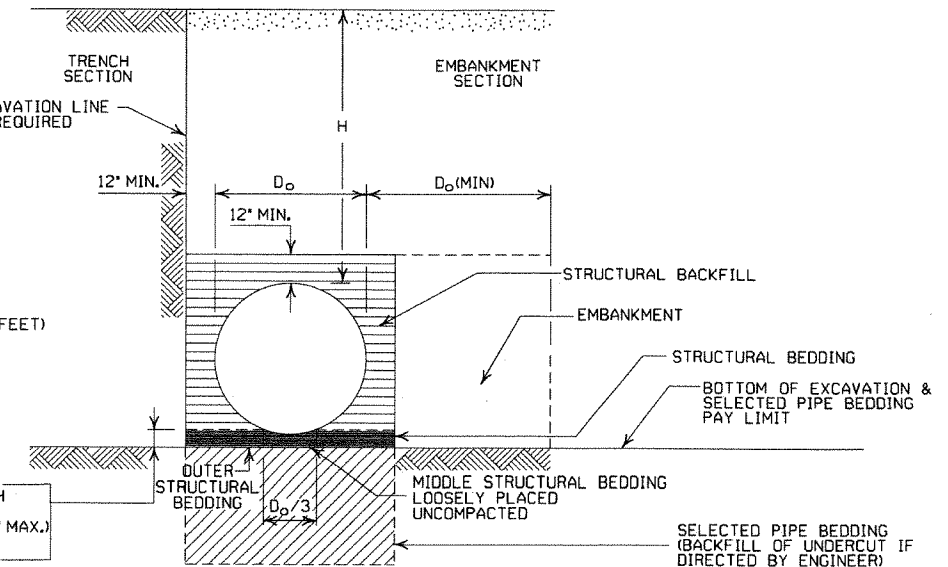
NOTE: STRUCTURAL BACKFILL AND STRUCTURAL BEDDING MATERIAL WILL NOT BE PAID FOR SEPARATELY, BUT COMPENSATION WILL BE CONSIDERED TO BE INCLUDED IN THE PRICE BID PER LINEAR FOOT OF METAL PIPE.

INSTALLATION TYPE	MATERIAL REQUIREMENTS FOR STRUCTURAL BACKFILL AND STRUCTURAL BEDDING
TYPE 1	AGGREGATE BASE COURSE (CLASS 4, 5, 6, OR 7)
TYPE 2	SELECTED MATERIALS (CLASS SM-1, SM-2, OR SM-4) OR TYPE 1 INSTALLATION MATERIAL ③

③ SM-3 WILL NOT BE ALLOWED.

- LEGEND -
- D_o = OUTSIDE DIAMETER OF PIPE
MAX. = MAXIMUM
MIN. = MINIMUM
- ===== = STRUCTURAL BACKFILL MATERIAL
- ===== = UNDISTURBED SOIL
- EQUIV. DIA. = EQUIVALENT DIAMETER
- H = FILL COVER HEIGHT OVER PIPE (FEET)

IN SOIL-MIN. EQUALS TWICE CORRUGATION DEPTH
IN ROCK-MIN. EQUALS GREATER OF:
1/2" PER FOOT OF FILL OVER PIPE (24" MAX.)
TWICE CORRUGATION DEPTH



EMBANKMENT AND TRENCH INSTALLATIONS

1. STRUCTURAL BACKFILL, EMBANKMENT, AND OUTER STRUCTURAL BEDDING MATERIAL SHALL BE COMPACTED TO 95% OF THE MAXIMUM DENSITY ACCORDING TO THE TYPE OR CLASS OF MATERIAL USED.
2. INSTALLATION TYPE 1 OR 2 MAY BE USED FOR CORRUGATED STEEL OR ALUMINUM PIPE (ROUND).
3. INSTALLATION TYPE 1 SHALL BE USED FOR CORRUGATED STEEL OR ALUMINUM PIPE ARCHES WITH 2 3/8" X 1/2" CORRUGATION.
4. INSTALLATION TYPE 1 OR 2 MAY BE USED FOR CORRUGATED STEEL OR ALUMINUM PIPE ARCHES WITH 3" X 1" OR 5" X 1" CORRUGATION.

GENERAL NOTES

1. METAL PIPE CULVERT CONSTRUCTION SHALL CONFORM TO ARKANSAS STATE HIGHWAY AND TRANSPORTATION DEPARTMENT STANDARD SPECIFICATIONS FOR HIGHWAY CONSTRUCTION (2003 EDITION), WITH APPLICABLE SUPPLEMENTAL SPECIFICATIONS AND SPECIAL PROVISIONS. UNLESS OTHERWISE NOTED IN THE PLANS, SECTION AND SUBSECTION REFER TO THE STANDARD CONSTRUCTION SPECIFICATIONS.
2. METAL PIPE CULVERT DESIGN SHALL CONFORM TO AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS, FIFTH EDITION (2010) WITH 2010 INTERIMS.
3. METAL PIPE CULVERT MATERIALS AND INSTALLATIONS SHALL CONFORM TO SECTION 606 AND JOB SPECIAL PROVISION "METAL PIPE".
4. ALL PIPE SHALL BE PROTECTED DURING CONSTRUCTION BY A COVER SUFFICIENT TO PREVENT DAMAGE FROM PASSAGE OF EQUIPMENT.
5. THE MINIMUM TRENCH WIDTH SHALL BE THE OUTSIDE DIAMETER OF THE PIPE PLUS 24 INCHES. THE MAXIMUM ALLOWABLE TRENCH WIDTH SHALL BE THE MINIMUM WIDTH PRACTICABLE FOR WORKING CONDITIONS.
6. MULTIPLE PIPE CULVERTS SHALL BE INSTALLED WITH A MINIMUM CLEARANCE OF 24 INCHES BETWEEN STRINGS OF PIPE. REFER TO STD. DWG. FES-2 FOR MINIMUM CLEARANCE WHERE FLARED END SECTIONS ARE USED.
7. IMPERVIOUS MATERIAL SHOULD BE PLACED AS DIRECTED BY THE ENGINEER AT THE ENDS OF THE CULVERT TO PREVENT LOSS OF STRUCTURAL BEDDING WHEN PERVIOUS MATERIAL IS USED FOR STRUCTURAL BEDDING AND/OR BACKFILL.
8. WHEN DIRECTED BY THE ENGINEER, UNSUITABLE MATERIAL THAT IS ENCOUNTERED AT THE BOTTOM OF THE EXCAVATED TRENCH (BELOW THE AREA IDENTIFIED AS "STRUCTURAL BEDDING" ABOVE) WILL BE EXCAVATED AND REPLACED WITH SELECTED PIPE BEDDING. THE QUANTITY OF MATERIAL REQUIRED TO BACKFILL THE UNDERCUT AREA UP TO THE SELECTED PIPE BEDDING PAY LIMIT DESIGNATED ABOVE WILL BE MEASURED AND PAID FOR AS "SELECTED PIPE BEDDING."
9. WHEN THE EXISTING MATERIAL EXCAVATED FOR THE PIPE TRENCH IS DETERMINED BY THE ENGINEER TO BE UNSUITABLE FOR BACKFILLING THE PIPE (ABOVE THE AREA IDENTIFIED AS STRUCTURAL BACKFILL), BORROW MATERIAL OR MATERIAL FROM THE ROADWAY EXCAVATION WILL BE USED TO BACKFILL THE PIPE. IF SUITABLE MATERIAL IS NOT AVAILABLE, THE ENGINEER MAY AUTHORIZE THE USE OF "SELECTED PIPE BACKFILL."

CORRUGATED ALUMINUM PIPE (ROUND)

PIPE DIAMETER (INCHES)	① MINIMUM COVER TOP OF PIPE TO TOP OF GROUND "H" (FEET)	MAX. FILL HEIGHT "H" ABOVE TOP OF PIPE (FEET)				
		METAL THICKNESS IN INCHES				
		0.060	0.075	0.105	0.135	0.164
		2 3/8 INCH BY 1/2 INCH CORRUGATION RIVETED OR HELICAL LOCK-SEAM				
12	1	45	45			
18	2	30	30	52		
24	2	22	22	39	41	
30	2		18	31	32	34
36	2.5		15	26	27	28
42	2			43	43	44
48	2			40	41	43
54	2			35	37	38
60	2				33	34
66	2					31
72	2					29

EQUIVALENT METAL THICKNESSES AND GAUGES

METAL THICKNESS IN INCHES			GAUGE NUMBER
STEEL		ALUMINUM	
ZINC COATED	UNCOATED		
0.064	0.0598	0.060	16
0.079	0.0747	0.075	14
0.109	0.1046	0.105	12
0.138	0.1345	0.135	10
0.168	0.1644	0.164	8

CORRUGATED METAL PIPE ARCHES

EQUIV. DIA. (INCHES)	PIPE DIMENSION SPAN X RISE (INCHES)	MINIMUM CORNER RADIUS (INCHES)	STEEL				ALUMINUM			
			MIN. THICKNESS REQUIRED INCHES	① MIN. HEIGHT OF FILL, "H" (FT.)	MAX. HEIGHT OF FILL, "H" (FT.)	MIN. THICKNESS REQUIRED INCHES	① MIN. HEIGHT OF FILL, "H" (FT.)	MAX. HEIGHT OF FILL, "H" (FT.)		
				INSTALLATION	INSTALLATION		INSTALLATION	INSTALLATION		
				TYPE 1	TYPE 1		TYPE 1	TYPE 1		
			2 3/8 INCH BY 1/2 INCH CORRUGATION RIVETED, WELDED, OR HELICAL LOCK-SEAM				2 3/8 INCH BY 1/2 INCH CORRUGATION RIVETED OR HELICAL LOCK-SEAM			
15	17x13	3	0.064	2	15	0.060	2	15		
18	21x15	3	0.064	2	15	0.060	2	15		
21	24x18	3	0.064	2.25	15	0.060	2.25	15		
24	28x20	3	0.064	2.5	15	0.075	2.5	15		
30	35x24	3	0.079	3	12	0.075	3	12		
36	42x29	3 1/2	0.079	3	12	0.105	3	12		
42	49x33	4	0.079	3	12	0.105	3	12		
48	57x38	5	0.109	3	13	0.135	3	13		
54	64x43	6	0.109	3	14	0.135	3	14		
60	71x47	7	0.138	3	15	0.164	3	15		
66	77x52	8	0.168	3	15					
72	83x57	9	0.168	3	15					
			② 3 INCH BY 1 INCH OR 5 INCH BY 1 INCH CORRUGATION RIVETED, WELDED, OR HELICAL LOCK-SEAM							
			INSTALLATION				INSTALLATION			
			TYPE 2		TYPE 1	TYPE 2		TYPE 1		
36	40x31	5	0.079	3	2	12	15			
42	46x36	6	0.079	3	2	13	15			
48	53x41	7	0.079	3	2	13	15			
54	60x46	8	0.079	3	2	13	15			
60	66x51	9	0.079	3	2	13	15			
66	73x55	12	0.079	3	2	15	15			
72	81x59	14	0.079	3	2	15	15			
78	87x63	14	0.079	3	2	15	15			
84	95x67	16	0.109	3	2	15	15			
90	103x71	16	0.109	3	2	15	15			
96	112x75	18	0.109	3	2	15	15			
102	117x79	18	0.109	3	2	15	15			
108	128x83	18	0.138	3	2	15	15			

① FOR MINIMUM COVER VALUES, "H" SHALL

② WHERE THE STANDARD 2 2/3" x 1/2" CORRUGATION WITH A 3" x 1" OR 5" x 1" CORRUGATION OR GREATER THAN THE MAXIMUM FILL

① FOR MINIMUM COVER VALUES, "H" SHALL INCLUDE A MINIMUM 12" OF PAVEMENT AND/OR BASE.

② WHERE THE STANDARD 2 2/3" X 1/2" CORRUGATION AND GAUGE IS SPECIFIED FOR A GIVEN DIAMETER, A PIPE OF THE SAME DIAMETER WITH A 3" X 1" OR 5" X 1" CORRUGATION MAY BE SUBSTITUTED, PROVIDING IT IS GAUGED FOR A FILL HEIGHT CONDITION EQUAL TO OR GREATER THAN THE MAXIMUM FILL HEIGHT CONDITION FOR THE SPECIFIED GAUGE AND CORRUGATION.

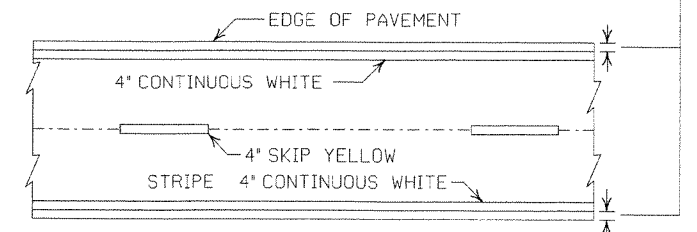
12-15-II	REVISED FOR LRFD DESIGN SPECS	
3-30-00	REVISED INSTALLATIONS	
11-06-97	ISSUED	
DATE	REVISION	DATE FILMED

ARKANSAS STATE HIGHWAY COMMISSION
METAL PIPE CULVERT FILL HEIGHTS & BEDDING
STANDARD DRAWING PCM-1

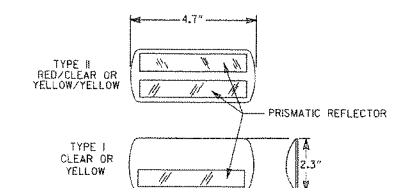
NOTES:

1. ALL LINES SHALL HAVE A WIDTH OF 4 INCHES.
2. THE THICKNESS AND RATE OF PAINT APPLICATION SHALL BE AS SPECIFIED IN SECTION 718 OF THE STANDARD SPECIFICATIONS.
3. THIS DRAWING SHALL BE USED IN CONJUNCTION WITH THE LATEST REVISED ADDITION OF THE "MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES."
4. RAISED PAVEMENT MARKERS SHALL BE CENTERED BETWEEN SKIP LINES ON 40 FEET SPACING UNLESS OTHERWISE SHOWN ON THE PLANS.

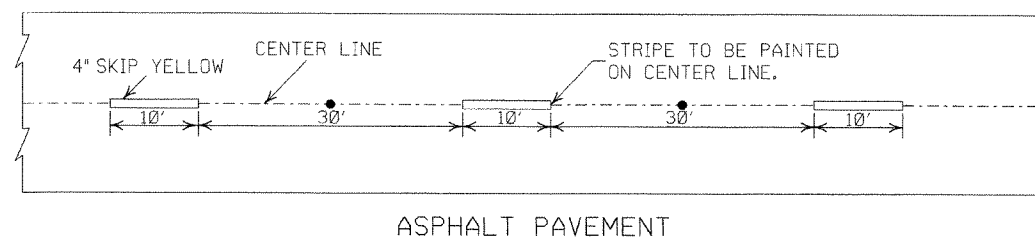
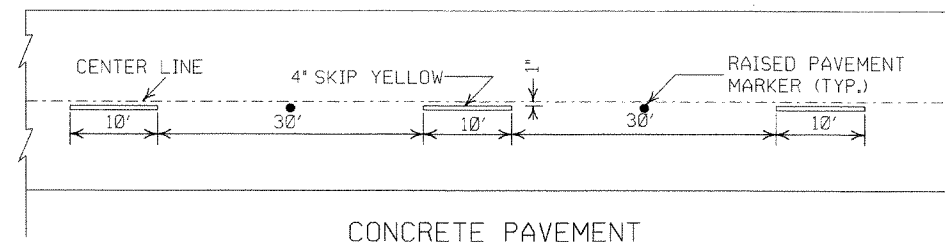
2" FOR ASPHALT OR CONCRETE PAVEMENT
6" FOR BITUMINOUS SURFACE TREATMENT



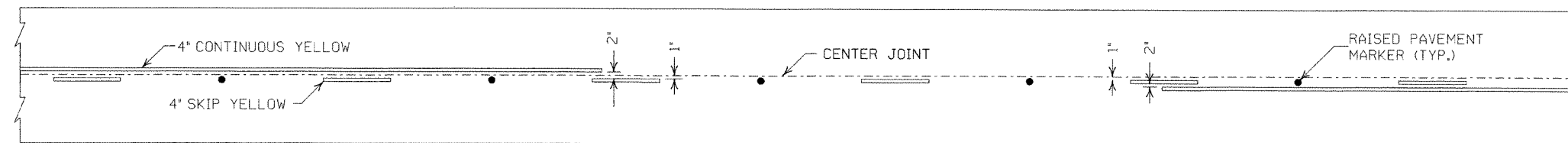
PAVEMENT EDGE LINE MARKING



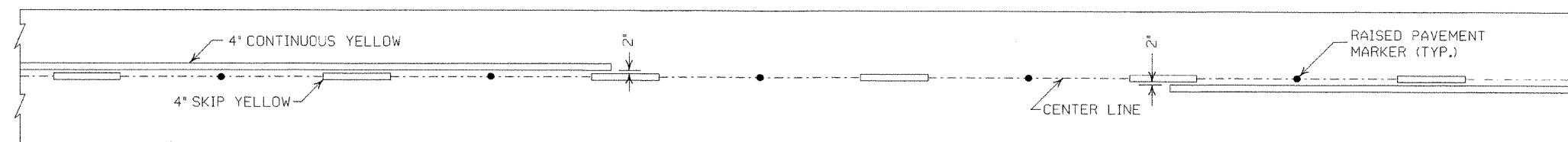
NOTE:
THE RED LENS OF THE
TYPE II R.P.M. SHALL
FACE THE INCORRECT
TRAFFIC MOVEMENT.

DETAIL OF
STANDARD
RAISED PAVEMENT MARKERS

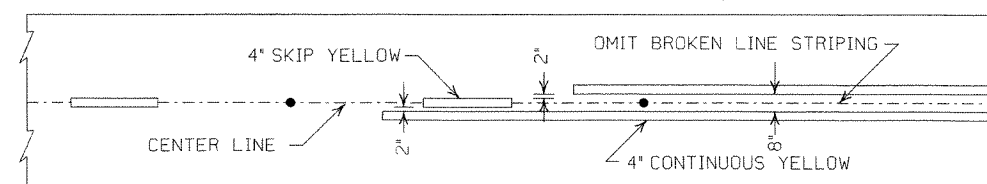
BROKEN LINE STRIPING



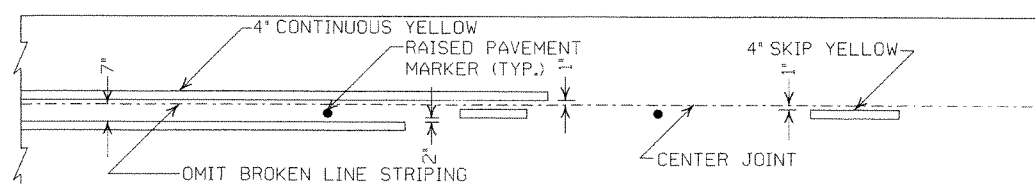
SOLID LINE STRIPING ON CONCRETE PAVEMENT



SOLID LINE STRIPING ON ASPHALT PAVEMENT

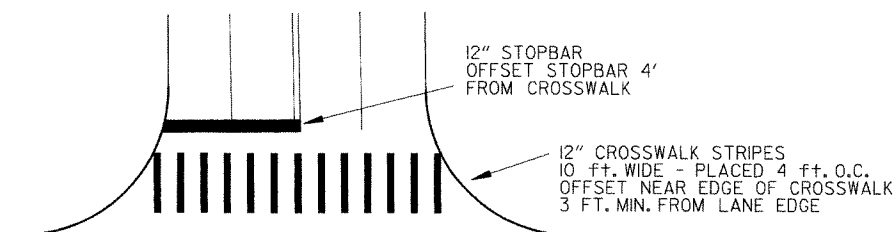


ASPHALT PAVEMENT



CONCRETE PAVEMENT

STRIPING AT ADJACENT NO PASSING LANES



CROSSWALK AND STOPBAR DETAILS

GENERAL NOTES:

THIS DRAWING SHOULD BE CONSIDERED AS TYPICAL ONLY AND THE FINAL LOCATION OF THE STRIPING AND RAISED PAVEMENT MARKERS SHALL BE DETERMINED BY THE ENGINEER.

THIS DRAWING SHOULD BE USED IN CONJUNCTION WITH THE "MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES", LATEST REVISION.

NOTE:
DIMENSIONS SHOWN FOR RAISED PAVEMENT MARKERS ARE TYPICAL. THE CONTRACTOR MAY SUBSTITUTE SIMILAR MARKERS WITH THE APPROVAL OF THE ENGINEER. REQUESTING APPROVAL FOR SIMILAR MARKERS MAY BE MADE BY REFERRING TO THE AHTD QUALIFIED PRODUCTS LIST.

11-17-10	REVISED GENERAL NOTES & REMOVED PLOWABLE PVT MKRS	
11-18-04	REVISED NOTE 2 & GENERAL NOTES	
8-22-02	ADDED CROSSWALK & STOPBAR DTLS.	
7-02-98	ADDED DETAILS OF STD. RAISED PAV'T. MARKERS	
4-26-96	REV. NOTES 3&4; ADDED R.P.M.	
9-30-80	DRAWN	1-9-30-80
DATE	REVISION	FILMED

ARKANSAS STATE HIGHWAY COMMISSION

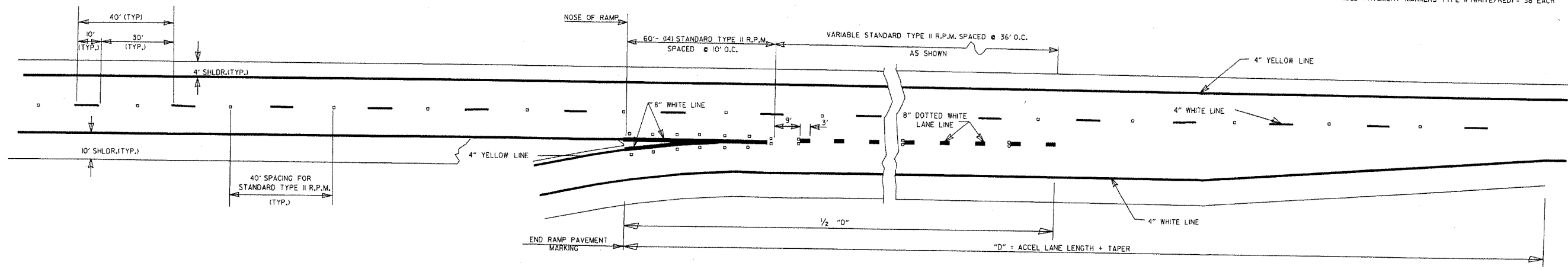
PAVEMENT MARKING DETAILS

STANDARD DRAWING PM-1

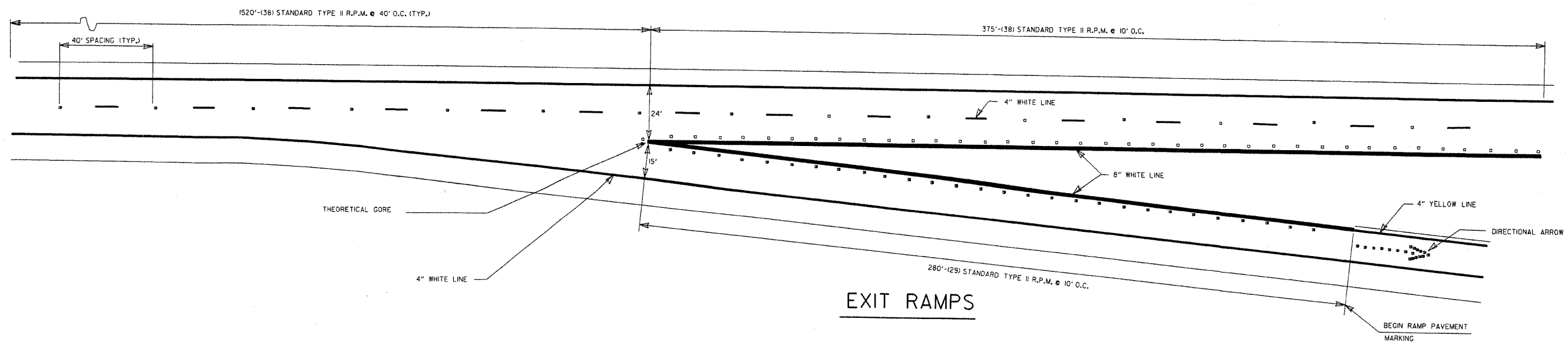
PAVEMENT MARKING QUANTITIES
(BASED ON 700' ACCEL. LANE + 300' TAPER)

ENTRANCE RAMP
8" WHITE = 228 LIN. FT.
RAISED PAVEMENT MARKERS TYPE II (WHITE/RED) = 38 EACH

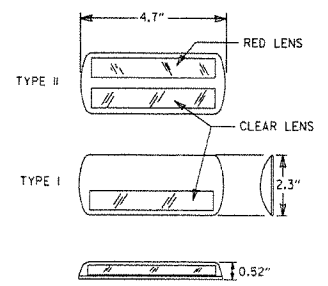
EXIT RAMP
4" WHITE = 280 LIN. FT.
8" WHITE = 655 LIN. FT.
RAISED PAVEMENT MARKERS TYPE II (WHITE/RED) = 38 EACH
RAISED PAVEMENT MARKERS TYPE II (WHITE/RED) = 48 EACH
RAISED PAVEMENT MARKERS TYPE II (WHITE/RED) = 38 EACH



ENTRANCE RAMP

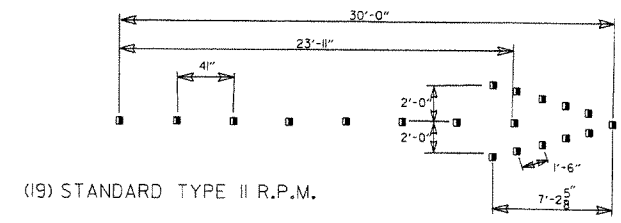


EXIT RAMP



DETAIL OF
STANDARD
RAISED PAVEMENT MARKERS

NOTE:
THE RED LENS OF THE
TYPE II R.P.M. SHALL
FACE THE INCORRECT
TRAFFIC MOVEMENT.



(19) STANDARD TYPE II R.P.M.

DIRECTIONAL ARROWS

GENERAL NOTES:
THIS DRAWING SHOULD BE CONSIDERED AS TYPICAL ONLY
AND THE FINAL LOCATION OF THE STRIPING AND PAVEMENT
MARKERS SHALL BE DETERMINED BY THE ENGINEER.

THIS DRAWING SHOULD BE USED IN CONJUNCTION WITH
THE "MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES",
LATEST REVISION.

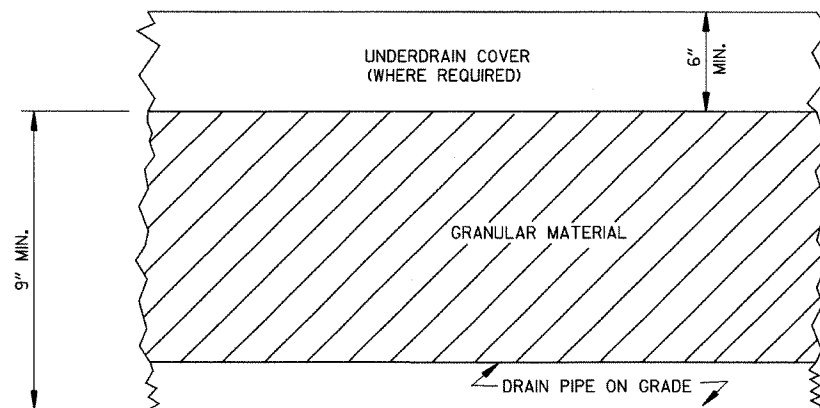
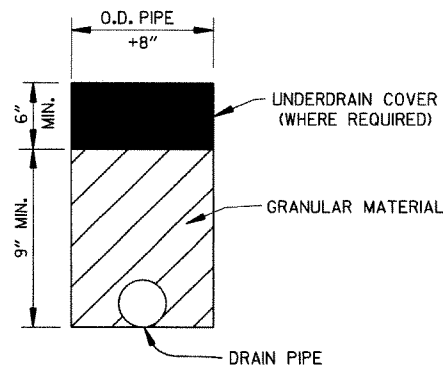
NOTE:
DIMENSIONS SHOWN FOR RAISED PAVEMENT MARKERS ARE
TYPICAL. THE CONTRACTOR MAY SUBSTITUTE SIMILAR
MARKERS WITH THE APPROVAL OF THE ENGINEER, REQUESTING
APPROVAL FOR SIMILAR MARKERS MAY BE MADE BY REFERRING
TO THE AHTD QUALIFIED PRODUCTS LIST.

7/26/12	REVISED RPM NOTATION	
12-15-11	REVISED RPMs ACCORDING TO LATEST POLICY	
11-17-10	REMOVED PLOWABLE PAVEMENT MARKERS	
6-3-10	REVISED PER 2009 MUTCD	
11-18-04	REVISED NOTES	
8-22-02	ADDED & REVISED NOTES; REV. ENTRANCE & EXIT RAMP	
5-18-00	REMOVED HASHMARKS	
7-02-98	CHANGED TYPES TO ROMAN NUMERALS	
4-26-96	ADDED DIMENSIONS & QUANTITIES; REVISED LANE WIDTH ON EXIT RAMP	
2-2-95	PLACED IN USE	2-2-95
DATE	REVISION	FILMED

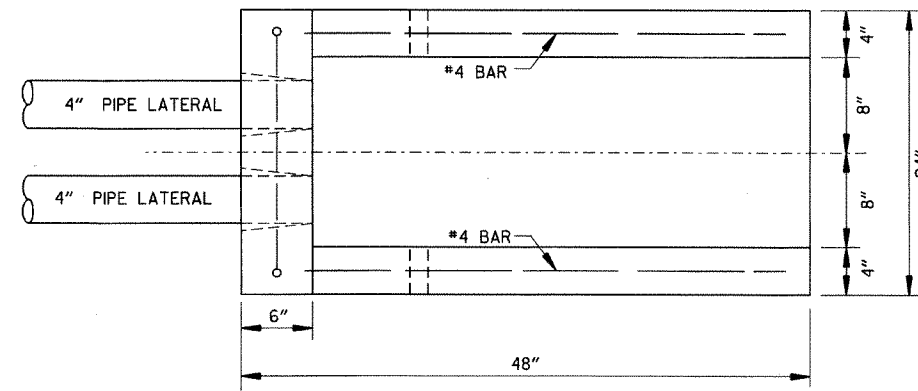
ARKANSAS STATE HIGHWAY COMMISSION
PERMANENT PAVEMENT MARKING
ON ACCESS CONTROLLED ROADWAYS

STANDARD DRAWING PM-2

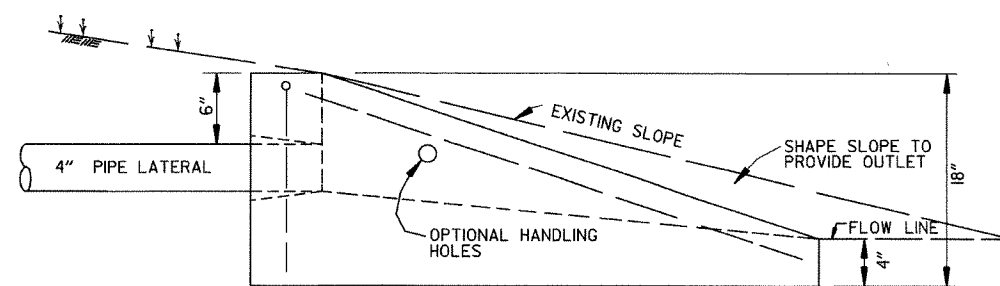
- NOTE:
1. GRANULAR BACKFILL TO BE SUBSIDIARY TO PIPE UNDERDRAIN.
 2. UNLESS OTHERWISE SPECIFIED ON THE PLANS, THE UNDERDRAIN COVER SHALL BE THOROUGHLY COMPACTED EARTH AND SHALL BE SUBSIDIARY TO PIPE UNDERDRAIN.
 3. GRANULAR MATERIAL SHALL BE WRAPPED WITH GEOTEXTILE FABRIC. LAP FABRIC 12" OR THE WIDTH OF THE TRENCH AT THE TOP.



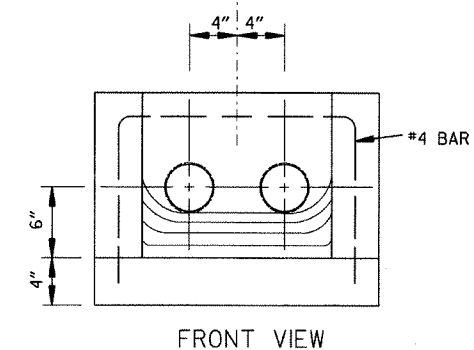
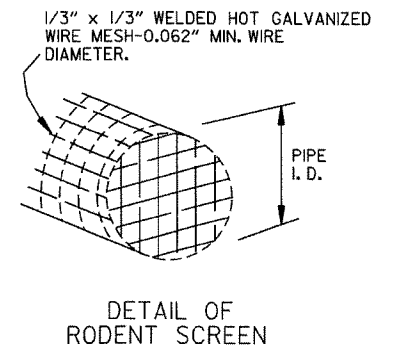
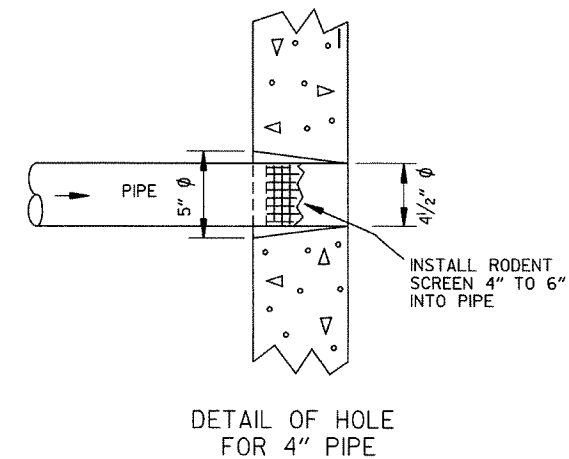
DETAILS OF PIPE UNDERDRAIN



PLAN VIEW



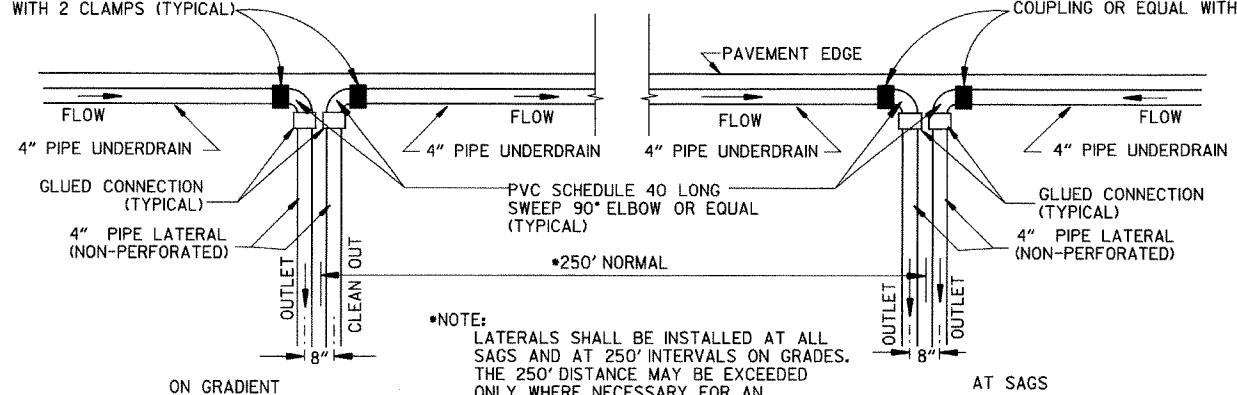
SIDE VIEW



FERNCO 1056-44 (4" CI/PLASTIC) OR FERNCO 1051-44 (4" AC/DIOR 4" CI/PLASTIC) COUPLING OR EQUAL WITH 2 CLAMPS (TYPICAL)

UNDERDRAIN OUTLET PROTECTORS

FERNCO 1056-44 (4" CI/PLASTIC) OR FERNCO 1051-44 (4" AC/DIOR 4" CI/PLASTIC) COUPLING OR EQUAL WITH 2 CLAMPS (TYPICAL)



DETAIL OF PIPE UNDERDRAIN LATERALS WHEN PLACED ALONG PAVEMENT EDGE

NOTE: PVC PIPE FOR LATERALS SHALL MEET THE REQUIREMENTS OF ASTM D 1785 (LATEST REVISION) FOR SCHEDULE 40 PIPE.

4-10-03	REVISED NOTE 3	
1-12-00	REVISED DETAIL OF UNDERDRAIN LATERALS	
11-18-98	REVISED NOTE	
10-18-96	REVISED MIN. DEPTH & GEOTEXTILE FABRIC	
4-26-96	ADDED LATERAL NOTE; 5 1/2" TO 5"	
11-22-95	REVISED LATERALS	
7-20-95	REVISED LATERALS & ADDED NOTE	
11- 3-94	REVISED FOR DUAL LATERALS	11- 3-94
10- 1-92	SUBSTITUTED GEOTEXTILE	10- 1-92
8-15-91	ADDED POLYETHYLENE PIPE	8-15-91
11- 8-90	DELETED ALTERNATE NOTE	11- 8-90
1-25-90	ADDED 4" SNAP ADAPTER	1-25-90
11-30-89	DEL. (SUBGRADE); ADDED (WHERE REQUIRED)	11-30-89
7-15-88	ISSUED P.L.M.	647-7-15-88
DATE	REVISION	DATE FILMED

ARKANSAS STATE HIGHWAY COMMISSION

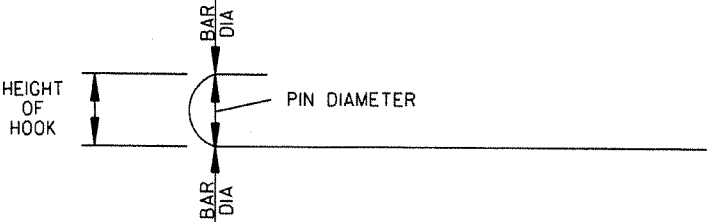
DETAILS OF PIPE UNDERDRAIN

STANDARD DRAWING PU-1

STEEL FABRICATION: REINFORCING STEEL FABRICATION SHALL CONFORM TO THE DIMENSIONS LISTED IN THE TABLE BELOW:

BAR SIZE	PIN DIAMETER	HOOK EXTENSION "K"
3	2 1/4"	4"
4	3 "	4 1/2"
5	3 3/4"	5"
6	4 1/2"	6"
7	5 1/4"	7"
8	6"	8"

IF THE OVERALL HEIGHT OF THE HOOK (SEE DIAGRAM BELOW) FOR A "b", "bi", "b2" or "b3" BENT BAR IS GREATER THAN THE CORRESPONDING TOP OR BOTTOM SLAB THICKNESS, LESS 2 3/4 INCHES, EACH BENT BAR SHALL BE REPLACED WITH ONE HOOKED BAR AND ONE STRAIGHT BAR, USING LENGTHS AS SHOWN IN THE TABLE BELOW. THE TWO BARS SHALL BE THE SAME DIAMETER AS, AND PLACED AT THE SAME SPACING AS, THE "b", "bi", "b2" OR "b3" BENT BARS THEY REPLACE.



NOTE: DIMENSIONS OF BARS ARE MEASURED OUT TO OUT OF BARS.

OVERALL HEIGHT OF HOOKED BAR DIAGRAM

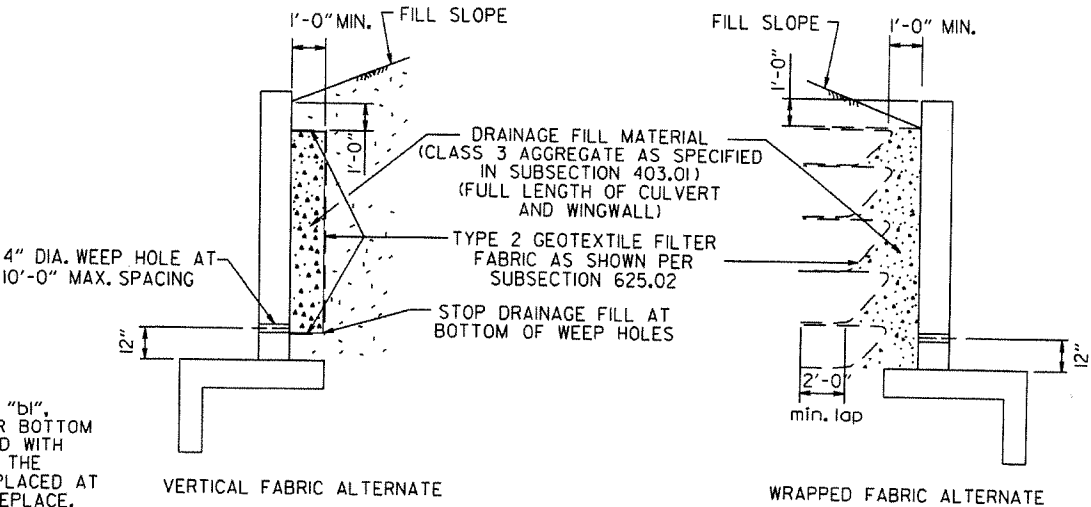
THE HOOKED BARS SHALL BE PLACED IN THE BOTTOM OF THE TOP SLAB AND THE TOP OF THE BOTTOM SLAB. THE STRAIGHT BARS SHALL BE PLACED IN THE TOP OF THE TOP SLAB AND THE BOTTOM OF THE BOTTOM SLAB. SEE TABLE BELOW FOR LENGTHS OF REPLACEMENT HOOKED AND STRAIGHT BARS.

FOR SKEWED CULVERTS, THE REPLACEMENT STRAIGHT BAR MAY HAVE TO BE CUT IN FIELD TO FIT.

REPLACEMENT BAR LENGTHS TABLE

BAR SIZE: "b", "bi", "b2" OR "b3"	LENGTH OF HOOKED BAR	LENGTH OF STRAIGHT BAR
*4	L + 1' - 0"	SEE "c" BAR LENGTH
*5	L + 1' - 2"	SEE "c" BAR LENGTH
*6	L + 1' - 4"	SEE "c" BAR LENGTH
*7	L + 1' - 8"	SEE "c" BAR LENGTH
*8	L + 1' - 10"	SEE "c" BAR LENGTH
*9	L + 2' - 6"	SEE "c" BAR LENGTH

L = "OW" - 3 INCHES



WINGWALL & CULVERT DRAINAGE DETAIL

REINFORCED CONCRETE BOX CULVERT GENERAL NOTES

CONCRETE SHALL BE CLASS S WITH A MINIMUM 28 DAY COMPRESSIVE STRENGTH OF 3500 PSI. REINFORCING STEEL SHALL BE AASHTO M 31 OR M 53, GRADE 60.

CONSTRUCTION AND MATERIALS FOR WINGWALL & CULVERT DRAINAGE, INCLUDING WEEP HOLES AND GRANULAR MATERIAL, SHALL BE SUBSIDIARY TO THE BID ITEM, "CLASS S CONCRETE".

MEMBRANE WATERPROOFING SHALL CONFORM TO THE REQUIREMENTS OF SECTION 815 OF THE STANDARD SPECIFICATIONS.

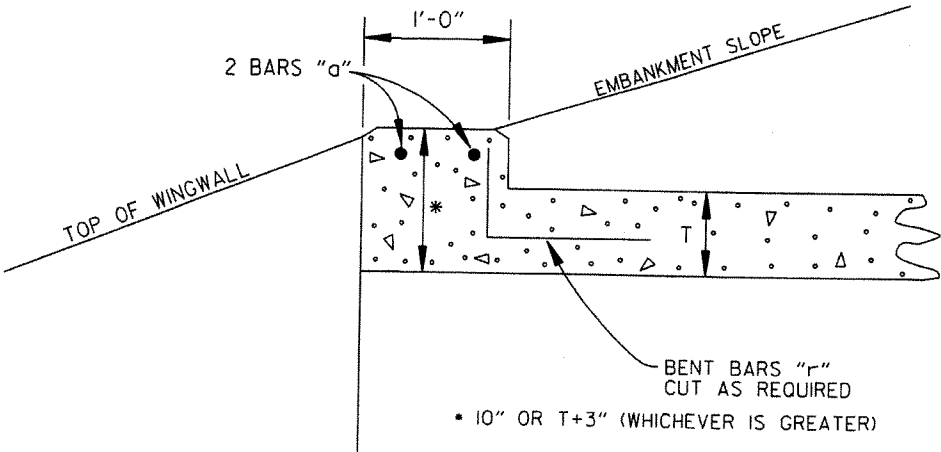
MEMBRANE WATERPROOFING SHALL BE APPLIED TO ALL CONSTRUCTION JOINTS IN THE TOP SLAB AND THE SIDEWALLS OF R.C. BOX CULVERTS AS DIRECTED BY THE ENGINEER. NO PAYMENT SHALL BE MADE FOR THIS ITEM, BUT PAYMENT WILL BE CONSIDERED TO BE INCLUDED IN THE VARIOUS ITEMS BID FOR THE R.C. BOX CULVERT.

REINFORCING STEEL TOLERANCES: THE TOLERANCES FOR REINFORCING STEEL SHALL MEET THOSE LISTED IN "MANUAL OF STANDARD PRACTICE" PUBLISHED BY CONCRETE REINFORCING STEEL INSTITUTE (CRSI) EXCEPT THAT THE TOLERANCE FOR TRUSS BARS SUCH AS FIGURE 3 ON PAGE 7-4 OF THE CRSI MANUAL SHALL BE MINUS ZERO TO PLUS 1/2 INCH.

WEEP HOLES IN BOX CULVERT WALLS SHALL HAVE A MAXIMUM HORIZONTAL SPACING OF 10'-0" AND SHALL BE SPACED TO CLEAR ALL REINFORCING STEEL. THE DRAIN OPENING SHALL BE 4" DIAMETER AND SHALL BE PLACED 12" ABOVE THE TOP OF THE BOTTOM SLAB.

WEEP HOLES IN WINGWALLS SHALL HAVE A MAXIMUM HORIZONTAL SPACING OF 10'-0" AND SHALL BE SPACED TO CLEAR ALL REINFORCING STEEL. THERE SHALL BE A MINIMUM OF TWO (2) WEEP HOLES IN EACH WINGWALL. THE DRAIN OPENING SHALL BE 4" DIAMETER AND SHALL BE PLACED 12" ABOVE THE TOP OF THE WINGWALL FOOTING.

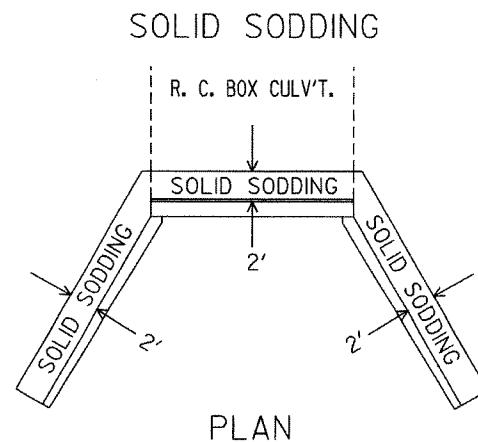
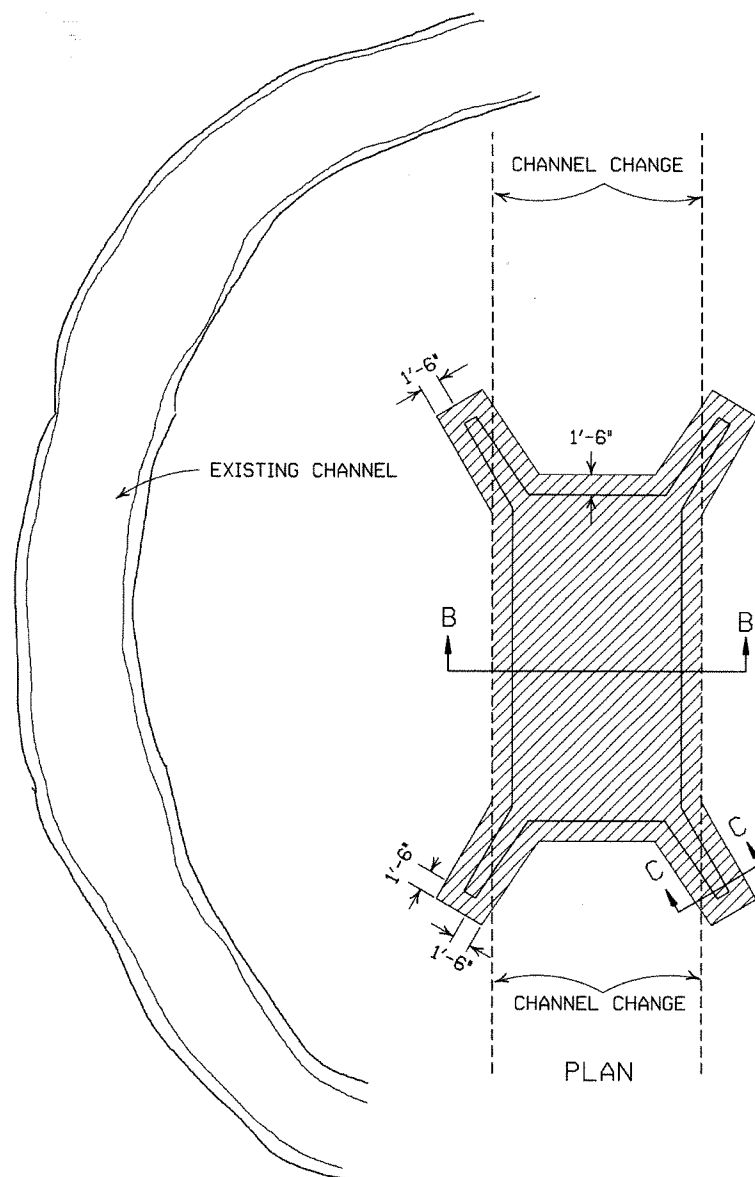
THE REQUIREMENTS SHOWN ON THIS DRAWING SHALL SUPERCEDE THE CORRESPONDING REQUIREMENTS ON ALL REINFORCED CONCRETE BOX CULVERT STANDARD DRAWINGS.



NOTE: FOR ALL SKEWED R.C. BOX CULVERTS THE LENGTH "K" OF THE MODIFIED HEADWALL SHALL BE EQUAL TO THE ROADWAY LENGTH "RL". THE ENDS OF THE HEADWALL SHALL BE CONSTRUCTED PARALLEL TO THE SKEW ANGLE OF THE BOX CULVERT.

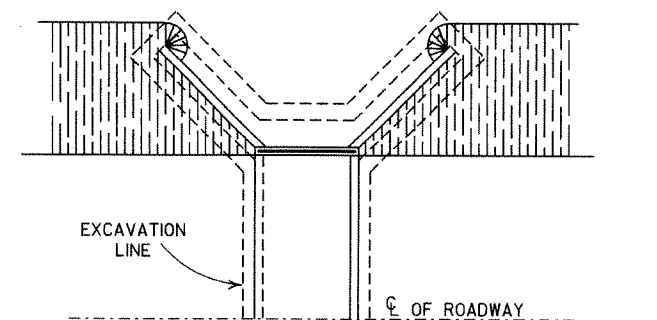
R.C. BOX CULVERT HEADWALL MODIFICATIONS

7/26/12	REV. DRAINAGE FILL MATERIAL & DETAIL		ARKANSAS STATE HIGHWAY COMMISSION
12/15/11	REQUIRE WEEP HOLES IN BOX CULVERT WALLS		
5-25-06	REV. GEN. NOTES AND DETAILS FOR WEEP HOLES; BAR DIAGRAM		
11-16-01	ADDED WINGWALL DRAINAGE DETAIL/EDITED GEN. NOTES		
10-18-96	REV. ASTM REF. TO AASHTO & ADDED BAR DIAGRAM		
10-12-95	MOVED SOLID SODDING DETAIL TO RCB-2		
6-2-94	ADDED SOLID SODDING PLAN DETAIL		
8-5-93	REVISED PIN DIAMETER TO SPECS.		
8-15-91	DRAWN AND ISSUED		
DATE	REVISION	DATE FILMED	
			REINFORCED CONCRETE BOX CULVERT DETAILS
			STANDARD DRAWING RCB-1



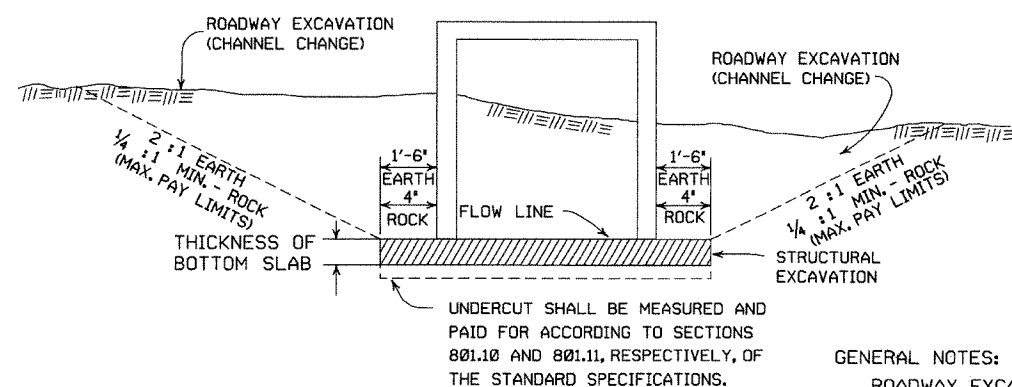
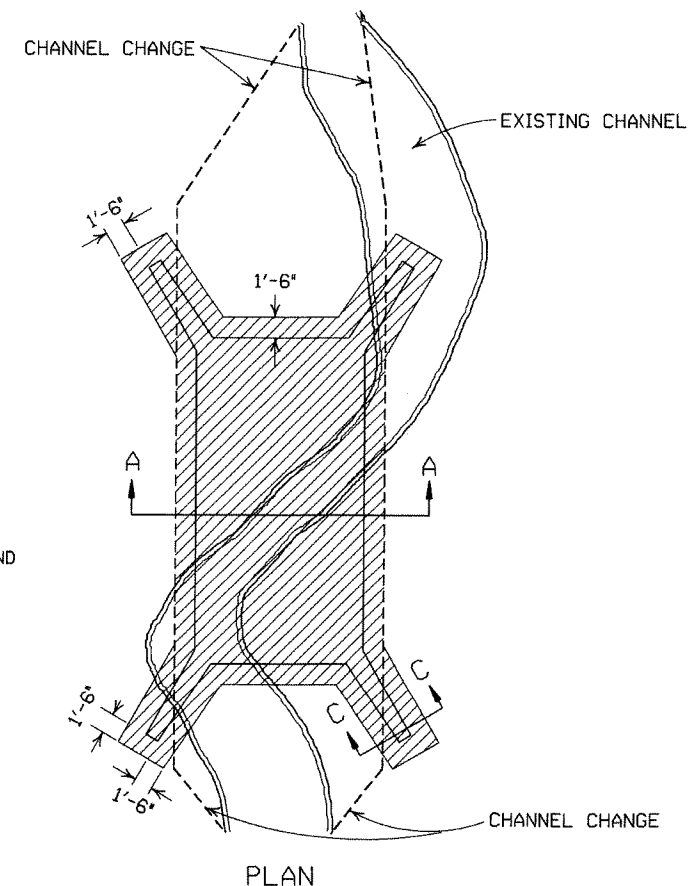
PARTIAL SECTION SHOWING SOLID SODDING AT HEADWALLS AND WING WALLS

NOTE: LENGTH MEASURED ALONG THE CENTER OF 2' STRIP OF SOLID SODDING.

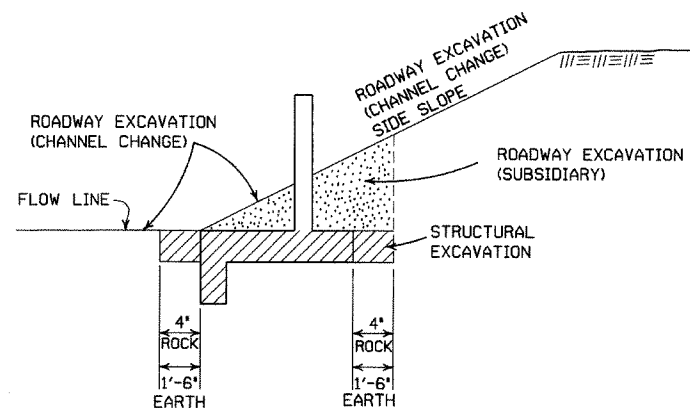


LONGITUDINAL SECTION

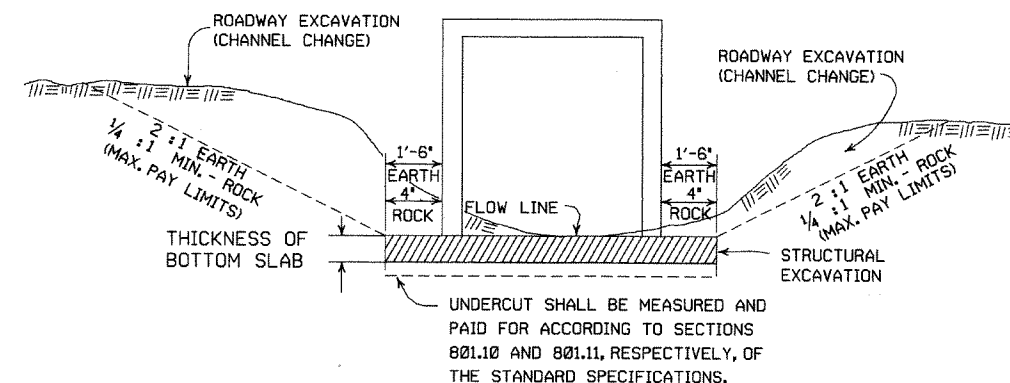
BACKFILL DETAILS FOR BOX CULVERT



DETAILS FOR NEW CHANNELS



SECTION C-C



DETAILS THROUGH EXISTING CHANNELS

GENERAL NOTES:

ROADWAY EXCAVATION (CHANNEL CHANGE) WILL BE PAID FOR AT R.C. BOX CULVERT LOCATIONS. IT WILL BE PAID TO THE LIMITS ACTUALLY CUT AND WILL BE CONFINED TO THAT PORTION OF THE INDICATED AREA THAT IS ABOVE THE FLOW LINE. ROADWAY EXCAVATION (CHANNEL CHANGE) SHALL BE MEASURED BY CROSS SECTIONS AND VOLUMES COMPUTED BY AVERAGE END AREA METHOD. ALL CHANNEL CHANGES SHALL BE BROUGHT TO GRADE PRIOR TO MAKING ANY EXCAVATION FOR STRUCTURES.

EXCAVATION FOR STRUCTURES WILL BE PAID FOR AT ALL R.C. BOX CULVERT LOCATIONS. IT WILL BE PAID TO THE LIMITS SHOWN AND SHALL BE CONFINED TO THAT PORTION OF THE INDICATED AREA THAT IS BELOW THE CHANNEL FLOW LINE.

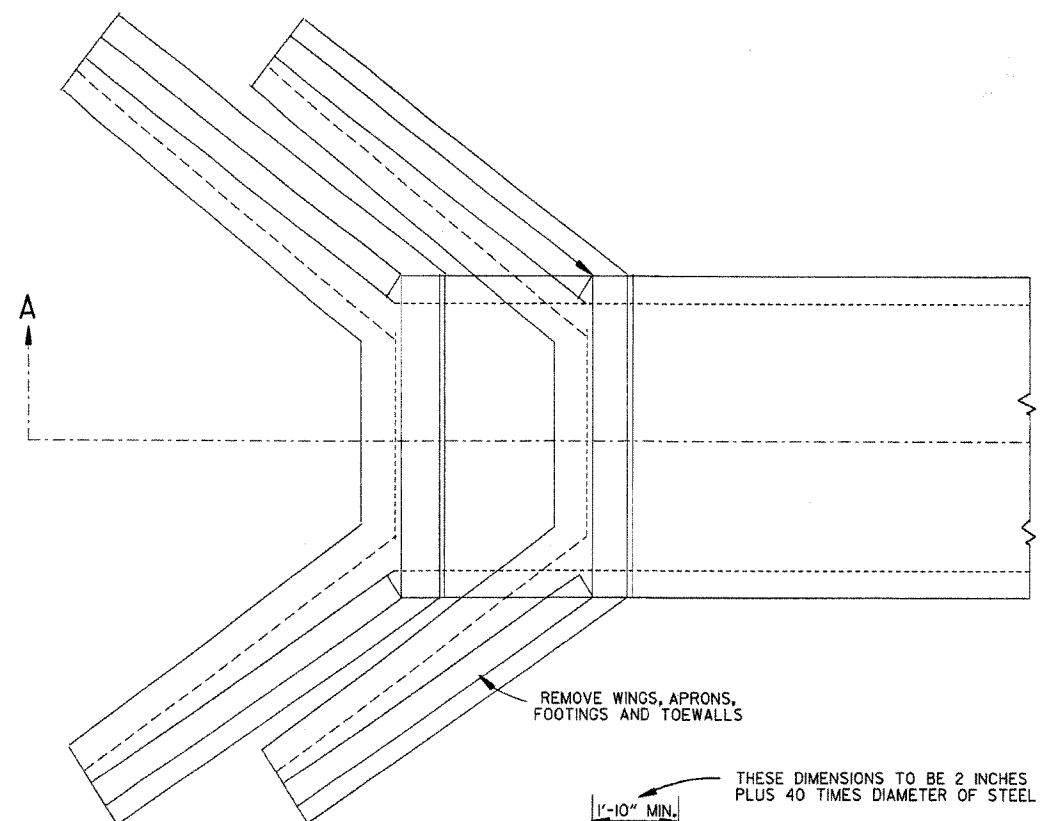
ROADWAY EXCAVATION SHOWN IN SECTION C-C ABOVE AS SUBSIDIARY WILL NOT BE MEASURED OR PAID FOR DIRECTLY, BUT PAYMENT WILL BE CONSIDERED TO BE INCLUDED IN THE VARIOUS ITEMS OF EXCAVATION.

11-20-03	REVISED SECTION A-A NOTE	
8-22-02	REVISED SECTION B-B NOTE	
10-12-95	COMBINED 1891B AND 1888A	
1-4-83	REVISED GENERAL NOTES AND ADDED MAXIMUM PAY LIMIT NOTES.	674-1-4-83
2-2-76	EXCAV. PAY LIMITS	917-2-2-76
10-2-72	REVISED AND REDRAWN	564-10-16-72
DATE	REVISION	FILMED

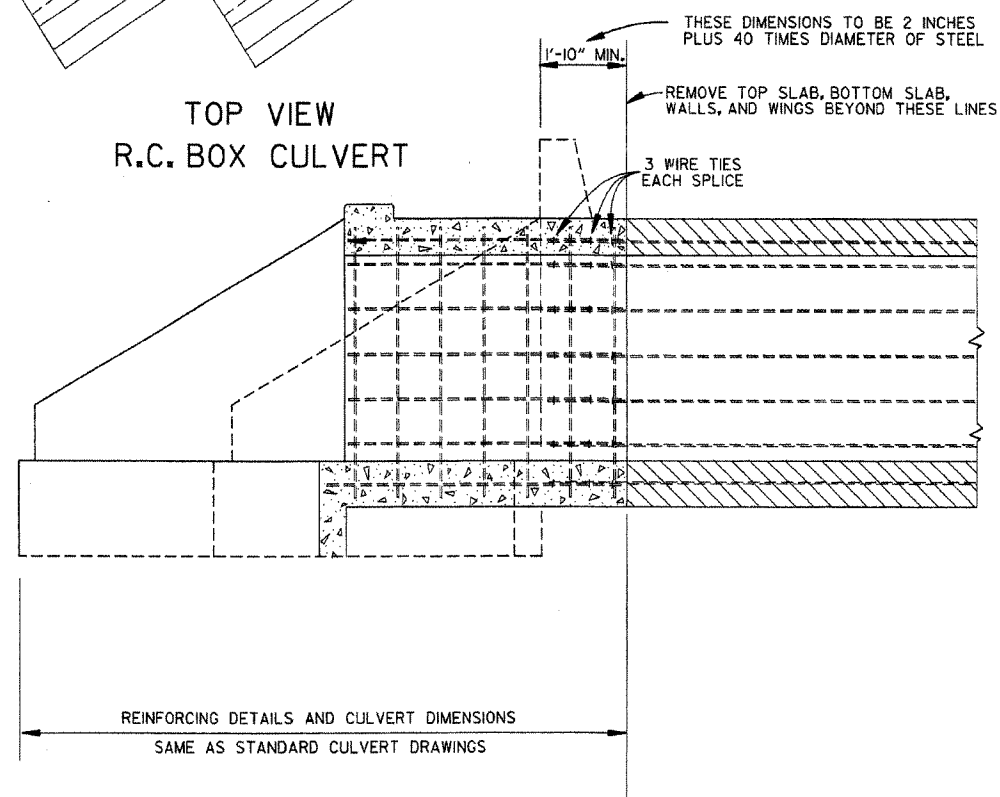
ARKANSAS STATE HIGHWAY COMMISSION

EXCAVATION PAY LIMITS, BACKFILL, & SOLID SODDING FOR BOX CULVERTS

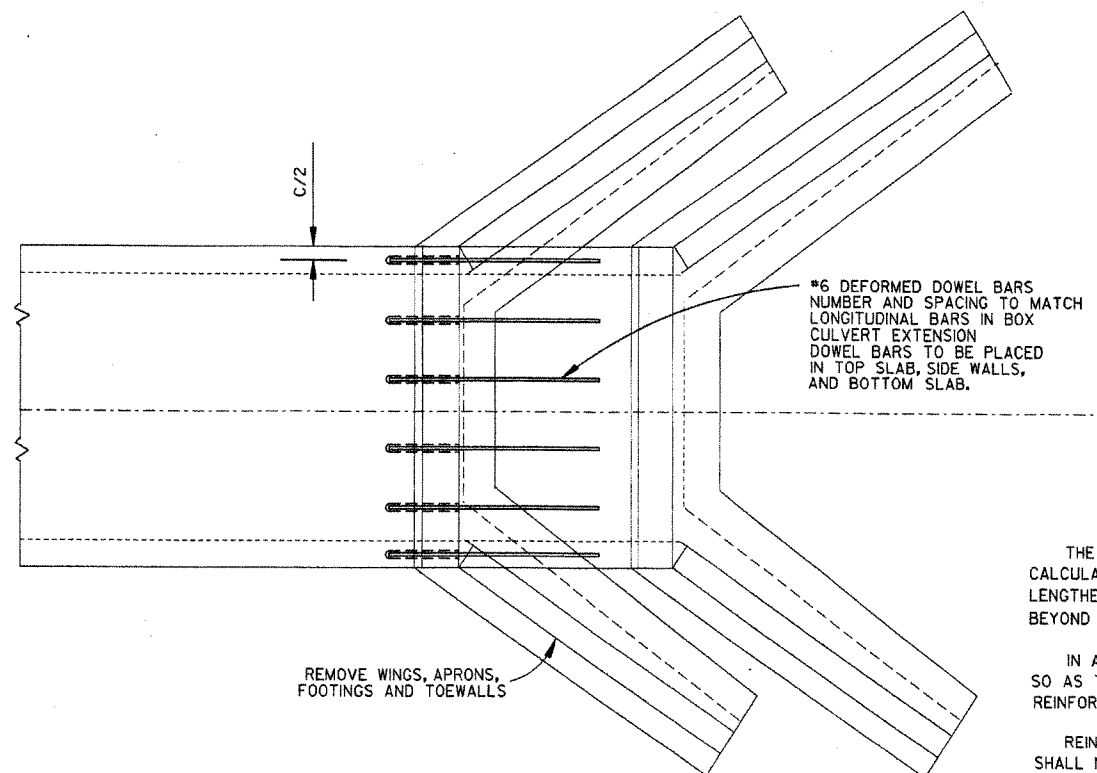
STANDARD DRAWING RCB-2



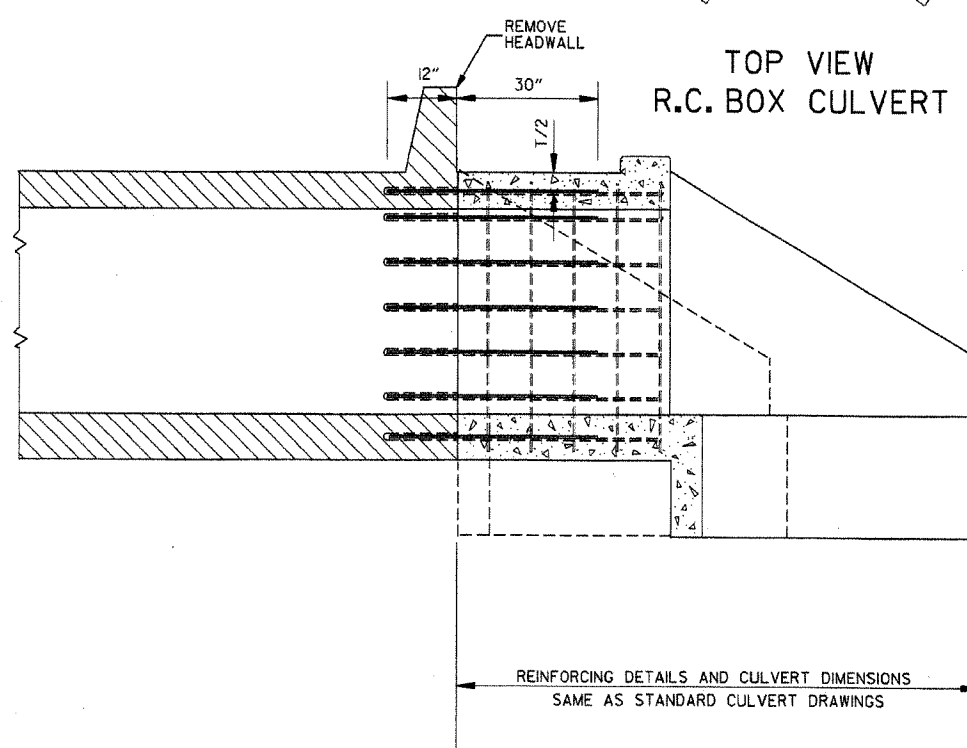
TOP VIEW
R.C. BOX CULVERT



SECTION A-A
METHOD 1



TOP VIEW
R.C. BOX CULVERT



SECTION A-A
METHOD 2

GENERAL NOTES

THE RESIDENT ENGINEER WILL MAKE INDIVIDUAL CALCULATIONS OF QUANTITIES FOR EACH STRUCTURE LENGTHENED, MAKING NO ALLOWANCE FOR OVERBREAKAGE BEYOND THE LINES INDICATED.

IN ALL INSTANCES CONCRETE SHALL BE REMOVED SO AS TO PERMIT FULL 40 DIAMETER SPLICE OF REINFORCING STEEL.

REINFORCING STEEL REMOVED FROM EXISTING STRUCTURE SHALL NOT BE REUSED IN CONSTRUCTING EXTENSION.

ON R.C. BOX CULVERTS THAT HAVE AN EXISTING CONCRETE APRON, THE CONCRETE APRON SHALL BE REMOVED WITH THE WINGS. THE COST OF REMOVING ALL OLD CONCRETE WILL BE INCLUDED IN THE PRICE BID PER CUBIC YARD FOR NEW CONCRETE OF THE CLASS SPECIFIED AND NO ADDITIONAL COMPENSATION WILL BE ALLOWED.

MATERIALS FOR SECURING DOWEL BARS SHALL MEET THE REQUIREMENTS OF SECTION 507.02 OF THE STANDARD SPECIFICATIONS.

DOWEL BARS SHALL BE INSTALLED AS FOLLOWS: THE DRILLING PROCEDURE SHALL BE APPROVED BY THE ENGINEER, THE FILLING SYSTEM SHALL BE APPROVED BY THE ENGINEER, AND SHALL BE AN INJECTION-TYPE SYSTEM WHICH WILL INSURE THAT SUFFICIENT MATERIAL IS INJECTED SO IT COMPLETELY SURROUNDS THE BARS AND FILLS THE HOLES.

THE CONTRACTOR SHALL HAVE THE OPTION OF USING EITHER METHOD 1 OR METHOD 2, REGARDLESS OF WHICH METHOD IS USED, PAY QUANTITIES WILL BE CALCULATED BASED ON METHOD 1.

NOTE:
NO PART OF THIS STANDARD IS TO BE USED FOR ANY DETAILS RELATIVE TO NEW CONSTRUCTION.
SEE STANDARD DRAWING LISTED IN TABULATION OF STRUCTURES FOR ALL NEW CONSTRUCTION DETAILS.

ARKANSAS STATE HIGHWAY COMMISSION		
METHOD OF EXTENDING EXISTING R.C. BOX CULVERTS		
STANDARD DRAWING RCB-3		
10-12-95	CHANGED DRAWING # FROM 144-A	
4-1-93	ADDED GENERAL NOTE	
10-1-92	ADDED ALT. METHOD OF EXTENSION	
11-30-89	REDRAWN	
1-4-83	ELIMINATED CONCRETE CLASS	
12-20-56	RETRACED	
DATE	REVISION	DATE FILM

SUPERELEVATION TABLE FOR ONE - WAY TRAFFIC

[illegible]

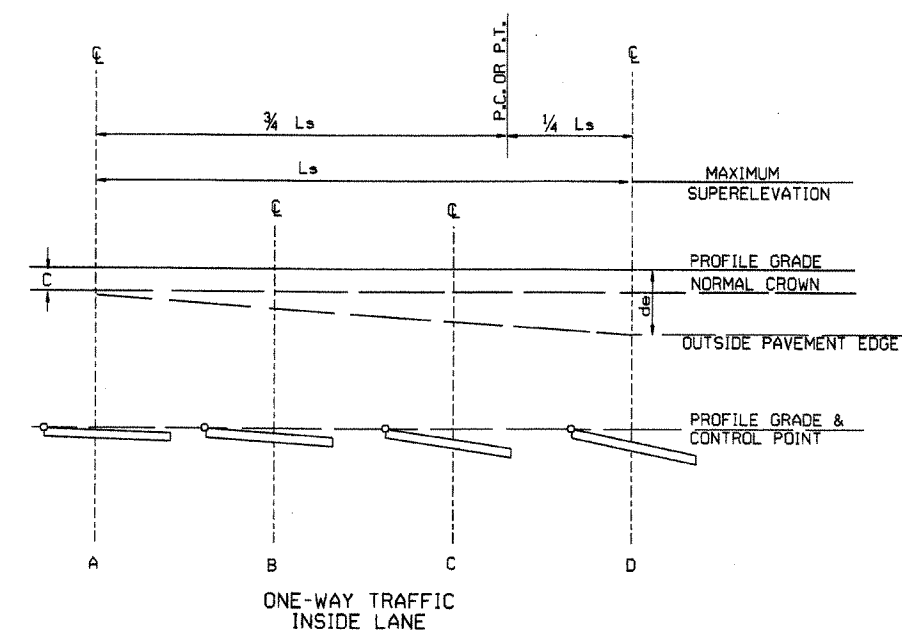
ABBREVIATIONS

NC - NORMAL CROWN
RC - REVERSE CROWN, SUPERELEVATION AT NORMAL CROWN SLOPE
S - SUPERELEVATION
L - DISTANCE FROM BEGINNING OF SUPERELEVATION TRANSITION
TO ANY POINT (FT.)
d - WIDTH OF PAVEMENT
e - MAXIMUM RATE OF SUPERELEVATION (FT. PER FT.)
Ls - LENGTH OF SUPERELEVATION TRANSITION (FT.)
C - NORMAL CROWN (FT.)

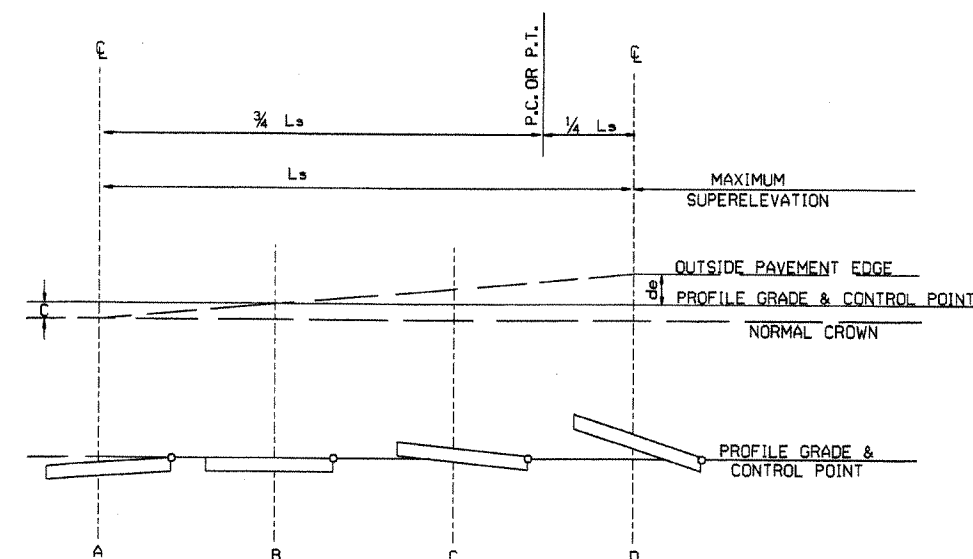
GENERAL NOTES

1. ON PAVEMENT WITH ONE-WAY TRAFFIC, THE SUPERELEVATION SHALL BE REVOLVED ON THE PROFILE GRADE POINT.
2. SUPERELEVATION VALUES SHOWN ON THE CROSS SECTIONS ARE VALUES (+) OR (-) TO BE ADDED OR SUBTRACTED FROM THE POINT OF CONTROL.
3. LENGTHS FOR L_s MAY BE ROUNDED IN MULTIPLES OF 25 FT. OR 50 FT. TO PERMIT SIMPLER CALCULATIONS.
4. MINIMUM L_s VALUES MAY BE USED FOR RAMPS; DESIRABLE VALUES SHALL APPLY TO MAIN LANES.
5. DIVIDED PAVEMENTS WIDER THAN 4 LANES SHALL HAVE ADDITIONAL TRANSITION LENGTHS AS FOLLOWS:

6 LANE DIVIDED-----+20%
8 LANE DIVIDED-----+50%



$$\text{SUPERELEVATION FORMULA} = S = - \frac{L(d-C)}{L_s} - C$$



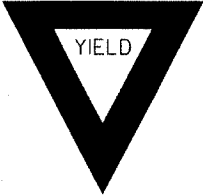
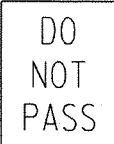
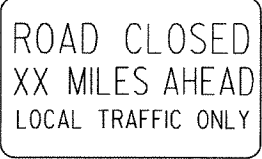
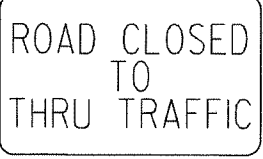
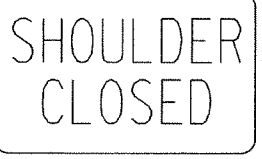
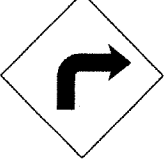
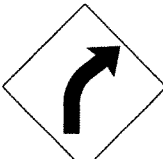
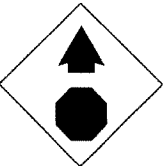
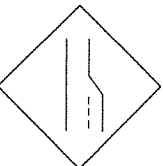
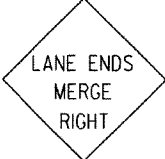
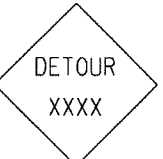
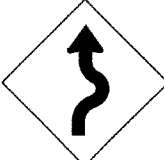
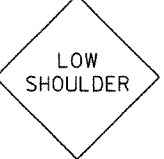
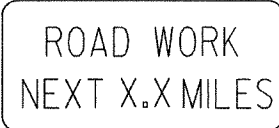
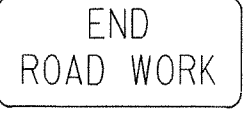
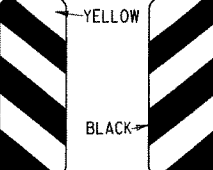
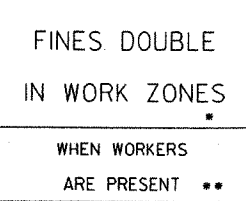
SUPERELEVATION FORMULA = $S = + \frac{L(d_e + C)}{L_s} - C$

01-09-87	ISSUED	578-1-15-87
DATE	REVISION	DATE FILMED

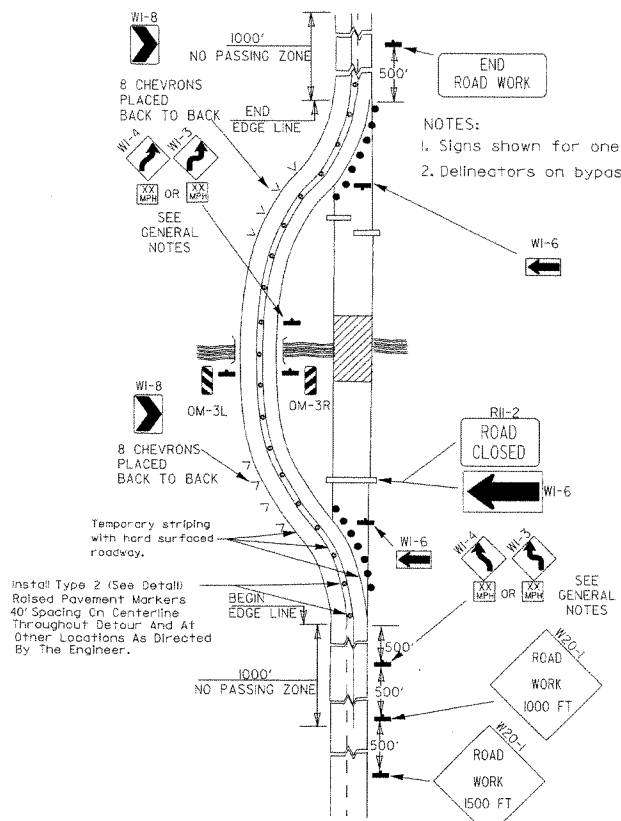
ARKANSAS STATE HIGHWAY COMMISSION

TABLES AND METHOD OF SUPERELEVATION FOR ONE-WAY TRAFFIC

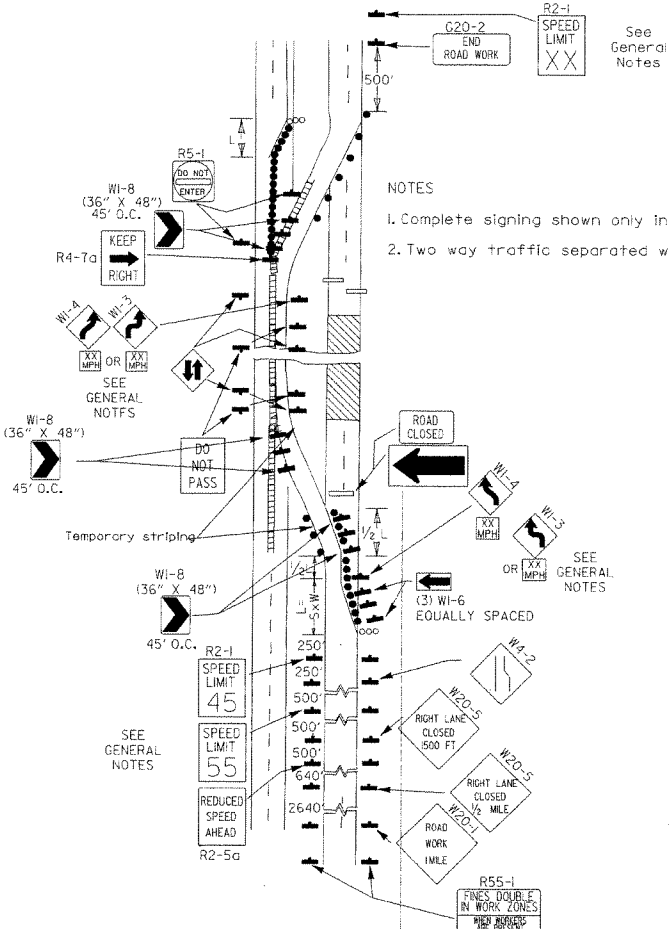
STANDARD DRAWING SE-1

RI-1  STANDARD 30"x30" EXPRESSWAY 36"x36" SPECIAL 48"x48"	RI-2  STD. 36"x36"x36" EXPWY. 48"x48"x48" FWY. 60"x60"x60"	R2-1  STD. 24"x30" EXPWY. 36"x48" FWY. 48"x60"	R2-5A  STD. 24"x30" EXPWY. 36"x48" FWY. 48"x60"	R2-5C  STD. 24"x30" EXPWY. 36"x48" FWY. 48"x60"	R4-1  STD. 24"x30" EXPWY. 36"x48" FWY. 48"x60"	R4-2  STD. 24"x30" EXPWY. 36"x48" FWY. 48"x60"	ADVANCE DISTANCES (XXXX) 500 FT 1/2 MILE 1000 FT 3/4 MILE 1500 FT 1 MILE AHEAD GENERAL NOTES: 1. ALL TRAFFIC CONTROL DEVICES USED ON ROAD CONSTRUCTION SHALL CONFORM TO THE MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES, LATEST EDITION, AND TO THE STANDARD HIGHWAY SIGNS, LATEST EDITION, OR AS APPROVED BY THE FEDERAL HIGHWAY ADMINISTRATION. 2. TRAFFIC CONTROL DEVICES SHALL BE SET UP JUST BEFORE THE START OF CONSTRUCTION OPERATIONS AND SHALL BE PROPERLY MAINTAINED DURING THE TIME SUCH CONDITIONS EXIST. THEY SHALL REMAIN IN PLACE ONLY AS LONG AS NEEDED AND REMOVED THEREAFTER. 3. EXISTING SIGNS AND CONSTRUCTION SIGNS SHALL BE KEPT IN PROPER POSITION, AND BE CLEAN AND LEGIBLE AT ALL TIMES. SIGNS THAT DO NOT APPLY TO EXISTING CONDITIONS SHALL BE REMOVED. SIGNS THAT ARE DAMAGED, DEFACED, OR THAT ACCUMULATE DIRT DURING CONSTRUCTION SHALL BE CLEANED, REPAIRED, OR REPLACED. 4. SIGNS ARE USUALLY MOUNTED ON A SINGLE POST, ALTHOUGH THOSE WIDER THAN 36" OR LARGER THAN 10 SO. FT. SHALL BE MOUNTED ON TWO POSTS OR ABOVE A TYPE III BARRICADE. 5. SIGN POSTS DIRECT BURIED IN SOIL SHALL BE 2 LB. MINIMUM CHANNEL POST OR 4"x4" WOOD POSTS. CHANNEL POSTS SHALL BE PAINTED GREEN. WOOD POSTS SHALL BE PAINTED WHITE. ALL POSTS SHALL BE NEATLY CONSTRUCTED, AND SHALL BE REPLUMBED, CLEANED, OR REPAIRED AS NEEDED FOR THE DURATION OF THE JOB. THERE SHALL NOT BE MORE THAN 2 POSTS IN A 7' PATH FOR WOOD OR CHANNEL POSTS. ANY CHANNEL POST SPLICE SHALL BE IN ACCORDANCE WITH STANDARD DRAWING TC-3. 6. POST MOUNTED SIGNS IN RURAL AREAS SHALL BE CONSTRUCTED WITH THE NEAR EDGE OF THE SIGN FROM 6 TO 12 FEET FROM THE PAVEMENT EDGE. SIGNS IN URBAN AREAS AND BARRICADE MOUNTED SIGNS SHALL BE MOUNTED A MINIMUM OF 2 FEET FROM THE PAVEMENT EDGE. 7. ALL POST AND BARRICADE MOUNTED SIGNS MOUNTED IN URBAN AREAS SHALL BE MOUNTED A MINIMUM DISTANCE OF 7' FROM THE BOTTOM OF THE SIGN TO THE ROADWAY SURFACE. ALL POST AND BARRICADE MOUNTED SIGNS MOUNTED IN RURAL AREAS SHALL BE MOUNTED A MINIMUM DISTANCE OF 7' FROM THE BOTTOM OF THE SIGN TO THE ROADWAY SURFACE, EXCEPT A MINIMUM OF 6' SHALL BE USED WHEN MOUNTING AN ADVISORY SIGN BELOW A WARNING SIGN. TEMPORARY SIGNS MAY BE MOUNTED ON PORTABLE SUPPORTS FOR INTERMEDIATE TERM STATIONARY WORK CONDITIONS. THE SIGNS MINIMUM MOUNTING HEIGHT SHALL BE 5'. RETROREFLECTIVE DEVICES SHALL BE USED. TEMPORARY SIGNS MAY BE MOUNTED ON PORTABLE SUPPORTS FOR SHORT-TERM, SHORT DURATION, AND MOBILE CONDITIONS. THEY SHALL BE NO LESS THAN ONE (1) FOOT ABOVE THE TRAVELED WAY. LONG-TERM STATIONARY SIGNS SHALL BE DIRECT BURIED IN SOIL, UNLESS CONDITIONS NECESSITATE THE USE OF PORTABLE SIGNS, OR AS APPROVED BY THE ENGINEER. CONCRETE PADS, CONCRETE OR ROCK BALLAST, OR OTHER SOLID MATERIALS SHALL NOT BE UTILIZED WITH PORTABLE SIGN SUPPORTS. 8. FLAGGERS SHALL USE REFLECTORIZED STOP-SLOW PADDLES. FLAGS MAY BE USED ONLY FOR EMERGENCY SITUATIONS. 9. MOST OF THE SIGNS SHOWN ARE ORIENTED TO THE RIGHT. HOWEVER, THIS DOES NOT PRECLUDE THE USE OF MIRROR IMAGES OF THESE SIGNS WHERE THE REVERSE ORIENTATION MIGHT BETTER CONVEY TO MOTORISTS THE PROPER DIRECTION OF MOVEMENT. 10. R55-1 SIGNS SHALL BE PLACED AT LEAST 1500' BUT NOT MORE THAN 1 MILE IN ADVANCE OF THE WORK ZONE. IF A SPEED LIMIT REDUCTION IS IN EFFECT, THE SIGN SHALL BE PLACED A MINIMUM OF 500' IN ADVANCE OF THE "REDUCED SPEED AHEAD" SIGN. • NOTE: SUPPORTS FOR SIGNS, BARRICADES, AND VERTICAL PANELS THAT ARE DIFFERENT FROM THE REQUIREMENTS SHOWN IN NOTES 4 & 5, BUT MEET THE REQUIREMENTS OF NCHRP-350 OR MANUAL FOR ASSESSING SAFETY HARDWARE (MASH), WILL BE ACCEPTED. COMPLIANCE WITH THE REQUIREMENTS OF NCHRP-350 OR MANUAL FOR ASSESSING SAFETY HARDWARE (MASH) IS REQUIRED FOR ALL PROJECTS.
R5-1  STD. 30"x30" EXPWY. 36"x36" SPECIAL 48"x48"	R11-2  48"x30"	R11-3A  60"x30"	R11-4  60"x30"	RSP-1  48"x30"	WI-1  STD. 36"x36" FWY. 48"x48"	WI-2  STD. 36"x36" FWY. 48"x48"	
WI-3  STD. 48"x48"	WI-4  STD. 48"x48"	WI-6  STD. 48"x24" SPECIAL 60"x30"	WI-8  STD. 18"x24" SPECIAL 24"x30" EXPWY. 30"x36" FWY. 36"x48"	W3-1  STD. 36"x36" SPECIAL 48"x48"	W3-2  STD. 36"x36" SPECIAL 48"x48"	W4-2  STD. 36"x36" FWY. 48"x48"	
W5-1  STD. 36"x36" SPECIAL 48"x48"	W6-3  EXPWY. 36"x36" SPECIAL 48"x48"	W8-7  EXPWY. 36"x36" FWY. 48"x48"	W9-2  STD. 36"x36" FWY. 48"x48"	W13-1  STD. 24"x24"	W20-1  STD. 48"x48"	W20-2  STD. 48"x48"	W20-3  STD. 48"x48"
W20-4  STD. 48"x48"	W20-5  STD. 48"x48"	W20-7a  500 FEET 24" W16-2 STD. 36"x36" FWY. 48"x48"	W21-2  STD. 30"x30" SPECIAL 36"x36"	W21-5  STD. 30"x30" SPECIAL 36"x36"	W24-1  STD. 36"x36"	W1-4b  STD. 48"x48"	R56-1  STD. 18"x18"
W8-11  STD. 36"x36" FWY. 48"x48"	W8-9  STD. 36"x36" FWY. 48"x48"	G20-1  60"x24"	G20-2  48"x24"	OM-3L OM-3R  12"x36"	M4-9  STD. 30"x24" SPECIAL 48"x36" SPECIAL 60"x48"	M4-10  48"x18"	R55-1  WHEN WORKERS ARE PRESENT ** 36"x60" • USE 6" C LETTERS ** USE 4" D LETTERS

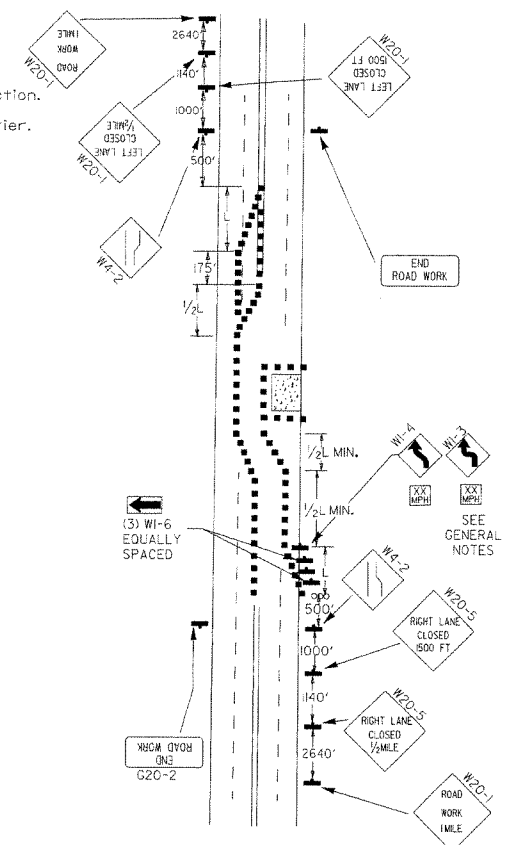
12-15-8	REVISED W24-1	
11-17-10	DELETED W8-9a & ADDED W8-9	
10-15-09	ADDED REFERENCE TO MASH & ADDED SIGN W24-1	
4-17-08	REVISED SIGN DESIGNATIONS	
11-18-04	REVISED NOTES	
10-9-03	REVISED NOTE 1	
11-16-01	REVISED NOTE 7	
9-28-00	REVISED NOTE	
11-18-98	ADDED NOTE	
6-26-97	REVISED NOTE 5	
4-03-97	REVISED NOTE 5	
10-18-96	ADDED CONTROLLED ACCESS HWY. SIGN & TO NOTE 7	
10-12-95	ADDED R55-1	
6-8-95	REVISED TO CORRECT SIGN ILLUSTRATIONS	6-8-95
2-2-95	REVISED PER PART VI, MUTCD SEPT. 3, 1993	
8-15-91	DRAWN AND PLACED IN USE	
DATE	REVISION	FILMED



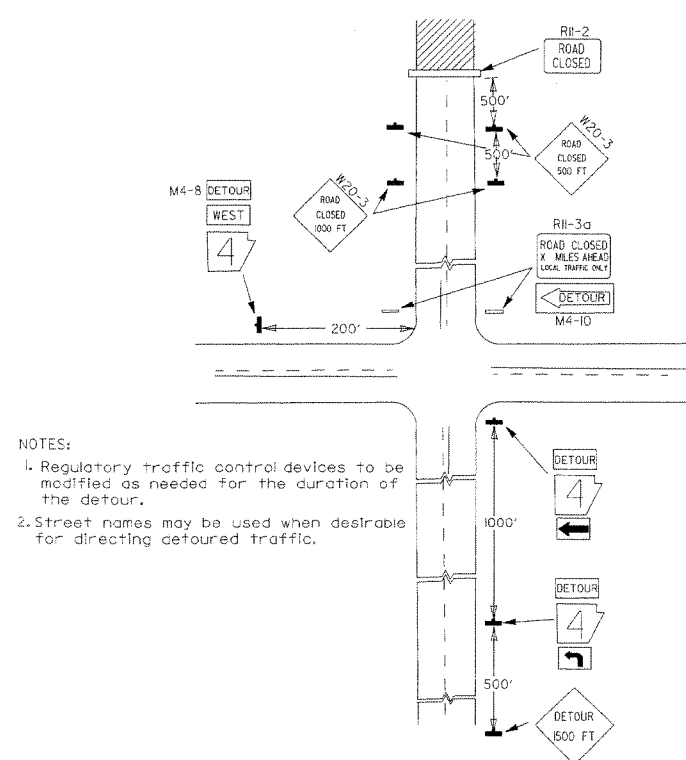
(A) Typical application of traffic control devices on a 2-lane highway where the entire roadway is closed and a bypass detour is provided.



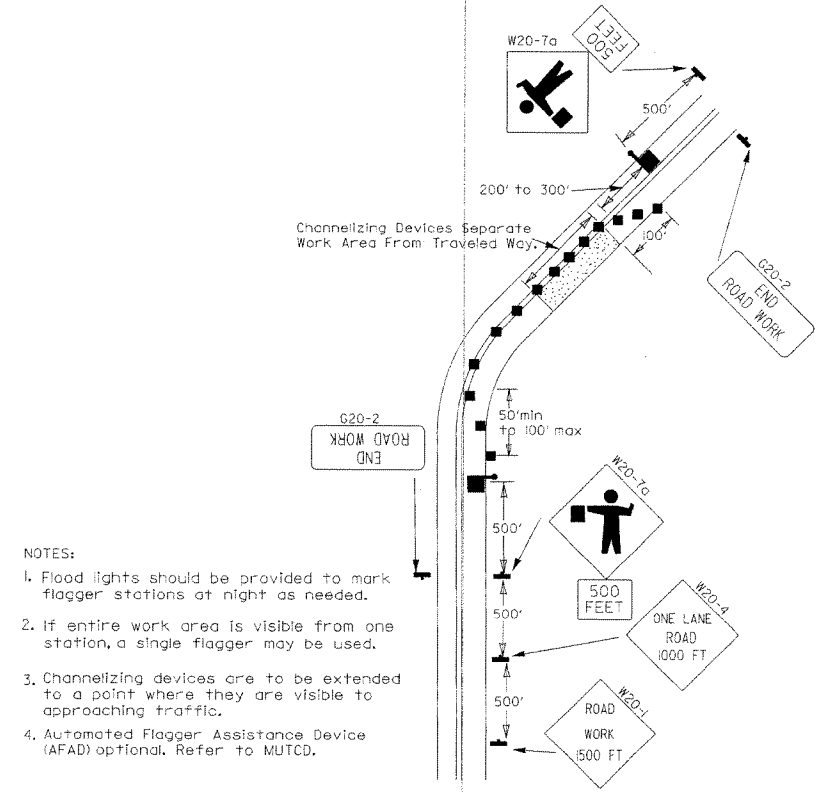
(B) Typical application - 4-lane divided roadway where one roadway is closed.



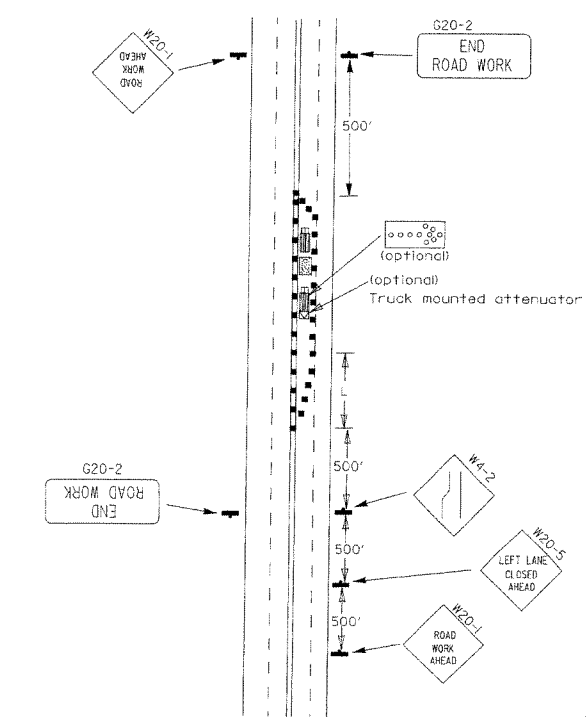
(C) Typical application - 4-lane undivided roadway where half of the roadway is closed.



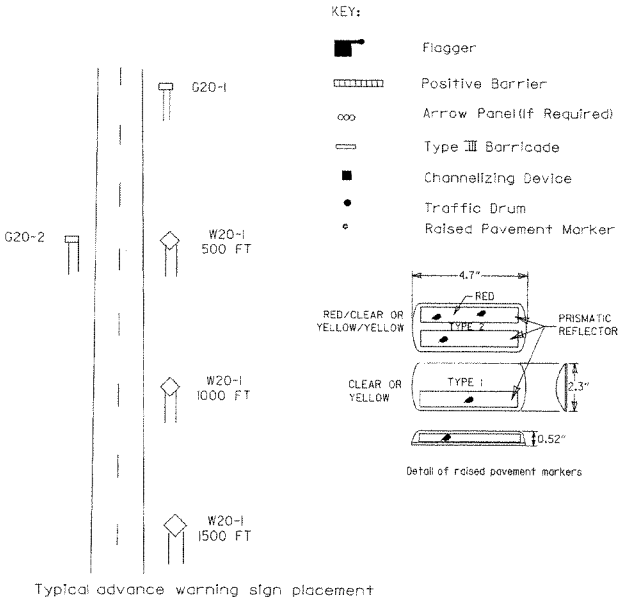
(D) Typical application - roadway closed beyond detour point.



(E) Typical application of traffic control devices on 2-lane highway where one lane is closed and flagging is provided.



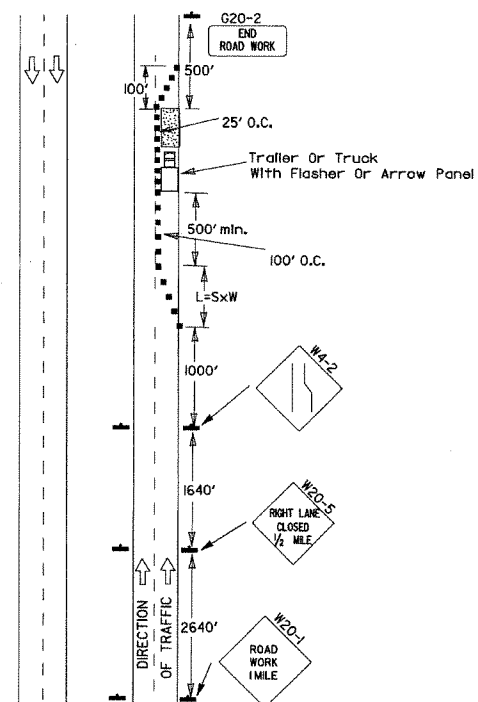
(F) Typical application - 4-lane undivided roadway with inside lane closed.



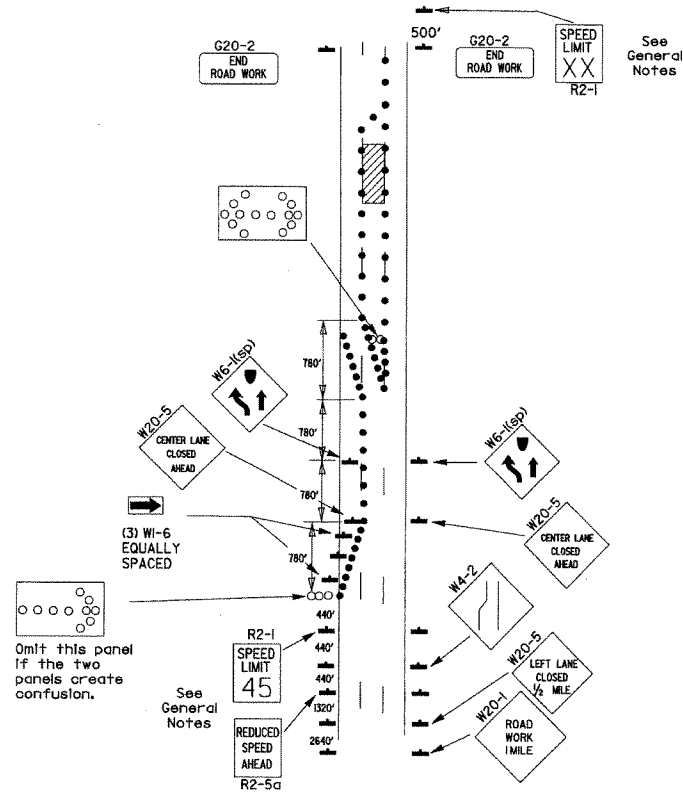
Taper formulae:
 $L = S \times W$ for speeds of 45mph or more.
 $L = \frac{WS^2}{60}$ for speeds of 40mph or less.
Where:
L = Minimum length of taper.
S = Numerical value of posted speed limit prior to work or 85th percentile speed.
W = Width of offset.

GENERAL NOTES:
1. Advisory speed posted on W1-3 or W1-4 curve warning signs to be determined at site. Use W1-4 when speed is greater than 30mph and W1-3 when 30mph or less.
2. When the existing speed limit is 55mph and the plans require a speed limit of 45mph, the R2-1(55) shall be omitted and the R2-5A shall be installed at that location. Additional R2-1(45) speed limit signs shall be installed at a maximum of 1/2 mile intervals. At the end of the work area a R2-1(45) shall be installed to match original speed limit.
3. When the existing speed limit is 65mph and the plans require a speed limit of 55mph, the R2-1(65) shall be omitted. Additional R2-1(55) speed limit signs shall be installed at a maximum of 1/2 mile intervals. At the end of the work area a R2-1(55) shall be installed to match original speed limit.
4. The maximum spacing between channelizing devices in a taper should be approximately equal in feet to the speed limit. Beyond the taper, maximum spacing shall be two times the speed limit, or as directed by the Engineer.
5. Warning lights and/or flags may be mounted to signs or channelizing devices at night as needed.
6. Pavement markings no longer applicable which might create confusion in the minds of vehicle operators shall be removed or obliterated as soon as practicable.
7. Trailer mounted devices such as arrow panels and portable changeable message signs shall be delineated by affixing conspicuity material in a continuous line on the face of the trailer. When placed on or adjacent to the shoulder and not behind a positive barrier, these devices shall be delineated by placing five (5) traffic drums, equally spaced along the traffic side of the device.

3-11-10	ADDED (AFAD)	
11-20-08	REVISED SIGN DESIGNATIONS	
11-18-04	ADDED GENERAL NOTE	
10-18-96	ADDED R55-1	
4-26-96	CORRECTED (a) BEHIND G20-2	
6-8-95	CORRECTED SIGN IDENT. ON W1-4A	6-8-95
2-2-95	REVISED PER PART VI, MUTCD, SEPT. 3, 1993	
8-5-91	DRAWN AND PLACED IN USE	
DATE	REVISION	FILMED



(A) Typical application - daytime maintenance operations of short duration on a 4-lane divided roadway where half of the roadway is closed.



(B) Typical application - 3-lane oneway roadway where center lane is closed.

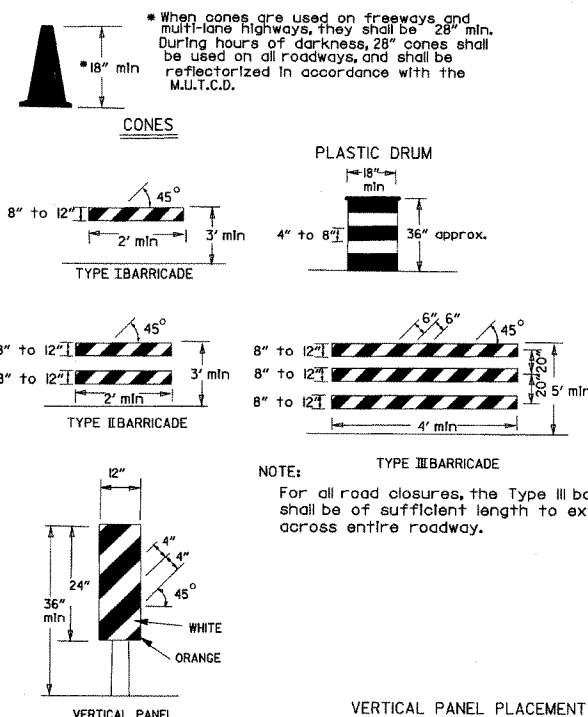
KEY:

- Arrow Panel (if Required)
- Channelizing Device
- Traffic drum

GENERAL NOTES:

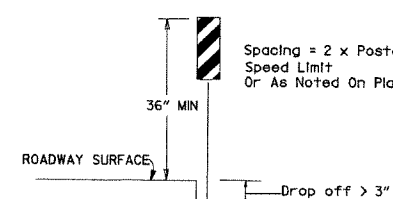
- A speed limit reduction may be implemented ONLY when designated in the plan or when recommended by the Roadway Design Division.
- When the existing speed limit is 55mph and the plans require a speed limit of 45mph, the R2-1(55) shall be omitted and the R2-5a shall be installed at that location. Additional R2-145mph speed limit signs shall be installed at a maximum of 1 mile intervals. At the end of the work area a R2-1(XX) shall be installed to match original speed limit.
- When the existing speed limit is 65mph and the plans require a speed limit of 55mph, the R2-1(45) shall be omitted. Additional R2-155mph speed limit signs shall be installed at a maximum of 1 mile intervals. At the end of the work area a R2-1(XX) shall be installed to match original speed limit.
- The maximum spacing between channelizing devices in a taper should be approximately equal in feet to the speed limit. Beyond the taper, maximum spacing shall be two times the speed limit or as directed by the Engineer.
- Warning lights and/or flags may be mounted to signs or channelizing devices at night as needed.
- Pavement markings no longer applicable which might create confusion in the minds of vehicle operators shall be removed or obliterated as soon as practicable.
- The G20-1 sign will be required on jobs of over two miles in length. When the lane closure is not at the beginning of the project, the G20-1 sign shall be erected 125' in advance of the job limit. Additional W20-1 (1 MILE) signs are not required in advance of lane closures that begin inside the project limits.
- Flaggers shall use STOP/SLOW paddles for controlling traffic through work zones. Flags may be used only for emergency situations.
- All plastic drums and cones shall meet the requirements of NCHRP-350 or Manual for Assessing Safety Hardware (MASH).
- Trailer mounted devices such as arrow panels and portable changeable message signs shall be delineated by affixing conspicuity material in a continuous line on the face of the trailer. When placed on or adjacent to the shoulder and not behind a positive barrier, these devices shall be delineated by placing five (5) traffic drums, equally spaced along the traffic side of the device.

Channelizing devices



NOTE:
For all road closures, the Type II barricades shall be of sufficient length to extend across entire roadway.

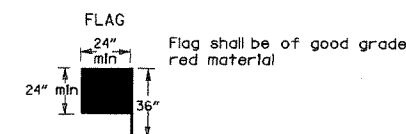
VERTICAL PANEL PLACEMENT



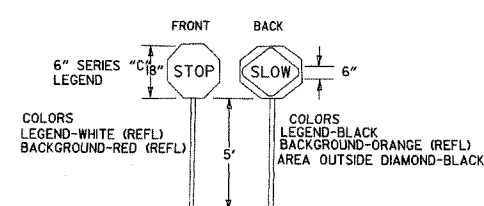
TRAFFIC CONTROL DEVICES FOR VERTICAL PAVEMENT DIFFERENTIALS

VERTICAL DIFFERENTIAL	LOCATIONS	TRAFFIC CONTROL
1" to 3"	Centerline, lane lines	W8-11
1" to 3"	Edge of shoulder	W8-9
Greater than 3"	Lane lines	Standard lane closure required
Greater than 3"	Edge of traveled lane	*RSP-1 and vertical panels, drums or concrete barrier
Greater than 3"	Edge of shoulder	*Vertical panels, drums or concrete barrier

* When shown on the plans concrete barrier will be used.
When the shoulder area is used as part of the traveled lane and there is insufficient width to place drums on the remaining shoulder width, then vertical panels shall be used.

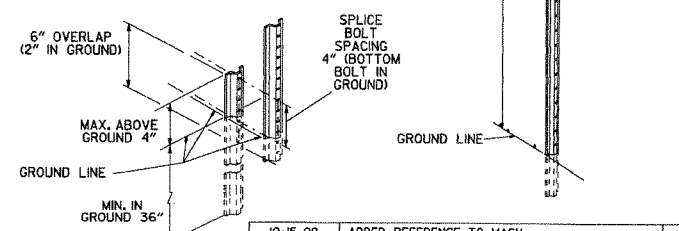


STOP SLOW PADDLE



DETAIL OF SPLICES

NOTES:
USE SPLICES ONLY WHEN NECESSARY FOR INSTALLATION. TYPICAL INSTALLATION SHOULD HAVE NO SPLICES (SEE STD. DRAWING NO. SHS-2)
NORMAL INSTALLATIONS WILL REQUIRE 1/4" DIA. BOLTS TO MOUNT SIGNS TO POST AND 5/16" DIA. BOLTS TO ASSEMBLE THE VARIOUS POST SUPPORTS. EACH OF THESE BOLTS SHALL BE CARRIAGE BOLTS.
SIGN POSTS SHALL BE PAINTED GREEN; SIGNS SHALL NOT BE PAINTED, AND ALL SIGN POSTS SHALL BE PLUMB.



10-15-09	ADDED REFERENCE TO MASH	
11-20-08	REVISED SIGN DESIGNATIONS	
11-18-04	ADDED NOTE	
10-1-98	ADDED NOTE	
4-03-97	ADDED (SP) TO W6-1 & REVISED TRAFFIC CONTROL DEVICES NOTE	
10-18-96	ADDED R55-1	
10-12-95	MOVED UPPER SPLICE	
6-8-95	REVISED SPLICE DETAIL, TEXT	6-8-95
2-2-95	REVISED PER PART VI, MUTCD, SEPT. 3, 1993	
8-15-91	DRAWN AND PLACED IN USE	
DATE	REVISION	FILMED

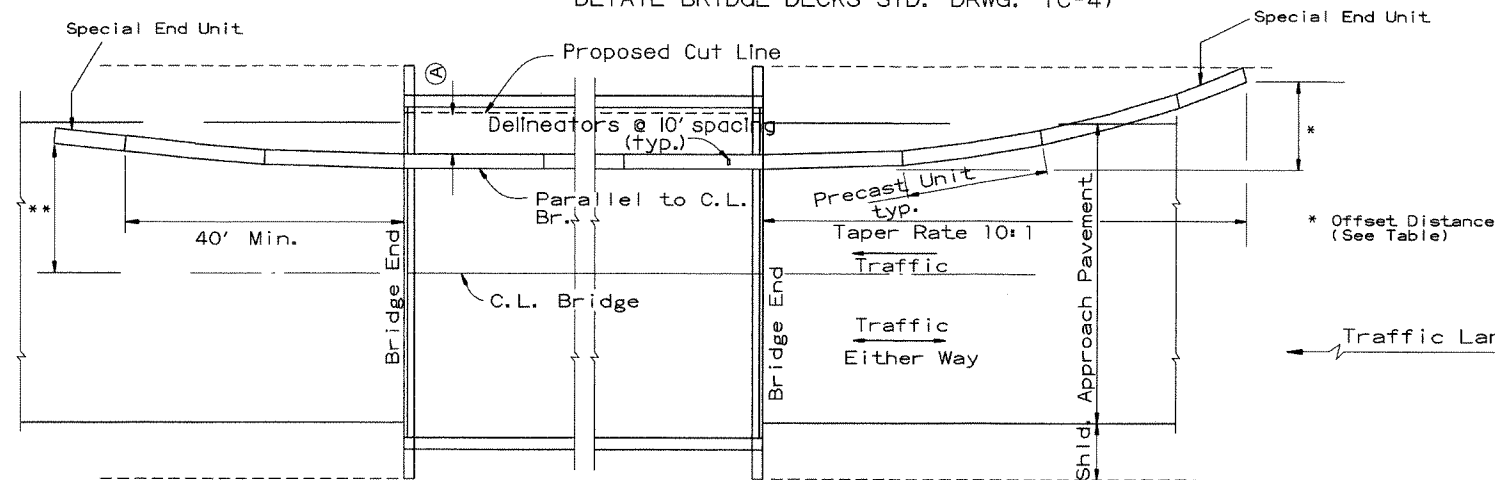
ARKANSAS STATE HIGHWAY COMMISSION
STANDARD TRAFFIC CONTROLS
FOR HIGHWAY CONSTRUCTION

STANDARD DRAWING TC-3

(C) Typical application - construction operations of intermediate to long term duration on a 4-lane divided roadway where half of the roadway is closed.

(D) Typical application - closing multiple lanes of a multi-lane highway.

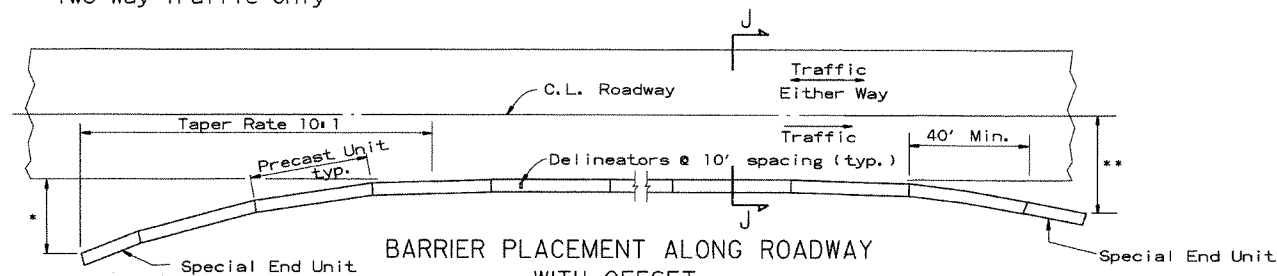
(A) 4 feet or greater preferred. If less than 4 feet, Precast Units shall be connected to slab (SEE BARRIER STABILIZATION DETAIL-BRIDGE DECKS STD. DRWG. TC-4)



BARRIER PLACEMENT ALONG BRIDGE WITH OFFSET

No Scale

** Offset Distance for Two Way Traffic Only



BARRIER PLACEMENT ALONG ROADWAY WITH OFFSET

No Scale

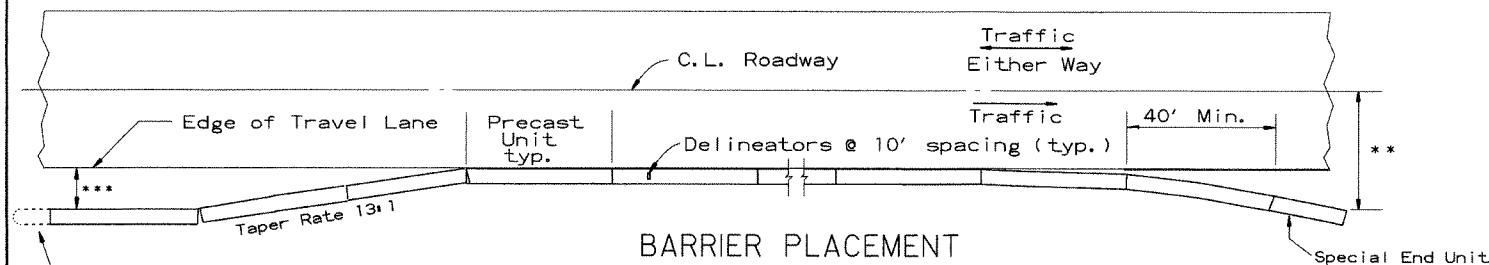
* Offset Distance (See Table)

** Offset Distance For Two Way Traffic Only

Offset Distance Table

Speed (MPH)	Offset Distance (FT.)
≤ 45	12
> 45	18

If offset distance is not attainable, then see 'Barrier Placement With Attenuator' Detail shown below.

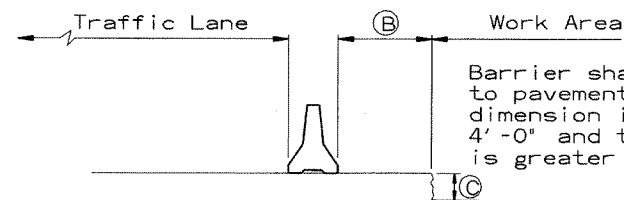


BARRIER PLACEMENT WITH ATTENUATOR

No Scale

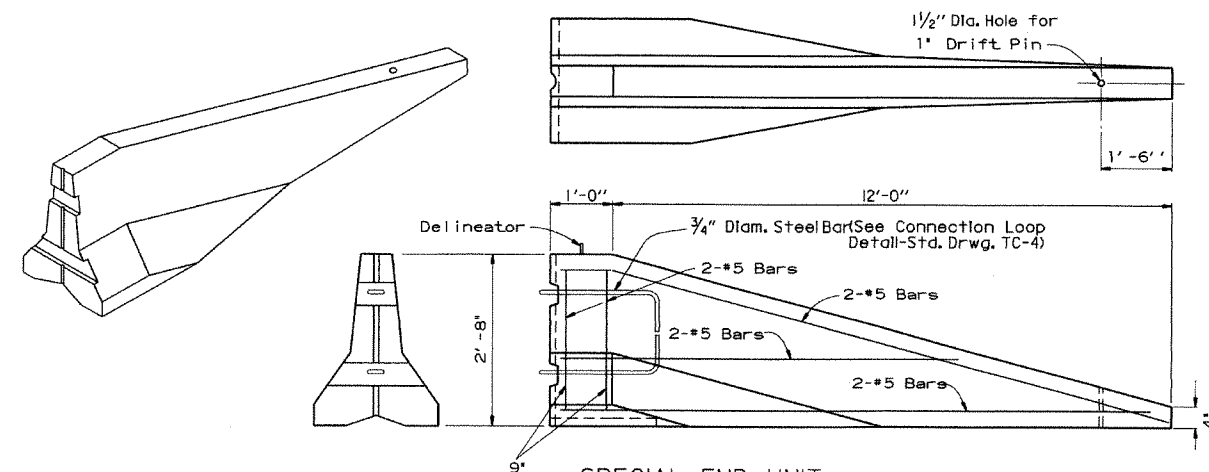
***Min. 3'-0" From Edge of Travel Lane to Nearest Edge of Attenuator

** Offset Distance For Two Way Traffic Only



SECTION J-J

No Scale



SPECIAL END UNIT

No Scale

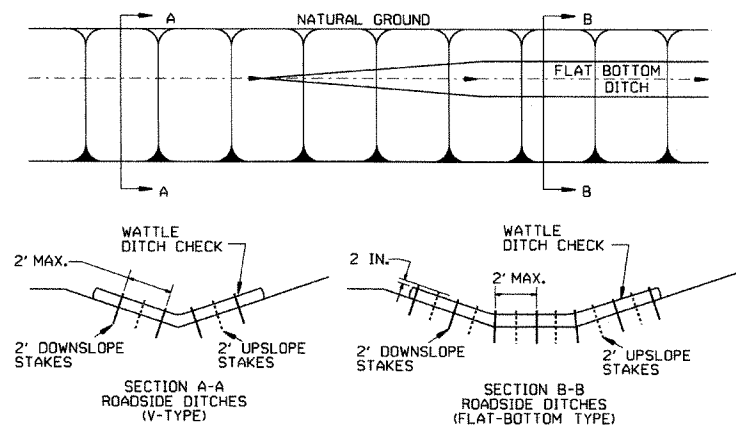
General Notes

When shown on the Plans, the ends of the Temporary Precast Concrete Barrier shall be protected with an NCHRP-350 or Manual For Assessing Safety Hardware (MASH) approved Crash Cushion. Payment for Crash Cushions shall be made under the item of "Temporary Impact Attenuation Barrier."

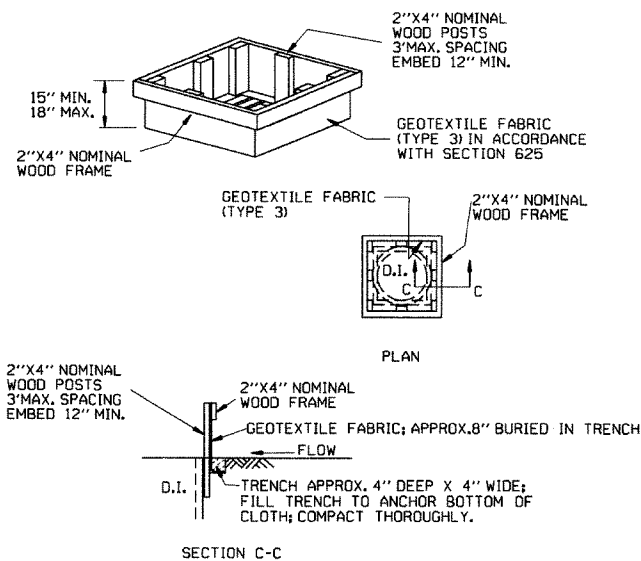
ARKANSAS STATE HIGHWAY COMMISSION		
STANDARD TRAFFIC CONTROLS FOR HIGHWAY CONSTRUCTION - TEMPORARY PRECAST BARRIER		
STANDARD DRAWING TC-5		
10-15-09	ADDED REFERENCE TO MASH	
5-25-06	REVISED BARRIER PLACEMENT	
8-22-02	ISSUED NEW DRAWING	
DATE	REVISION	FILMED

GENERAL NOTES

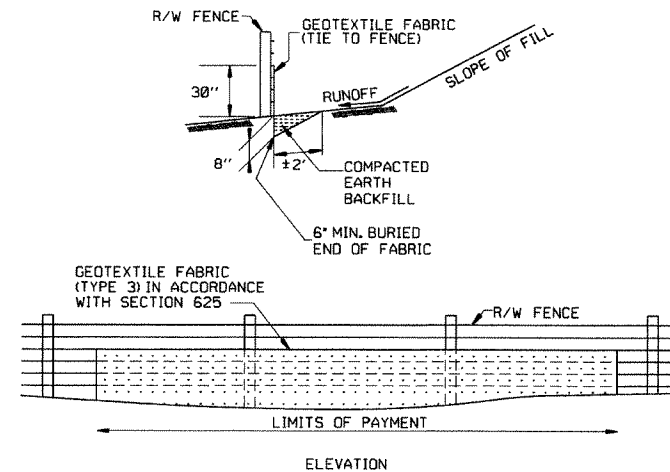
INSTALL A MINIMUM OF 2 UPSLOPE STAKES AND 4 DOWNSLOPE STAKES AT AN ANGLE TO WEDGE WATTLE TO BOTTOM OF DITCH.



WATTLE DITCH CHECK (E-1)



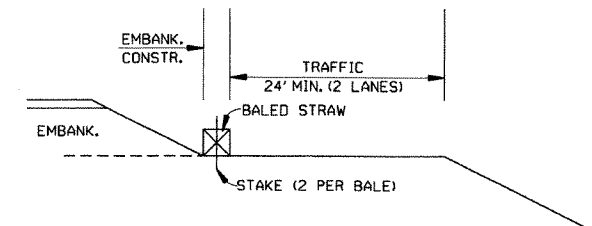
DROP INLET SILT FENCE (E-7)



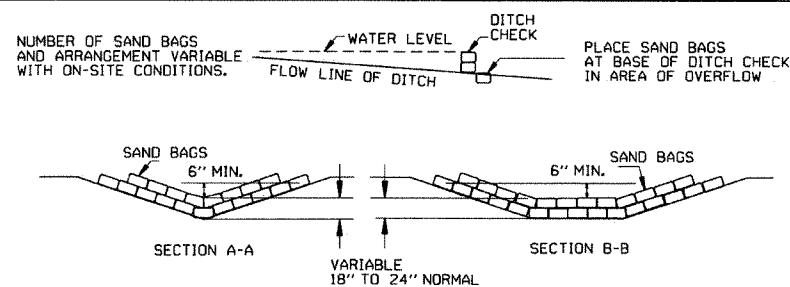
SILT FENCE ON R/W FENCE (E-4)

GENERAL NOTES
GEOTEXTILE FABRIC SHALL BE SPLICED TOGETHER WITH A SEWN SEAM ONLY AT A SUPPORT POST, OR TWO SECTIONS OF FENCE MAY BE OVERLAPPED INSTEAD. PAYMENT OF ADDITIONAL MATERIAL FOR OVERLAP WILL NOT BE MADE.

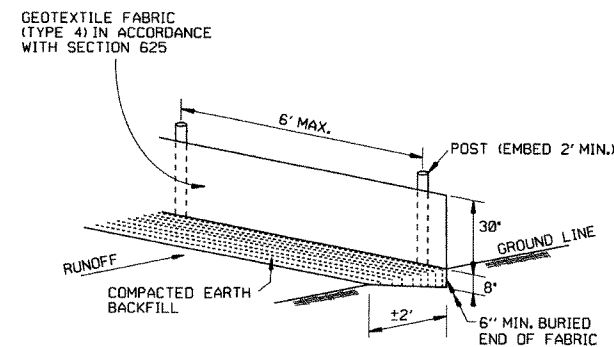
- GENERAL NOTES
1. STRAW BALES SHALL BE INSTALLED SO THAT THE BINDINGS ARE ORIENTED AROUND THE SIDES RATHER THAN ALONG THE TOPS AND BOTTOMS OF THE BALES. THE BALES SHALL BE A MINIMUM OF 30 INCHES IN LENGTH.
 2. NO GAPS SHALL BE LEFT BETWEEN BALES.
 3. BALED STRAW FILTER BARRIERS COMPLETED AND ACCEPTED WILL BE MEASURED BY THE BALE IN PLACE AS AUTHORIZED BY THE ENGINEER AND WILL BE PAID FOR AT THE CONTRACT UNIT PRICE BID PER BALE FOR BALED STRAW DITCH CHECKS.



BALED STRAW FILTER BARRIER (E-2)

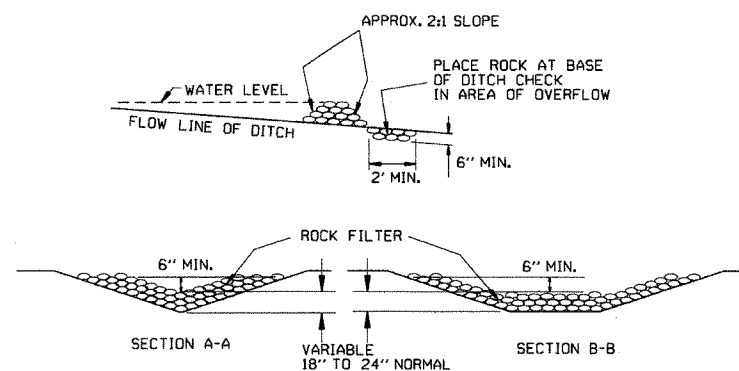


SAND BAG DITCH CHECK (E-5)



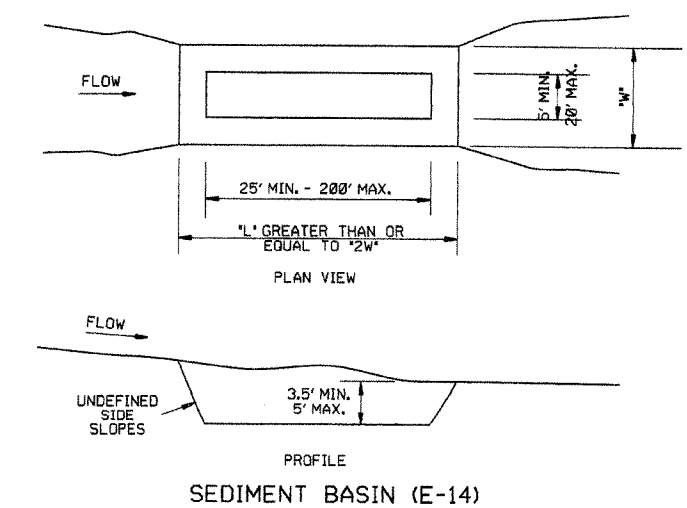
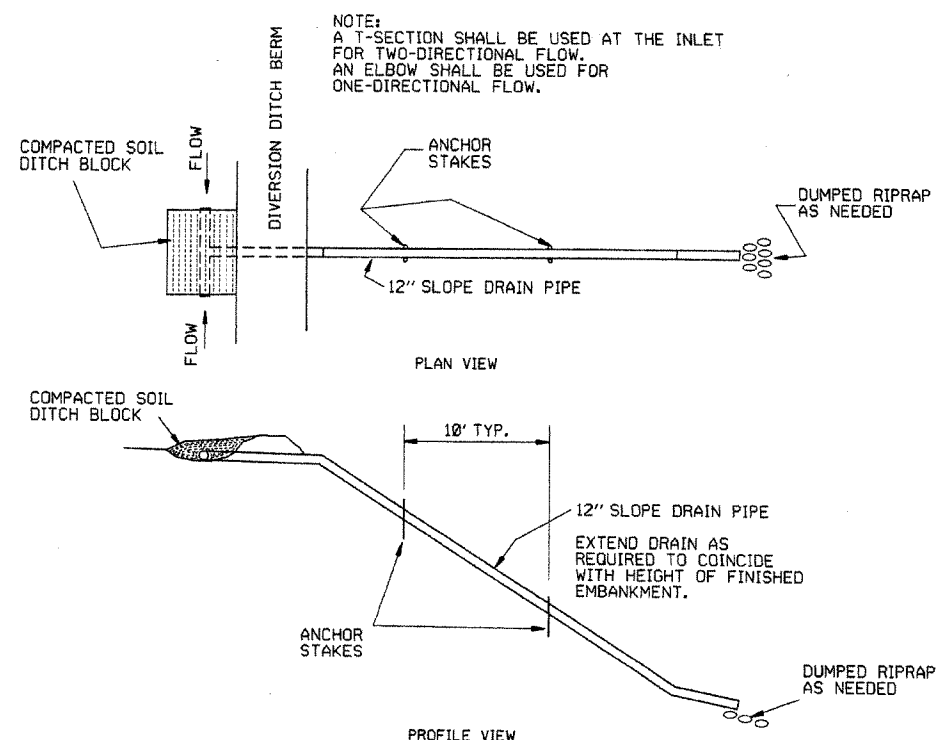
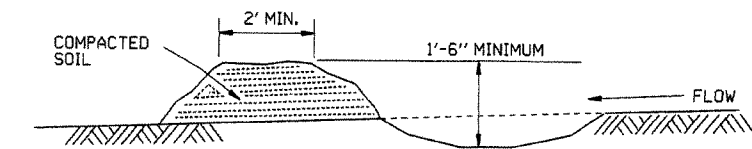
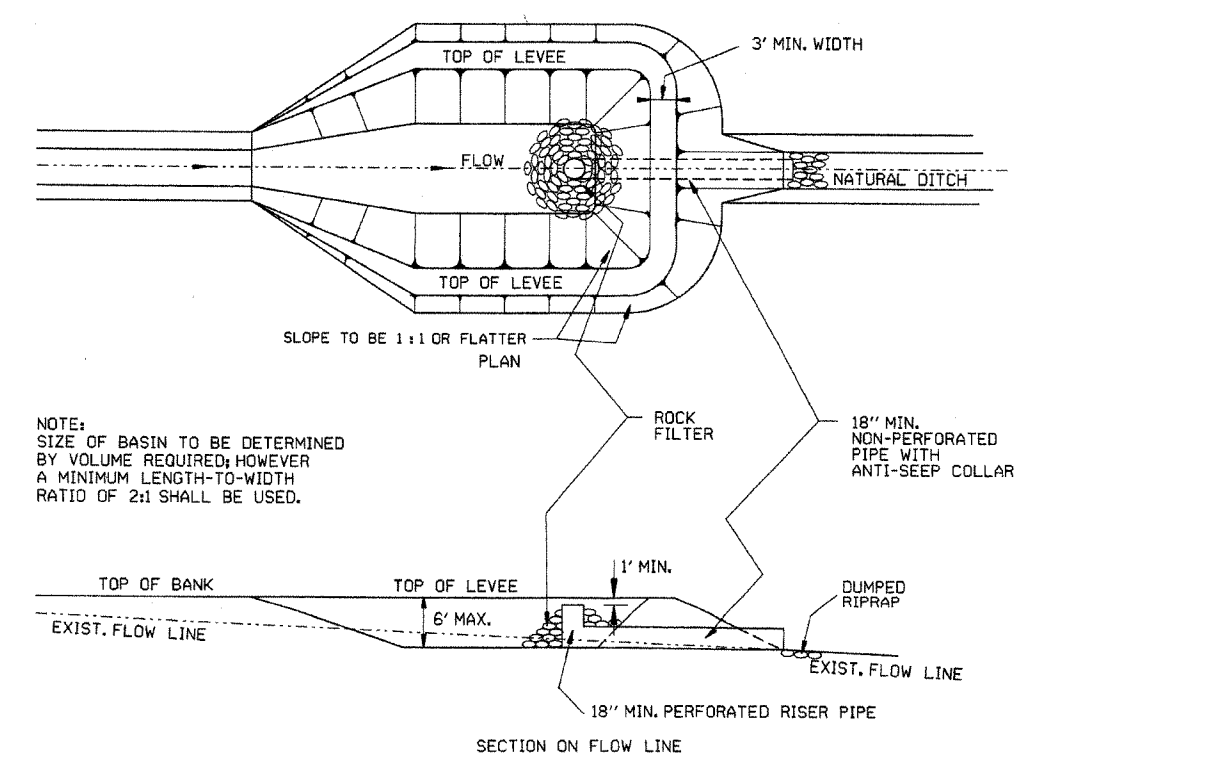
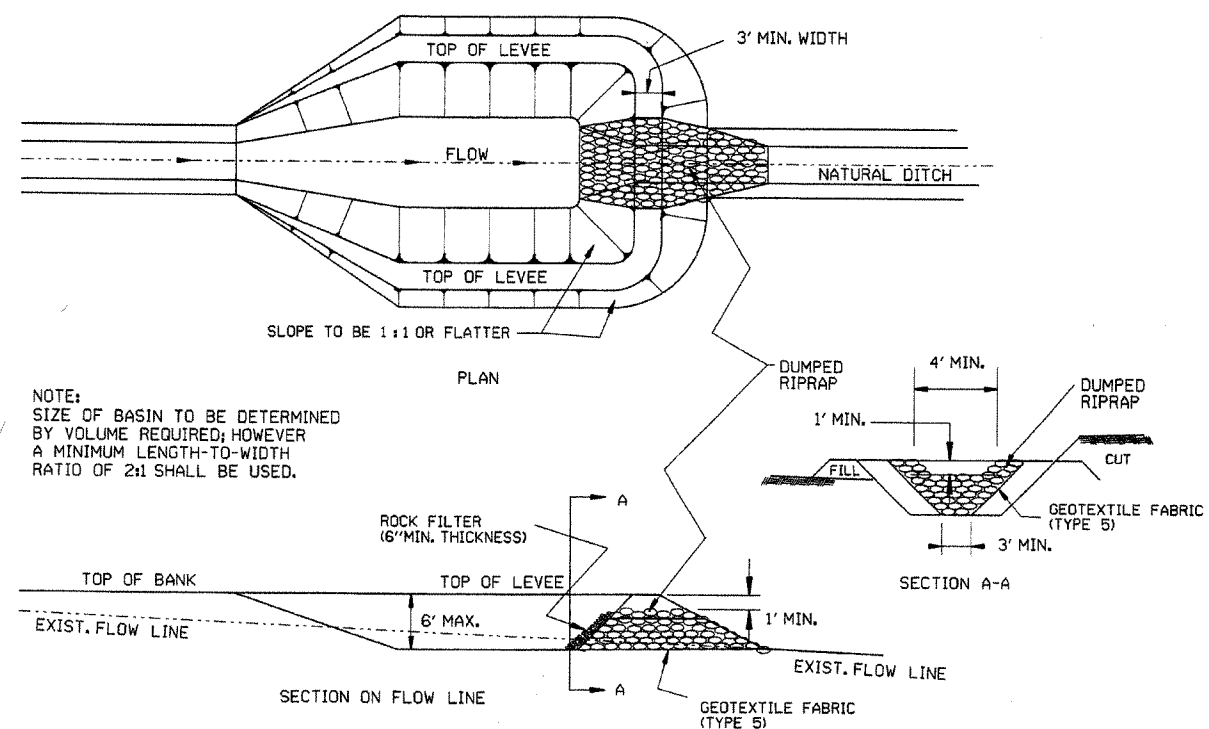
SILT FENCE (E-11)

GENERAL NOTES
GEOTEXTILE FABRIC SHALL BE SPLICED TOGETHER WITH A SEWN SEAM ONLY AT A SUPPORT POST OR TWO SECTIONS OF FENCE MAY BE OVERLAPPED INSTEAD. PAYMENT OF ADDITIONAL MATERIAL FOR OVERLAP WILL NOT BE MADE.



ROCK DITCH CHECK (E-6)

12-15-11	DELETED BALED STRAW DITCH CHECK & ADDED WATTLE DITCH CHECK		ARKANSAS STATE HIGHWAY COMMISSION
11-18-98	ADDED NOTES		
7-02-98	ADDED BALED STRAW FILTER BARRIER (E-2)		
7-20-95	REVISED SILT FENCE E-4 AND E-11	7-20-95	
7-15-94	REV. E-4 & E-11 MIN. 13' BURIED END OF FABRIC		
6-2-94	REVISED E-1, 4, 7 & 11; DELETED E-2 & 3	6-2-94	
4-1-93	REDRAWN		
10-1-92	REDRAWN		
8-2-76	ISSUED R.D.M.	298-7-28-76	
DATE	REVISION	FILMED	STANDARD DRAWING TEC-1

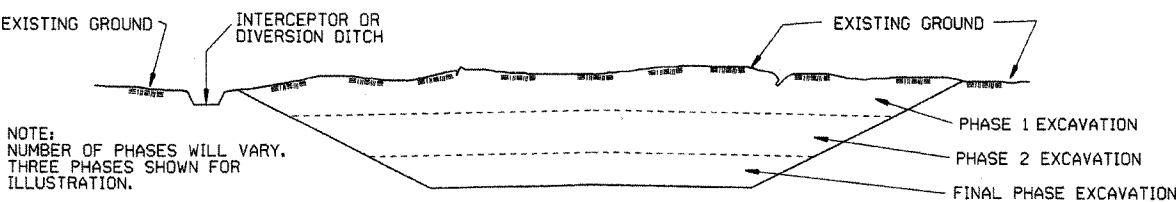


ARKANSAS STATE HIGHWAY COMMISSION			
TEMPORARY EROSION CONTROL DEVICES			
STANDARD DRAWING TEC-2			
6-2-94	Revised E-8 & E-12; Added E-14 & Deleted E-13		
4-1-93	ISSUED		
DATE	REVISION		FILMED

CLEARING AND GRUBBING

- CONSTRUCTION SEQUENCE
1. PLACE PERIMETER CONTROLS (I.E. SILT FENCES , DIVERSION DITCHES, SEDIMENT BASINS, ETC.)
 2. PERFORM CLEARING AND GRUBBING OPERATION.

EXCAVATION

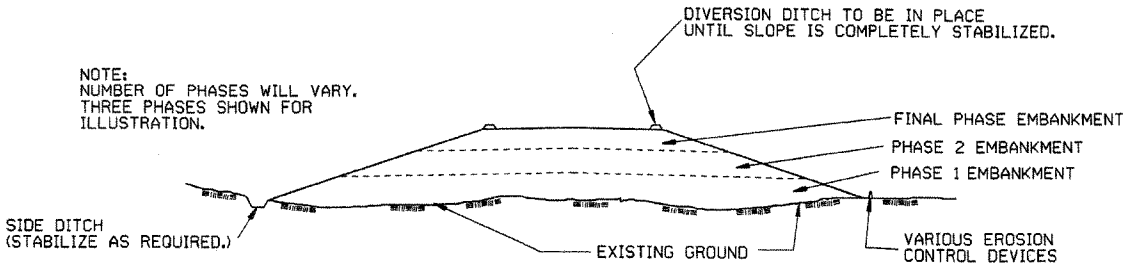


GENERAL NOTE

ALL CUT SLOPES SHALL BE DRESSED, PREPARED, SEEDED, AND MULCHED AS THE WORK PROGRESSES. SLOPES SHALL BE EXCAVATED AND STABILIZED IN EQUAL INCREMENTS NOT TO EXCEED 25 FEET, MEASURED VERTICALLY.

- CONSTRUCTION SEQUENCE
1. EXCAVATE AND STABILIZE INTERCEPTOR AND/OR DIVERSION DITCHES.
 2. PERFORM PHASE 1 EXCAVATION. PLACE PERMANENT OR TEMPORARY SEEDING.
 3. PERFORM PHASE 2 EXCAVATION. PLACE PERMANENT OR TEMPORARY SEEDING.
 4. PERFORM FINAL PHASE OF EXCAVATION. PLACE PERMANENT OR TEMPORARY SEEDING. STABILIZE DITCHES. CONSTRUCT DITCH CHECKS, DIVERSION DITCHES, SEDIMENT BASINS, OR OTHER EROSION CONTROL DEVICES AS REQUIRED.

EMBANKMENT



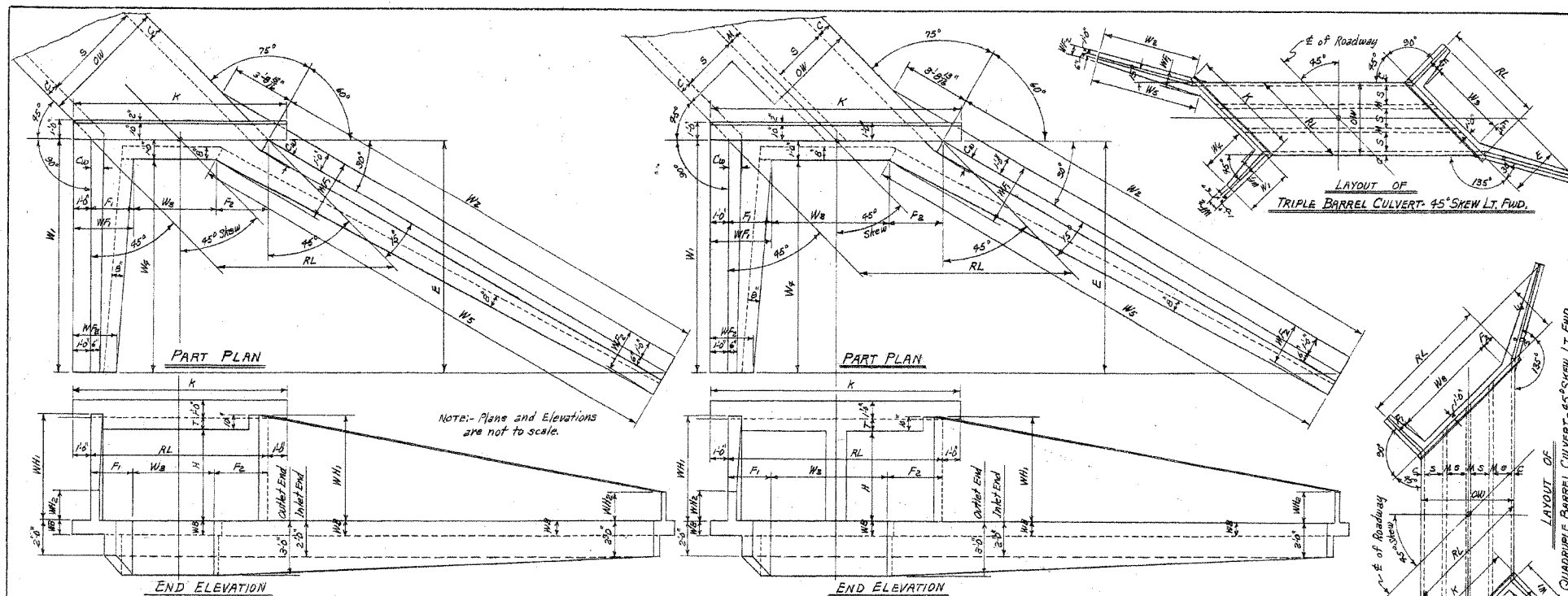
GENERAL NOTE

ALL EMBANKMENT SLOPES SHALL BE DRESSED, PREPARED, SEEDED, AND MULCHED AS THE WORK PROGRESSES. SLOPES SHALL BE CONSTRUCTED AND STABILIZED IN EQUAL INCREMENTS NOT TO EXCEED 25 FEET, MEASURED VERTICALLY.

- CONSTRUCTION SEQUENCE
1. CONSTRUCT DIVERSION DITCHES, DITCH CHECKS, SEDIMENT BASINS, SILT FENCES, OR OTHER EROSION CONTROL DEVICES AS SPECIFIED.
 2. PLACE PHASE 1 EMBANKMENT WITH PERMANENT OR TEMPORARY SEEDING. PROVIDE DIVERSION DITCHES AND SLOPE DRAINS IF EMBANKMENT CONSTRUCTION IS TO BE TEMPORARILY ABANDONED FOR A PERIOD OF GREATER THAN 21 DAYS.
 3. PLACE PHASE 2 EMBANKMENT WITH PERMANENT OR TEMPORARY SEEDING. PROVIDE DIVERSION DITCHES AND SLOPE DRAINS IF EMBANKMENT CONSTRUCTION IS TO BE TEMPORARILY ABANDONED FOR A PERIOD OF GREATER THAN 21 DAYS.
 4. PLACE FINAL PHASE OF EMBANKMENT WITH PERMANENT OR TEMPORARY SEEDING. PLACE DIVERSION DITCHES AND SLOPE DRAINS AND MAINTAIN UNTIL ENTIRE SLOPE IS STABILIZED.

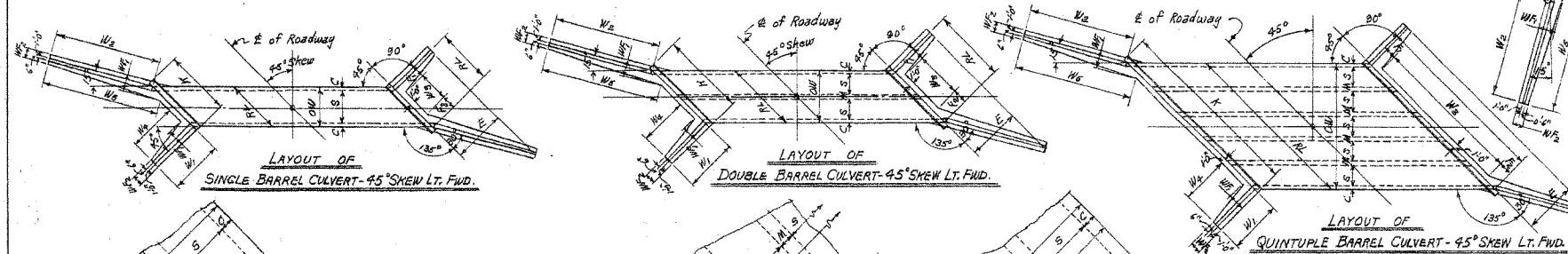
			ARKANSAS STATE HIGHWAY COMMISSION
			TEMPORARY EROSION CONTROL DEVICES
11-03-94	CORRECTED SPELLING		STANDARD DRAWING TEC-3
6-2-94	Drawn & Issued	6-2-94	
DATE	REVISION	FILMED	

FED. ROAD No.	STATE	FED. AID PROJECT	FISCAL YEAR	SHEET No.	TOTAL SHEETS
6	ARK.			208	
JOB No.					



SINGLE BARREL CULVERT-45° SKEW RIGHT FORWARD
Details of Culvert with 45° Skew Left Forward is reversed, see Layout below.

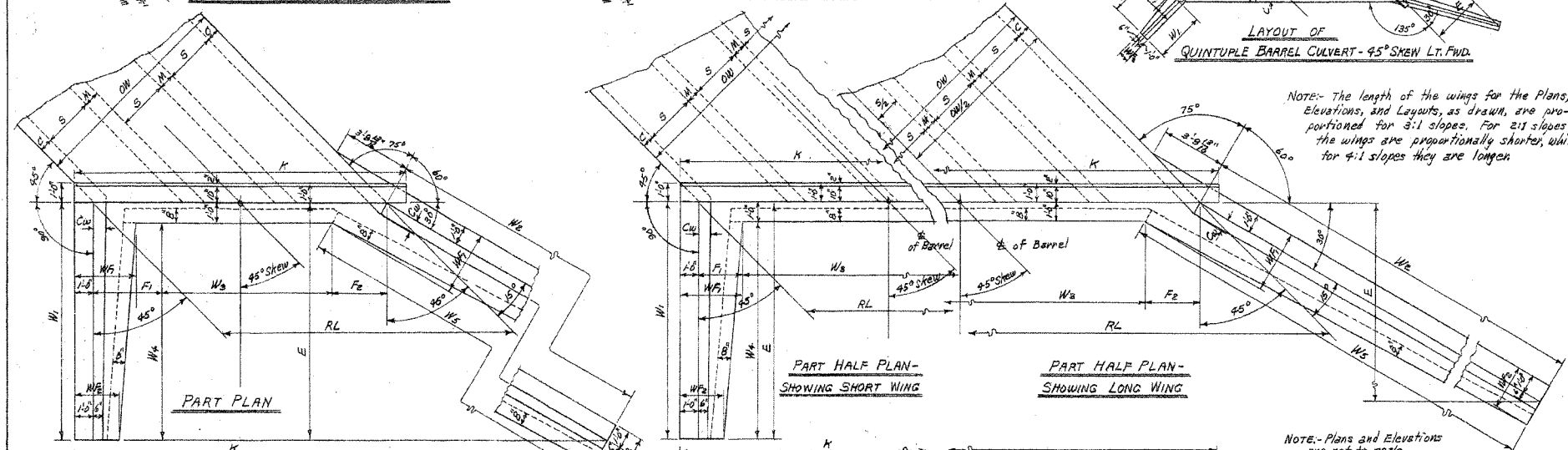
DOUBLE BARREL CULVERT-45° SKEW RIGHT FORWARD
Details of Culvert with 45° Skew Left Forward is reversed, see Layout below.



LAYOUT OF SINGLE BARREL CULVERT-45° SKEW LT. FWD.

LAYOUT OF DOUBLE BARREL CULVERT-45° SKEW LT. FWD.

LAYOUT OF QUINTUPLE BARREL CULVERT-45° SKEW LT. FWD.



PART PLAN

PART HALF PLAN-SHOWING SHORT WING

PART HALF PLAN-SHOWING LONG WING

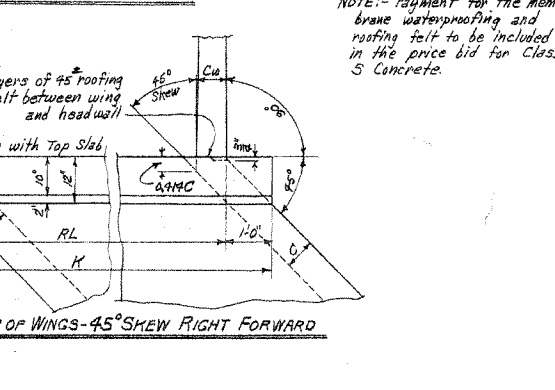
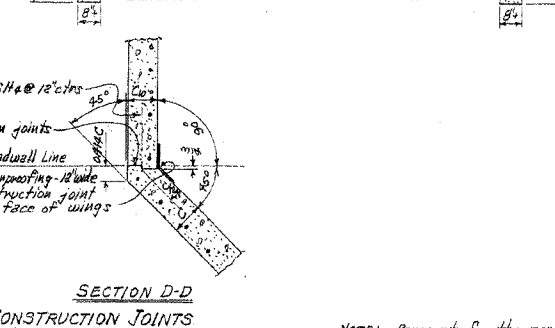
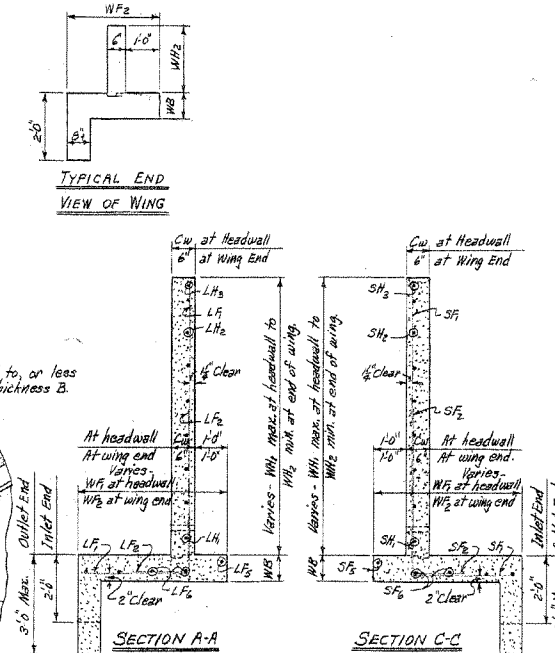
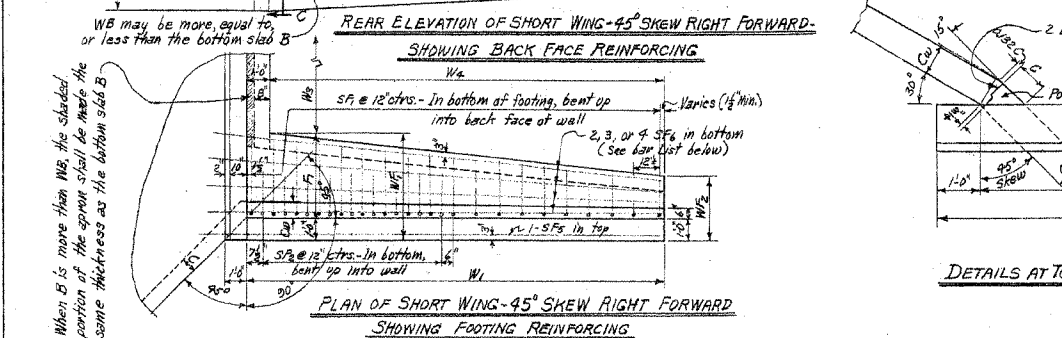
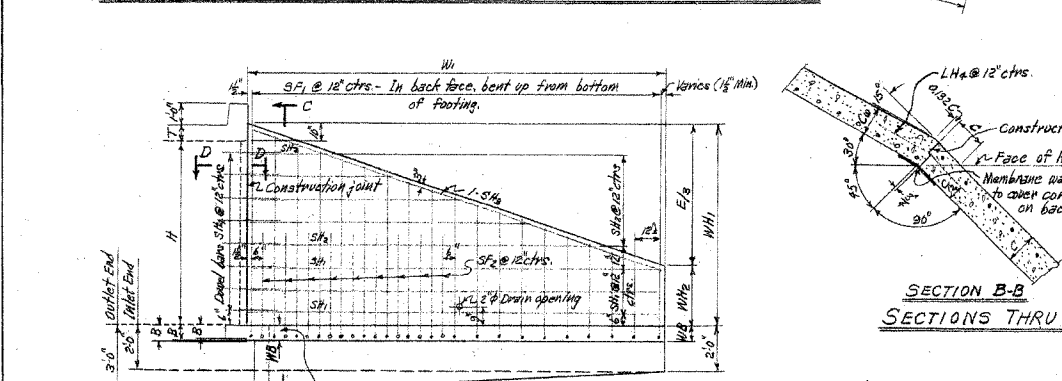
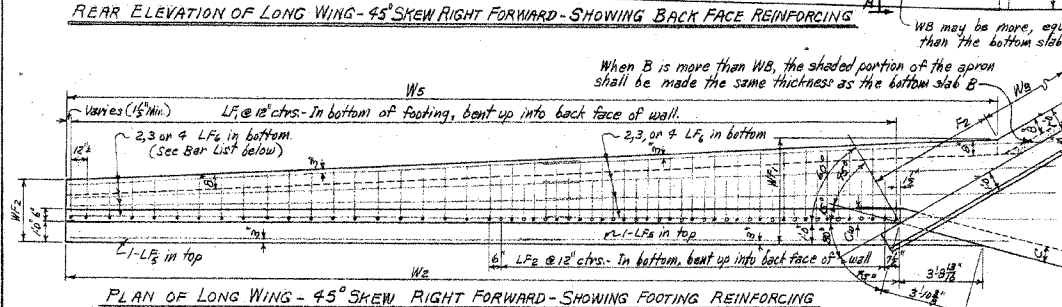
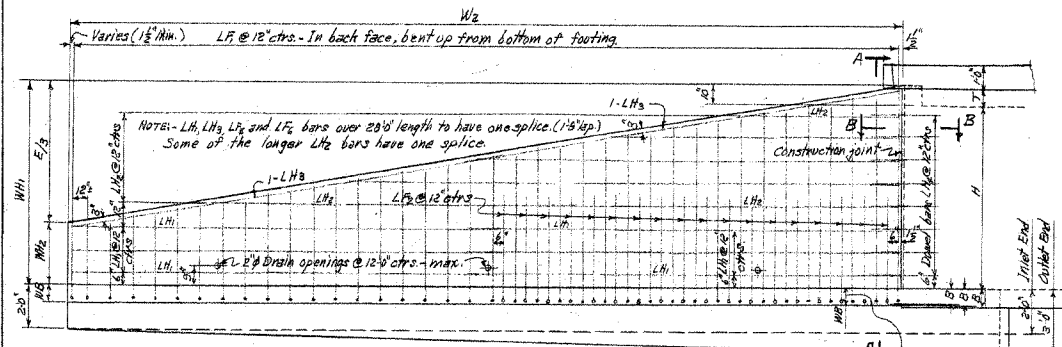
TRIPLE BARREL CULVERT-45° SKEW RIGHT FORWARD
Details of Culvert with 45° Skew Left Forward is reversed, see Layout at top center of sheet.

QUADRUPLE BARREL CULVERT-45° SKEW RT. FWD.
Details of Culvert with 45° Skew Left Forward is reversed, see Layout at above center.

QUINTUPLE BARREL CULVERT-45° SKEW RT. FWD.
Details of Culvert with 45° Skew Left Forward is reversed, see Layout above.

ROADWAY LENGTH RL				HEADWALL LENGTH K				APRON DIMENSION W ₃															
RL = OW + 141.421				K = RL + (2'-0")				W ₃ = RL - (F + F ₂)															
USE WITH DRAWING NO.	CLEAR SPAN	CLEAR HEIGHT	SUM OF DIMENSIONS	SINGLE BARREL CULVERT				DOUBLE BARREL CULVERT				TRIPLE BARREL CULVERT				QUADRUPLE BARREL CULVERT				QUINTUPLE BARREL CULVERT			
				S	H	F ₁	F ₂	OW	RL	K	W ₃	OW	RL	K	W ₃	OW	RL	K	W ₃	OW	RL	K	W ₃
4	2	2'-3"	5'-0"	7'-0"	9'-0"	9'-8"	13'-8"	11'-4"	14'-4"	20'-3"	22'-3"	18'-8"	19'-0"	26'-10"	28'-10"	24'-8"	23'-8"	33'-5"	35'-5"	31'-2"	34'-2"		
	3	3'-3"	5'-0"	"	"	3'-9"	9'-8"	"	10'-2"	14'-4"	"	17'-0"	19'-0"	"	"	22'-8"	"	"	"	30'-2"	"		
	4	4'-3"	5'-0"	"	"	2'-0"	9'-8"	"	9'-4"	14'-4"	"	16'-0"	19'-0"	"	"	22'-8"	"	"	"	29'-2"	"		
	5	5'-3"	5'-0"	7'-0"	9'-0"	1'-9"	13'-8"	15'-8"	14'-4"	20'-3"	22'-3"	15'-0"	19'-0"	26'-10"	28'-10"	21'-8"	23'-8"	33'-5"	35'-5"	28'-2"	30'-2"		
	6	6'-3"	5'-0"	7'-3"	9'-3"	1'-0"	3'-11"	14'-6"	15'-0"	7'-9"	14'-8"	22'-9"	14'-8"	19'-5"	27'-5"	20'-5"	21'-2"	28'-2"	30'-2"	34'-8"	36'-8"		
	3	3'-3"	4'-0"	8'-5"	10'-5"	5'-2"	13'-8"	16'-8"	13'-8"	17'-0"	24'-4"	22'-3"	21'-3"	32'-8"	34'-8"	24'-8"	26'-8"	36'-2"	42'-4"	37'-0"	39'-0"		
	4	4'-3"	4'-0"	"	"	4'-2"	13'-8"	"	12'-2"	17'-4"	"	20'-8"	23'-4"	"	"	28'-8"	"	"	"	36'-8"	"		
	5	5'-3"	4'-0"	8'-5"	10'-5"	3'-2"	13'-8"	16'-8"	18'-8"	11'-2"	17'-4"	24'-4"	19'-3"	23'-0"	32'-6"	34'-6"	27'-8"	29'-8"	40'-6"	42'-6"	35'-8"	37'-8"	
5	6	6'-3"	6'-2"	8'-8"	10'-8"	2'-5"	11'-0"	16'-10"	18'-10"	17'-0"	17'-8"	27'-0"	23'-5"	33'-5"	35'-10"	28'-2"	29'-2"	41'-3"	43'-3"	34'-10"	36'-10"		
	7	7'-9"	6'-8"	8'-0"	10'-0"	1'-0"	18'-1"	17'-1"	19'-1"	9'-10"	17'-10"	25'-4"	24'-4"	7'-4"	23'-8"	33'-7"	35'-7"	26'-9"	28'-9"	41'-0"	43'-0"		
	3	3'-3"	7'-0"	9'-0"	11'-0"	6'-7"	13'-8"	19'-3"	21'-8"	16'-3"	28'-9"	30'-9"	25'-3"	27'-3"	36'-10"	34'-10"	37'-10"	39'-10"	48'-4"	50'-4"			
	4	4'-3"	7'-0"	"	"	5'-7"	13'-8"	"	15'-0"	20'-8"	"	24'-8"	27'-8"	"	"	33'-10"	33'-8"	"	"	43'-4"	"		
	5	5'-3"	7'-0"	9'-0"	11'-0"	4'-7"	13'-8"	19'-3"	21'-8"	20'-8"	28'-9"	30'-9"	23'-5"	28'-10"	38'-10"	40'-10"	32'-11"	33'-8"	47'-7"	49'-7"			
	6	6'-3"	7'-2"	10'-5"	12'-5"	3'-10"	13'-11"	19'-11"	21'-11"	13'-5"	20'-8"	32'-2"	31'-2"	22'-4"	27'-5"	38'-2"	37'-2"	34'-4"	35'-3"	50'-3"	52'-0"		
	7	7'-9"	7'-8"	10'-5"	18'-3"	2'-3"	14'-11"	19'-11"	21'-11"	12'-11"	20'-11"	31'-7"	31'-7"	22'-4"	27'-5"	38'-2"	37'-2"	34'-4"	35'-3"	50'-3"	52'-0"		
	8	8'-3"	7'-6"	10'-4"	12'-4"	1'-10"	14'-2"	22'-0"	22'-4"	10'-9"	21'-0"	28'-0"	31'-0"	31'-0"	22'-4"	27'-5"	38'-2"	37'-2"	34'-4"	35'-3"	50'-3"	52'-0"	
6	4	4'-3"	8'-0"	11'-2"	13'-2"	7'-0"	15'-8"	22'-10"	24'-10"	17'-0"	28'-4"	33'-0"	35'-0"	28'-8"	31'-0"	43'-0"	45'-0"	38'-8"	40'-8"	56'-0"	58'-0"		
	5	5'-3"	8'-0"	11'-3"	13'-3"	6'-0"	15'-8"	22'-10"	24'-10"	17'-0"	28'-4"	33'-0"	35'-0"	27'-8"	31'-0"	43'-0"	45'-0"	38'-8"	40'-8"	56'-0"	58'-0"		
	6	6'-3"	8'-2"	11'-4"	13'-4"	5'-3"	15'-8"	22'-10"	24'-10"	16'-3"	28'-8"	33'-5"	35'-5"	21'-2"	31'-5"	44'-5"	46'-5"	38'-8"	40'-8"	54'-8"	56'-8"		
	7	7'-9"	8'-3"	11'-8"	13'-8"	1'-11"	18'-1"	22'-9"	24'-9"	16'-11"	23'-11"	33'-9"	35'-9"	24'-0"	31'-9"	44'-9"	46'-9"	39'-7"	41'-7"	54'-7"	56'-7"		
	8	8'-3"	8'-4"	11'-8"	13'-8"	2'-6"	16'-2"	22'-10"	24'-10"	13'-7"	24'-0"	33'-11"	35'-11"	24'-0"	31'-10"	44'-10"	46'-10"	39'-8"	41'-8"	54'-8"	56'-8"		
	4	4'-3"	9'-0"	12'-3"	14'-3"	14'-0"	18'-8"	27'-10"	29'-10"	20'-8"	32'-0"	37'-0"	39'-0"	31'-0"	41'-10"	45'-10"	51'-10"	53'-10"	61'-10"	63'-10"	71'-10"	73'-10"	
	5	5'-3"	9'-2"	12'-10"	14'-10"	7'-8"	19'-11"	25'-4"	27'-4"	19'-0"	26'-8"	39'-8"	41'-8"	32'-5"	35'-5"	50'-11"	52'-11"	44'-8"	46'-8"	68'-8"	70'-8"		
	7	7'-9"	9'-3"	13'-11"	15'-11"	5'-3"	18'-1"	22'-7"	24'-7"	17'-9"	26'-11"	38'-0"	40'-0"	30'-3"	35'-9"	52'-11"	54'-11"	44'-7"	46'-7"	68'-7"	70'-7"		
7	8	8'-3"	9'-4"	14'-2"	16'-2"	3'-10"	18'-2"	25'-6"	27'-6"	16'-2"	22'-10"	34'-0"	36'-0"	28'-11"	31'-0"	52'-8"	54'-8"	41'-5"	43'-5"	63'-5"	65'-5"		
	9	9'-3"	10'-2"	14'-6"	16'-6"	4'-10"	18'-6"	26'-2"	28'-2"	16'-6"	22'-10"	33'-8"	35'-8"	30'-5"	32'-8"	50'-8"	52'-8"	42'-9"	44'-9"	62'-9"	64'-9"		
	6	6'-3"	10'-8"	14'-6"	16'-6"	5'-2"	20'-1"	28'-0"	30'-0"	16'-6"	22'-10"	34'-3"	36'-3"	30'-9"	32'-8"	50'-8"	52'-8"	42'-9"	44'-9"	62'-9"	64'-9"		
	7	7'-9"	10'-8"	14'-6"	16'-6"	4'-8"	20'-1"	28'-0"	30'-0"	16'-6"	22'-10"	34'-3"	36'-3"	30'-9"	32'-8"	50'-8"	52'-8"	42'-9"	44'-9"	62'-9"	64'-9"		
	8	8'-3"	10'-6"	14'-7"	16'-7"	5'-4"	20'-1"	28'-0"	30'-0"	16'-6"	22'-10"	34'-3"	36'-3"	30'-9"	32'-8"	50'-8"	52'-8"	42'-9"	44'-9"	62'-9"	64'-9"		
	6	6'-3"	11'-2"	15'-9"	17'-9"	10'-4"	21'-11"	31'-0"	33'-0"	25'-8"	32'-8"	46'-8"	48'-8"	36'-11"	41'-11"	62'-11"	64'-11"	56'-2"	58'-2"	76'-2"	78'-2"		
	7	7'-9"	11'-3"	15'-10"	17'-10"	8'-3"	22'-1"	31'-2"	33'-2"	25'-8"	32'-11"	46'-8"	48'-8"	36'-11"	41'-11"	62'-11"	64'-11"	56'-2"	58'-2"	76'-2"	78'-2"		
	8	8'-3"	11'-4"	15'-10"	17'-10"	6'-3"	22'-2"	31'-2"	33'-2"	25'-8"	32'-11"	46'-8"	48'-8"	36'-11"	41'-11"	62'-11"	64'-11"	56'-2"	58'-2"	76'-2"	78'-2"		
9	6	6'-3"	12'-4"	17'-5"	19'-5"	11'-2"	24'-2"	34'-2"	36'-2"	27'-11"	34'-0"	50'-11"	52'-11"	44'-7"	46'-7"	68'-7"	70'-7"	54'-4"	56'-4"	74'-4"	76'-4"		
	7	7'-9"	12'-4"	"	"	8'-8"	24'-2"	"	"	26'-5"	34'-0"	"	"	48'-11"	47'-0"	"	"	59'-8"	61'-8"	79'-8"	81'-8"		
	8	8'-3"	12'-4"	"	"	8'-8"	24'-2"	"	"	26'-5"	34'-0"	"	"	48'-11"	47'-0"	"	"	59'-8"	61'-8"	79'-8"	81'-8"		
	6	6'-3"	12'-4"	17'-5"	19'-5"	11'-2"	24'-2"	34'-2"	36'-2"	27'-11"	34'-0"	50'-11"	52'-11"	44'-7"	46'-7"	68'-7"	70'-7"	54'-4"	56'-4"	74'-4"	76'-4"		
	7	7'-9"	12'-4"	"	"	8'-8"	24'-2"	"	"	26'-5"	34'-0"	"	"	48'-11"	47'-0"	"	"	59'-8"	61'-8"	79'-8"	81'-8"		
	8	8'-3"	12'-4"	"	"	8'-8"	24'-2"	"	"	26'-5"	34'-0"	"	"	48'-11"	47'-0"	"	"	59'-8"	61'-8"	79'-8"	81'-8"		
	6	6'-3"	12'-4"	17'-5"	19'-5"	11'-2"	24'-2"	34'-2"	36'-2"	27'-11"	34'-0"	50'-11"	52'-11"	44'-7"	46'-7"	68'-7"	70'-7"	54'-4"	56'-4"	74'-4"	76'-4"		
	7	7'-9"	12'-4"	"	"	8'-8"	24'-2"	"	"	26'-5"	34'-0"	"	"	48'-11"	47'-0"	"	"	59'-8"	61'-8"	79'-8"	81'-8"		
10	6	6'-3"	12'-4"	17'-5"	19'-5"	11'-2"	24'-2"	34'-2"	36'-2"	27'-11"	34'-0"	50'-11"	52'-11"	44'-7"	46'-7"	68'-7"	70'-7"	54'-4"	56'-4"	74'-4"	76'-4"		
	7	7'-9"	12'-4"	"	"	8'-8"	24'-2"	"	"	26'-5"	34'-0"	"	"	48'-11"	47'-0"	"	"	59'-8"	61'-8"	79'-8"	81'-8"		
	8	8'-3"	12'-4"	"	"	8'-8"	24'-2"	"	"	26'-5"	34'-0"	"	"	48'-11"	47'-0"	"	"	59'-8"	61'-8"	79'-8"	81'-8"		
	6	6'-3"	12'-4"	17'-5"	19'-5"	11'-2"	24'-2"	34'-2"	36'-2"	27'-11"	34'-0"	50'-11"	52'-11"	44'-7"	46'-7"	68'-7"	70'-7"	54'-4"	56'-4"	74'-4"	76'-4"		
	7	7'-9"	12'-4"	"	"	8'-8"	24'-2"	"	"	26'-5"	34'-0"	"	"	48'-11"	47'-0"	"	"	59'-8"	61'-8"	79'-8"	81'-8"		
	8	8'-3"	12'-4"	"	"	8'-8"	24'-2"	"	"	26'-5"	34'-0"	"	"	48'-11"	47'-0"	"	"	59'-8"	61'-8"	79'-8"	81'-8"		
	6	6'-3"	12'-4"	17'-5"	19'-5"	11'-2"	24'-2"	34'-2"	36'-2"	27'-11"	34'-0"	50'-11"	52'-11"	44'-7"	46'-7"	68'-7"	70'-7"	54'-4"	56'-4"	74'-4"	76'-4"		
	7	7'-9"	12'-4"	"	"	8'-8"	24'-2"	"	"	26'-5"	34'-0"	"	"	48'-11"	47'-0"	"	"	59'-8"	61'-8"	79'-8"	81'-8"		
11	6	6'-3"	12'-4"	17'-5"	19'-5"	11'-2"	24'-2"	34'-2"	36'-2"	27'-11"	34'-0"	50'-11"	52'-11"	44'-7"	46'-7"	68'-7"	70'-7"	54'-4"	56'-4"	74'-4"	76'-4"		
	7	7'-9"	12'-4"	"	"	8'-8"	24'-2"	"	"	26'-5"	34'-0"	"	"	48'-11"	47'-0"	"	"	59'-8"	61'-8"	79'-8"	81'-8"		
	8	8'-3"	12'-4"	"	"	8'-8"	24'-2"	"	"	26'-5"	34'-0"	"	"	48'-11"	47'-0"	"	"	59'-8"	61'-8"	79'-8"	81'-8"		
	6	6'-3"	12'-4"	17'-5"	19'-5"	11'-2"	24'-2"	34'-2"	36'-2"	27'-11"	34'-0"	50'-11"	52'-11"	44'-7"	46'-7"	68'-7"	70'-7"	54'-4"	56'-4"	74'-4"	76'-4"		
	7	7'-9"	12'-4"	"	"	8'-8"	24'-2"	"	"	26'-5"	34'-0"	"	"	48'-11"	47'-0"	"	"	59'-8"	61'-8"	79'-8"	81'-8"		
	8	8'-3"	12'-4"	"	"	8'-8"	24'-2"	"	"	26'-5"	34'-0"	"	"	48'-11"	47'-0"	"	"	59'-8"	61'-8"	79'-8"	81'-8"		
	6	6'-3"	12'-4"	17'-5"	19'-5"	11'-2"	24'-2"	34'-2"	36'-2"	27'-11"	34'-0"	50'-11"	52'-11"	44'-7"	46'-7"	68'-7"	70'-7"	54'-4"	56'-4"	74'-4"	76'-4"		
	7	7'-9"	12'-4"	"	"	8'-8"	24'-2"	"	"	26'-5"	34'-0"	"	"	48'-11"	47'-0"	"	"	59'-8"	61'-8"	79'-8"	81'-8"		
12	6	6'-3"	12'-4"	17'-5"	19'-5"	11'-2"	24'-2"	34'-2"	36'-2"	27'-11"	34'-0"	50'-11"	52'-11"	44'-7"	46'-7"	68'-7"	70'-7"	54'-4"	56'-4"	74'-4"	76'-4"		
	7	7'-9"	12'-4"	"	"	8'-8"	24'-2"	"	"	26'-5"	34'-0"	"	"	48'-11"	47'-0"	"	"	59'-8"	61'-8"	79'-8"	81'-8"		
	8	8'-3"	12'-4"	"	"	8'-8"	24'-2"	"	"	26'-5"	34'-0"	"	"	48'-11"	47'-0"	"	"	59'-8"	61'-8"	79'-8"	81'-8"		
	6	6'-3"	12'-4"	17'-5"	19'-5"	11'-2"	24'-2"	34'-2"	36'-2"	27'-11"	34'-0"	50'-11"	52'-11"	44'-7"	46'-7"	68'-7"	70'-7"	54'-4"	56'-4"	74'-4"	76'-4"		
	7	7'-9"	12'-4"	"	"	8'-8"	24'-2"	"	"	26'-5"	34'-0"	"	"	48'-11"	47'-0"	"	"	59'-8"	61'-8"	79'-8"	81'-8"		
	8	8'-3"	12'-4"	"	"	8'-8"	24'-2"	"	"	26'-5"	34'-0"	"	"	48'-11"	47'-0"	"	"	59'-8"	61'-8"	79'-8"	81'-8"		
	6	6'-3"	12'-4"	17'-5"	19'-5"	11'-2"	24'-2"	34'-2"	36'-2"	27'-11"	34'-0"	50'-11"	52'-11"	44'-7"	46'-7"	68'-7"	70'-7"	54'-4"	56'-4"	74'-4"	76'-4"		
	7	7'-9"	12'-4"	"	"	8'-8"	24'-2"	"	"	26'-5"	34'-0"	"	"	48'-11"	47'-0"	"	"	59'-8"	61'-8"	79'-8"	81'-8"		
13	6	6'-3"	12'-4"	17'-5"	19'-5"	11'-2"	24'-2"	34'-2"	36'-2"	27'-11"	34'-0"	50'-11"	52'-11"	44'-7"	46'-7"	68'-7"	70'-7"	54'-4"	56'-4"	74'-4"	76'-4"		
	7	7'-9"	12'-4"	"	"	8'-8"	24'-2"	"	"	26'-5"	34'-0"	"	"	48'-11"	47'-0"	"	"	59'-8"	61'-8"	79'-8"	81'-8"		
	8	8'-3"	12'-4"	"	"	8'-8"	24'-2"	"	"	26'-5"	34'-0"	"	"	48'-11"	47'-0"	"	"	59'-8"	61'-8"	79'-8"	81'-8"		
	6																						

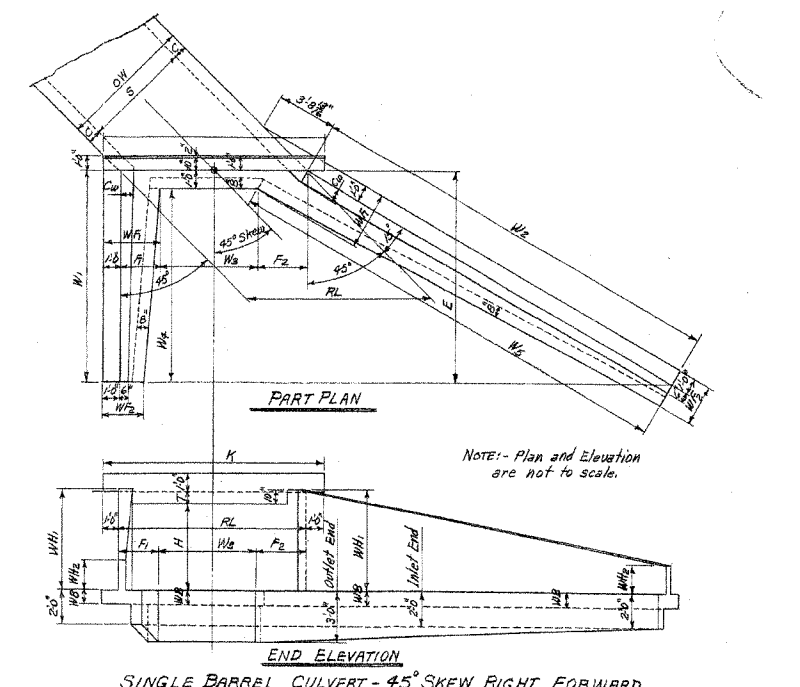
FED. ROAD No.	STATE	FED. AID PROJECT	FISCAL YEAR	SHEET No.	TOTAL SHEETS
6	ARK.			209	
JOB No.					



REGULAR WING DIMENSIONS - 3:1 SLOPES

CLEAR HEIGHT OF BOX	THICKNESS OF WING FOOTING	THICKNESS OF WING AT HEADWALL	WING WALL HEIGHTS	WIDTHS OF WING FOOTINGS		FOOTING DIMENSIONS - PARALLEL WISE		PERPENDICULAR DIST. FROM HEADWALL TO END OF WING	LENGTHS OF WING WALLS		INSIDE FOOTING DIMENSION		* QUANTITY PER WING CLASS 5 CONCRETE				
				AT HEADWALL	AT END OF WING	AT HEADWALL	AT END OF WING		SHORT WING	LONG WING	SHORT WING	LONG WING	SHORT WING	LONG WING	SHORT WING	LONG WING	
				INLET END	OUTLET END	INLET END	OUTLET END										
H	WB	CW	WH ₁	WH ₂	WF ₁	WF ₂	F ₁	F ₂	E	W ₁	W ₂	W ₃	W ₄	CWYD	CWYD	CWYD	CWYD
2'	7"	6"	2'0"	0'8"	2'4"	2'0"	1'4"	0'10"	6'6"	6'6"	13'0"	5'6"	13'3"	0.793	1.572	0.889	1.791
3'	7"	6"	3'0"	1'0"	2'8"	2'4"	1'8"	1'7"	8'6"	8'6"	17'0"	7'6"	17'10"	1.120	2.364	1.229	2.590
4'	7"	6"	4'0"	1'4"	3'0"	2'8"	2'0"	2'3"	10'6"	10'6"	21'0"	9'6"	22'5"	1.567	3.295	1.700	3.577
5'	7"	6"	5'0"	1'8"	3'4"	2'4"	2'4"	2'10"	12'6"	12'6"	25'0"	11'6"	27'0"	2.082	4.367	2.241	4.705
			5'6"	1'8"	3'4"	2'4"	2'4"	2'10"	12'6"	12'6"	25'0"	11'6"	27'0"	2.166	4.551	2.385	4.989
6'	8"	7"	6'0"	2'0"	3'8"	2'6"	2'8"	3'7"	14'6"	14'6"	29'0"	13'6"	31'7"	2.699	6.035	3.078	6.460
			6'6"	2'0"	3'8"	2'6"	2'8"	3'7"	14'6"	14'6"	29'0"	13'6"	31'7"	2.752	6.180	3.185	6.678
			6'6"	2'0"	3'8"	2'6"	2'8"	3'7"	14'6"	14'6"	29'0"	13'6"	31'7"	3.009	6.305	3.192	6.700
7'	8"	7"	7'0"	2'4"	4'0"	2'6"	3'2"	4'7"	16'6"	16'6"	33'0"	15'6"	36'5"	3.394	7.930	4.008	8.380
			7'6"	2'4"	4'0"	2'6"	3'2"	4'7"	16'6"	16'6"	33'0"	15'6"	36'5"	3.670	8.093	4.078	8.548
8'	9"	8"	8'0"	2'8"	4'0"	2'8"	3'6"	5'7"	18'6"	18'6"	37'0"	17'6"	41'4"	4.055	10.186	5.080	10.651

* Quantity per wing does not include headwall or that portion of apron or tee wall for the length W3.



DETAILS OF CULVERT WITH 45° SKEW LEFT FORWARD IS REVERSED, SEE DRAWING NO. W-X-45.

TYPICAL WING DETAILS

NOTE: For remainder of General Plans and Elevations of Single, Double, Triple, Quadruple and Quintuple Span Culverts, see Std. Drawing No. W-X-45. For values of RL, K and W3 for each box, see above Std. also.

MEMBRANE: A membrane waterproofing 18" wide, consisting of three layers of waterproofing asphalt and two alternate layers of treated cotton fabric shall be applied to the back face of wing to cover the construction joints in wings.

REVISIONS: - Membrane Added. 5-10-66 W.C.H.

QUANTITIES

CLASS S CONCRETE - 4 WINGS

CLEAR SPAN	CLEAR HEIGHT	THICKNESS OF WING AT HEADWALL	THICKNESS OF WING FOOTING	STEEL FOR 4 WINGS	HEADWALLS, WING WALLS, FOOTINGS, TEES AND APRONS				
					SINGLE BARREL CULVERT	DOUBLE BARREL CULVERT	TRIPLE BARREL CULVERT	QUADRUPLE BARREL CULVERT	QUINTUPLE BARREL CULVERT
S	H	CW	WB	LB	CWYD	CWYD	CWYD	CWYD	CWYD
2'	6'	7"	1'2"	6'15"	7.50	0.84	10.22	11.58	
3'	6'	7"	2'0"	8'15"	9.79	1.14	12.50	13.86	
4'	6'	7"	2'8"	11'15"	12.18	1.34	15.20	16.56	
5'	6'	7"	3'6"	14'15"	15.60	1.64	18.32	19.68	
6'	6'	7"	4'4"	17'15"	20.42	2.04	22.44	23.80	
7'	6'	7"	5'2"	20'15"	25.24	2.44	26.56	27.92	
8'	6'	7"	6'0"	23'15"	30.06	2.84	31.68	33.04	
9'	6'	7"	6'8"	26'15"	34.88	3.24	36.80	38.16	
10'	6'	7"	7'6"	29'15"	39.70	3.64	41.92	43.28	
11'	6'	7"	8'4"	32'15"	44.52	4.04	47.04	48.40	
12'	6'	7"	9'2"	35'15"	49.34	4.44	52.16	53.52	

* For reinforcing steel in Headwalls and Aprons, see Drawing Nos. of Barrel Sections listed below.

GENERAL NOTES: -

CONCRETE: All concrete to be Class S, and shall be poured in the dry. All exposed corners to have 3/4" chamfers.

REINFORCING STEEL: Reinforcing steel to be deformed bars of intermediate or hard grade.

CONSTRUCTION JOINTS: Construction joints between wingwall, footings and side walls shall be only where shown on plans.

SPECIFICATIONS: Arkansas State Highway Commission Standard Specifications for Highway Construction and applicable Special Provisions.

UNIT STRESSES: -

Class S Concrete (f' = 1200 psi)

Reinforcing Steel (f' = 20,000 psi)

NOTE: This drawing to be used in conjunction with Standard Barrel Sections, Drawing No. -

SINGLES	DOUBLES	TRIPLES	QUADRUPLES	QUINTUPLES
R-145X-0	R-245X-01	R-345X-01	R-445X-01	R-545X-01
R-145X-1	R-245X-02	R-345X-02	R-445X-02	R-545X-02
R-145X-2	R-245X-03	R-345X-03	R-445X-03	R-545X-03

CLASS S CONCRETE

ARKANSAS STATE HIGHWAY COMMISSION

DETAILS OF STANDARD WINGS

FOR

REINFORCED CONCRETE BOX CULVERTS

45° SKEW

4, 5, 6, 7, 8, 9, 10, 11 & 12 SPANS

3:1 SLOPES

SINGLES, DOUBLES, TRIPLES, ALL DEPTHS OF COVER

QUADRUPLES & QUINTUPLES FOR H = 6'-0" OR LESS

STANDARD DRAWING NO. W-X-453-1

Designed By: W.C.H. 5-13-63

Drawn By: W.C.H. 6-5-64

Checked By: J.C.M. 7-21-64

Quantities By: W.C.H. 7-2-64

BAR LIST FOR ONE SHORT AND ONE LONG WING - 2 EACH REQUIRED																																					
CLEAR HEIGHT	WING LOCATION	SF ₁ & LF ₁								SF ₂ & LF ₂				SF ₃ & LF ₃		SF ₄ & LF ₄		SH ₁ & LH ₁		SH ₂ & LH ₂		SH ₃ & LH ₃		SH ₄ & LH ₄				BAR BENDING DIAGRAM	QUANTITY								
		BENT								BENT				STRAIGHT		STRAIGHT		STRAIGHT		BENT		BENT															
		In bottom of footing, bent up into back face of wing. One bar (or two bars) of each length.								In bottom of footing, bent up into back face of wing. 4Hr. with #5 bars				Longitudinal in bottom of wing footing 10L.		Longitudinal in bottom of wing footing 10L.		Horizontal in back face of wing.		Horizontal in back face of wing One bar of each length.		In back face of top-on slope		Downs thru construction joint at headwall.													
H	SHORT OR LONG	SIZE	SPACING	NO. BARS	LENGTHS VARY	X	Y	SIZE	SPACING	NO. BARS	LENGTH	X	Y	SIZE	NO. BARS	LENGTH	SIZE	SPACING	NO. BARS	LENGTH	SIZE	SPACING	NO. BARS	LENGTH	SIZE	SPACING	NO. BARS	LENGTH	X								
					MIN. MAX.	MIN. MAX.	MIN. MAX.																														
2'	Short	#3	12"	7	1'2" 3'4"	0'8"	1'0"	1'0"	3'0"						#3	1	8'4"	#3	2	8'0"	#3	12"	1	6'3"	#3	12"	1	6'3"	#3	1	6'3"						
	Long	#3	12"	14	1'5" 5'10"	0'8"	1'0"	0'10"	3'0"						#3	1	7'10"	#3	2	15'4"	#3	12"	1	12'8"	#3	12"	1	12'8"	#3	1	12'8"						
3'	Short	#3	12"	9	2'11" 5'3"	0'10"	1'4"	1'4"	4'0"						#3	1	8'10"	#3	2	15'0"	#3	12"	1	8'5"	#3	12"	2	6'9"	3'9"	#3	1	8'8"					
	Long	#3	12"	18	1'4" 5'3"	0'10"	1'4"	1'2"	4'0"						#3	1	10'4"	#3	2	15'0"	#3	12"	1	14'6"	#3	1	14'6"	#3	1	14'6"	#3	1	14'6"				
4'	Short	#3	12"	11	2'6" 6'7"	0'10"	1'8"	1'8"	5'0"						#3	1	10'4"	#3	2	15'0"	#3	12"	1	10'4"	#3	12"	3	9'10"	#3	1	9'10"						
	Long	#3	12"	22	2'4" 6'6"	0'10"	1'8"	1'6"	5'0"						#3	1	8'6"	#3	3	20'0"	#3	12"	2	18'0"	#3	12"	3	18'0"	#3	1	5'0"	#3	1	5'0"			
5'	Short	#3	12"	13	2'11" 7'10"	1'0"	2'0"	2'0"	6'0"	#3	12"	3	3'6"	1'0"	2'4"	#3	1	14'4"	#3	3	20'0"	#3	12"	2	12'9"	#3	12"	3	9'10"	#3	1	12'11"	#3	1	12'11"		
	Long	#3	12"	26	2'9" 7'4"	1'0"	1'10"	1'10"	6'0"	#3	12"	6	3'4"	1'10"	2'6"	#3	2	15'4"	#3	3	15'0"	#3	12"	2	20'9"	#3	12"	3	18'6"	6'6"	#3	1	2'5"	#3	1	2'5"	
6'	Short	#4	12"	15	3'4" 9'10"	1'2"	2'4"	2'4"	7'11"	#4	12"	3	3'6"	1'4"	3'6"	#4	1	16'4"	#4	3	15'0"	#4	12"	2	20'9"	#4	12"	4	12'3"	3'3"	#4	1	16'0"	#4	1	16'0"	
	Long	#4	12"	30	3'4" 9'10"	1'2"	2'4"	2'10"	7'11"	#4	12"	6	3'6"	1'4"	3'10"	#4	2	17'0"	#4	3	18'0"	#4	12"	4	15'0"	#4	12"	4	24'4"	6'4"	#4	2	15'5"	#4	3	12'6"	
7'	Short	#4	12"	17	3'11" 10'10"	1'3"	2'6"	2'6"	8'1"	#4	12"	6	3'4"	1'4"	4'10"	#4	1	18'4"	#4	4	18'0"	#4	12"	2	16'9"	#4	12"	5	15'3"	3'3"	#4	1	17'10"	#4	1	2'8"	
	Long	#4	12"	34	3'11" 10'10"	1'3"	2'6"	2'7"	8'1"	#4	12"	12	5'0"	1'5"	4'10"	#4	2	18'0"	#4	4	20'0"	#4	12"	4	17'0"	#4	12"	5	15'3"	4'4"	#4	2	15'5"	#4	12"	7	"
8'	Short	#4	12"	19	4'6" 12'5"	1'5"	3'4"	3'4"	9'2"	#4	12"	9	6'6"	1'10"	4'10"	#4	1	20'0"	#4	4	18'0"	#4	12"	3	18'0"	#4	12"	5	15'3"	3'3"	#4	1	19'10"	#4	12"	0	"
	Long	#4	12"	38	4'6" 12'5"	1'5"	3'4"	3'0"	9'2"	#4	12"	18	6'6"	1'10"	4'10"	#4	2	20'0"	#4	4	18'0"	#4	12"	6	19'0"	#4	12"	5	15'3"	6'4"	#4	4	12'0"	#4	12"	0	"
Notes: 1. Bars are to be bent up at 90 degrees with one bar in each wing. 2. Bars are to be bent up at 90 degrees with one bar in each wing. 3. Bars are to be bent up at 90 degrees with one bar in each wing. 4. Bars are to be bent up at 90 degrees with one bar in each wing. 5. Bars are to be bent up at 90 degrees with one bar in each wing. 6. Bars are to be bent up at 90 degrees with one bar in each wing. 7. Bars are to be bent up at 90 degrees with one bar in each wing. 8. Bars are to be bent up at 90 degrees with one bar in each wing. 9. Bars are to be bent up at 90 degrees with one bar in each wing. 10. Bars are to be bent up at 90 degrees with one bar in each wing. 11. Bars are to be bent up at 90 degrees with one bar in each wing. 12. Bars are to be bent up at 90 degrees with one bar in each wing. 13. Bars are to be bent up at 90 degrees with one bar in each wing. 14. Bars are to be bent up at 90 degrees with one bar in each wing. 15. 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NOTE: Bars for short wing shall be marked with prefix letter S, while those for long wing shall be marked with letter L.

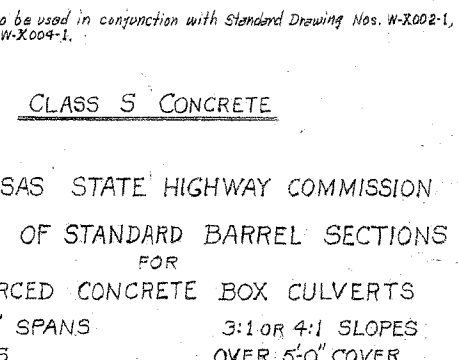
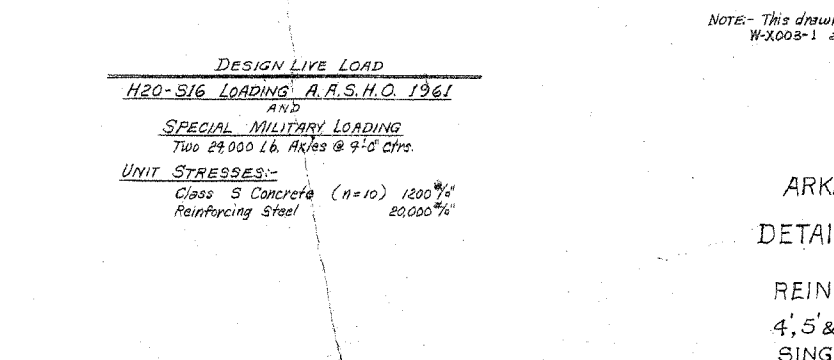
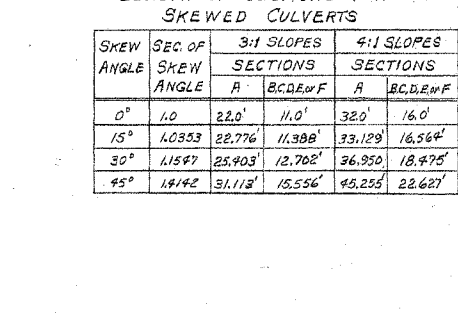
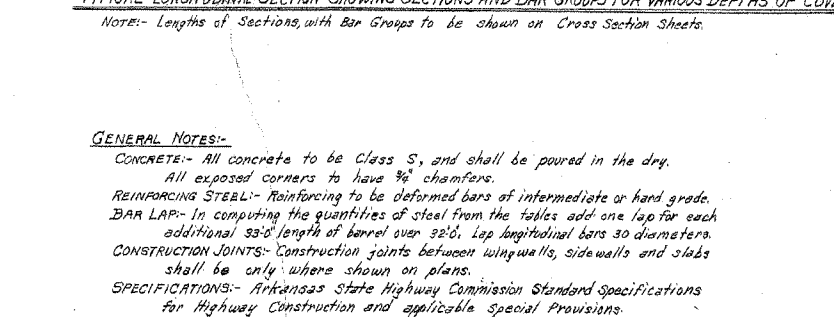
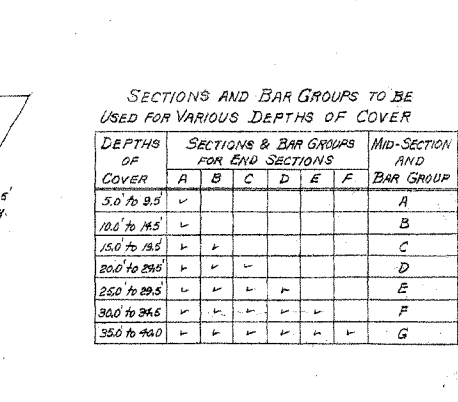
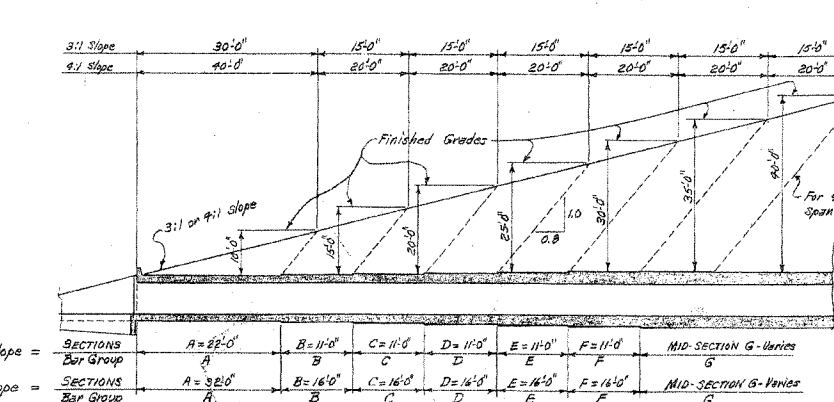
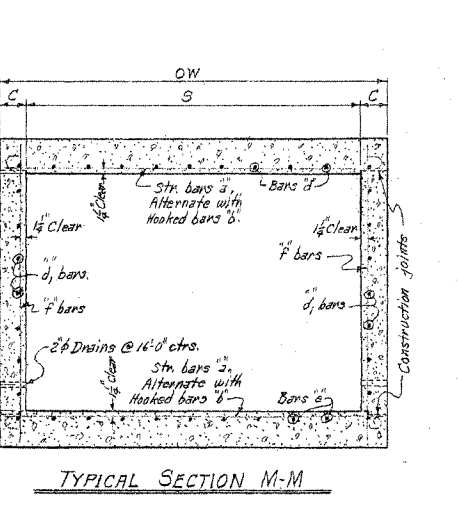
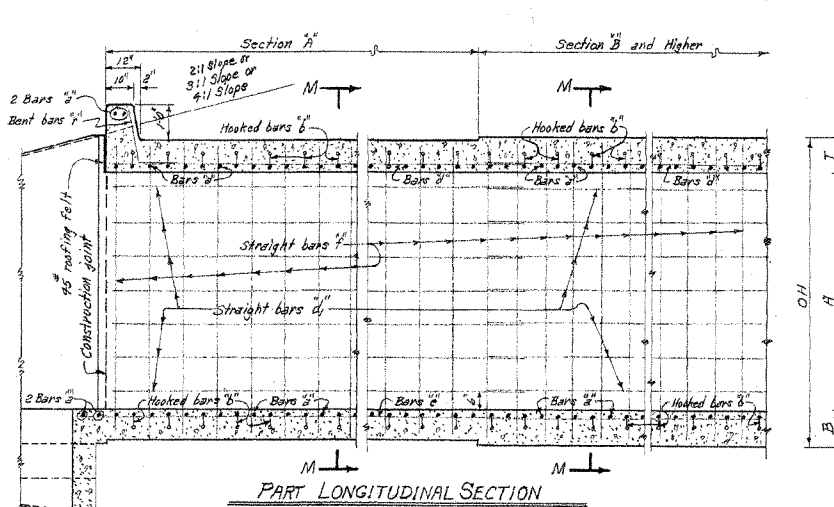
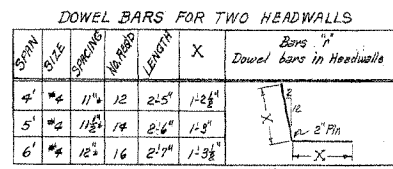
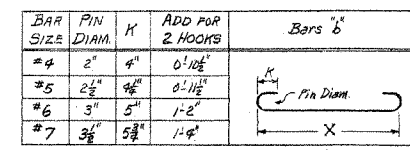
* Length without splice. Bars over 28' length may be spliced (1-8' lap).

BAR LIST FOR VARIOUS SECTIONS OF BARREL

SECTION & BAR GROUP	LENGTH OF SECTION	DEPTH OF COVER	CLEAR SPAN	CLEAR HEIGHT	STRAIGHT		BENT- See Diagram at Right		STRAIGHT		STRAIGHT	
					In Top and Bottom Slabs of Barrel		In Top and Bottom Slabs of Barrel		Longitudinal in Top Slab of Barrel		Longitudinal in Bottom Slab of Barrel	
					NO. REIN. REQ'D	LENGTH	NO. REIN. REQ'D	LENGTH	NO. REIN. REQ'D	LENGTH	NO. REIN. REQ'D	LENGTH
SECTION A & BAR GROUP A	31' = 32'-0"	4'-0"	10'-0"	10'-0"	48	6.8	48	6.8	48	6.8	48	6.8
					48	6.8	48	6.8	48	6.8	48	6.8
					48	6.8	48	6.8	48	6.8	48	6.8
					48	6.8	48	6.8	48	6.8	48	6.8
					48	6.8	48	6.8	48	6.8	48	6.8
					48	6.8	48	6.8	48	6.8	48	6.8
					48	6.8	48	6.8	48	6.8	48	6.8
					48	6.8	48	6.8	48	6.8	48	6.8
					48	6.8	48	6.8	48	6.8	48	6.8
					48	6.8	48	6.8	48	6.8	48	6.8
SECTION B & BAR GROUP B	31' = 32'-0"	4'-0"	10'-0"	10'-0"	48	6.8	48	6.8	48	6.8	48	6.8
					48	6.8	48	6.8	48	6.8	48	6.8
					48	6.8	48	6.8	48	6.8	48	6.8
					48	6.8	48	6.8	48	6.8	48	6.8
					48	6.8	48	6.8	48	6.8	48	6.8
					48	6.8	48	6.8	48	6.8	48	6.8
					48	6.8	48	6.8	48	6.8	48	6.8
					48	6.8	48	6.8	48	6.8	48	6.8
					48	6.8	48	6.8	48	6.8	48	6.8
					48	6.8	48	6.8	48	6.8	48	6.8
SECTION C & BAR GROUP C	31' = 32'-0"	4'-0"	10'-0"	10'-0"	48	6.8	48	6.8	48	6.8	48	6.8
					48	6.8	48	6.8	48	6.8	48	6.8
					48	6.8	48	6.8	48	6.8	48	6.8
					48	6.8	48	6.8	48	6.8	48	6.8
					48	6.8	48	6.8	48	6.8	48	6.8
					48	6.8	48	6.8	48	6.8	48	6.8
					48	6.8	48	6.8	48	6.8	48	6.8
					48	6.8	48	6.8	48	6.8	48	6.8
					48	6.8	48	6.8	48	6.8	48	6.8
					48	6.8	48	6.8	48	6.8	48	6.8

DIMENSIONS QUANTITIES

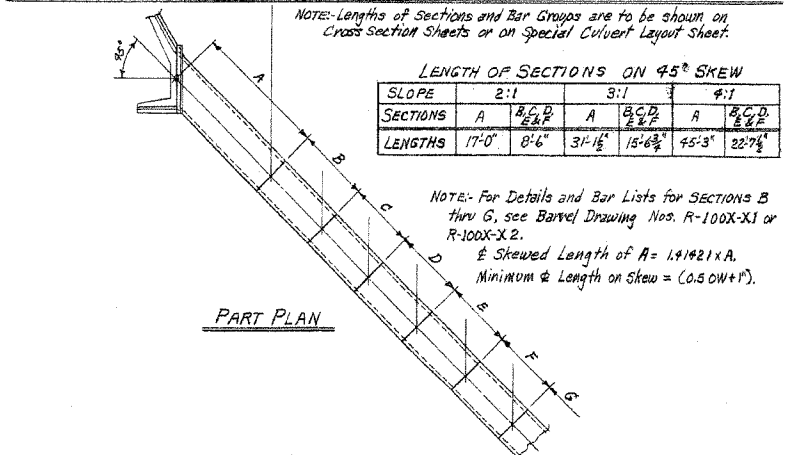
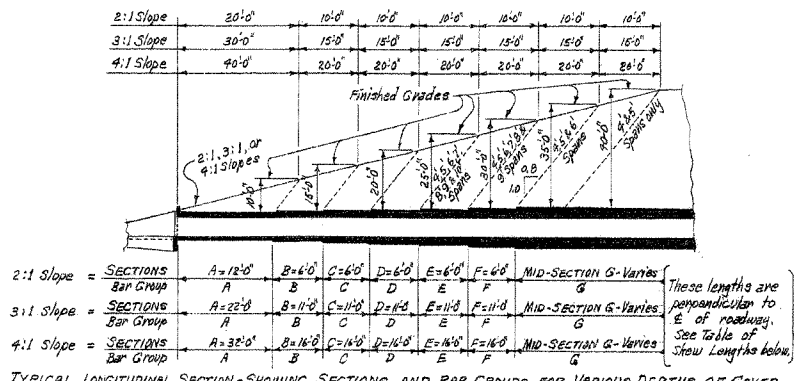
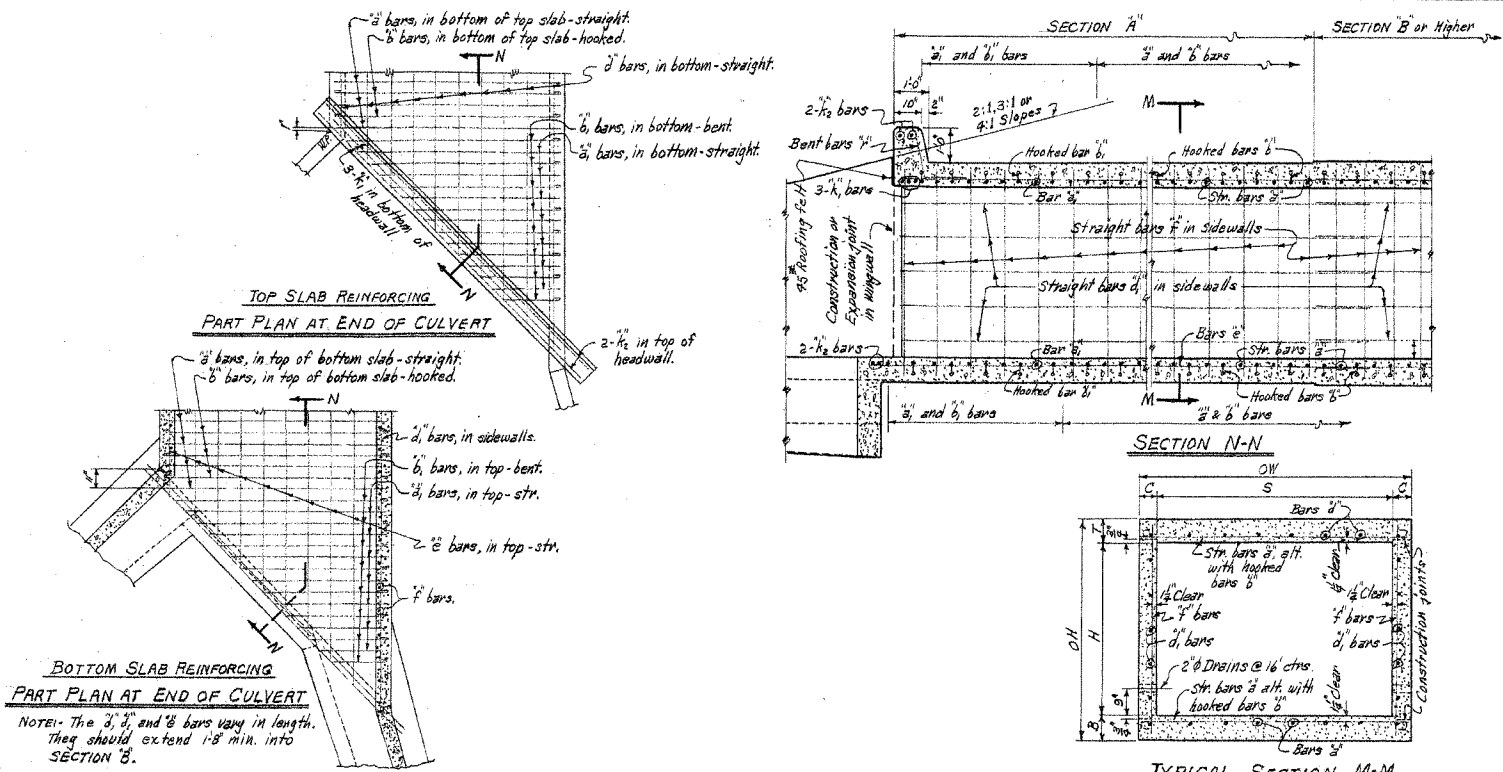
SECTION & BAR GROUP	LENGTH OF SECTION	DEPTH OF COVER	CLEAR SPAN	CLEAR HEIGHT	STRAIGHT		BENT- See Diagram at Right		STRAIGHT		STRAIGHT	
					In Top and Bottom Slabs of Barrel		In Top and Bottom Slabs of Barrel		Longitudinal in Top Slab of Barrel		Longitudinal in Bottom Slab of Barrel	
					NO. REIN. REQ'D	LENGTH	NO. REIN. REQ'D	LENGTH	NO. REIN. REQ'D	LENGTH	NO. REIN. REQ'D	LENGTH
SECTION A & BAR GROUP A	31' = 32'-0"	4'-0"	10'-0"	10'-0"	48	6.8	48	6.8	48	6.8	48	6.8
					48	6.8	48	6.8	48	6.8	48	6.8
					48	6.8	48	6.8	48	6.8	48	6.8
					48	6.8	48	6.8	48	6.8	48	6.8
					48	6.8	48	6.8	48	6.8	48	6.8
					48	6.8	48	6.8	48	6.8	48	6.8
					48	6.8	48	6.8	48	6.8	48	6.8
					48	6.8	48	6.8	48	6.8	48	6.8
					48	6.8	48	6.8	48	6.8	48	6.8
					48	6.8	48	6.8	48	6.8	48	6.8
SECTION B & BAR GROUP B	31' = 32'-0"	4'-0"	10'-0"	10'-0"	48	6.8	48	6.8	48	6.8	48	6.8
					48	6.8	48	6.8	48	6.8	48	6.8
					48	6.8	48	6.8	48	6.8	48	6.8
					48	6.8	48	6.8	48	6.8	48	6.8
					48	6.8	48	6.8	48	6.8	48	6.8
					48	6.8	48	6.8	48	6.8	48	6.8
					48	6.8	48	6.8	48	6.8	48	6.8
					48	6.8	48	6.8	48	6.8	48	6.8
					48	6.8	48	6.8	48	6.8	48	6.8
					48	6.8	48	6.8	48	6.8	48	6.8



Designed By: M.C.H. 9-5-62 Checked By: R.H.S. 11-8-62
 Drawn By: M.C.H. 11-10-62 Checked By: J.M. 11-12-62
 Quantities By: M.C.H. 11-7-62 Checked By: J.M. 11-16-62

ARKANSAS STATE HIGHWAY COMMISSION
 DETAILS OF STANDARD BARREL SECTIONS
 FOR
 REINFORCED CONCRETE BOX CULVERTS
 4', 5' & 6' SPANS
 SINGLES
 3:1 OR 4:1 SLOPES
 OVER 5'-0" COVER
 STANDARD DRAWING NO. R-100X-X1

FED. ROAD No.	STATE	FED. AID PROJECT	FISCAL YEAR	SHEET No.	TOTAL SHEETS
6	ARK.			211	
JOB No.					



LENGTH OF SECTIONS ON 45° SKEW						
SLOPE	2:1		3:1		4:1	
SECTIONS	A	$\frac{B+C}{2} D$	A	$\frac{B+C}{2} D$	A	$\frac{B+C}{2} D$
LENGTHS	17'-0"	8'-6"	31'- $\frac{1}{2}$ "	15'-6 $\frac{3}{4}$ "	45'-3"	22'-7 $\frac{1}{2}$ "

SECTIONS AND BAR GROUPS TO BE USED FOR VARIOUS DEPTHS OF COVER

DEPTHS OF COVER	SECTIONS & BAR GROUPS FOR END SECTIONS						MID-SECTION AND BAR GROUP		DEPTHS OF COVER	SECTIONS & BAR GROUPS FOR END SECTIONS						MID-SECTION AND BAR GROUP
	A	B	C	D	E	F				A	B	C	D	E	F	
5.0' to 9.5'	A						A	OR ALT.	5.0' to 9.5'	A						A
10.0' to 14.5'	A						B		10.0' to 14.5'	A						B
15.0' to 19.5'	A	B					C		15.0' to 19.5'	A	B					C
20.0' to 24.5'	A	B	C				D		20.0' to 24.5'	A	2@C					D
25.0' to 29.5'	A	B	C	D			E		25.0' to 29.5'	A	B	2@D				E
30.0' to 34.5'	A	B	C	D	E		F		30.0' to 34.5'	A	2@C		2@E			F
35.0' to 40.0'	A	B	C	D	E	F	G		35.0' to 40.0'	A	B	2@D		2@F		G

BAR SIZE	PIN DIAM.	K	ADD FOR 2 HOOPS	BENDING DIAGRAM FOR BARS ϕ	BAR SIZE	PIN DIAM.	K	ADD FOR 1 HOOP	BENDING DIAGRAM FOR BARS ϕ
#4	2"	4"	0'-10 $\frac{1}{2}$ "		#4	2"	4"	0'-5 $\frac{1}{2}$ "	
#5	2 $\frac{1}{2}$ "	4 $\frac{1}{2}$ "	0'-11 $\frac{1}{2}$ "		#5	2 $\frac{1}{2}$ "	4 $\frac{1}{2}$ "	0'-5 $\frac{3}{4}$ "	
#6	3"	5"	1'-2"		#6	3"	5"	0'-7"	
#7	3 $\frac{1}{2}$ "	5 $\frac{3}{4}$ "	1'-4"		#7	3 $\frac{1}{2}$ "	5 $\frac{3}{4}$ "	0'-8"	

DOWEL BARS FOR ONE HEADWALL					
SPAN	SIZE	SPACING	No. BARS	LENGTH	Dowel bars "n" in One Headwall
4'	#4	12"s	8	2'5"	1-2'6"
5'	#4	12"s	9	2'6"	1-3'
6'	#4	12"s	11	2'7"	1-3'6"
7'	#4	12"s	12	2'8"	1-4'
8'	#4	12"s	14	2'10"	1-4'5"
9'	#4	12"s	15	3'0"	1-6'
10'	#4	12"s	17	3'2"	1-7'

DESIGN LIVE LOAD
H20-S16 LOADING A.A.S.H.O. 1961
AND
SPECIAL MILITARY LOADING
Two 24,000 lb. Axles @ 4'-0" cts.
UNIT STRESSES:-
Class 5 Concrete ($n=10$) 1200 psi
Reinforcing Steel 20,000 psi

[illegible]

These bars are in the skewed portion of barrel only. The length of "a" bars and overall length "X" of "b" bars vary by 1'-0" for 12" spacing and 0'-11" for 11" spacing.

MAX. DESIGN DEPTH OF COVER	BARREL DIMENSIONS											* QUANTITIES-ONE SECTION "A"						
	CLEAR SPANS		CLEAR HEIGHT	SAFT. DRAWING	OVERALL WIDTH	THICKNESS OF TOP SLAB	THICKNESS OF SIDEWALLS	THICKNESS OF BOTTOM SLAB	OVERALL HEIGHT	ROADWAY LENGTH	LENGTH OF HEADWALL	CLASS 5 CONCRETE			REINFORCING STEEL			
	BARREL ONLY											BARREL, APRON AND HEADWALL						
	2:1	3:1	4:1	2:1	3:1	4:1												
D	S	H	A	OW	T	C	B	OH	RL	K	CuYd	CuYd	CuYd	L.B.	L.B.	L.B.		
SECTION "A" = 10'-0"	1 @ 4'	2'	8	5'-0"		6"		3'-0"	7'-0"	9'-0"	4.67	8.51	12.94	555	931	1321		
		3'	12	5'-0"		6"		3'-0"	"	"	5.30	9.71	14.12	593	1017	1446		
		4'	16	5'-0"		6"		3'-0"	"	"	5.93	10.64	15.79	641	1103	1571		
		5'	20	5'-0"		6"		3'-0"	"	"	6.56	12.01	17.49	690	1189	1694		
	1 @ 5'	5'	24	5'-2"		7"		7'-0"	7'-3"	9'-3"	7.94	14.99	21.18	747	1282	1893		
		6'	15	6'-0"		6"		4'-2"	8'-5"	10'-5"	6.29	11.52	16.74	667	1085	1506		
		6'	25	6'-0"		6"		5'-2"	"	"	6.92	12.67	18.42	915	1471	2216		
		6'	30	6'-2"		7"		7'-2"	8'-5"	10'-5"	8.94	16.37	23.00	1036	1655	2515		
	1 @ 6'	7'	35	7'-0"		7 1/2"		8'-0"	8'-10"	10'-10"	10.10	18.49	26.68	1114	1917	2753		
		3'	18	7'-0"		6"		5'-3"	9'-0"	11'-0"	7.90	13.59	19.48	1108	1922	2729		
		4'	24	7'-0"		6"		5'-3"	"	"	8.02	14.63	21.36	1156	2008	2854		
		5'	30	7'-0"		7 1/2"		6'-3"	"	"	8.65	15.84	23.03	1205	2094	2974		
1 @ 7'	6'	36	7'-2"		7"		7'-3"	10'-15"	12'-15"	10.05	18.39	26.74	1271	2187	3193			
	7'	42	7'-3"		7 1/2"		8'-3"	10'-3"	12'-3"	11.22	20.54	29.36	1361	2352	3389			
	8'	48	7'-4"		7 1/2"		9'-3"	10'-3"	12'-4"	12.50	22.38	32.36	1532	2657	3836			
	5'	28	8'-0"		6"		5'-4"	11'-3"	13'-3"	9.45	17.30	25.12	1339	2275	3214			
1 @ 8'	5'	35	8'-0"		6"		6'-4"	"	"	10.08	18.06	26.89	1387	2361	3339			
	6'	42	8'-2"		7"		7'-4"	11'-6"	13'-6"	11.48	21.01	30.54	1455	2643	3508			
	7'	49	8'-3"		7 1/2"		8'-4"	11'-8"	13'-8"	12.65	22.16	32.67	1530	2597	3698			
	8'	56	8'-4"		7 1/2"		9'-4"	11'-9"	13'-9"	13.92	25.49	37.06	1683	2812	4044			
1 @ 9'	9'	63	8'-6"		8"		10'-4"	12'-0"	14'-0"	15.86	29.07	42.22	1803	3094	4391			
	4'	32	9'-0"		6"		5'-6"	12'-0"	14'-0"	11.02	20.17	29.32	1469	2647	3915			
	5'	40	9'-2"		7"		6'-2"	12'-10"	14'-10"	12.33	22.57	32.91	1711	2853	4085			
	6'	48	9'-2"		7"		7'-6"	"	"	13.07	23.94	34.80	1759	2945	4213			
1 @ 10'	7'	56	9'-3"		8 1/2"		8'-6"	13'-1"	15'-1"	14.25	26.38	37.92	1820	3049	4364			
	8'	64	9'-4"		8 1/2"		9'-6"	13'-2"	15'-2"	15.54	28.45	41.00	2001	3372	4830			
	9'	72	9'-6"		9"		10'-6"	13'-5"	15'-5"	17.09	30.86	44.52	2117	3542	5095			
	10'	80	9'-8"		10"		11'-6"	13'-5"	15'-8"	19.68	35.38	52.32	2359	3914	5490			
1 @ 11'	5'	45	10'-2"		7"		6'-											

* For remainder of quantities see Std. Wing and Barrel Drawings listed below.
Total steel quantities listed above include one lap of longitudinal bars, plus an add'l lap for 4:1 slopes.

CONCRETE:- All concrete to be Class S, and shall be poured in the dry. All exposed corners to have $\frac{3}{4}$ chamfers.
REINFORCING STEEL:- Reinforcing to be deformed bars of intermediate or hard grade.
CONSTRUCTION JOINTS:- Construction joints between wingwalls, side walls and slabs shall be only where shown on plans.
SPECIFICATIONS:- Arkansas State Highway Commission Standard Specifications for Highway Construction and applicable Special Provisions.

NOTE: This drawing to be used in conjunction with Standard Wing Drawing Nos. W-X452-1 or W-X452-2; W-X453-1 or W-X453-2, and W-X454-1 or W-X454-2. Also W-X45.

For Barrels of 4'5" & 6" Spans, See Drawing No. R-100X-X1.
For Barrels of 7'6", 9' & 10' Spans, See Drawing No. R-100X-X2.

CLASS S CONCRETE

ARKANSAS STATE HIGHWAY COMMISSION

DETAILS OF STANDARD BARREL SECTIONS
FOR
REINFORCED CONCRETE BOX CULVERTS
45° SKEW SECTIONS A' ONLY

4'5", 6', 7', 8', 9' & 10' SPANS 2:1, 3:1 OR 4:1 SLOPES
SINGLES OVER 5'-0" COVER

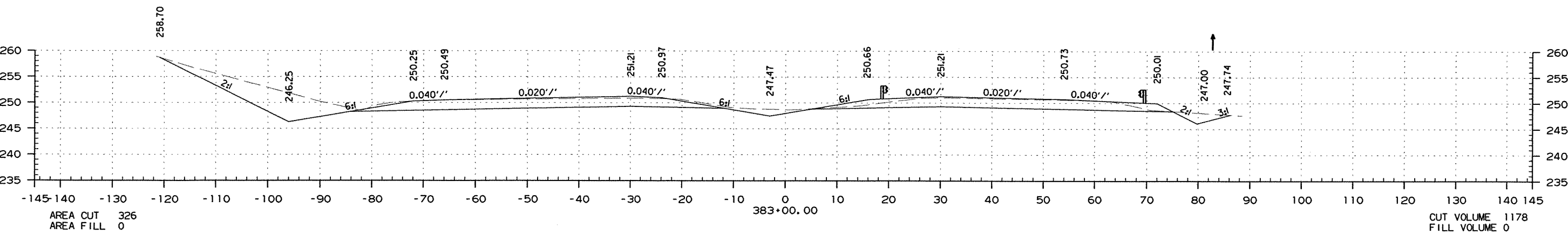
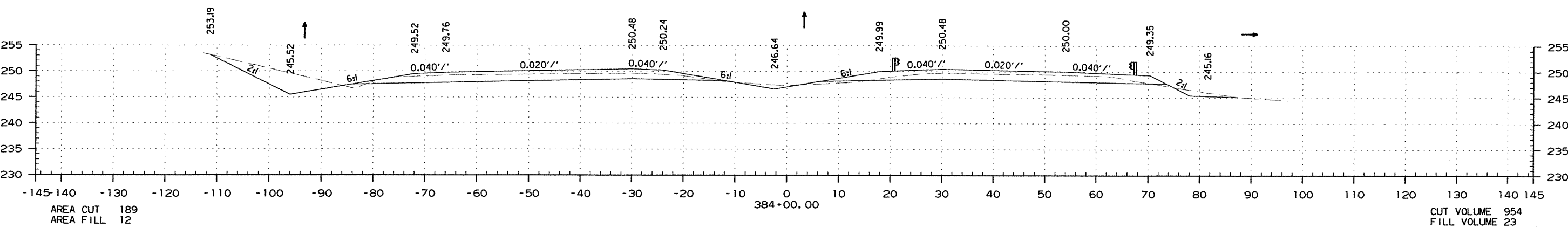
STANDARD DRAWING NO. R-145X-1

Designed by:- W.C.H.	9-5-62.	Checked by:- R.H.S.	11-8-62
Drawn by:- W.C.H.	8-10-64.	Checked by:-	
Quantities by:- W.C.H.	10-22-64.	Checked by:-	

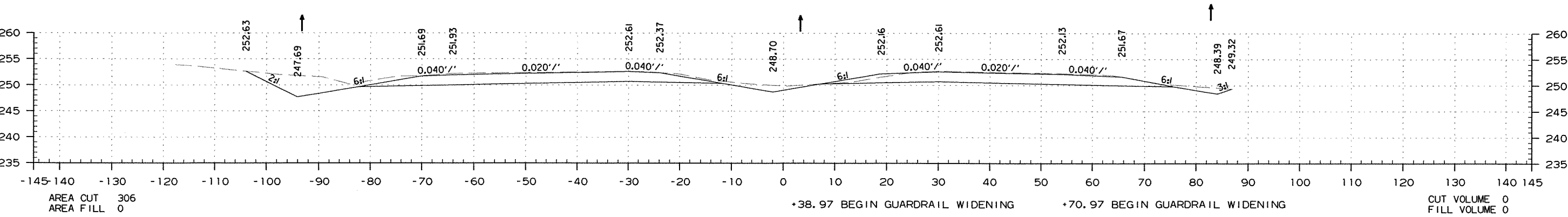
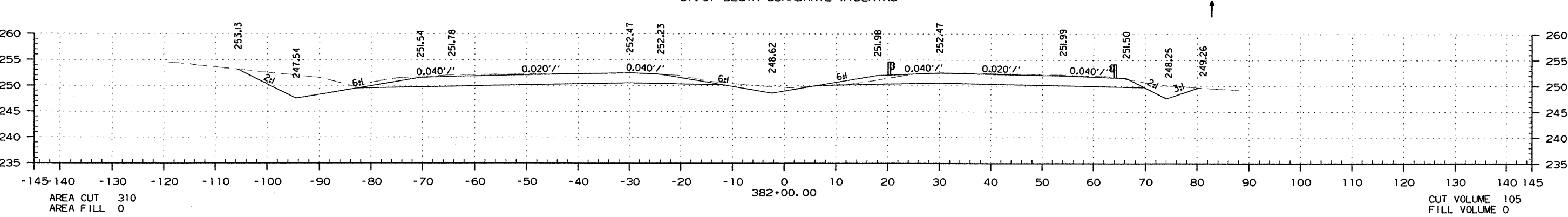
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DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB NO. BB0614		212	219	

2 CROSS SECTIONS



+37.97 BEGIN GUARDRAIL WIDENING

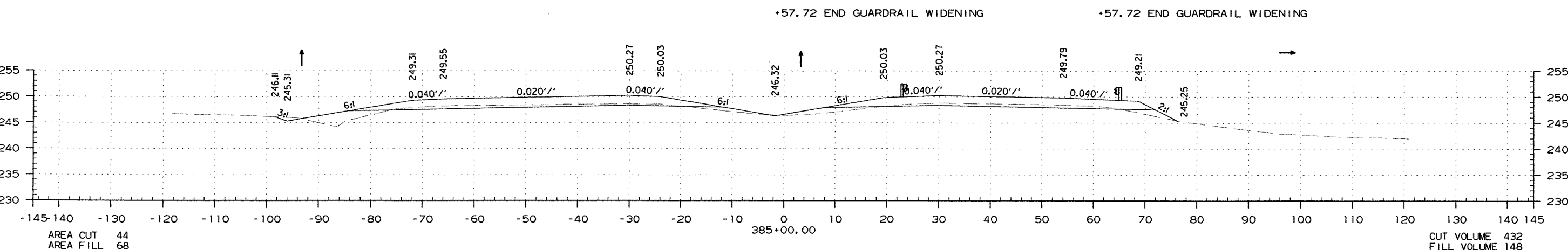
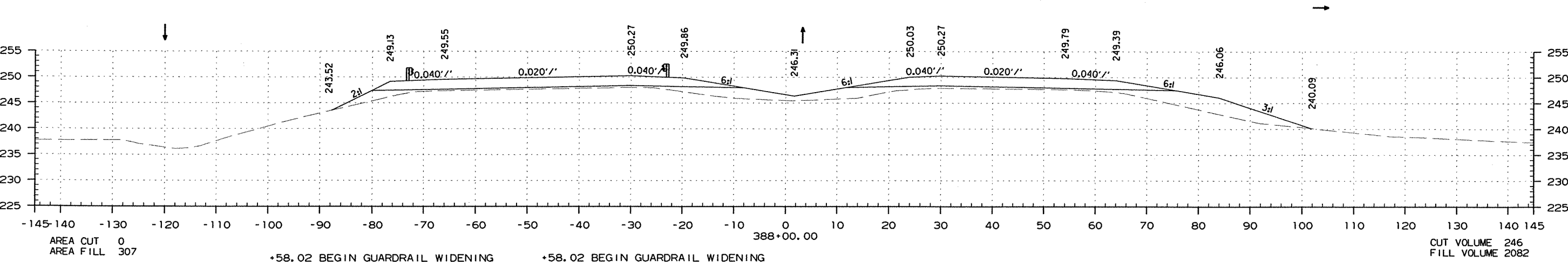


STA. 381+90.00
BEGIN 1-530 PROFILE GRADE

STA. 381+90.00 TO STA. 384+00.00

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB NO.		BB0614	213	219

2 CROSS SECTIONS

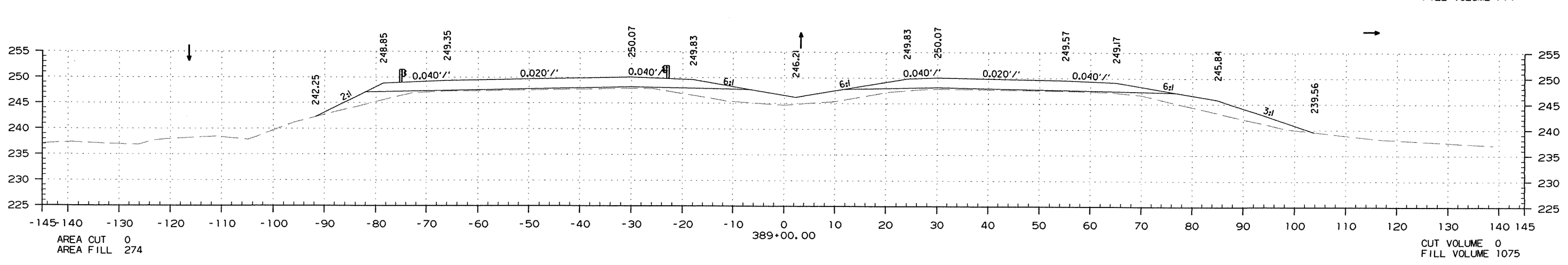
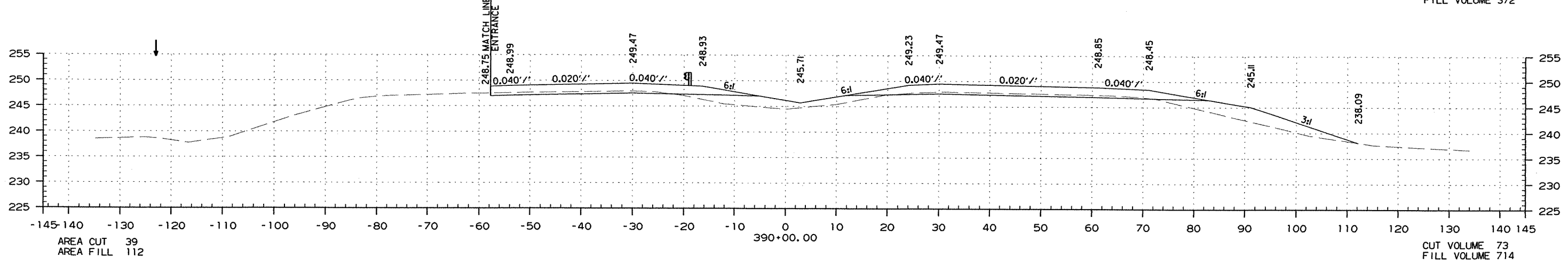
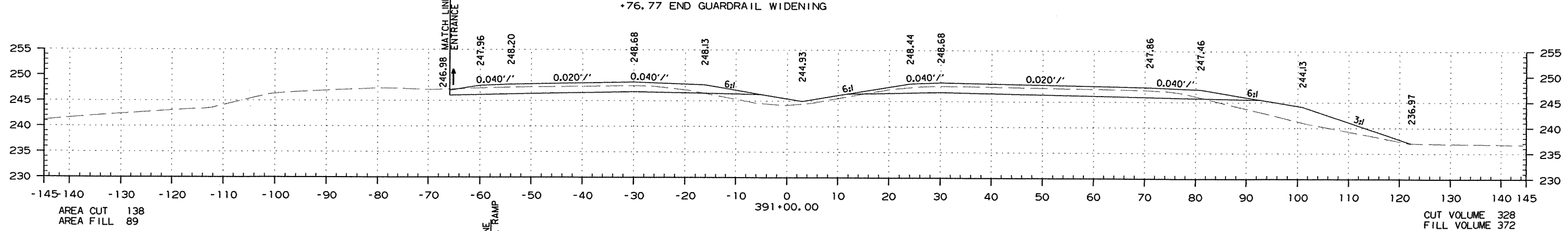
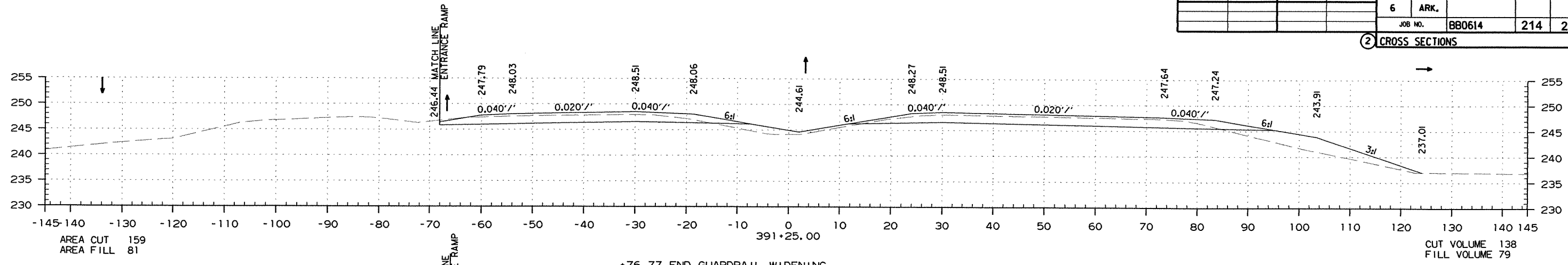


STA. 385+00.00 TO STA. 388+00.00

10/8/2012
RB0614.DGN

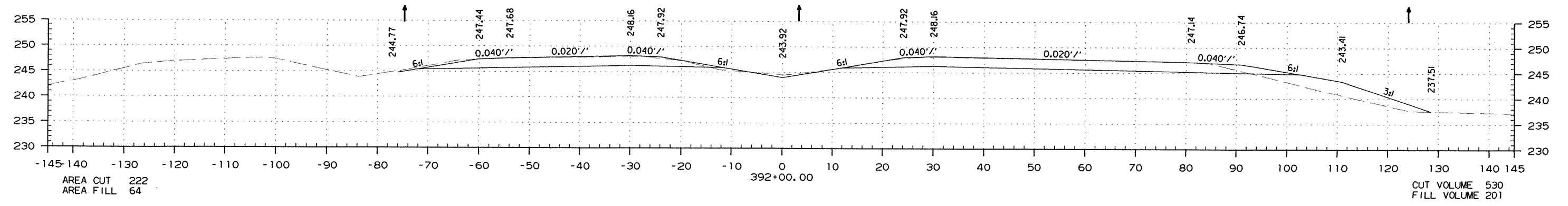
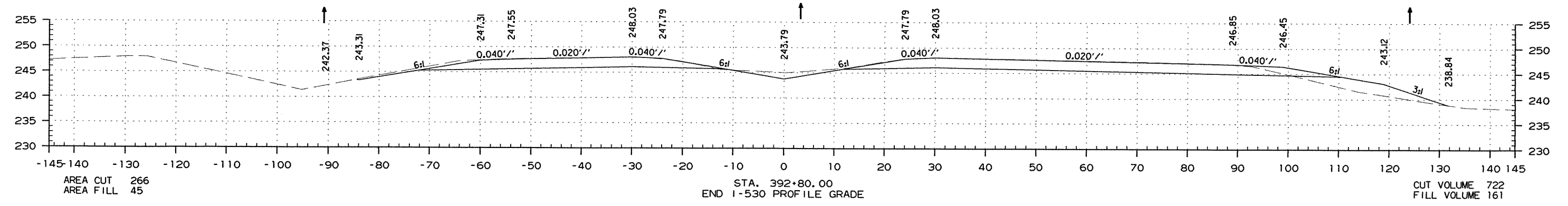
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				6	ARK.			
				JOB NO. BB0614		214	219	

2 CROSS SECTIONS



STA. 389+00.00 TO STA. 391+25.00

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB NO.		BB0614	215	219
② CROSS SECTIONS								

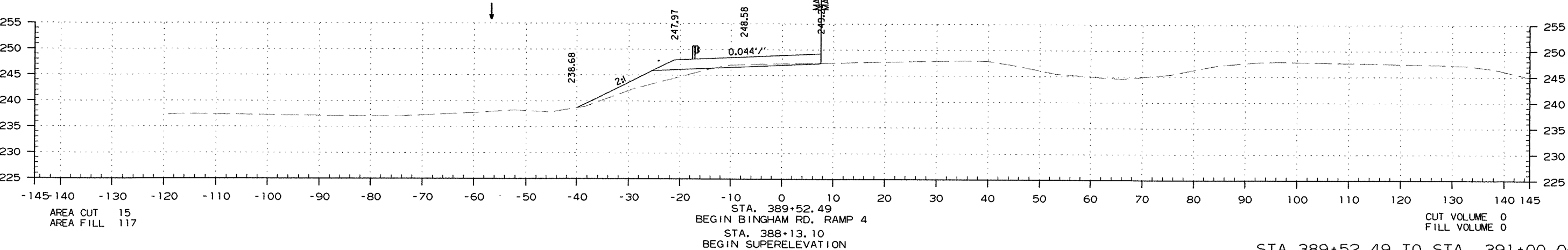
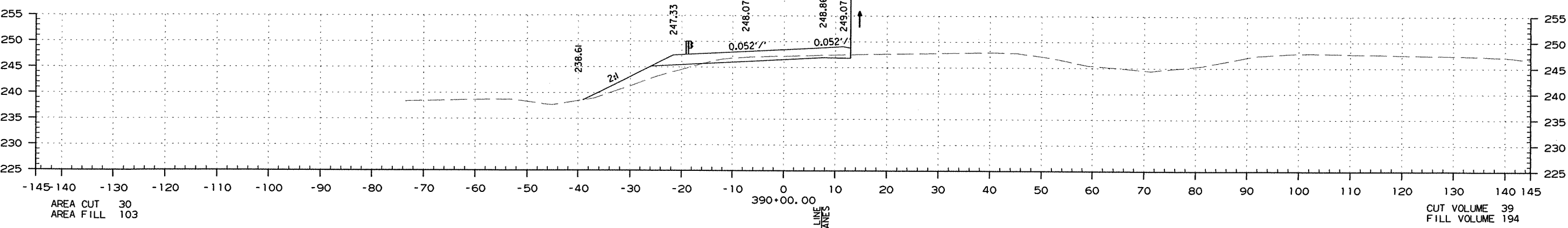
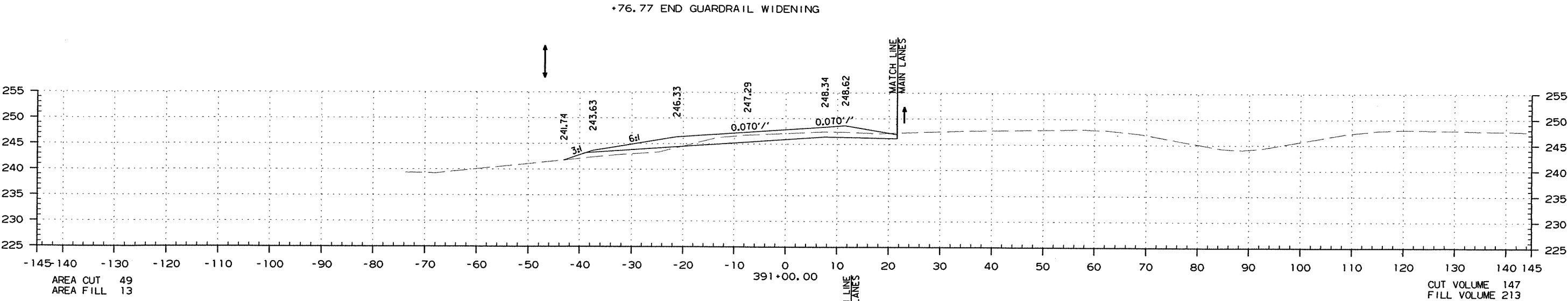


STA. 392+00.00 TO STA. 392+80.00

10/8/2012
RB0614.DGN

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB NO. 880614		216	219	

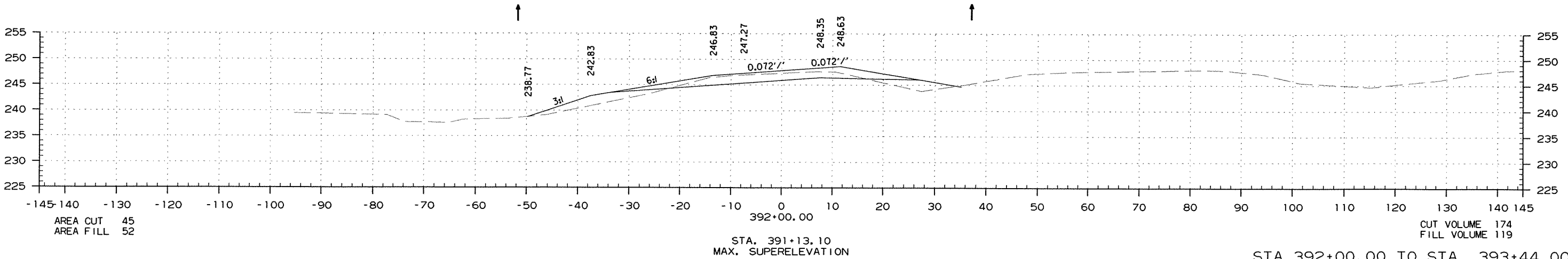
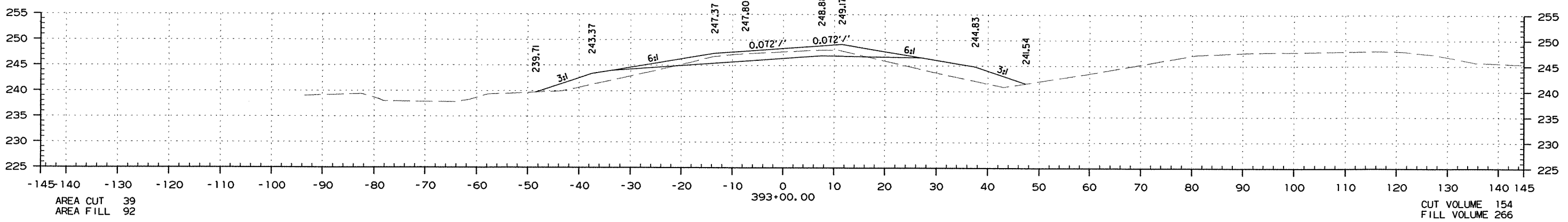
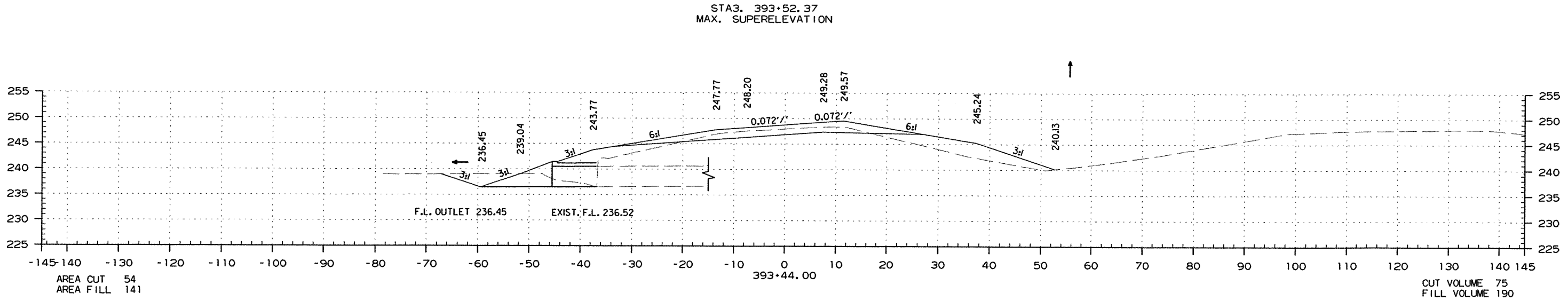
2 CROSS SECTIONS



10/8/2012
R880614.DGN

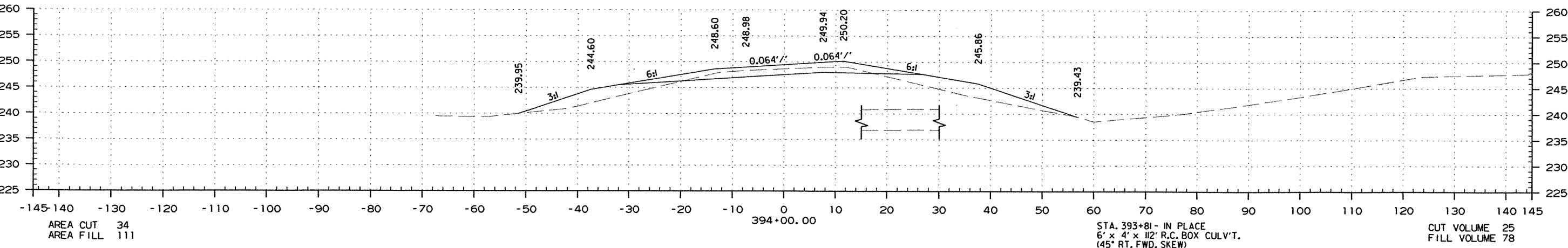
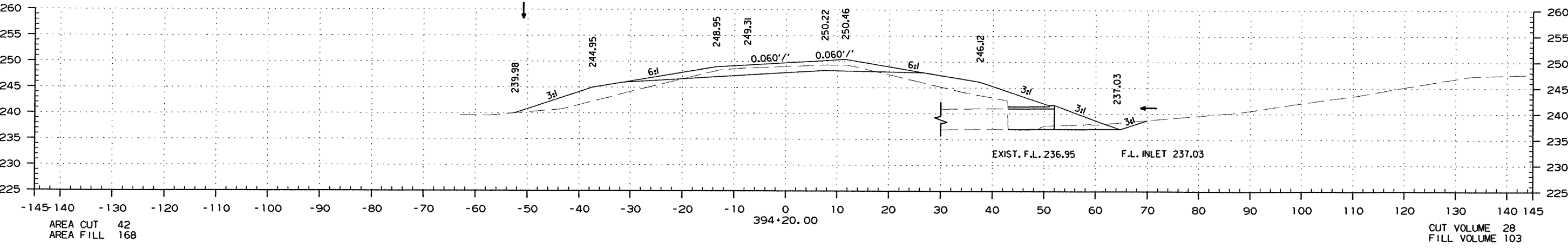
DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB NO. BB0614		217	219	

2 CROSS SECTIONS

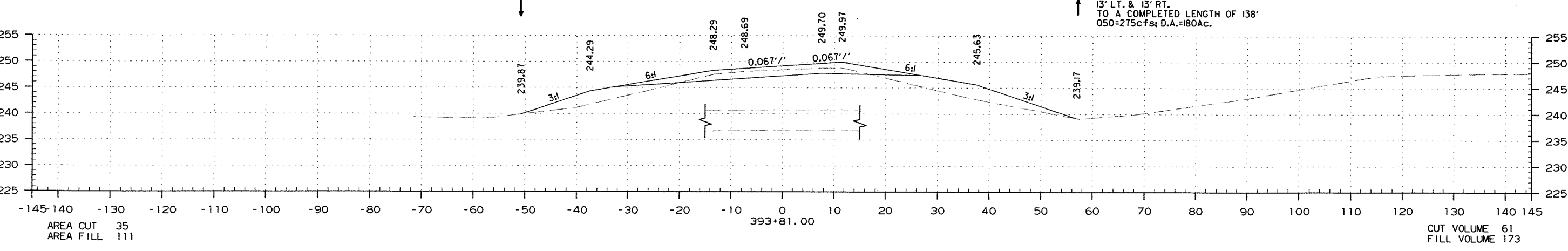


DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
						JOB NO. 880614	218	219

2 CROSS SECTIONS



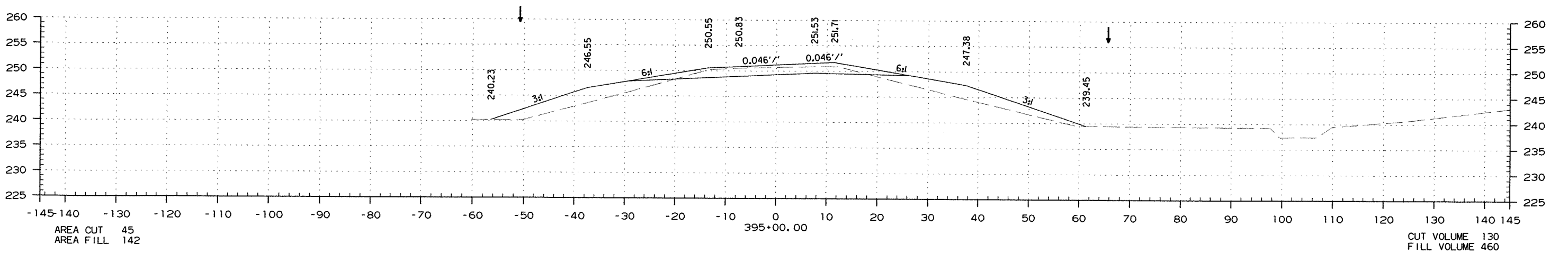
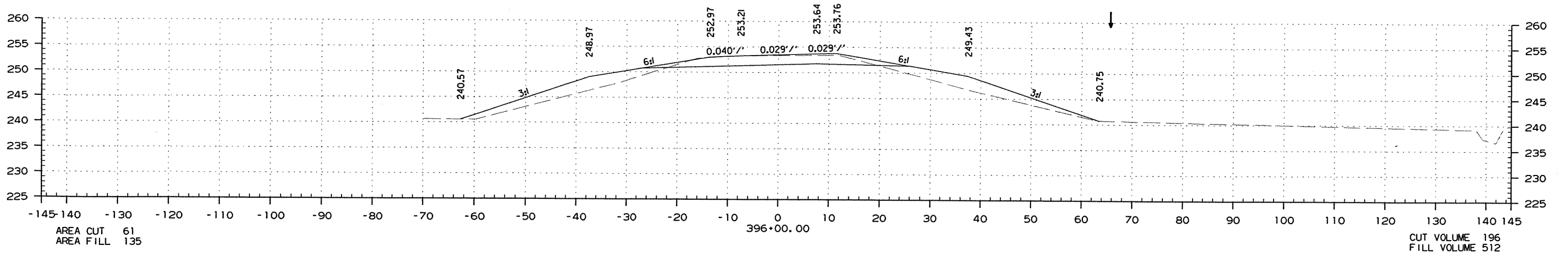
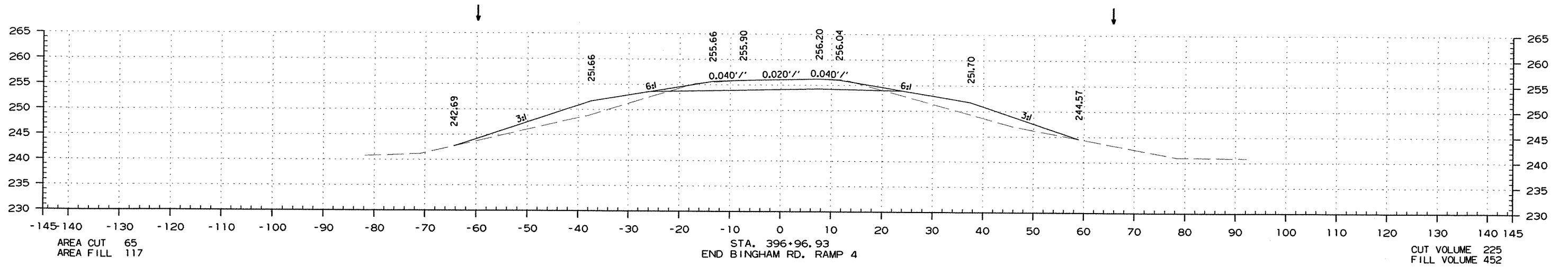
STA. 393+81 - IN PLACE
6' x 4' x 112' R.C. BOX CULV'T.
(45° RT. FWD. SKEW)
RETAIN AND EXTEND
13' LT. & 13' RT.
TO A COMPLETED LENGTH OF 138'
Q50=275cfs; D.A.=180Ac.



STA. 393+81.00 TO STA. 394+20.00

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB NO.	BB0614		219	219

② CROSS SECTIONS



STA. 395+00.00 TO STA. 396+96.93

10/8/2012

RB0614.DGN